



# DEVELOPMENT APPLICATION

City of Rockwall  
Planning and Zoning Department  
385 S. Goliad Street  
Rockwall, Texas 75087

STAFF USE ONLY  
PLANNING & ZONING CASE NO.

**NOTE:** THE APPLICATION IS NOT CONSIDERED ACCEPTED BY THE CITY UNTIL THE PLANNING DIRECTOR AND CITY ENGINEER HAVE SIGNED BELOW.

DIRECTOR OF PLANNING:

CITY ENGINEER:

PLEASE CHECK THE APPROPRIATE BOX BELOW TO INDICATE THE TYPE OF DEVELOPMENT REQUEST [SELECT ONLY ONE BOX]:

## PLATTING APPLICATION FEES:

- ☐ MASTER PLAT (\$100.00 + \$15.00 ACRE)<sup>1</sup>
- ☐ PRELIMINARY PLAT (\$200.00 + \$15.00 ACRE)<sup>1</sup>
- ☐ FINAL PLAT (\$300.00 + \$20.00 ACRE)<sup>1</sup>
- ☐ REPLAT (\$300.00 + \$20.00 ACRE)<sup>1</sup>
- ☐ AMENDING OR MINOR PLAT (\$150.00)
- ☐ PLAT REINSTATEMENT REQUEST (\$100.00)

## SITE PLAN APPLICATION FEES:

- ☐ SITE PLAN (\$250.00 + \$20.00 ACRE)<sup>1</sup>
- ☐ AMENDED SITE PLAN/ELEVATIONS/LANDSCAPING PLAN (\$100.00)

## ZONING APPLICATION FEES:

- ☐ ZONING CHANGE (\$200.00 + \$15.00 ACRE)<sup>1</sup>
- ☒ SPECIFIC USE PERMIT (\$200.00 + \$15.00 ACRE)<sup>1 & 2</sup>
- ☐ PD DEVELOPMENT PLANS (\$200.00 + \$15.00 ACRE)<sup>1</sup>

## OTHER APPLICATION FEES:

- ☐ TREE REMOVAL (\$75.00)
- ☐ VARIANCE REQUEST/SPECIAL EXCEPTIONS (\$100.00)<sup>2</sup>

### NOTES:

<sup>1</sup>: IN DETERMINING THE FEE, PLEASE USE THE EXACT ACREAGE WHEN MULTIPLYING BY THE PER ACRE AMOUNT. FOR REQUESTS ON LESS THAN ONE ACRE, ROUND UP TO ONE (1) ACRE.  
<sup>2</sup>: A \$1,000.00 FEE WILL BE ADDED TO THE APPLICATION FEE FOR ANY REQUEST THAT INVOLVES CONSTRUCTION WITHOUT OR NOT IN COMPLIANCE TO AN APPROVED BUILDING PERMIT.

## PROPERTY INFORMATION [PLEASE PRINT]

ADDRESS 1540 I-30 Rockwall, TX 75087

SUBDIVISION Rockwall Recreation Addition

LOT 1 BLOCK 1

GENERAL LOCATION Located off I-30 Service Road & Commerce Street

## ZONING, SITE PLAN AND PLATTING INFORMATION [PLEASE PRINT]

CURRENT ZONING L1

CURRENT USE Auto Dealer

PROPOSED ZONING

PROPOSED USE

ACREAGE 7.17

LOTS [CURRENT]

LOTS [PROPOSED]

☒ **SITE PLANS AND PLATS:** BY CHECKING THIS BOX YOU ACKNOWLEDGE THAT DUE TO THE PASSAGE OF HB3167 THE CITY NO LONGER HAS FLEXIBILITY WITH REGARD TO ITS APPROVAL PROCESS, AND FAILURE TO ADDRESS ANY OF STAFF'S COMMENTS BY THE DATE PROVIDED ON THE DEVELOPMENT CALENDAR WILL RESULT IN THE DENIAL OF YOUR CASE.

## OWNER/APPLICANT/AGENT INFORMATION [PLEASE PRINT/CHECK THE PRIMARY CONTACT/ORIGINAL SIGNATURES ARE REQUIRED]

☐ OWNER 1540 East IH 30 Rockwall, LLC

☒ APPLICANT The Charles Morgan Group, LP

CONTACT PERSON Chase Cooley

CONTACT PERSON Davin Marceau

ADDRESS

ADDRESS

CITY, STATE & ZIP

CITY, STATE & ZIP

PHONE

PHONE

E-MAIL

E-MAIL

## NOTARY VERIFICATION [REQUIRED]

BEFORE ME, THE UNDERSIGNED AUTHORITY, ON THIS DAY PERSONALLY APPEARED CHASE F. COOLEY [OWNER] THE UNDERSIGNED, WHO STATED THE INFORMATION ON THIS APPLICATION TO BE TRUE AND CERTIFIED THE FOLLOWING:

I HEREBY CERTIFY THAT I AM THE OWNER FOR THE PURPOSE OF THIS APPLICATION; ALL INFORMATION SUBMITTED HEREIN IS TRUE AND CORRECT; AND THE APPLICATION FEE OF \$ 1125 TO COVER THE COST OF THIS APPLICATION, HAS BEEN PAID TO THE CITY OF ROCKWALL ON THIS THE 11th DAY OF December, 2025. BY SIGNING THIS APPLICATION, I AGREE THAT THE CITY OF ROCKWALL (I.E. "CITY") IS AUTHORIZED AND PERMITTED TO PROVIDE INFORMATION CONTAINED WITHIN THIS APPLICATION TO THE PUBLIC. THE CITY IS ALSO AUTHORIZED AND PERMITTED TO REPRODUCE ANY COPYRIGHTED INFORMATION SUBMITTED IN CONJUNCTION WITH THIS APPLICATION, IF SUCH REPRODUCTION IS ASSOCIATED OR IN RESPONSE TO A REQUEST FOR PUBLIC INFORMATION.

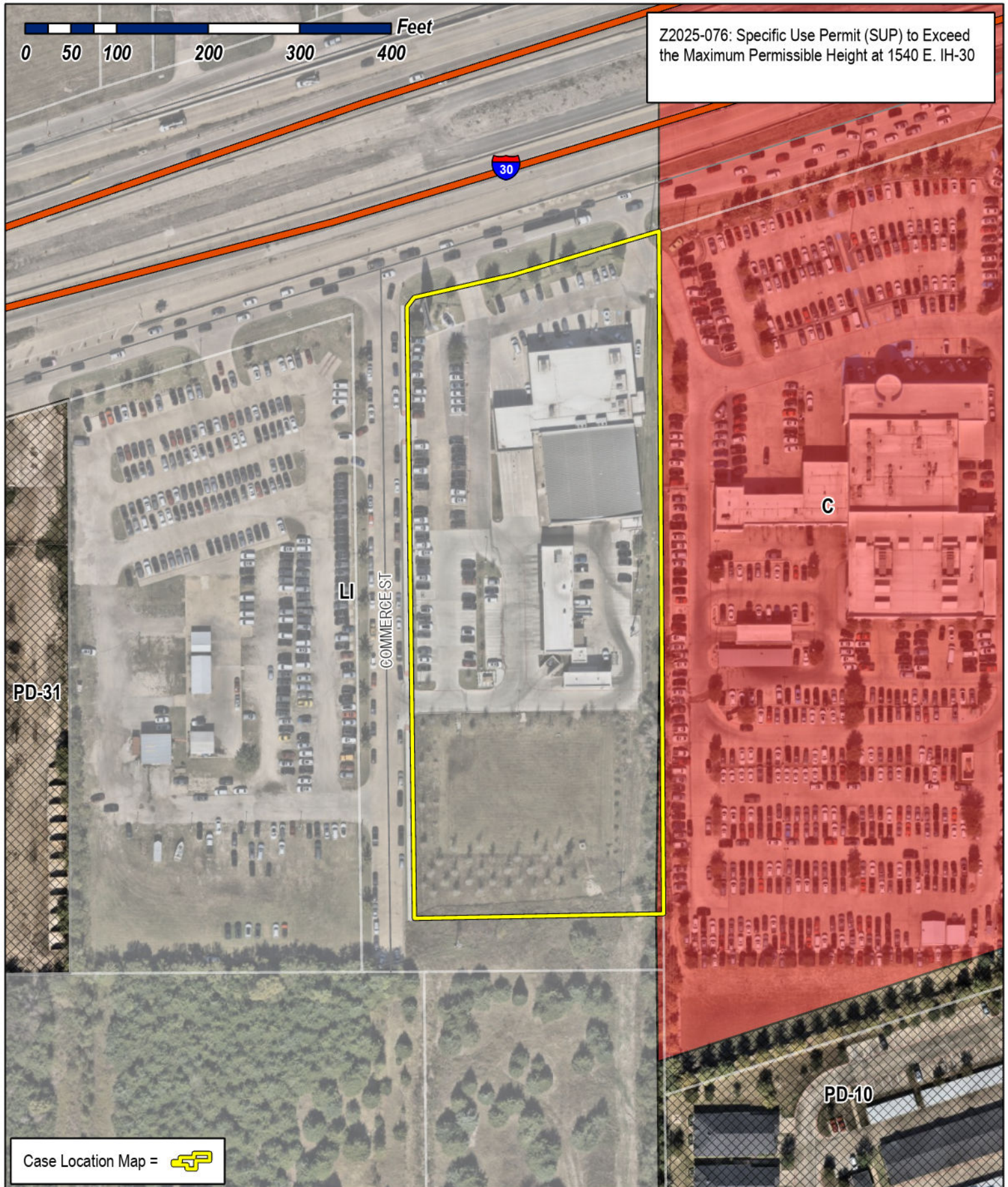
GIVEN UNDER MY HAND AND SEAL OF OFFICE ON THIS THE 11th DAY OF December, 2025.

