

96-37  
Target

Page 1 of 4

City of Rockwall (3/87)

APPLICATION AND FINAL PLAT CHECKLIST

Date 5-22-96

Name of Proposed Development STEGER TOWNE CROSSING PHASE I.

Name of Developer STEGER TOWNE CROSSING, L.P.

Address 5025 ARAPAHO ROAD, 407 Phone 214/789-2977  
DALLAS, TEXAS 75248

Owner of Record 740/3097, LIMITED PARTNERSHIP, A TEXAS LIMITED PARTNERSHIP  
O.L. STEGER, III, GENERAL PARTNER

Address 504 W. RUSK, ROCKWALL, TEXAS 75087 Phone 214/722-3334

Name of Land Planner/Surveyor/Engineer LAWRENCE A. CATES & ASSOCIATES, INC.

Address 14200 MIDWAY ROAD, 122 Phone 214/385-2272  
DALLAS, TEXAS 75244

Total Acreage 32.4557 AC. Current Zoning A & C

Number of Lots/Units NINE (9)

Signed *Lawrence A. Cates*

The Final Plat shall generally conform to the Preliminary Plat, as approved by the City Council and shall be drawn to legibly show all data on a satisfactory scale, usually not smaller than one inch equals 100 feet. The Final Plat shall be submitted on a drawing which is 18" x 24".

The following Final Plat Checklist is a summary of the requirements listed under Section VIII of the Rockwall Subdivision Ordinance. Section VIII should be reviewed and followed when preparing a Final Plat. The following checklist is intended only as a reminder and a guide for those requirements.

Information

Provided of	Not
<u>Shown on Plat</u>	<u>Applicable</u>

XX \_\_\_\_\_

1. Title or name of development, written and graphic scale, north point, date of plat and key map

XX \_\_\_\_\_

2. Location of the development by City, County and State.

Page 2 of 4

\_\_\_\_\_ XX \_\_\_\_\_

3. Location of development tied to a USGS monument, Texas highway monument or other approved benchmark

XX \_\_\_\_\_

4. Accurate boundary survey and property description with tract boundary lined indicated by heavy lines

\_\_\_\_\_ XX \_\_\_\_\_

5. If no engineering is provided show contours of 5 ft. intervals

XX \_\_\_\_\_

6. Accurate plat dimensions with all engineering information necessary to reproduce plat on the ground

XX \_\_\_\_\_

7. Approved name and right-of-way width of each street, both within an adjacent to the development

XX \_\_\_\_\_

8. Locations, dimensions and purposes of any easements or other rights-of-way

XX \_\_\_\_\_

9. Identification of each lot or site and block by letter and number and building lines

\_\_\_\_\_ \_\_\_\_\_

10. Record owners of contiguous parcels of unsubdivided land, names and lot patterns of contiguous subdivisions, approved Concept Plans, reference recorded subdivision plats or adjoining platted land by record name and by deed record volume and page

XX \_\_\_\_\_

11. Boundary lines, dimensions and descriptions of open spaces to be dedicated for public use of the inhabitants of the development

\_\_\_\_\_ \_\_\_\_\_

12. Certificate of dedication of all streets, alleys, parks and other public uses signed by the owner or owners (see wording)

Page 3 of 4

x \_\_\_\_\_

13. Designation of the entity responsible for the operation and maintenance of any commonly held property and a waiver releasing the City of such responsibility, a waiver releasing the City for damages in establishment or alteration of graded (see wording)

x \_\_\_\_\_

14. Statement of developer responsibility for storm drainage improvements (see wording)

x \_\_\_\_\_

15. Instrument of dedication or adoption signed by the owner or owners (see wording)

x \_\_\_\_\_

16. Space for signatures attesting approval of the plat (see wording)

y \_\_\_\_\_

17. Seal and signature of the surveyor and/or engineer responsible for surveying the development and/or the preparation of the plat (see wording)

x \_\_\_\_\_

18. Compliance with all special requirements developed in preliminary plat review

xx \_\_\_\_\_

19. Statements indicating that no building permits will be issued until all public improvements are accepted by the City (see wording)

\_\_\_\_\_

20. Submit along with plat a calculation sheet indicating the area of each lot

\_\_\_\_\_ x

21. Attach copy of any proposed deed restrictions for proposed subdivision

**City of Rockwall  
Planning And Zoning Commission**

**Agenda Date:** June 11, 1996

**Applicant:** Weber & Company

**Agenda Item:** **96-37-FP/SP/LP** Consider approval of a request from Weber and Company for a final plat, site plan and landscape plan for Steger Towne Crossing Phase I.

**Action Needed:** Discuss and consider approval of the request

**Background Information:**

**PLAT**  
This property is part of the Steger Towne Crossing development. This plat contains 9 lots as part of the first phase of the development. Cross access and fire lanes are provided to serve this site. The proposed anchor tenants on the property include Target and Albertson's. We are finalizing the 15' R.O.W dedication for FM-740.

**LANDSCAPE PLAN**  
At the work session there was some discussion regarding clustering the trees. Staff reviewed this and was unable to find an acceptable alternative that would brake up this massive parking lot.

Additional screening has been added to the rear of the Albertson's store and along the chain link fence behind the Target on the north side of Steger Towne Drive.

**SITE PLAN**  
The site plan meets the parking and site requirements for the commercial zoning district.

**Recommendation:** Staff recommends approval of this request with the following conditions;

1. 15' R.O.W. dedication for FM-740 prior to the plat being filed.
2. Approval of the engineering plans.

3. The existing temporary fire lane and access drives on the Food Lion site be abandon or reconfigured prior to the construction of that portion of Steger Towne Drive.
4. The developer of the Steger Towne Crossing allow the property behind Albertson's to petition for cross access to the center.

**CITY OF ROCKWALL  
City Council Agenda**

**Agenda Date:** June 17, 1996

**Agenda No.** V.F.

**Agenda Item:** **PZ-37-FP/SP/LP** Hold Public Hearing Regarding Request for Sign Variance Consider Approval of a Request from Weber and Company for a Final Plat, Site Plan, Landscape Plan and Sign Plan for Stegar Towne Crossing Phase I Generally located on the east side of FM-740 South of I-30 and Take Any Necessary Action.