OWNER'S SIGNATURE

NOTARY PUBLIC IN AND FOR THE STATE OF TEXAS







# City of Rockwall

Planning & Zoning Department  
385 S. Goliad Street  
Rockwall, Texas 75087  
(P): (972) 771-7745  
(W): [www.rockwall.com](http://www.rockwall.com)

The City of Rockwall GIS maps are continually under development and therefore subject to change without notice. While we endeavor to provide timely and accurate information, we make no guarantees. The City of Rockwall makes no warranty, express or implied, including warranties of merchantability and fitness for a particular purpose. Use of the information is the sole responsibility of the user.



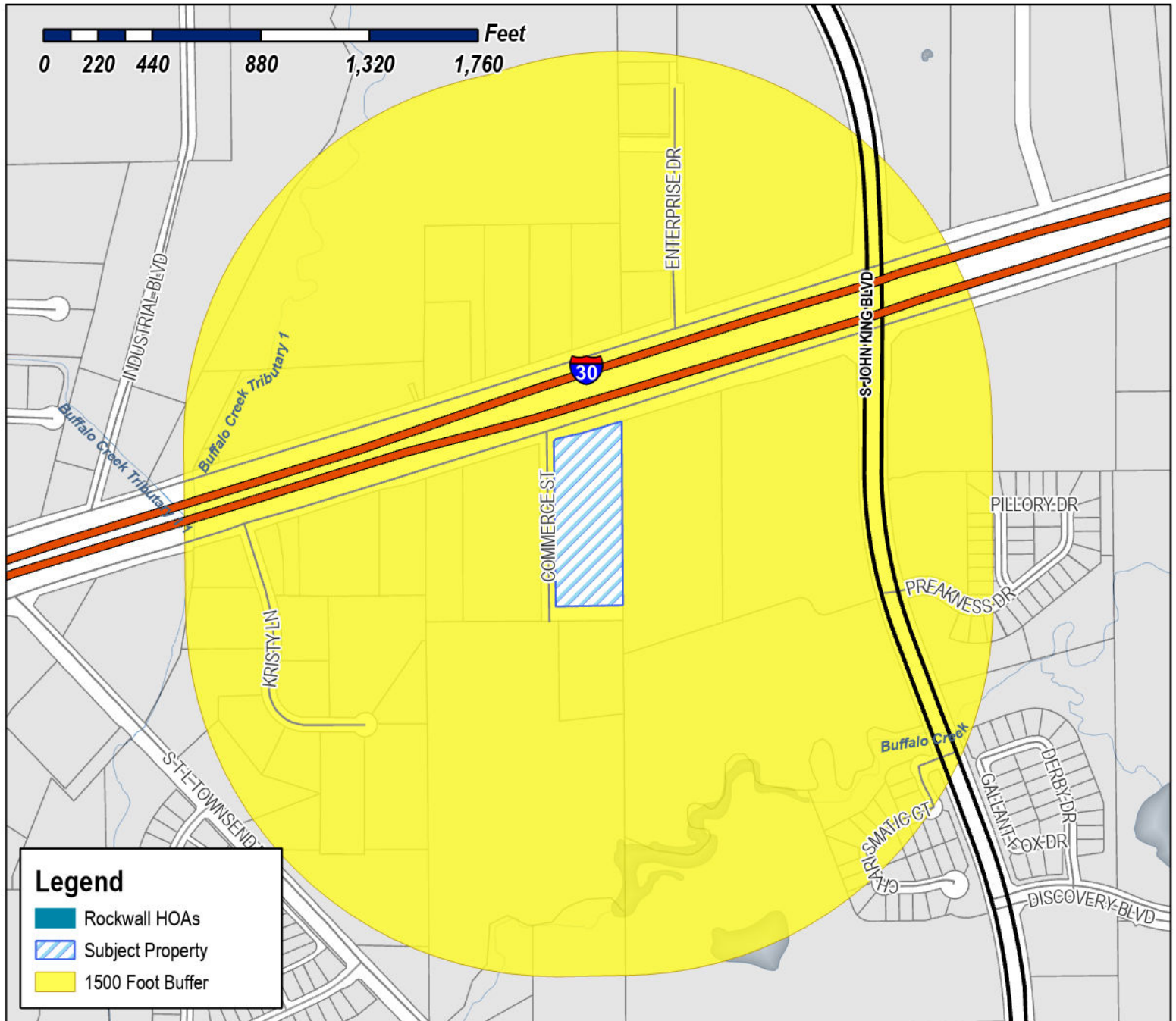




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**Case Number:** Z2025-076  
**Case Name:** Specific Use Permit (SUP) to Exceed the Maximum Permissible Height  
**Case Type:** Zoning  
**Zoning:** Light Industrial (LI) District  
**Case Address:** 1540 I-30

**Date Saved:** 12/15/2025  
For Questions on this Case Call (972) 771-7745

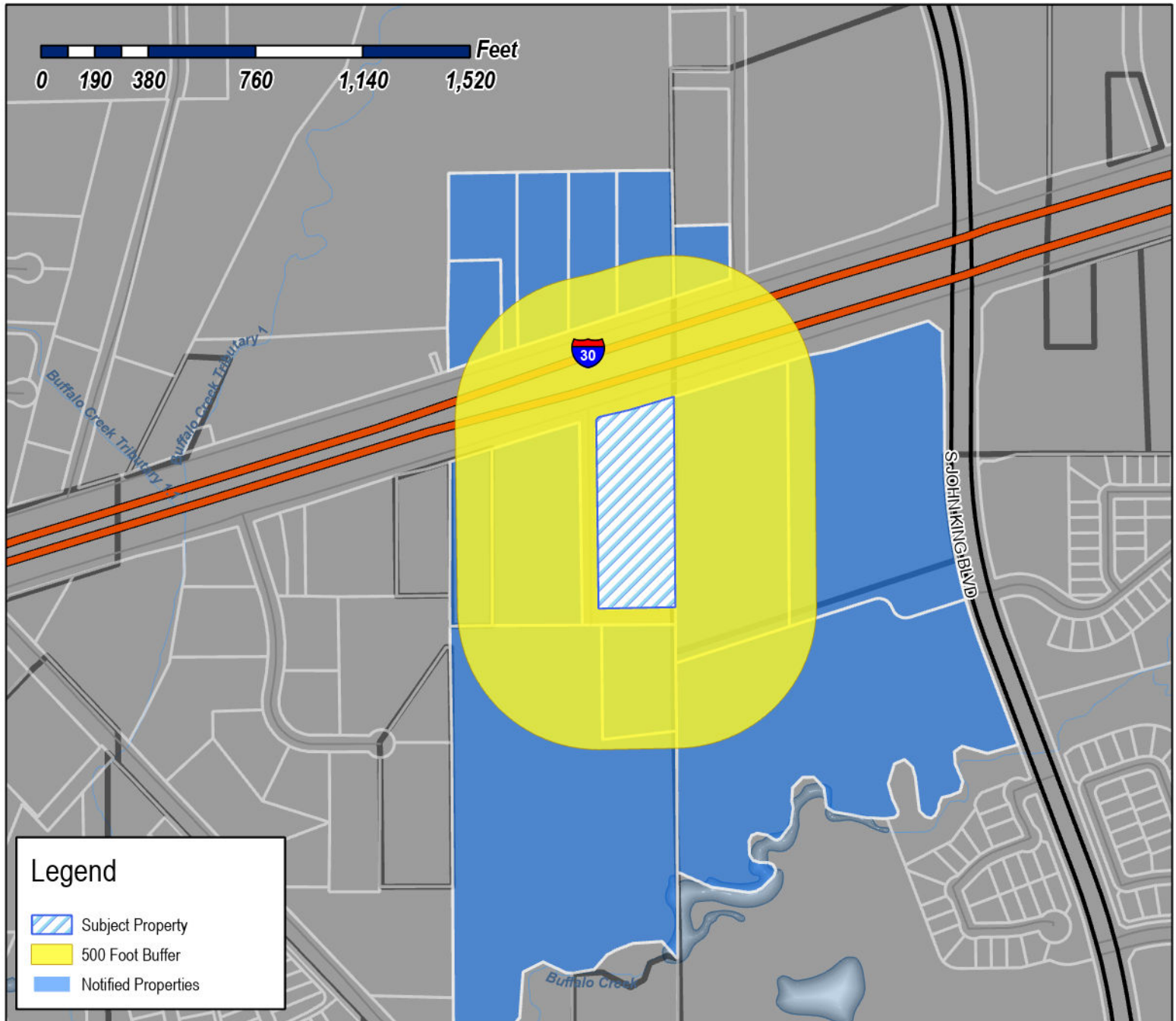




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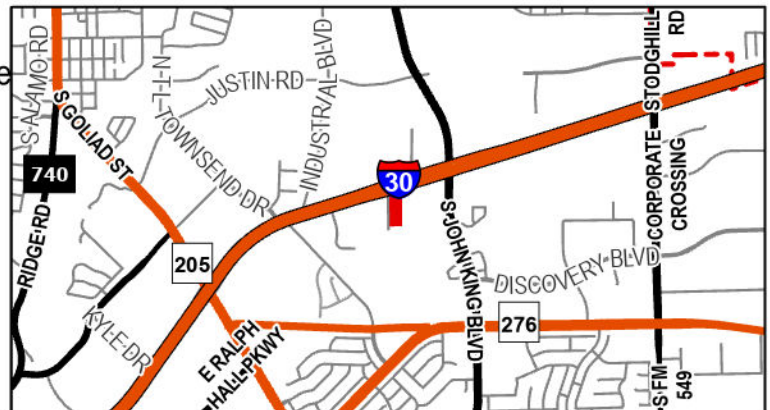
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RESIDENT  
130 NORTH SERVICE RD  
ROCKWALL, TX 75087

LITHIA REAL ESTATE INC  
150 N BARTLETT STREET  
MEDFORD, OR 97501

RESIDENT  
1520 E I30  
ROCKWALL, TX 75087

RESIDENT  
1530 S I30  
ROCKWALL, TX 75087

RESIDENT  
1535 I30  
ROCKWALL, TX 75087

RESIDENT  
1540 I30  
ROCKWALL, TX 75087

RESIDENT  
1545 E INTERSTATE 30  
ROCKWALL, TX 75087

RESIDENT  
1550 E I30  
ROCKWALL, TX 75087

RESIDENT  
1551 E I30  
ROCKWALL, TX 75087

AM ROCKWALL INVESTMENTS LP  
A TEXAS LTD PARTNERSHIP  
1551 E Interstate 30 Ste A  
Rockwall, TX 75087