**Item Generated By:**

**Action Needed:**

**Background Information:**

**Attachments:**

1. Copy of Planning & Zoning Recommendations

**City of Rockwall  
City Council**

**Agenda Date:** June 17, 1996

**Applicant:** Weber & Company

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**Action Needed:** Discuss and consider approval of the request

**Background Information:**

**PLAT**

This property is part of the Steger Towne Crossing development. This plat contains 9 lots as part of the first phase of the development. Cross access and fire lanes are provided to serve this site. The proposed anchor tenants on the property include Target and Albertson's. We are finalizing the 15' R.O.W dedication for FM-740. The right of way for FM-740 will need to be dedicated by separate instrument.

**LANDSCAPE PLAN**

At the Commission work session there was some discussion regarding clustering the trees instead of the proposed diamond tree islands. Staff reviewed this and was unable to find an acceptable alternative that would brake up this massive parking lot.

Additional screening has been added to the rear of the Albertson's store and along the chain link fence behind the Target on the north side of Steger Towne Drive.

This landscaping plan was revised to match the Boston Market landscaping to achieve a uniform planting along Steger Towne Drive.

**SITE PLAN**

The site plan meets the parking and site requirements for the commercial zoning district. The anchor stores are still finalizing the plans for exterior materials and colors. Staff has met with

the architects for the anchor stores, and the building plans are still being designed and reviewed by their corporate offices.

**Staff Recommendation:**

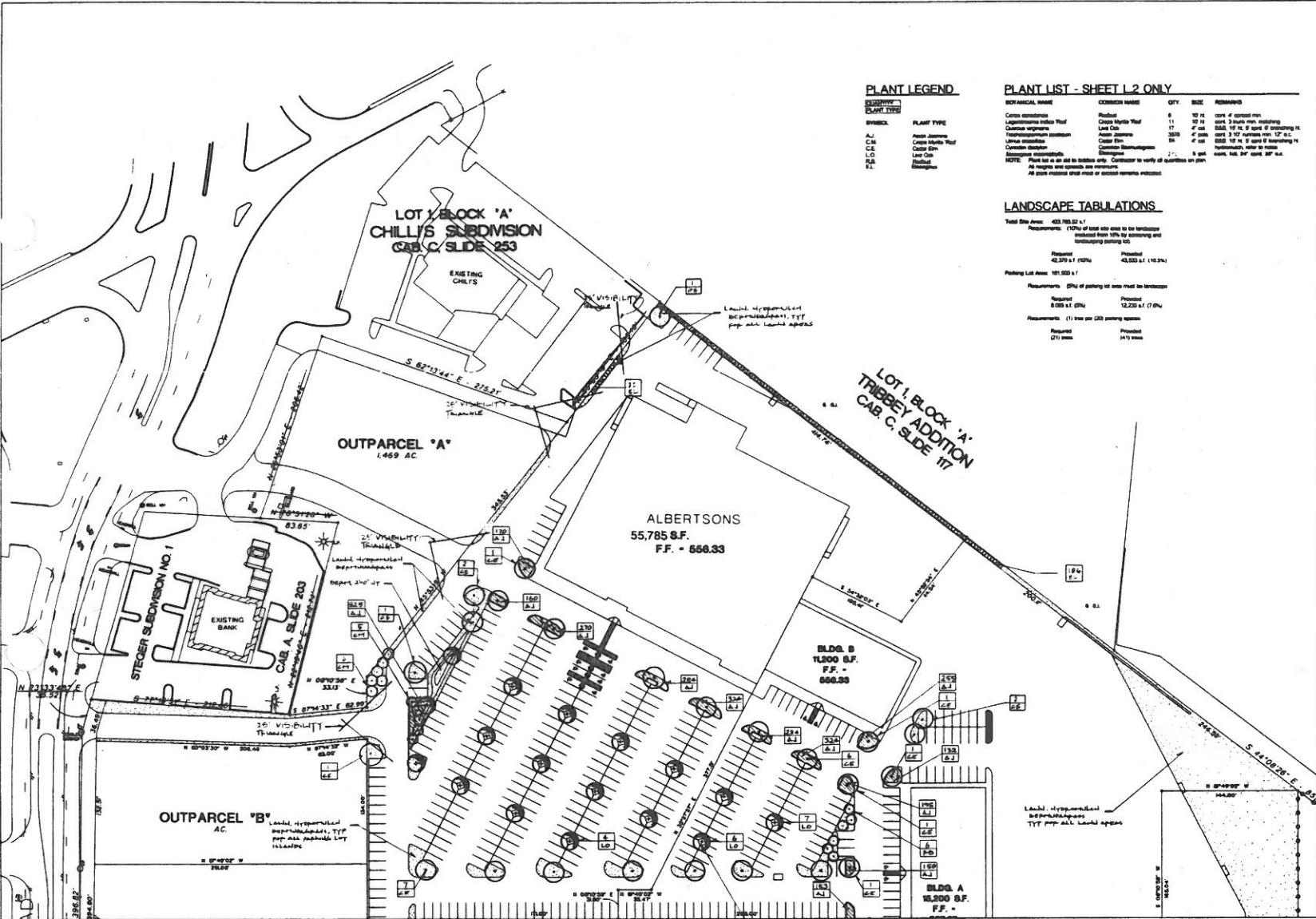
Staff recommends approval of this request with the following conditions;

1. 15' R.O.W. dedication by separate instrument for FM-740 prior to the plat being filed.
2. Approval of the engineering plans.
3. The existing temporary fire lane and access drives on the Food Lion site be abandon or reconfigured prior to the construction of that portion of Steger Towne Drive.
4. The developer of the Steger Towne Crossing allow the property behind Albertson's to petition for cross access to the center.

**P & Z Recommendation:**

Approval with staff conditions.





**PLANT LEGEND**

SYMBOL	PLANT TYPE
A.J.	Asian Jasmine
C.M.	Crema Marula Tree
C.E.	Castor Bean
L.O.	Live Oak
R.B.	Rubber Tree
E.L.	Elm

**PLANT LIST - SHEET L.2 ONLY**

SCIENTIFIC NAME	COMMON NAME	QTY	SIZE	REMARKS
Cornus canadensis	Rubus	8	10 ft	corn 4 spread min
Lagerströmia indica	Crab Apple	11	10 ft	corn 3 bush min, spreading
Quercus virginiana	Live Oak	17	4 cal	30-40 ft, 15 ft, 5' apart if spreading 14
Thymocarpus americanus	Asian Jasmine	3000	4 cal	corn 3' 1/2' minimum min. 12' o.c.
Alnus incana	Castor Bean	80	4 cal	30-40 ft, 15 ft, 5' apart if spreading 14
Castanea coccinea	Castor Bean	21	8 gal	hydraulic, water to main
Stemodia americana	Stemodia	21	8 gal	water, 1/2" dia, 1/2" apart 10' o.c.

NOTE: Plant list is an aid to bidding only. Contractor to verify quantities on plan.  
All heights and spacings are minimums.  
All plant material shall meet or exceed minimum standards.

**LANDSCAPE TABULATIONS**

Total Site Area: 423,765 sq. ft.  
Measurements: (10% of total site area to be landscaped, including 10% for screening and landscaping parking lot)

Requested	Provided
42,376 sq. ft. (10%)	42,376 sq. ft. (10.2%)

Parking Lot Area: 197,500 sq. ft.  
Measurements: (2% of parking lot area must be landscaped)

Requested	Provided
3,950 sq. ft. (2%)	3,220 sq. ft. (1.6%)

Measurements: (1) Area per ADA parking spaces

Requested	Provided
(21) trees	(41) trees



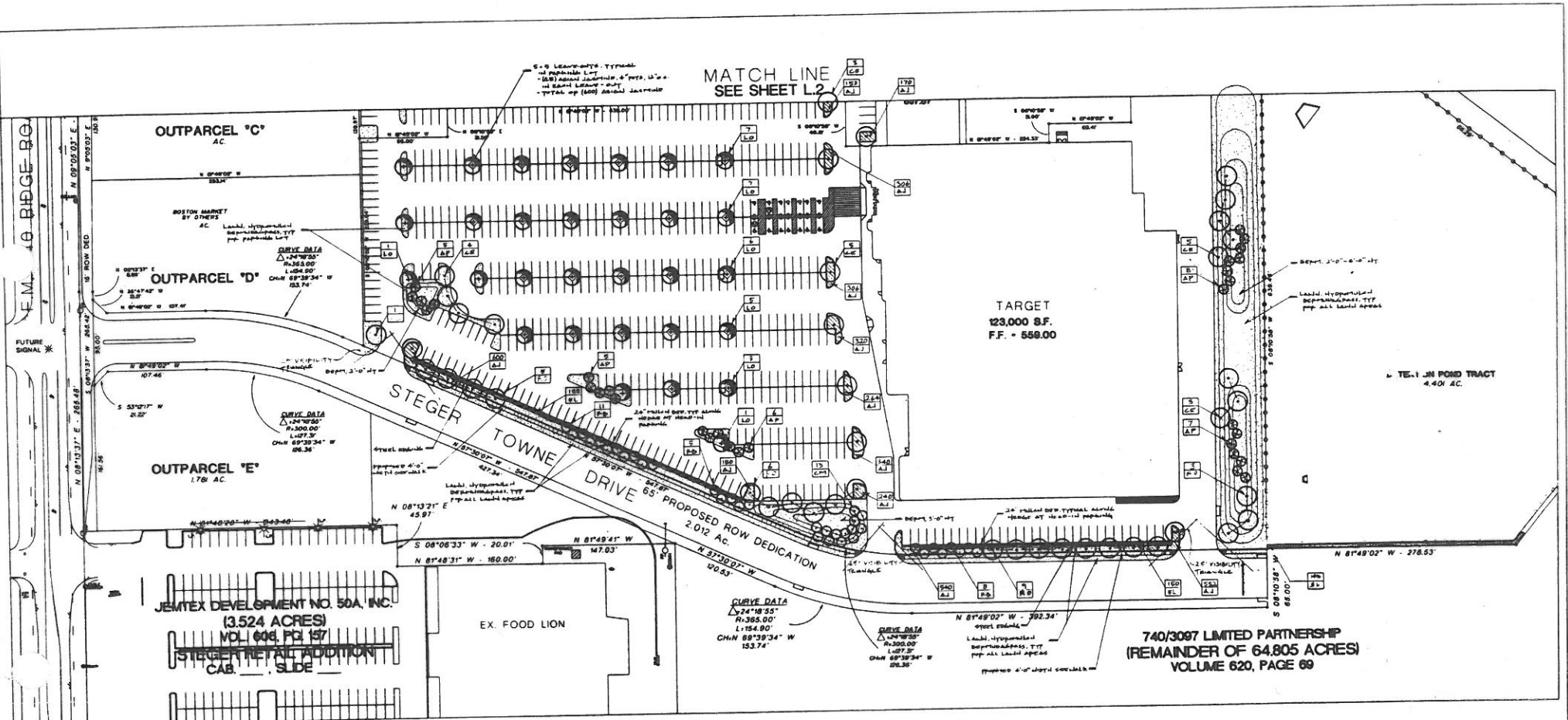
STEVEN H. BANTZ, INC.  
The Design Building  
705 McKinney Avenue  
Suite 420, LB 107  
Dallas, Texas 75202  
Tel: 214 971 0282  
Fax: 214 971 0242



MATCH LINE  
SEE SHEET L.1

5'-0" LEAVE JOINTS TYPICAL  
IN PARALLEL LOT  
(20') MINIMUM SPACING, 4' FROM CURB  
1/4" EXACT LEAVE-JOINT  
- TOTAL OF (1500) ADJUST JOINTS

LANDSCAPE PLAN - ALBERTSONS						
STEGER TOWNE CROSSING						
F.M. 740 (RIDGE ROAD) AND I.H. 30						
CITY OF ROCKWALL, TEXAS						
LAWRENCE A. CATES & ASSOC., INC. CONSULTING ENGINEERS						
DESIGN	DRAWN	DATE	SCALE	NOTES	FILE	NO.
SA	MA	21 MAY 06	1/8" = 1'	DP	0000	L2



**PLANT LEGEND**

SYMBOL	PLANT TYPE
AJ	Asian Jasmine
AP	Asian Pine
CA	Crab Apple Tree
CE	China Elm
LI	Lilac
LO	Low G
RS	Rose
RD	Shrubland Red Oak

**PLANT LIST - SHEET L.1 ONLY**

SYMBOL	PLANT TYPE	QUANTITY	SIZE	REMARKS
CA	Crab Apple Tree	24	10" H	4" x 4" spread min.
CE	China Elm	446	5" H	4" x 4" spread min.
LI	Lilac	13	10" H	4" x 4" spread min.
LO	Low G	30	4" H	3" x 3" spread min.
RS	Rose	21	4" H	3" x 3" spread min.
RD	Shrubland Red Oak	23	4" H	3" x 3" spread min.