RESIDENT  
1600 E INTERSTATE 30  
ROCKWALL, TX 75087

RESIDENT  
1650 S JOHN KING  
ROCKWALL, TX 75087

DVB FAMILY LIMITED PARTNERSHIP  
2421 KATHRYN DR  
HEATH, TX 75032

DVB FAMILY LIMITED PARTNERSHIP  
2421 KATHRYN DR  
HEATH, TX 75032

C2LA, LLC  
382 Ranch Trl  
Rockwall, TX 75032

H E B LP  
646 SOUTH FLORES STREET  
SAN ANTONIO, TX 78204

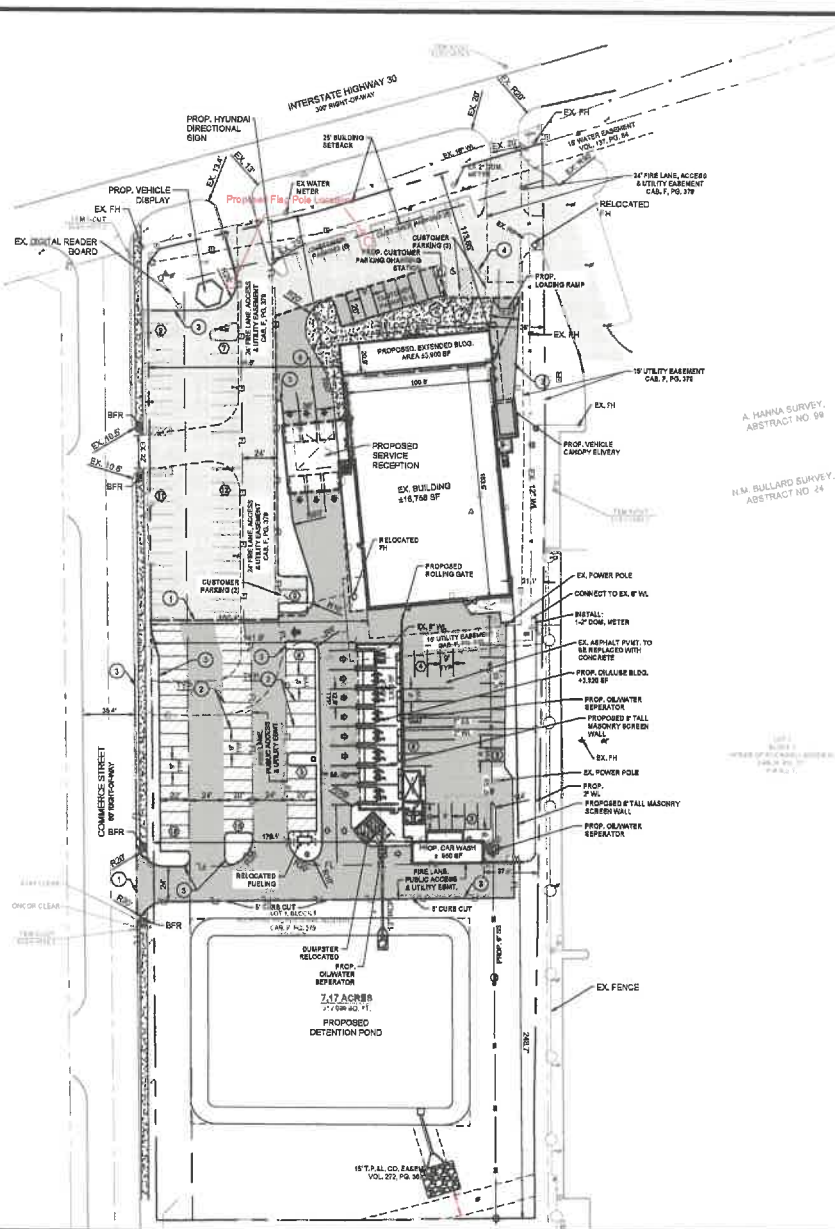
DYNACAP HOLDINGS LTD &  
CHARLES SMITH  
709 W Rusk St Ste B  
Rockwall, TX 75087

ZBH/1535 E INTERSTATE 30 LTD  
9669 JOURDAN WAY  
DALLAS, TX 75230











1540 EAST IH 30 ROCKWALL LLC  
M/R  
ROCKWALL, TX 75087

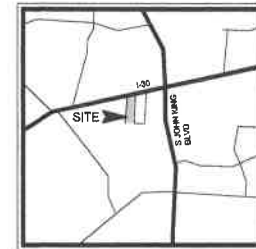
STAR HUBBARD LLC  
C/O STEADFAST COMPANIES  
PO BOX 530292  
BIRMINGHAM, AL 35253





PARKING DATA TABLE	
PARKING REQ.	
SALES FLOOR 1/ 250 SF	15 SPACES
OFFICE SP 1/ 300 SF	5 SPACES
1 PER 2 BAY	8 SPACES
TOTAL:	27 SPACES
PARKING PROVIDED	
DISPLAY PARKING	55 SPACES
CUSTOMER PARKING	27 (2 ADA) SPACES
SERVICE/STORAGE PARKING	24 SPACES

CONSTRUCTION SCHEDULE	
	EXISTING CONCRETE PAVEMENT
	FIRE LANE AND DRIVE ABLE 8' x 3000 PSI SACK/1" #9 ON 16" ON 4" FINE STABILIZED SUBGRADE
	PARKING 5' 3000 PSI (8.5 SACK/1" #9 ON 4" FINE STABILIZED SUBGRADE)
	PROPOSED DUMPSTER ENCLOSURE MIN. THICK
	PROPOSED CONCRETE SIDEWALK PER DETAIL SHEET C-10
	PROPERTY LINE
	PROPOSED CONCRETE CURB AND GUTTER
	PROPOSED FIRE LANE
	PROPOSED DRIVE/CUT
	PARKING COUNT



### VICINITY MAP

N.T.S.

CONSTRUCTION SCHEDULE	
①	PROP. BAW CUT FULL DEPTH EXISTING PAVEMENT
②	PROP. 4" PARKING STALL STRIPING COLOR: WHITE (TYP)
③	PROP. CONCRETE CURB & GUTTER
④	EX. HANDICAP SYMBOL
⑤	EX. HANDICAP SIGN
⑥	PROP. PAVEMENT STRIPING
⑦	PROP. PROPOSED BOLLARD

1. A SEPARATE SIGNAGE PERMIT IS REQUIRED BY THE CITY OF ROCKWALL.
2. ALL WALLS 3' IN HEIGHT OR GREATER SHALL BE DESIGNED BY AN ENGINEER, LICENSED IN THE STATE OF TEXAS.
3. ALL WALLS MUST BE ROCK OR STONE FACE. NO WALLS SHALL BE SMOOTH CONCRETE.
4. NO TREES SHALL BE PLANTED WITHIN 10' OF NON-STEEL ENCASED PUBLIC UTILITIES.
5. BOD MUST BE LAID WITHIN ALL DISTURBED R.O.W. BEFORE ACCEPTANCE OF PROJECT.

CLAY COOLEY HYUNDAI  
ROCKWALL TX 75087

LEGAL DESCRIPTION AND OR ADDRESS:

ROCKWALL RECREATION ADDITION  
LOT 1 BLOCK 1  
7.17 AC (32,108 SF)

CASE#:

CLAY COOLEY AUTO  
1391 S AIRPORT FREEWAY  
IRVING, TX 75062

SELLER#:

CLAYMOORE ENGINEERING, INC.  
1903 CENTRAL DRIVE, SUITE #400  
ROCKFORD, TX 76081  
PH: 817-281-0522

CASE NUMBER

22021-049

I HEREBY CERTIFY THAT THE ABOVE AND FOREGOING SITE IS PLAN FOR  
A DEVELOPMENT IN THE CITY OF ROCKWALL, TEXAS, WAS APPROVED  
BY THE PLANNING AND ZONING COMMISSION OF THE CITY OF  
ROCKWALL, ON THE \_\_\_\_\_ DAY OF \_\_\_\_\_

WITNESS OUR HANDS THIS \_\_\_\_\_ DAY OF \_\_\_\_\_

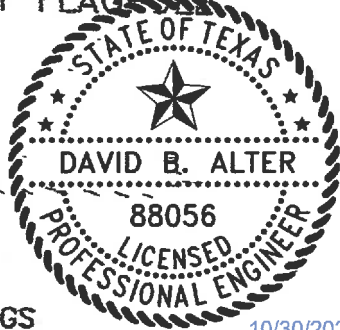
PLANNING AND ZONING COMMISSION, CHAIRMAN

\_\_\_\_\_  
DIRECTOR OF PLANNING AND ZONING

CITY SITE PLAN

SHEET  
SP-1

# DIMINISHING SECTION STEEL CONCEALED HALYARD GROUND SET FLAGPOLE



## STANDARD FITTINGS

10/30/2025

**FINIAL:** (The flagpole division of NAAMM recommends that no Finial, other than the internal Halyard Truck itself be used on the Concealed Halyard Flagpole.)

**TRUCK:** Heavy duty, cast aluminum, internal halyard type, revolving non-fouling, gold powder coated, equipped with upper and lower sealed bearings assemblies and a stainless steel spindle for flagpoles up to 130'. Larger flagpoles will come with two spun steel hemispheres, automotive type sealed bearing assemblies and a welded in steel spindle.

**HALYARD:** Halyard and flag arrangement are 1/8" and 3/16" stainless steel aircraft cable, with attachment ends crimped over 1/8" stainless steel yokes and joined by a stainless steel quicklink. Cable is two piece, joined by a stainless steel swivel at mid-point. The flag arrangement is sized to accommodate an appropriate size flag and is supplied complete with two heavy duty swivel snaps, neoprene coated counterweight, and beaded nylon retainer rings.