**HYDROMULCH NOTES**

- Refer to plan for locations of hydromulch seeding.
- Grass seed shall be sown in full and moist lawn type seed, delivered to the site in its original unopened container, and shall meet area type seed.
- Flow - shall be one hundred (100%) coarse West Co. Fibre, delivered to the site in its original unopened container. Use "Control" or "Erosion".
- Flow - shall be delivered to the site in its original unopened container, and shall be "Terra-Tek One", as manufactured by Chem-Systems, Inc. or equal.
- Hydromulch shall be applied at a rate of two (2) pounds per one thousand (1000) sq. ft. of area to be seeded.
- If installation occurs between September 1 and April 1, or after the break of last frost, all hydromulch areas to be Water Regimes. Seeded at a rate of four (4) pounds per one thousand (1000) square feet. Contractor shall be required to perform the following watering regimen:
- All lawn areas to be hydromulched, shall have one hundred (100%) coarse coverage per to full coverage.
- Until full germination, and until an approved grade of grass is achieved, irrigation planning by watering, mowing, weeding, spraying, clipping and replacing as necessary to keep areas in a vigorous, healthy condition. Irrigation period shall extend from time of seeding completion to final acceptance.
- Contractor to establish a clear planting of grasses, free from weeds and debris. Any part of the area failing to show uniform cover shall be reseeded, and such reseeded area complete until a crop has been established. Seeded lawn areas will not be allowed.
- In the event established grass is not available prior to the time of seeding, the contractor shall be required to install a temporary lawn cover, which shall be removed and replaced with a permanent lawn cover, which shall be removed and replaced with a permanent lawn cover.
- Minimum original grade of lawn areas after commencement of planting and during maintenance period. Possible surface erosion to take, ridge, trench, etc. Repair areas as required for final acceptance.
- Take adequate precautions to protect the work against damage. See backnotes and warning signs as necessary. No pre-emergent herbicides shall be used on any areas to be seeded.
- The work will be accepted when a continuous, unbroken stand of grass is achieved, as approved by the Landscape Architect and Owner.

**MAINTENANCE NOTES**

- The Owner, tenant and their agent, if any, shall be jointly and severally responsible for the maintenance of all landscape.
- All landscape shall be maintained in a neat and orderly manner at all times. The shall include mowing, edging, pruning, watering, weeding and other such activities common to landscape maintenance.
- All landscape areas shall be kept free of trash, litter, weeds, and other such material or debris to a depth of 2" below the surface.
- All plant material shall be maintained in a healthy and growing condition as a minimum for the season of the year.
- All plant material which shall be replaced with plant material of equal or better value.

**LANDSCAPE TABULATIONS**

**TARGET LANDSCAPE TABULATIONS**

Total Site Area: 437,196 S.F.

Requirements: 100% of total site area to be landscaped (includes lawn 10% by covering and landscaping planting list)

Required	Provided
43,719 S.F. (10%)	63,763 S.F. (14.6%)

Requirements: 60% of required landscape area shall be located in street park.

Required	Provided
21,859 S.F. (5%)	26,383 S.F. (6%)

Street Trees:

Requirements: (1) tree per 50 linear feet of frontage

Required	Provided
(21) trees	(21) trees

Parking Lot Area: 180,324 S.F.

Requirements: 60% of parking lot area must be landscaped

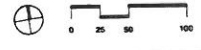
Required	Provided
8,719 S.F. (5%)	9,346 S.F. (5%)

Requirements: (1) tree per (25) parking spaces

Required	Provided
(21) trees	(21) trees

**LANDSCAPE NOTES**

- Contractor shall verify locations of all existing and proposed site elements and notify Architect of any discrepancies. Survey data of existing conditions was provided by others.
- Contractor shall locate all existing underground utility and notify Architect of any discrepancies. Contractor shall locate utility locations when working in the vicinity of underground utilities. Contractor shall obtain Engineers Certificate for extent of existing and proposed utilities.
- Contractor is responsible for obtaining all required landscape and erosion permits.
- Contractor to provide a minimum 2% slope away from all structures.
- All planting beds and lawn areas to be established by steel edging, unless noted otherwise.
- All planting beds to be mulched with 2" layer of specified mulch.
- All landscape areas to be 100% irrigated with an underground automatic irrigation system.
- All lawn areas to be hydromulched unless noted otherwise.



**CITY**  
 Landscape Architect  
 STEVEN M. ADAMS, INC.  
 The Gateway Building  
 202 Midway Avenue  
 Suite 408 LF 107  
 Dallas, Texas 75201  
 Tel: 214-871-0000  
 Fax: 214-871-0048

DESIGN	DRAWN	DATE	SCALE	NOTES	FILE	NO.
DA	MA	21 MAR 2007	1/8" = 1'-0"	SP	6000	L.1

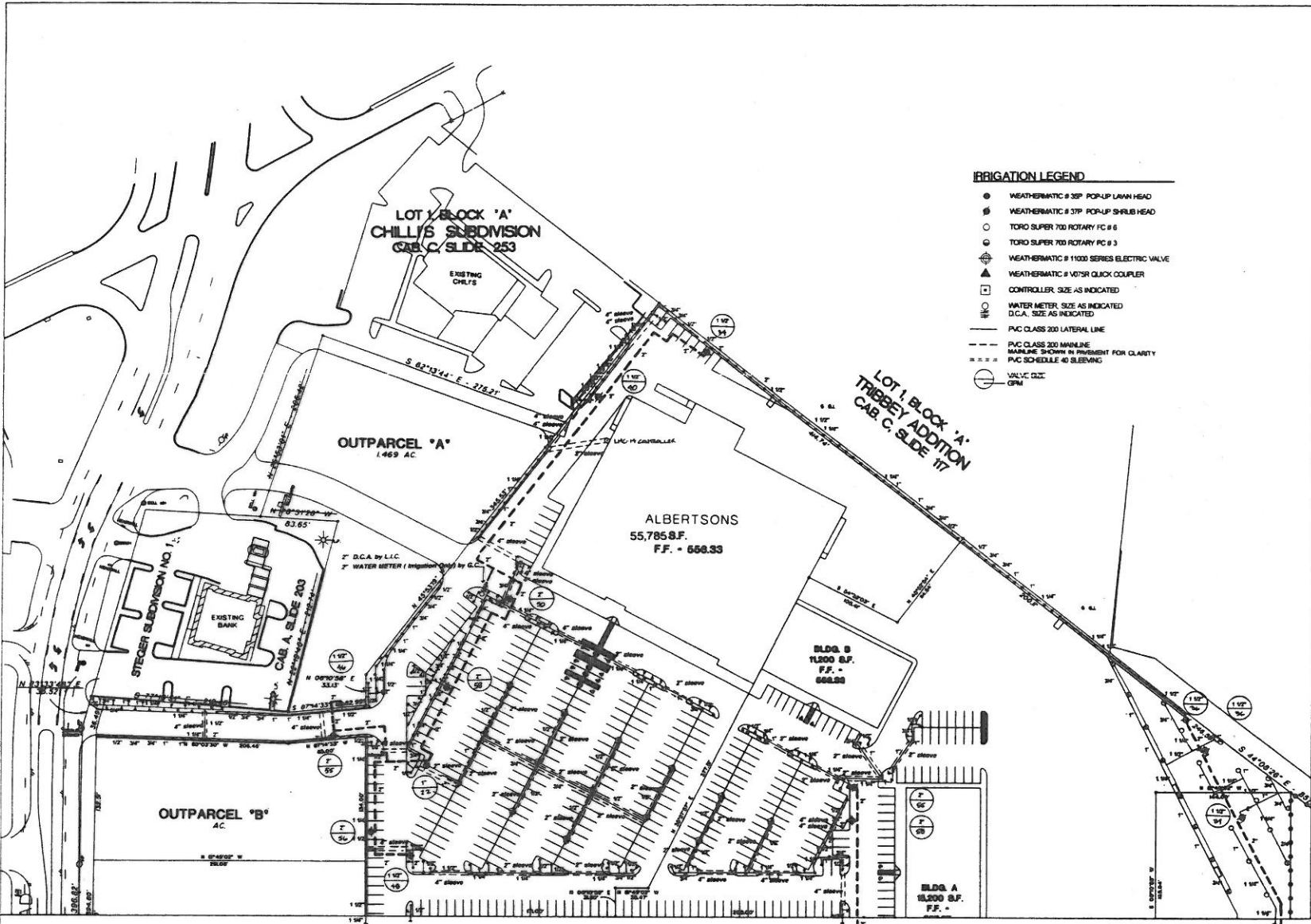
**LANDSCAPE PLAN - TARGET**

**STEGER TOWNE CROSSING**

**F.M. 740 (RIDGE ROAD) AND I.H. 30**

**CITY OF ROCKWALL, TEXAS**

**LAWRENCE A. GATES & ASSOC., INC.**  
 2008-2009  
 CONSULTING ENGINEER  
 DALLAS, TEXAS



- IRRIGATION LEGEND**
- WEATHERMATIC # 35P POP-UP LAWN HEAD
  - ⊗ WEATHERMATIC # 37P POP-UP SHRUB HEAD
  - TORO SUPER 700 ROTARY FC # 6
  - TORO SUPER 700 ROTARY FC # 3
  - ⚡ WEATHERMATIC # 11000 SERIES ELECTRIC VALVE
  - ⚡ WEATHERMATIC # V07SR QUICK COUPLER
  - CONTROLLER, SIZE AS INDICATED
  - ⊕ WATER METER, SIZE AS INDICATED
  - ⊕ D.C.A. SIZE AS INDICATED
  - PVC CLASS 200 LATERAL LINE
  - - - PVC CLASS 200 MAINLINE
  - MAINLINE SCHEDULE 40 SLEEVING FOR CLARITY
  - PVC SCHEDULE 40 SLEEVING
  - VALVE, SIZE AS INDICATED
  - GPM