**WINCH:** Stainless steel, direct drive, accessible for maintenance only through a reinforced access opening, which is covered by a removable door finished to match flagpole shaft and contains a six tumbler cylinder lock. Winch is gearless, operable only by a removable crank handle and locks in any position upon removal of the crank handle.

**FOUNDATION TUBE:** Fabricated from 16GA. galvanized steel, with a steel base plate whose square dimension is 4" larger than the I.D. of the sleeve. A setting plate 6" square is securely welded to the ground spike 6" below the base plate. The ground spike is 3/4" diameter and 36" in length.

CAST ALUMINUM REVOLVING,  
NON-FOULING, INTERNAL  
HALYARD TYPE TRUCK

FLAG  
ARRANGEMENT

PLEASE SPECIFY  
FLAG SIZE TO BE  
FLOWN

40' X 80'

NEOPRENE COATED 2"  
DIAMETER STEEL  
COUNTERWEIGHT

NYLON BEADED  
RETAINER RING

(4)

PERIPHERALLY  
REINFORCED ACCESS  
DOOR WITH SIX TUMBLER  
CYLINDER LOCK

STEEL  
FLASH COLLAR

3000 PSI CONCRETE

SHARPWOOD WEDGES  
(REMOVE AFTER  
TAMPING SAND)

FOUNDATION SLEEVE-16 GA.  
HOT DIP GALVANIZED STEEL

BLACK ASPHALTUM PAINT  
ON OUTSIDE SURFACES  
OF POLE BELOW GRADE

DRY SAND THOROUGHLY  
TAMPED AFTER ALIGNING  
POLE

STEEL CENTERING WEDGES

1/2" STEEL BASE PLATE

1/2" STEEL SUPPORT PLATE-  
WELDED TO GROUND SPIKE

7 X 19 MIL-  
CS424 STAINLESS  
STEEL AIRCRAFT  
CABLE

EXPOSED HEIGHT

OVERALL LENGTH

ACCESS DOOR 6'-0"

FOUNDATION

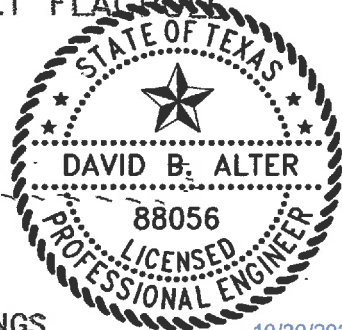
PROJECT NO.	GROUND SET DIM. SECTION STEEL FLAGPOLE
LOC. 1540-I 30	EXP. HT. 150' OVERALL HT. 116.5' NO. OF SEC. 5
ARCHT:	BUTT. DIA. 24" TOP DIA. 10 3/4" WALL THICKNESS .375"
CONT R:	SHIP IN 5 SEC. FINISH: Powder Coat
CUST: Clay Cooley Hyundai Rental	White

EXPOSED	OVERALL	TOP	BOTTOM	BUTT WALL	SHIP	NO. OF	FLAG	SHIPPING
HEIGHT	LENGTH	DIAMETER	DIAMETER	THICKNESS	SECTIONS	SECTIONS	SIZE	WEIGHT
150'	116.5'	10 3/4"	24"	.375"	5	5	40' X 80'	15,000#

**WARNING:** Extreme Caution should be exercised when installing flagpoles near overhead power lines, or in the vicinity of buried cables.



# DIMINISHING SECTION STEEL CONCEALED HALYARD GROUND SET FLAGPOLE



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10/30/2025

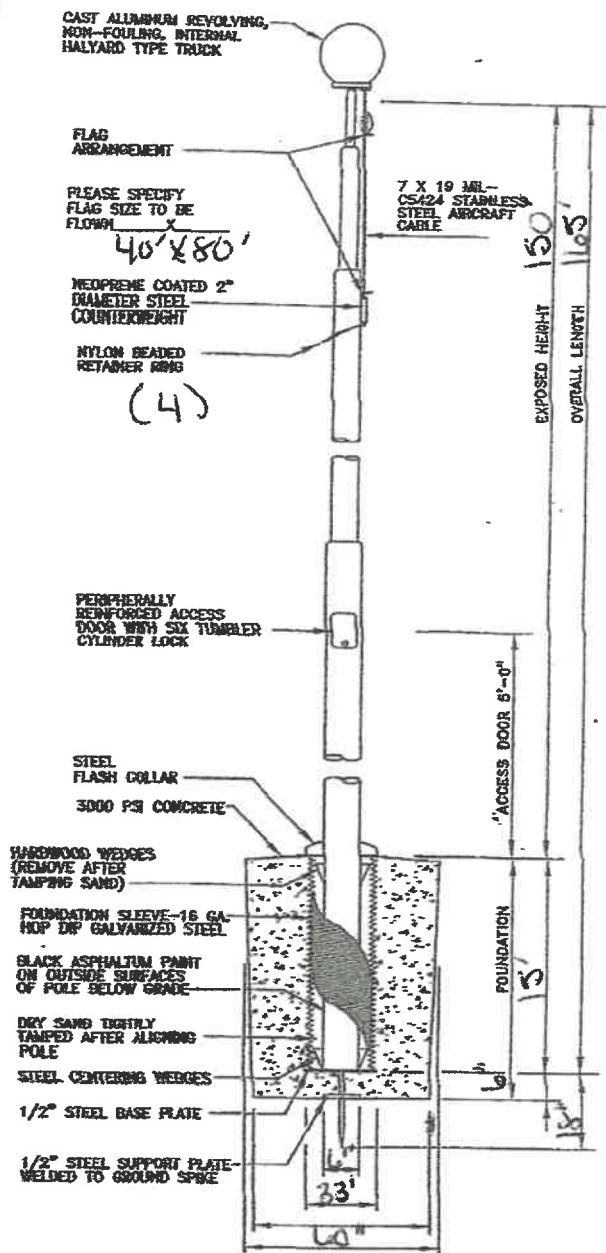
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US Flag,  
40' x 80'

PROJECT NO.	GROUND SET DIM. SECTION STEEL FLAGPOLE
LOC. 1540-I 30	EXP. HT. 150' OVERALL HT. 165' NO. OF SEC. 5
ARCHT:	BUTT. DIA. 24" TOP DIA. 10 3/4" WALL THICKNESS .375"
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HEIGHT	LENGTH	DIAMETER	DIAMETER	THICKNESS	SECTIONS	SECTIONS	SIZE	WEIGHT
150'	165'	10 3/4"	24"	.375"	5	5	40' x 80'	15,000 lbs

**WARNING:** Extreme Caution should be exercised when installing flagpoles near overhead power lines, or in the vicinity of buried cables.



# STRUCTURAL CALCULATIONS

*Project:*

**Clay Cooley Hyundai Flagpole & Footing**

1540 I-30

Rockwall, TX 75087

*Project Number:* 14520

*Prepared For:*

**Symonds Flags and Poles**

250 W. Airport Freeway

Irving, TX 75062

*Date:*

October 2025

*Prepared By:*

**Adam Pope, EIT**

*Reviewed By:*

**David B. Alter, PE**



10/30/2025

**Ensign Engineering**

45 West 10000 South, Suite 500

Sandy, Utah 84070

P: (801) 255-0529

F: (801) 255-4449

ensigneng.com

**ENSIGN**  
THE STANDARD IN ENGINEERING



## ASCE Hazards Report

**Address:**

1540 E Interstate 30  
Rockwall, Texas  
75087

**Standard:**

ASCE/SEI 7-22

**Risk Category:** II

**Soil Class:**

Default

**Latitude:**

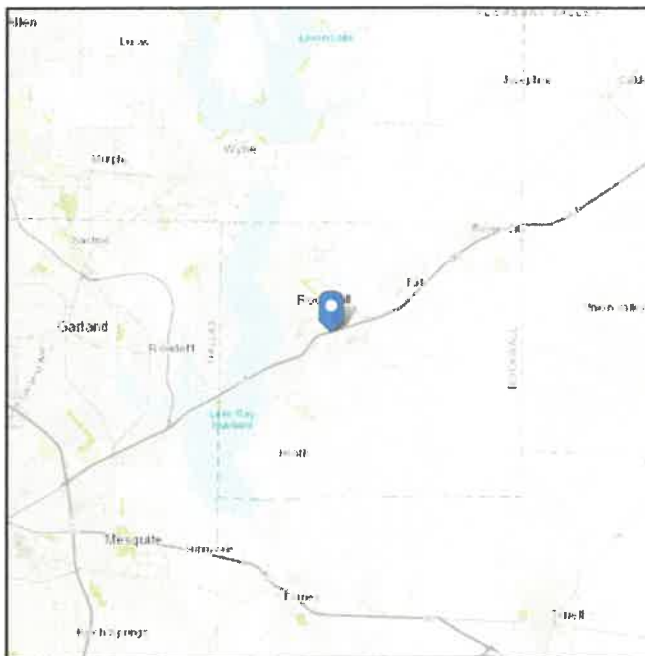
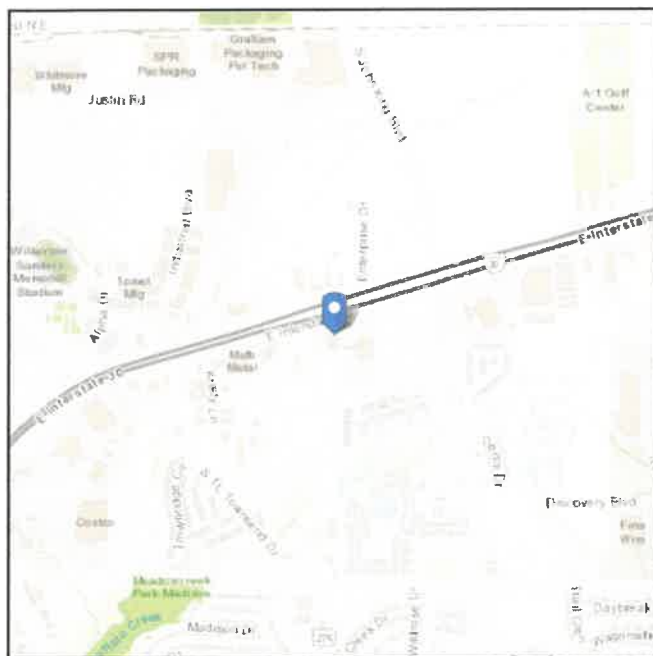
32.914362

**Longitude:**

-96.436478

**Elevation:**

573.0197118668139 ft  
(NAVD 88)



## Wind

**Results:**

Wind Speed	105 Vmph
10-year MRI	75 Vmph
25-year MRI	80 Vmph
50-year MRI	85 Vmph
100-year MRI	90 Vmph
300-year MRI	100 Vmph
700-year MRI	105 Vmph
1,700-year MRI	115 Vmph
3,000-year MRI	117 Vmph
10,000-year MRI	126 Vmph
100,000-year MRI	145 Vmph
1,000,000-year MRI	162 Vmph

Data Source:

ASCE/SEI 7-22, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2

Date Accessed:

Wed Oct 29 2025





Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-22 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years). Values for 10-year MRI, 25-year MRI, 50-year MRI and 100-year MRI are Service Level wind speeds, all other wind speeds are Ultimate wind speeds.