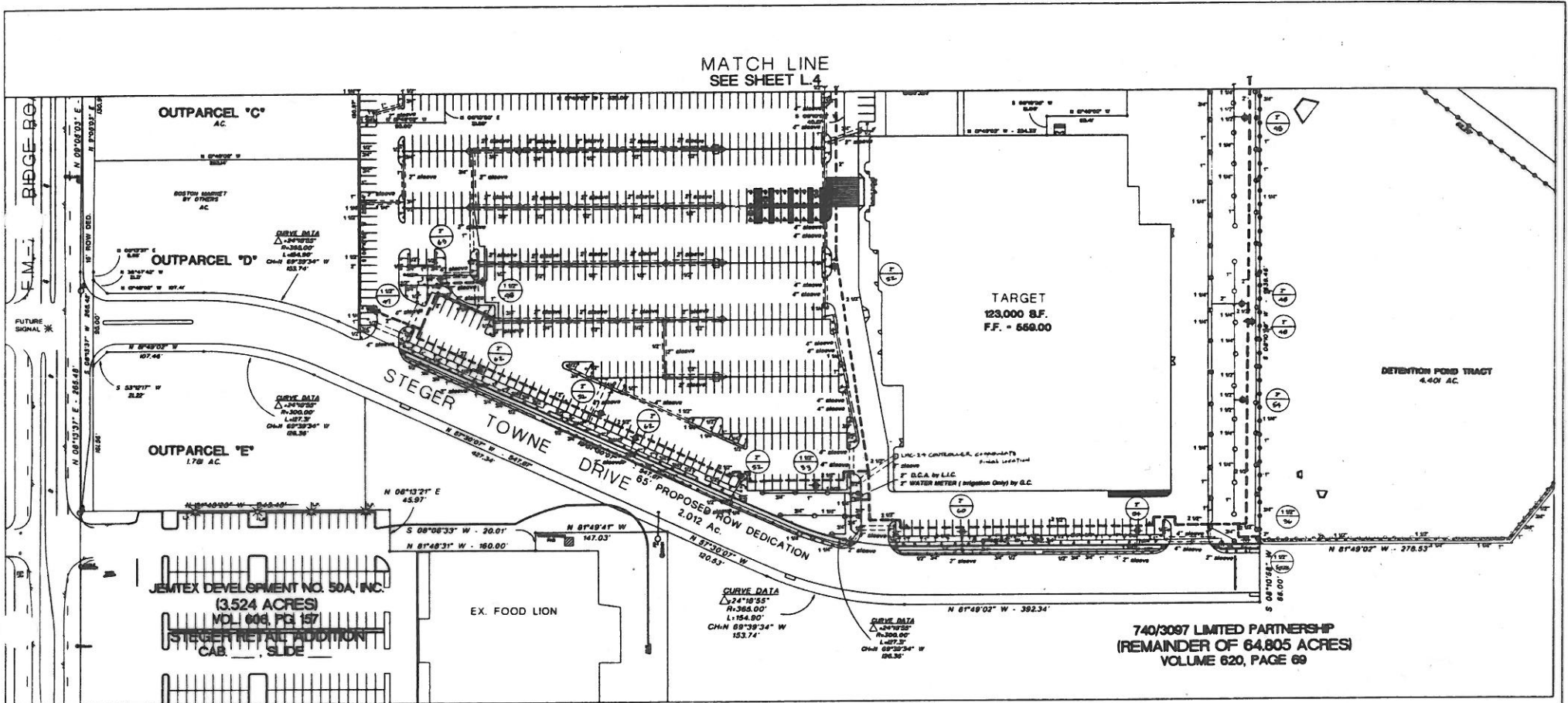
MATCH LINE  
SEE SHEET L.3



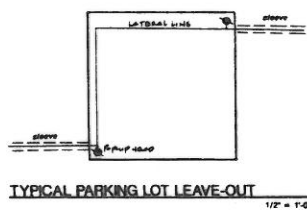
STEVEN EL SANKI, P.E.  
The Steady Building  
200 Millington Avenue  
Dallas, Texas 75202  
Tel: 214 071 0000  
Fax: 214 071 0000



IRRIGATION PLAN - ALBERTSONS						
STEGER TOWNE CROSSING						
F.M. 740 (RIDGE ROAD) AND I.H. 30						
CITY OF ROCKWALL, TEXAS						
LAWRENCE A. CATES & ASSOC., INC. ENGINEERING ARCHITECTS						
DESIGN	DRAWN	DATE	SCALE	NOTES	FILE	REV.
JW	JW	11 MAR 92	1/8" = 1'-0"	04	0000	L.4



740/3097 LIMITED PARTNERSHIP  
 (REMAINDER OF 64,805 ACRES)  
 VOLUME 620, PAGE 69



**IRRIGATION NOTES**

1. All station equipment numbers reference the manufacturer equipment catalog unless otherwise indicated.
2. LAWN SPRAY HEADS are BSP installed as per detail shown.
3. BRUSH SPRAY HEADS are BSP installed per detail shown.
4. ELECTRIC CONTROL VALVES shall be #11000CA installed per detail shown. Size valves as shown on plans. Valves shall be installed in valve boxes large enough to permit manual operation, removal of solenoid and/or valve cover without any depth excavation.
5. QUICK COUPLING VALVES shall be #11000CA installed per detail shown. Quick couplings shall be installed using 3/4" Schedule 40 elbows. Controller shall supply power with three (3) #1200PS couplings and three (3) #10 solid brass wire as part of this contract.
6. AUTOMATIC CONTROLLER shall be installed at location shown. Power (120V) shall be located in a junction box within five feet (5') of controller location by other trades.
7. All 90° and 45° valves using 1/2" to 1" 90° angle connector. All valve spigots are to be permanent and valves shall be installed by General Contractor. Solenoid valves shall be Schedule 40. Size as indicated on plans.
8. Test pipe shall be installed at location shown. Contractor shall verify static water pressure. If static pressure is less than 50 PSI, do not start work until notified to do so by Owner.
9. All main line and lateral piping shall have a minimum of 12 inches of cover. All piping under paving shall have a minimum of 18 inches of cover.
10. The irrigation contractor shall coordinate installation of the system with the landscape contractor so that all plant material will be installed in accordance with the intent of the plans and specifications.
11. The irrigation contractor shall submit the proper size and radius for each riser to insure 100% and proper coverage of all lawn areas and plant material. All risers shall be Weathermatic 400 risers. All risers in parking lot curbs and planting beds shall be low angle to minimize compaction on pavement surfaces. No riser will be allowed to spray on building.

**IRRIGATION LEGEND**

- WEATHERMATIC # 33P POP-UP LAWN HEAD
- WEATHERMATIC # 37P POP-UP BRUSH HEAD
- TORO SUPER 700 ROTARY FC # 8
- TORO SUPER 700 ROTARY FC # 3
- ⊕ WEATHERMATIC # 11000 SERIES ELECTRIC VALVE
- ▲ WEATHERMATIC # MOTOR QUICK COUPLER
- CONTROLLER, SIZE AS INDICATED
- WATER METER, SIZE AS INDICATED
- D.C.A. SIZE AS INDICATED
- PVC CLASS 200 LATERAL LINE
- PVC CLASS 200 MAINLINE
- MARKING SHOWN AS PRECEDENT FOR CLARITY
- PVC SCHEDULE 40 SLEEVING
- VALVE SIZE
- GPM



SEITP  
 Steven E. Parks, Inc.  
 The Academy Building  
 700 McKinney Avenue  
 Suite 401-107  
 Dallas, Texas 75202  
 Tel: 214 871 9555  
 Fax: 214 871 9556

IRRIGATION PLAN - TARGET					
STEEGER TOWNE CROSSING					
F.M. 740 (RIDGE ROAD) AND I.H. 30					
CITY OF ROCKWALL, TEXAS					
LAWRENCE A. GATES & ASSOC., INC.			PROJECT NUMBER		
1700 Rockwall Avenue			6740		
Rockwall, Texas 75087			Date: 05/21/2010		
DESIGN	DRAWN	DATE	SCALE	NOTES	FILE NO.
JW	JW	21 MAY 10	PHOT	0P	6740 L3

**IRRIGATION SPECIFICATIONS**

**SECTION 02800 - IRRIGATION SPECIFICATIONS**

**PART 1 - GENERAL**

- 1.01 SCOPE**
- A. Provide complete irrigation installation as detailed and specified herein, including furnishing of labor, materials, and equipment for the project installation. Work includes but is not limited to:
    1. Transferring and setting
    2. Assembling and installation
    3. Labor completion of installation, including cleaning, testing, and adjustment
    4. Including location of existing piping, removal and installation valves, wherever applicable to correct, and completely exact location of automatic valves.
- B. NOTE:** All devices as shown on plans will be furnished by General Contractor. Meter and power source to be provided by General Contractor.

**1.02 RELATED WORK SPECIFICATIONS**

- A. See irrigation Plans. See plans for controller, meter, and valves.