Site is not in a hurricane-prone region as defined in ASCE/SEI 7-22 Section 26.2.



The ASCE Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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## GENERAL PROJECT INFORMATION

Latitude: 32.91 North (Approximate)  
Longitude: -96.44 West (Approximate)

Client: Symonds Flags and Poles

## PROJECT DESCRIPTION

Structural Calculations for  $h := 150 \text{ ft}$  flag pole footing

## GENERAL DESIGN CRITERIA

Structure Type: Flag Pole  
Design Code: 2021 IBC / FP1001-07  
Risk Category: II

## DESIGN LOADS

### Wind Loads:

Wind Speed:  $WS := 105$  mph - 3 second gust (strength level)  
Exposure: C

## FOUNDATION CRITERIA & SPECIFICATIONS

Soils Report: Company: Presumptive  
Date: N/A  
Report/Project Number: N/A  
Contact: N/A

Allowable Bearing Pressure: 1500 psf

Passive Pressure:  $p_{\text{passive}} := 2 \cdot 150 \frac{\text{psf}}{\text{ft}} = 300 \frac{\text{psf}}{\text{ft}}$   
Increase Per IBC 1806.3.4

Coefficient of Friction,  $\mu$ : 0.35

Foundation Type:

Footing Type: Pole Footing

## FOUNDATION CRITERIA & SPECIFICATIONS

### CONCRETE & REINFORCING STEEL SPECIFICATIONS

Concrete Strength,  $f'_c$ :

Footings/Foundation Walls: 3000 psi

Steel Pole Segments: API 5Lx46 Grade B ( $f_y=46$  ksi)

## Flagpole Design

### Flagpole Geometry

$$h = 150 \text{ ft}$$

Total Pole Height from the Ground

$$w_1 := 24 \text{ in} \quad w_3 := 15 \text{ in}$$

Pole Widths

$$w_2 := 19.5 \text{ in} \quad w_4 := 10.75 \text{ in}$$

$$h_1 := 50 \text{ ft} \quad h_3 := 40 \text{ ft}$$

Respective Pole Heights

$$h_2 := 40 \text{ ft} \quad h_4 := 20 \text{ ft}$$

### Wind loads on pole (Per NAAMM / FP1001-07)

$$WS = 105$$

Wind speed (mph)

$$V := \sqrt{0.6} \cdot WS$$

Wind velocity ASCE 7-16 (mph) adjusted to ASCE 7-05

$$G := 1.14$$

Gust effect factor recommended for flag poles

### Segment 4 Calculations

$$z_4 := 140 \text{ ft}$$

Avg. Distance From Ground (ft)

$$d_4 := \frac{w_4}{\text{ft}} = 0.896$$

$$C_{deq4} := \frac{129}{(V \cdot d_4)^{1.3}} = 0.49$$

Drag coefficient FP1001 Table 3.2.4.

$$C_{d4} := \text{if}(C_{deq4} < 0.45, 0.45, \text{if}(C_{deq4} > 1.1, 1.1, C_{deq4})) = 0.489$$

$$C_{h4} := 2.01 \cdot \left( \frac{z_4}{900 \text{ ft}} \right)^{\frac{2}{9.5}} = 1.36$$

Height coefficient FG1001 Eq. 2

$$P_4 := 0.00256 \cdot V^2 \cdot C_{d4} \cdot C_{h4} \cdot G = 12.826$$

Wind Pressure (psf) FP1001 Eq. 1

$$A_4 := h_4 \cdot w_4 = 17.917 \text{ ft}^2$$

Pole Area

$$F_4 := P_4 \cdot \text{psf} \cdot A_4 = 229.799 \text{ lbf}$$

Wind Force as a Point Load (Lbs)

$$M_{b4} := A_4 \cdot P_4 \cdot \text{psf} \cdot z_4 = 32.172 \text{ kip} \cdot \text{ft}$$

Moment @ base resulting from segment 4



### Segment 3 Calculations

$$z_3 := 110 \text{ ft}$$

Avg. Distance From Ground (ft)

$$d_3 := \frac{w_3}{\text{ft}} = 1.25$$

$$C_{\text{deq}3} := \frac{129}{(V \cdot d_3)^{1.3}} = 0.32$$

Drag coefficient FP1001 Table 3.2.4.

$$C_{d3} := \text{if}(C_{\text{deq}3} < 0.45, 0.45, \text{if}(C_{\text{deq}3} > 1.1, 1.1, C_{\text{deq}3})) = 0.45$$

$$C_{h3} := 2.01 \cdot \left( \frac{z_3}{900 \text{ ft}} \right)^{\frac{2}{9.5}} = 1.29$$

Height coefficient FG1001 Eq. 2

$$P_3 := 0.00256 \cdot V^2 \cdot C_{d3} \cdot C_{h3} \cdot G = 11.218$$

Wind Pressure (psf) FP1001 Eq. 1

$$A_3 := h_3 \cdot w_3 = 50 \text{ ft}^2$$

Pole Area

$$F_3 := P_3 \cdot \text{psf} \cdot A_3 = 560.886 \text{ lbf}$$

Wind Force as a Point Load (Lbs)

$$M_{b3} := A_3 \cdot P_3 \cdot \text{psf} \cdot z_3 = 61.697 \text{ kip} \cdot \text{ft}$$

Moment @ base resulting from segment 3

### Segment 2 Calculations

$$z_2 := 70 \text{ ft}$$

Avg. Distance From Ground (ft)

$$d_2 := \frac{w_2}{\text{ft}} = 1.625$$

$$C_{\text{deq}2} := \frac{129}{(V \cdot d_2)^{1.3}} = 0.23$$

Drag coefficient FP1001 Table 3.2.4.

$$C_{d2} := \text{if}(C_{\text{deq}2} < 0.45, 0.45, \text{if}(C_{\text{deq}2} > 1.1, 1.1, C_{\text{deq}2})) = 0.45$$

$$C_{h2} := 2.01 \cdot \left( \frac{z_2}{900 \text{ ft}} \right)^{\frac{2}{9.5}} = 1.17$$

Height coefficient FG1001 Eq. 2

$$P_2 := 0.00256 \cdot V^2 \cdot C_{d2} \cdot C_{h2} \cdot G = 10.2$$

Wind Pressure (psf) FP1001 Eq. 1

$$A_2 := h_2 \cdot w_2 = 65 \text{ ft}^2$$

Pole Area

$$F_2 := P_2 \cdot \text{psf} \cdot A_2 = 662.968 \text{ lbf}$$

Wind Force as a Point Load (Lbs)

$$M_{b2} := A_2 \cdot P_2 \cdot \text{psf} \cdot z_2 = 46.408 \text{ kip} \cdot \text{ft}$$

Moment @ base resulting from segment 2

### Segment 1 Calculations

$$z_1 := 25 \text{ ft}$$

Avg. Distance From Ground (ft)

$$d_1 := \frac{w_1}{\text{ft}} = 2$$

$$C_{\text{deq1}} := \frac{129}{(V \cdot d_1)^{1.3}} = 0.17$$

Drag coefficient FP1001 Table 3.2.4.

$$C_{d1} := \text{if}(C_{\text{deq1}} < 0.45, 0.45, \text{if}(C_{\text{deq1}} > 1.1, 1.1, C_{\text{deq1}})) = 0.45$$

$$C_{h1} := \text{if}\left(z_1 < 16.4 \text{ ft}, 0.86, 2.01 \cdot \left(\frac{z_1}{900 \text{ ft}}\right)^{\frac{2}{9.5}}\right) = 0.95$$

Height coefficient FG1001 Eq. 2 (if  $z_1 < 16.4'$   $Ch = 0.86$ )

$$P_1 := 0.00256 \cdot V^2 \cdot C_{d1} \cdot C_{h1} \cdot G = 8.212$$

Wind Pressure (psf) FP1001 Eq. 1

$$A_1 := h_1 \cdot w_1 = 100 \text{ ft}^2$$

Pole Area

$$F_1 := P_1 \cdot \text{psf} \cdot A_1 = 821.184 \text{ lbf}$$

Wind Force as a Point Load (Lbs)

$$M_{b1} := A_1 \cdot P_1 \cdot \text{psf} \cdot z_1 = 20.53 \text{ kip} \cdot \text{ft}$$

Moment @ base resulting from segment 1

$$M_{\text{bpole}} := M_{b1} + M_{b2} + M_{b3} + M_{b4} = 160.807 \text{ kip} \cdot \text{ft}$$

### Wind loads on flag (per NAAMM / FP1001-07)

$$h_f := 40 \text{ ft} \quad w_f := 80 \text{ ft} \quad z_f := 130 \text{ ft}$$

Flag geometry

$$C_{hf} := 2.01 \cdot \left(\frac{z_f}{900 \text{ ft}}\right)^{\frac{2}{9.5}} = 1.34$$