**1.03 APPLICABLE STANDARDS**

- A. ASTM
  1. D2454 - Poly Vinyl Chloride (PVC) Plastic Pipe Fittings, Standard Specification 40
  2. D2455 - Poly Vinyl Chloride (PVC) Plastic Pipe Fittings, Socket Type, Schedule 40
  3. D2456 - Socket Capacity for Poly Vinyl Chloride (PVC) Plastic Pipe Fittings
  4. D2457 - Recommended Practices for Poly Vinyl Chloride (PVC) Plastic Pipe Fittings
  5. D2458 - Flaring Device - Combined Joints with Poly Vinyl Chloride (PVC) Pipe and Fittings

**1.04 MAINTENANCE AND WARRANTY**

- A. Materials and workmanship shall be fully guaranteed for one (1) year after final acceptance.
- B. Provide maintenance of system including cleaning and servicing of heads to compensate for stem growth, cleaning and adjustment of heads, testing and servicing of stem heads to compensate for stem growth, for one (1) year after completion of installation.
- C. Guarantee to include to repair and replacement of defective materials or workmanship, including repair of labor in connection.

**1.05 SUBMITTALS**

- A. Use of material differing in quality, size or performance from those specified will only be allowed upon written approval of Owner/Landscape Architect. The provision will be made to discontinue supply of material or device to correct any all provisions of materials and general design submitted to be processed by them submitted.
- B. Before issuing to make a substitution for specified materials shall submit manufacturer's catalog showing full specification of each type of substitute material as a substitute including drawings if the substitute indicates operating pressure or service.

Section 02800 - 01

**1.06 A. TESTS**

- A. Approval of substitute material shall be obtained from Owner/Landscape Architect of his responsibility to compensate for any material substitution. Material substitution will be made according to test of properly designed and specified system.
- B. It is the responsibility of the Irrigation Contractor to demonstrate that the installed irrigation system will operate according to intent of originally designed and specified system. If Irrigation Contractor makes any changes in head location or placement, coverage is to be the responsibility to notify the Landscape Architect in writing before any changes are made. Irrigation Contractor guarantee 100% coverage of all areas to be irrigated.

**1.07 TESTING**

- A. Pressure testing required with other trades including earthwork, paving, and plumbing to avoid unnecessary cutting, patching and boring.

**1.08 COORDINATION**

- A. Coordinate installation with other trades including earthwork, paving and plumbing to avoid unnecessary cutting, patching and boring.

**PART 2 - PRODUCTS**

- A. Sprinkler Heads. Sprinkler heads are that portion of piping from water source to the operating system. The portion of piping is subject to variation, being a closed portion of irrigation system. Physical heads are considered a part of irrigation main.
- B. Lateral Piping. Lateral piping is that portion of piping from operating system to operating heads. The portion of piping is not subject to variation, being an "open end" portion of irrigation system.

**2.01 POLY VINYL CHLORIDE PIPE**

- A. PVC pipe shall be installed in accordance with manufacturer's standards listed herein.
- B. Marking and identification. PVC pipe shall be continuously and permanently marked with the following information: manufacturer's name, pipe size, type of head, and material. SDP number, product identification number, and the NSF (National Sanitation Foundation) logo.
- C. PVC Pipe Fittings. Each size of the same material as the PVC pipe specified and shall be compatible with PVC pipe furnished.

**2.02 COPPER TUBING**

- A. Head, straight lengths of domestic manufacture only. No copper tube of foreign origin or any other origin having thin wall shall be used.

**2.03 COPPER TUBE FITTINGS**

- A. Cast brass or wrought copper, standard types.

**2.05 WIRE**

- A. Type US with 4.84" insulation which is Underwriters' Laboratory approved for 60°C, 600-volt use or other uses in a National Electric Code Class 9 Circuit 130 with MC or MC-B.

**2.06 SCHEDULE 40 PVC PIPE**

- A. Composition of Standard Schedule 40 PVC Fittings and PVC Lateral pipe standards. The fittings shall be made by the same manufacturer as the pipe. The pipe shall be made by the same manufacturer as the fittings.
- B. Polyethylene resin shall be 100% virgin and shall be used on all 100 and 100S pipe and fittings.

**2.07 MATERIALS**

- A. Sprinkler heads in head area as specified on plan.
- B. PVC Pipe. Class 200 SPP 2.
- C. Copper tubing. Type "M".
- D. Copper tubing. Type "M".
- E. Electric wire. See 1.05, Item 1.05.
- F. Water to be used for irrigation. See 1.05, Item 1.05.
- G. Water to be used for irrigation. See 1.05, Item 1.05.

**PART 3 - EXECUTION**

**3.01 INSTALLATION - GENERAL**

- A. Sprinkler. Before installation is started place a stake where each sprinkler is to be located in accordance with drawing. Staking shall be approved by Landscape Architect before installation.
- B. Excavation. Excavation are indicated and include depth, base, back fill, etc. for completion and include no backfill. Backfill shall be compacted to a minimum of 95% relative compaction. Excavation shall be protected with 2" x 4" boards and 2" x 4" posts. Backfill shall be compacted to a minimum of 95% relative compaction. Excavation shall be protected with 2" x 4" boards and 2" x 4" posts.
- C. Backfill. Backfill shall be compacted to a minimum of 95% relative compaction. Excavation shall be protected with 2" x 4" boards and 2" x 4" posts.
- D. Backfill. Backfill shall be compacted to a minimum of 95% relative compaction. Excavation shall be protected with 2" x 4" boards and 2" x 4" posts.

**3.02 PIPE INSTALLATION**

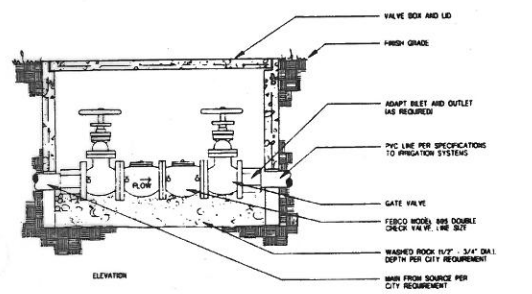
- A. Sprinkler Heads. Install a four (4") inch diameter branch with a minimum of twelve (12") inches of cover.
- B. Lateral Piping. Install a four (4") inch diameter branch with enough to allow for installation of sprinkler heads and valves. Do not use pipe with less than eight (8") inches of cover.
- C. Flanking. Remove water, sediment and debris from trenches. Provide for proper drainage for entire length of each pipe. Do not use pipe with less than eight (8") inches of cover.
- D. Flanking. Remove water, sediment and debris from trenches. Provide for proper drainage for entire length of each pipe. Do not use pipe with less than eight (8") inches of cover.