Height coefficient FG1001 Eq. 2

$$A_f := h_f \cdot w_f = 3200 \text{ ft}^2$$

Flag Area

$$F_f := 0.0014 \frac{\text{lbf}}{\text{ft}} \cdot V^2 \cdot \sqrt{A_f} \cdot C_{hf} \cdot G = 798.783 \text{ lbf}$$

Force on flag FG1001 Eq. 5  
(polyester flag = worst case)

$$M_{bf} := F_f \cdot z_f = 103.842 \text{ kip} \cdot \text{ft}$$

Moment @ base resulting from the flag

$$F_d := \frac{F_f}{h_f} = 19.97 \text{ plf}$$

### Moment @ Base

$$M_{\text{total}} := M_{\text{bf}} + M_{\text{bpole}} = 264.648 \text{ kip} \cdot \text{ft}$$

### Moment @ Base of Segment 2

$$M_2 := F_4 \cdot (z_4 - h_1) + F_3 \cdot (z_3 - h_1) + F_2 \cdot (z_2 - h_1) + F_f \cdot (z_f - h_1)$$

$$M_2 = 131.497 \text{ kip} \cdot \text{ft}$$

### Moment @ Base of Segment 3

$$M_3 := F_4 \cdot (z_4 - h_1 - h_2) + F_3 \cdot (z_3 - h_1 - h_2) + F_f \cdot (z_f - h_1 - h_2)$$

$$M_3 = 54.659 \text{ kip} \cdot \text{ft}$$

### Moment @ Base of Segment 4

$$M_4 := F_4 \cdot (z_4 - h_1 - h_2 - h_3) + F_d \cdot h_4 \cdot \left( \frac{h_4}{2} \right) = 6.292 \text{ kip} \cdot \text{ft}$$

Worst loading case for segment 4  
= full length of segment

### Section Modulus of Each Segment

$$T := 0.375 \text{ in}$$

$$S_1 := \frac{\pi \cdot (w_1^4 - (w_1 - 2T)^4)}{32 \cdot w_1} = 161.858 \text{ in}^3$$

$$S_3 := \frac{\pi \cdot (w_3^4 - (w_3 - 2T)^4)}{32 \cdot w_3} = 61.461 \text{ in}^3$$

$$S_2 := \frac{\pi \cdot (w_2^4 - (w_2 - 2T)^4)}{32 \cdot w_2} = 105.696 \text{ in}^3$$

$$S_4 := \frac{\pi \cdot (w_4^4 - (w_4 - 2T)^4)}{32 \cdot w_4} = 30.637 \text{ in}^3$$

$$Z_1 := \frac{w_1^3 - (w_1 - 2T)^3}{6} = 209.32 \text{ in}^3$$

$$Z_3 := \frac{w_3^3 - (w_3 - 2T)^3}{6} = 80.227 \text{ in}^3$$

$$Z_2 := \frac{w_2^3 - (w_2 - 2T)^3}{6} = 137.18 \text{ in}^3$$

$$Z_4 := \frac{w_4^3 - (w_4 - 2T)^3}{6} = 40.383 \text{ in}^3$$

### Bending Stress Check of Each Segment (16.1-F8 AISI)

$$E := 29000 \text{ ksi} \quad F_y := 42 \text{ ksi} \quad \Omega_b := 1.67$$

ALLOWABLE FACTORED  
STRENGTH PER CODE IS 40  
KSI FOR 42 KSI STEEL



#### Segment 4

$$D_t := \frac{w_4}{T} = 28.667 \quad F_8 := \frac{0.45 \cdot E}{F_y} = 310.714 \quad F_{cr} := \frac{0.33 \cdot E}{(D_t)} = 333.837 \text{ ksi}$$

$$F_{8\text{apply}} := \text{if}(D_t < F_8, \text{"PROCEED"}, \text{"REVISE"}) = \text{"PROCEED"}$$

#### Yielding

$$M_{ny} := F_y \cdot Z_4 = 141.34 \text{ kip} \cdot \text{ft} \quad \frac{\Omega_b \cdot M_4}{M_{ny}} = 0.074 \quad \text{Must be } < 1$$

#### Local Buckling

$$\lambda_p := 0.07 \cdot \frac{E}{F_y} = 48.333 \quad \lambda_r := 0.31 \cdot \frac{E}{F_y} = 214.048$$

$$\text{Check}_c := \text{if}(D_t < \lambda_p, \text{"Compact"}, \text{if}(D_t > \lambda_r, \text{"Slender"}, \text{"Non-compact"})) = \text{"Compact"}$$

$$M_{nb} := \text{if}(\text{Check}_c = \text{"Compact"}, \text{"N/A"}, \text{if}(\text{Check}_c = \text{"Slender"}, F_{cr} \cdot S_4, \left(\frac{0.021 \cdot E}{D_t} + F_y\right) \cdot S_4)) = \text{"N/A"}$$

#### Segment 3

$$D_t := \frac{w_3}{T} = 40 \quad F_8 := \frac{0.45 \cdot E}{F_y} = 310.714 \quad F_{cr} := \frac{0.33 \cdot E}{(D_t)} = 239.25 \text{ ksi}$$

$$F_{8\text{apply}} := \text{if}(D_t < F_8, \text{"PROCEED"}, \text{"REVISE"}) = \text{"PROCEED"}$$

#### Yielding

$$M_{ny} := F_y \cdot Z_3 = 280.793 \text{ kip} \cdot \text{ft} \quad \frac{\Omega_b \cdot M_3}{M_{ny}} = 0.325 \quad \text{Must be } < 1$$

#### Local Buckling

$$\lambda_p := 0.07 \cdot \frac{E}{F_y} = 48.333 \quad \lambda_r := 0.31 \cdot \frac{E}{F_y} = 214.048$$

$$\text{Check}_c := \text{if}(D_t < \lambda_p, \text{"Compact"}, \text{if}(D_t > \lambda_r, \text{"Slender"}, \text{"Non-compact"})) = \text{"Compact"}$$

$$M_{nb} := \text{if}(\text{Check}_c = \text{"Compact"}, \text{"N/A"}, \text{if}(\text{Check}_c = \text{"Slender"}, F_{cr} \cdot S_3, \left(\frac{0.021 \cdot E}{D_t} + F_y\right) \cdot S_3)) = \text{"N/A"}$$

### Segment 2

$$D_t := \frac{W_2}{T} = 52 \quad F_8 := \frac{0.45 \cdot E}{F_y} = 310.714 \quad F_{cr} := \frac{0.33 \cdot E}{(D_t)} = 184.038 \text{ ksi}$$

$$F_{8\text{apply}} := \text{if}(D_t < F_8, \text{"PROCEED"}, \text{"REVISE"}) = \text{"PROCEED"}$$

### Yielding

$$M_{ny} := F_y \cdot Z_2 = 480.129 \text{ kip} \cdot \text{ft} \quad \frac{\Omega_b \cdot M_2}{M_{ny}} = 0.457 \quad \text{Must be } < 1$$

### Local Buckling

$$\lambda_p := 0.07 \cdot \frac{E}{F_y} = 48.333 \quad \lambda_r := 0.31 \cdot \frac{E}{F_y} = 214.048$$

$$\text{Check}_c := \text{if}(D_t < \lambda_p, \text{"Compact"}, \text{if}(D_t > \lambda_r, \text{"Slender"}, \text{"Non-compact"})) = \text{"Non-compact"}$$

$$M_{nb} := \text{if}\left(\text{Check}_c = \text{"Compact"}, \text{"N/A"}, \text{if}\left(\text{Check}_c = \text{"Slender"}, F_{cr} \cdot S_2, \left(\frac{0.021 \cdot E}{D_t} + F_y\right) \cdot S_2\right)\right) = 15221234.198 \frac{\text{lb} \cdot \text{ft}^2}{s^2}$$

### Segment 1

$$D_t := \frac{W_1}{T} = 64 \quad F_8 := \frac{0.45 \cdot E}{F_y} = 310.714 \quad F_{cr} := \frac{0.33 \cdot E}{(D_t)} = 149531.25 \text{ psi}$$

$$F_{8\text{apply}} := \text{if}(D_t < F_8, \text{"PROCEED"}, \text{"REVISE"}) = \text{"PROCEED"}$$

### Yielding

$$M_{ny} := F_y \cdot Z_1 = 732.621 \text{ kip} \cdot \text{ft} \quad \frac{\Omega_b \cdot M_{b\text{total}}}{M_{ny}} = 0.603 \quad \text{Must be } < 1$$

### Local Buckling

$$\lambda_p := 0.07 \cdot \frac{E}{F_y} = 48.333 \quad \lambda_r := 0.31 \cdot \frac{E}{F_y} = 214.048$$

$$\text{Check}_c := \text{if}(D_t < \lambda_p, \text{"Compact"}, \text{if}(D_t > \lambda_r, \text{"Slender"}, \text{"Non-compact"})) = \text{"Non-compact"}$$

$$M_{nb} := \text{if}\left(\text{Check}_c = \text{"Compact"}, \text{"N/A"}, \text{if}\left(\text{Check}_c = \text{"Slender"}, F_{cr} \cdot S_1, \left(\frac{0.021 \cdot E}{D_t} + F_y\right) \cdot S_1\right)\right) = 694.852 \text{ kip} \cdot \text{ft}$$

$$\frac{\Omega_b \cdot M_{b\text{total}}}{M_{nb}} = 0.636 \quad \text{Must be } < 1$$

At maximum wind, the pole with attached flag would not fail. However, the flag is not expected to stay attached with winds exceeding 50 mph. Calculations without the flag follow. The assumption is that the flag will rip from the pole before it is able to transfer maximum load to the pole.

**FLAG SHALL BE REMOVED BEFORE WIND SPEEDS REACH 75 MPH.**

Moment @ Base w/out Flag

$$M_{\text{totalwof}} := M_{\text{pole}} = 160.807 \text{ kip} \cdot \text{ft}$$

Moment @ base of segment 2 w/out flag

$$M_{2\text{wof}} := F_4 \cdot (z_4 - h_1) + F_3 \cdot (z_3 - h_1) + F_2 \cdot (z_2 - h_1) = 67.594 \text{ kip} \cdot \text{ft}$$

Moment @ base of segment 3 w/out flag

$$M_{3\text{wof}} := F_4 \cdot (z_4 - h_1 - h_2) + F_3 \cdot (z_3 - h_1 - h_2) = 22.708 \text{ kip} \cdot \text{ft}$$

Moment @ base of segment 4 w/out flag

$$M_{4\text{wof}} := F_4 \cdot (z_4 - h_1 - h_2 - h_3) = 2.298 \text{ kip} \cdot \text{ft}$$

Loads on steel sections less than when the flag is present. **Steel is adequate by inspection.**



## Footing Design

$$b := 4.5 \text{ ft}$$

*Diameter of footing in ft*

$$P' := F_1 + F_2 + F_3 + F_4 + F_f = 3073.62 \text{ lbf}$$

*Applied lateral force in lbs*

$$h' := \frac{M_{\text{total}}}{P'} = 86.103 \text{ ft}$$

*Distance in ft from ground surface to point of application of "P"*

$$S_1 := 1191.237 \text{ psf}$$

*Allowable Lateral soil bearing pressure as set forth in the IBC, Section 1806.2 based on depth of one-third the depth of embedment in psf*

\*Note to Engineer: Iterate Lateral Bearing Soil Pressure

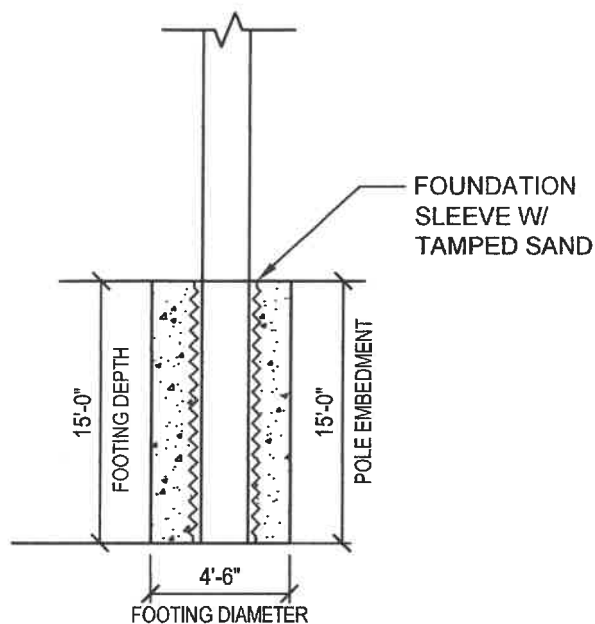
$$A := \frac{2.34 \cdot P'}{S_1 \cdot b} = 1.342 \text{ ft}$$

$$d := 0.5 \cdot A \cdot \left( 1 + \left( 1 + \left( \frac{4.36 \cdot h'}{A} \right)^{\frac{1}{2}} \right) \right) = 11.91 \text{ ft}$$

*Depth of embedment in earth in ft but not over 12 ft for purpose of computing lateral pressure (IBC 1807.3, EQ 18-1)*

Footing Depth to be: **12 ft** min

**WARNING: FLAG MUST BE  
REMOVED IF WINDS  
WILL EXCEED 75 MPH**

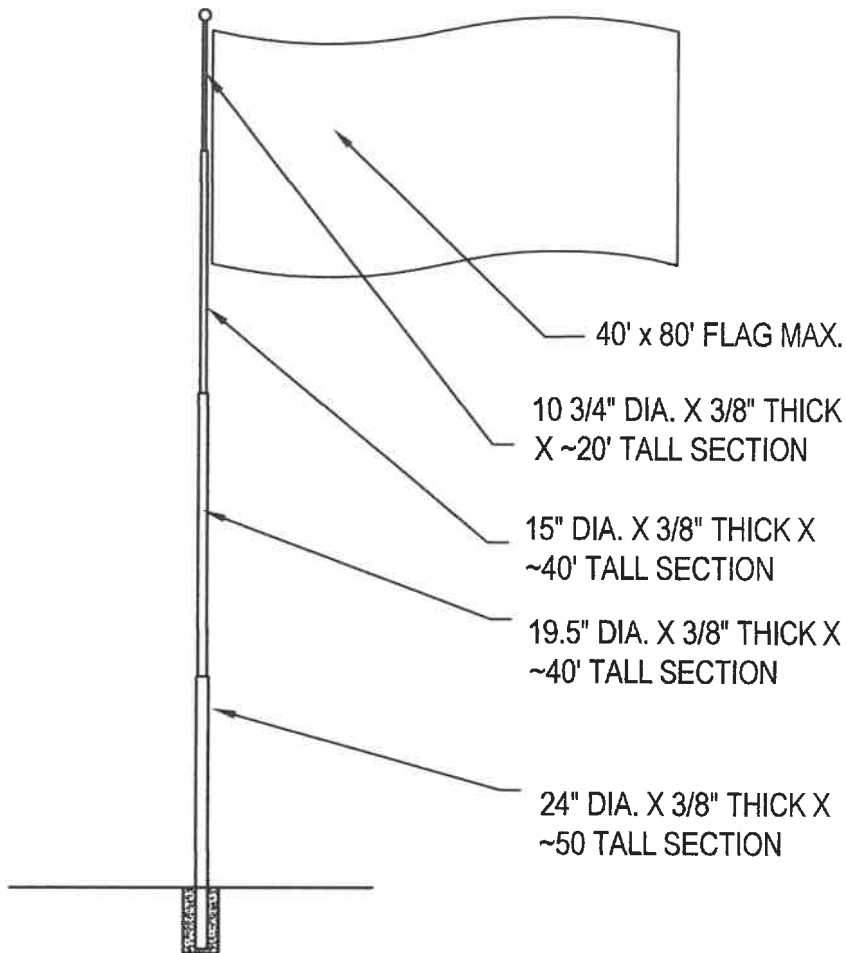


## FOOTING

FOUNDATION DESIGN IS BASED ON ASSUMED SOIL PROPERTIES. IF THE SOIL ENCOUNTERED IN THE EXCAVATION IS NOT SAND, SILTY SAND, CLAYER SAND, SILTY GRAVEL OR CLAYEY GRAVEL, PLEASE NOTIFY THE ENGINEER. ALSO, IF THE WATER TABLE IS ENCOUNTERED IN THE EXCAVATION, PLEASE CONTACT THE ENGINEER



10/30/2025



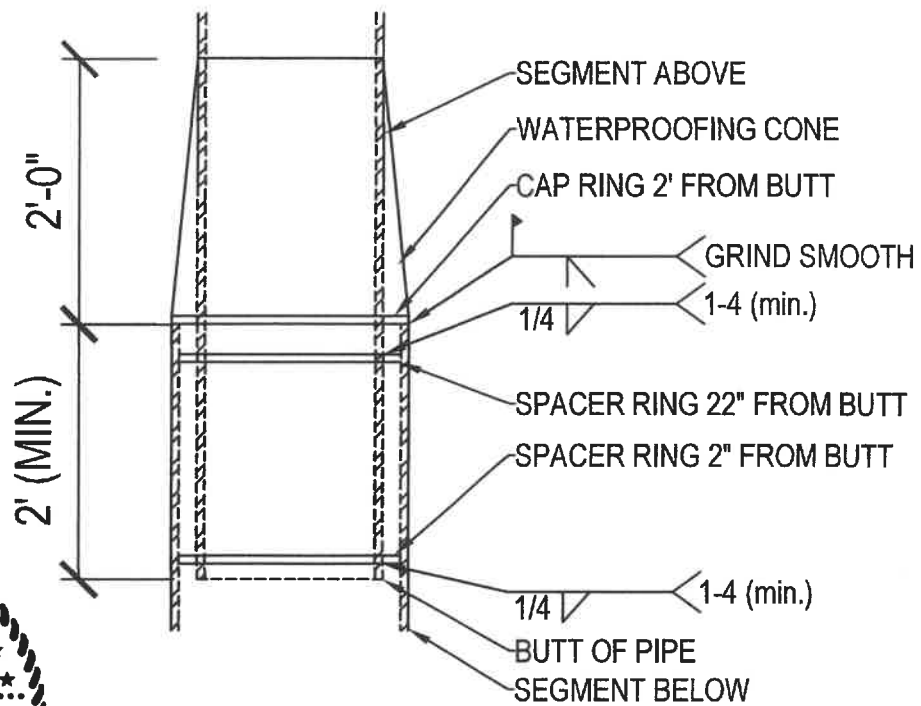
## ELEVATION

1

## FLAGPOLE

SCALE: N.T.S.

<p>SHEET:</p> <p><b>SK1</b></p>	<p>PROJECT NUMBER 14520</p> <p>DATE 10/29/2025</p> <p>DRAWN BY A. Pope</p> <p>PROJECT MANAGER D. Alter</p>	<p>PROJECT NAME:</p> <p><b>Clay Cooley Hyundai Flagpole &amp; Ftg</b></p> <p><b>1540 I-30</b></p> <p><b>Rockwall, TX 75087</b></p> <p>SHEET TITLE:</p>	<p><b>ENSIGN</b></p> <p>THE STANDARD IN ENGINEERING</p> <p>SALT LAKE CITY 45 WEST 10000 SOUTH Suite 500 Sandy, UT 84070 P. 801.255.0529 F. 801.255.4440 WWW.ENSIGNUTAH.COM</p>
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10/30/2025

1

## CONNECTION SLEEVE

SCALE: N.T.S.

SHEET:

SK2

PROJECT NUMBER  
14520  
DATE  
10/29/2025  
DRAWN BY  
A. Pope  
REVIEWED BY  
D. Alter

PROJECT NAME:

**Clay Cooley Hyundai Flagpole & Ftg**  
1540 I-30  
Rockwall, TX 75087

SHEET TITLE:

**ENSIGN**  
THE STANDARD IN ENGINEERING

SALT LAKE CITY  
45 WEST 10000 SOUTH  
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Sandy, UT 84070  
P. 801.255.0529  
F. 801.255.4449  
WWW.ENSIGNUTAH.COM



# **DIMINISHING SECTION STEEL CONCEALED HALYARD GROUND SET FLAGPOLE**

## **STANDARD FITTINGS**

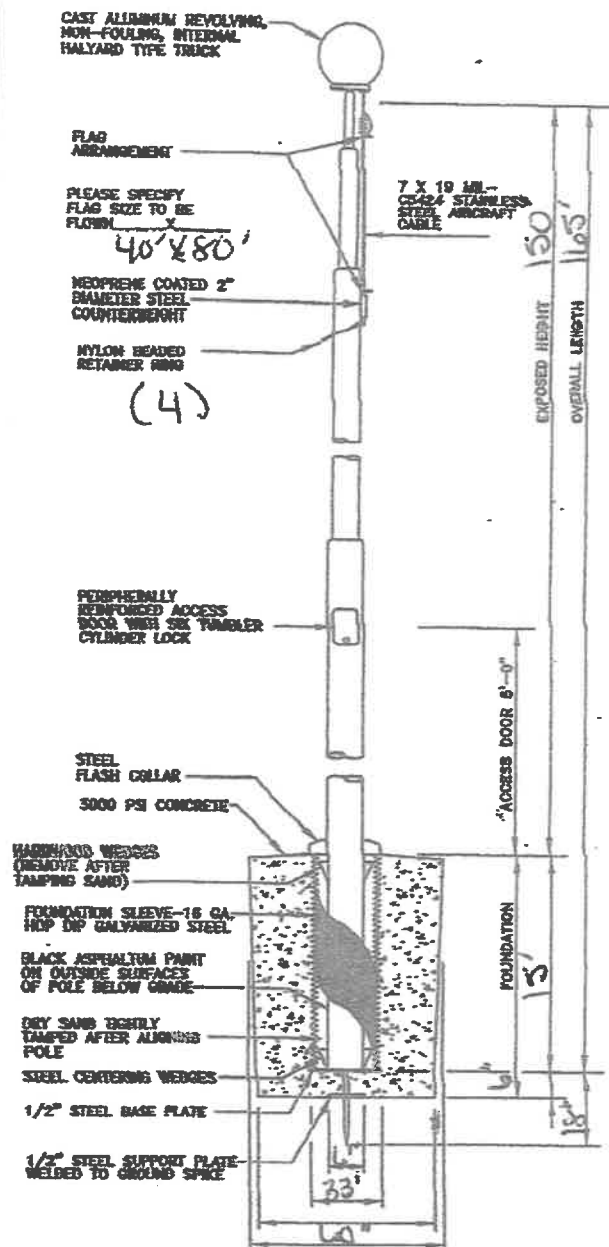
**FINIAL:** (The flagpole division of NAAMM recommends that no Finial, other than the internal Halyard Truck itself be used on the Concealed Halyard Flagpole.)

**TRUCK:** Heavy duty, cast aluminum, internal halyard type, revolving non-fouling, gold powder coated, equipped with upper and lower sealed bearings assemblies and a stainless steel spindle for flagpoles up to 130'. Larger flagpoles will come with two spun steel hemispheres, automotive type sealed bearing assemblies and a welded in steel spindle.

**HALYARD:** Halyard and flag arrangement are 1/8" and 3/16" stainless steel aircraft cable, with attachment ends crimped over 1/8" stainless steel yokes and joined by a stainless steel quicklink. Cable is two piece, joined by a stainless steel swivel at mid-point. The flag arrangement is sized to accommodate an appropriate size flag and is supplied complete with two heavy duty swivel snaps, neoprene coated counterweight, and beaded nylon retainer rings.

**WINCH:** Stainless steel, direct drive, accessible for maintenance only through a reinforced access opening, which is covered by a removable door finished to match flagpole shaft and contains a six tumbler cylinder lock. Winch is gearless, operable only by a removable crank handle and locks in any position upon removal of the crank handle.

**FOUNDATION TUBE:** Fabricated from 16GA. galvanized steel, with a steel base plate whose square dimension is 4" larger than the I.D. of the sleeve. A setting plate 6" square is securely welded to the ground spike 6" below the base plate. The ground spike is 3/4" diameter and 36" in length.



PROJECT NO.	GROUND SET DIM. SECTION STEEL FLAGPOLE		
LOC. 1540-I 30	EXP. HT. 150'	OVERALL HT. 116.5'	NO. OF SEC. 5
ARCHT:	BUTT. DIA. 24"	TOP DIA. 10 3/4"	WALL THICKNESS 3/16"
CONT. R:	SHIP IN 5	SEC.	FINISH: Powder Coat
CUST: Clay Cooley Hyundai	Rokwell	White	

EXPOSED	OVERALL	TOP	BOTTOM	BUTT WALL	SHIP	NO. OF	FLAG	SHIPPING
HEIGHT	LENGTH	DIAMETER	DIAMETER	THICKNESS	SECTIONS	SECTIONS	SIZE	WEIGHT
150'	116.5'	10 3/4"	24"	.375"	5	5	40' x 80'	15,000 lbs

**WARNING:** Extreme Caution should be exercised when installing flagpoles near overhead power lines, or in the vicinity of buried cables.

# DIMINISHING SECTION STEEL CONCEALED HALYARD GROUND SET FLAGPOLE

## STANDARD FITTINGS

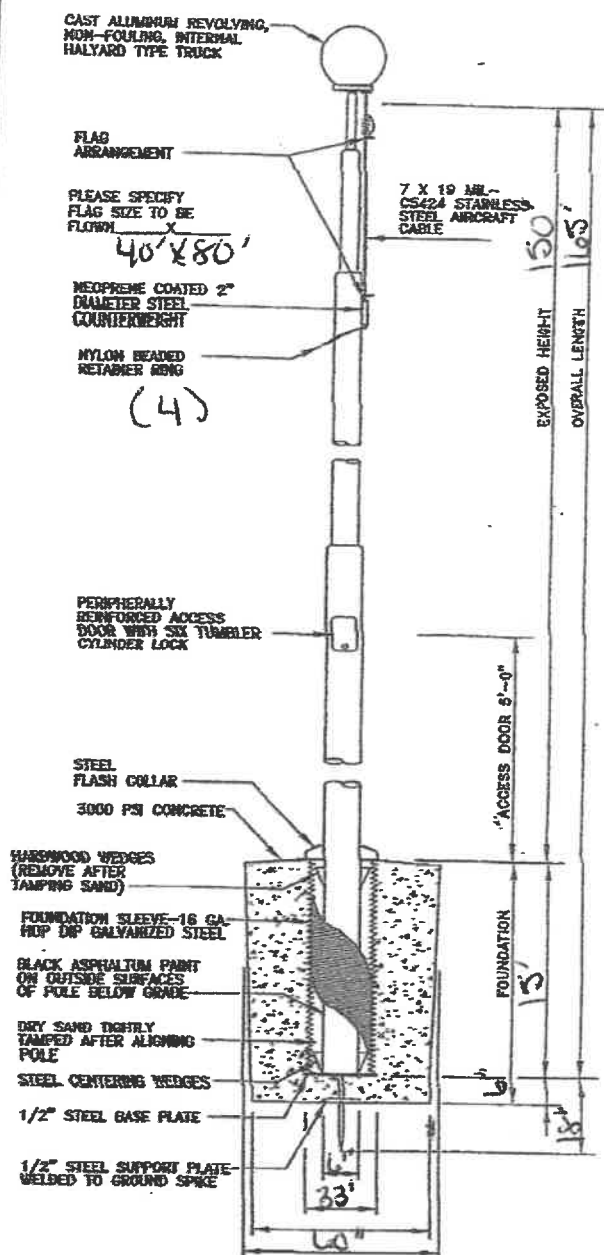
**FINIAL:** (The flagpole division of NAAMM recommends that no Finial, other than the Internal Halyard Truck itself be used on the Concealed Halyard Flagpole.)

**TRUCK:** Heavy duty, cast aluminum, internal halyard type, revolving non-fouling, gold powder coated, equipped with upper and lower sealed bearings assemblies and a stainless steel spindle for flagpoles up to 130'. Larger flagpoles will come with two spun steel hemispheres, automotive type sealed bearing assemblies and a welded in steel spindle.

**HALYARD:** Halyard and flag arrangement are 1/8" and 3/16" stainless steel aircraft cable, with attachment ends crimped over 1/8" stainless steel yokes and joined by a stainless steel quicklink. Cable is two piece, joined by a stainless steel swivel at mid-point. The flag arrangement is sized to accommodate an appropriate size flag and is supplied complete with two heavy duty swivel snaps, neoprene coated counterweight, and beaded nylon retainer rings.

**WINCH:** Stainless steel, direct drive, accessible for maintenance only through a reinforced access opening, which is covered by a removable access door finished to match flagpole shaft and contains a six tumbler cylinder lock. Winch is gearless, operable only by a removable crank handle and locks in any position upon removal of the crank handle.

**FOUNDATION TUBE:** Fabricated from 16GA. galvanized steel, with a steel base plate whose square dimension is 4" larger than the I.D. of the sleeve. A setting plate 6" square is securely welded to the ground spike 6" below the base plate. The ground spike is 3/4" diameter and 36" in length.



PROJECT NO.	GROUND SET DIM. SECTION STEEL FLAGPOLE		
LOC. 1540-130	EXP. HT. 150'	OVERALL HT. 165'	NO. OF SEC. 5
ARCHT:	BUTT. DIA. 24"	TOP DIA. 10 3/4"	WALL THICKNESS .375"
CONT R:	SHIP IN 5	SEC.	FINISH: Powder Coat
CUST: ClayCooley Hyundai Rental			White

EXPOSED	OVERALL	TOP	BOTTOM	BUTT WALL	SHIP	NO. OF	FLAG	SHIPPING
HEIGHT	LENGTH	DIAMETER	DIAMETER	THICKNESS	SECTIONS	SECTIONS	SIZE	WEIGHT
150'	165'	10 3/4"	24"	.375"	5	5	40'x80'	15,000 lbs

**WARNING:** Extreme Caution should be exercised when installing flagpoles near overhead power lines, or in the vicinity of buried cables.

US Flag,  
40' x 80'