Section 02800 - 02

Section 02800 - 03

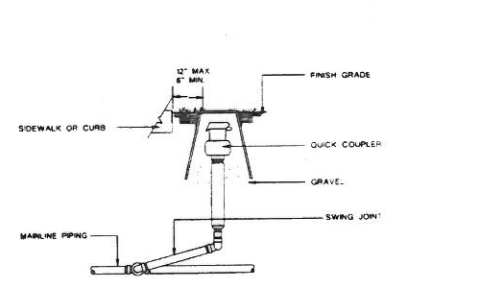
Section 02800 - 04

**IRRIGATION SPECIFICATIONS**



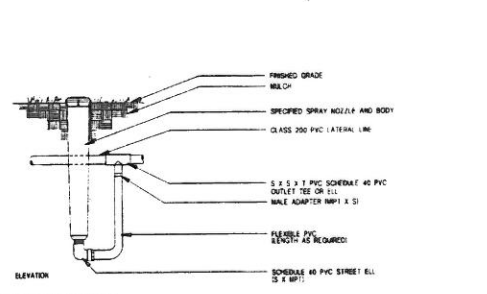
**BACKFLOW PREVENTER**

NOT TO SCALE



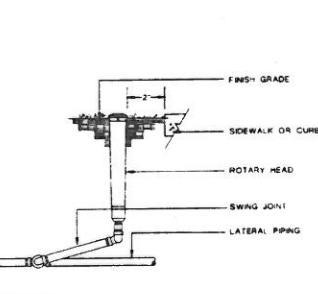
**QUICK COUPLER**

NOT TO SCALE



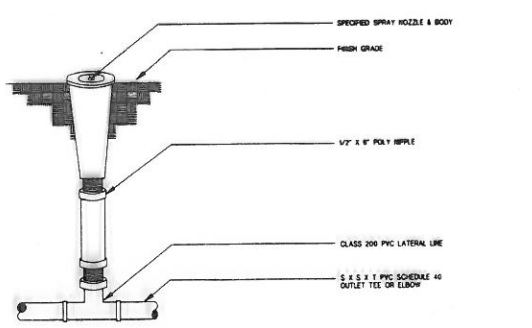
**HIGH RISER ASSEMBLY**

NOT TO SCALE



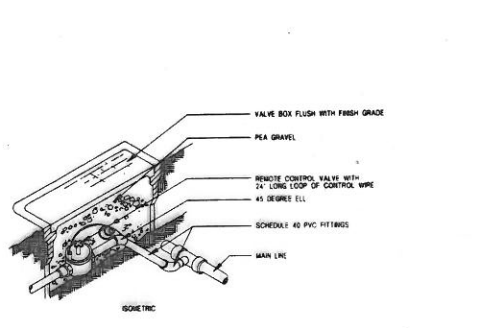
**ROTARY HEAD**

NOT TO SCALE



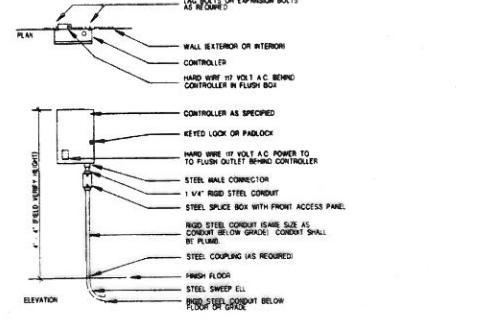
**POP - UP RISER ASSEMBLY**

NOT TO SCALE



**REMOTE CONTROL VALVE**

NOT TO SCALE



**WALL MOUNTED CONTROLLER**

NOT TO SCALE

IRRF landscape architecture  
STEVEN B. RANON, INC.  
The Inventory Building  
700 Valleyview Avenue  
Suite 408 LB 100  
Dallas, Texas 75202  
Tel: 214 871 0800  
Fax: 214 871 0845

REV.	DATE	REVISIONS

**IRRIGATION SPECIFICATIONS / DETAILS**

**STEGER TOWNE CROSSING**  
**F.M. 740 (RIDGE ROAD) AND I.H. 30**  
**CITY OF ROCKWALL, TEXAS**

LAWRENCE A. CATES & ASSOC. CONSULTING ENGINEERS  
11400 MIDWAY RD., STE. 172, (914) 286-2772 DALLAS, TEXAS

DESIGN	DRAWN	DATE	SCALE	NOTES	FILE	NO.
PLA	PLA	21 MAY 88	N.T.S.			L.6

City of Rockwall

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**FINAL DRAFT  
TRAFFIC IMPACT STUDY FOR THE  
PROPOSED STEGER TOWNE CROSSING  
IN ROCKWALL, TEXAS**

---

Prepared by:

**DeShazo, Tang & Associates, Inc.  
400 S. Houston St., Suite 330  
Dallas, Texas 75202**



January 24, 1996

City of Rockwall

---

**FINAL DRAFT  
TRAFFIC IMPACT STUDY FOR THE  
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IN ROCKWALL, TEXAS**

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Prepared by:

DeShazo, Tang & Associates, Inc.  
400 S. Houston St., Suite 330  
Dallas, Texas 75202



January 24, 1996

**Final Draft**

**Traffic Impact Study  
for the Proposed Steger Towne Crossing  
in Rockwall, Texas**

Prepared for:

Mr. Bill Crolley  
City of Rockwall

Prepared by:

DeShazo, Tang & Associates, Inc.  
400 South Houston Street, Suite 330  
Dallas, Texas 75202  
(214) 748-6740  
J95118

January 24, 1996



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3	City of Rockwall Thoroughfare Plan .....	5
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DeShazo, Tang & Associates, Inc.  
Engineers ♦ Planners  
400 S. Houston St, Suite 330  
Dallas, Texas 75202-4802  
214/748-6740 ♦ FAX 214/748-7037

## TECHNICAL MEMORANDUM

To: Mr. Bill Crolley  
City of Rockwall

From: DeShazo, Tang & Associates, Inc.

Date: January 24, 1996

Subject: **Traffic Impact Study for the Proposed Steger Towne Crossing in Rockwall, Texas; J95118**

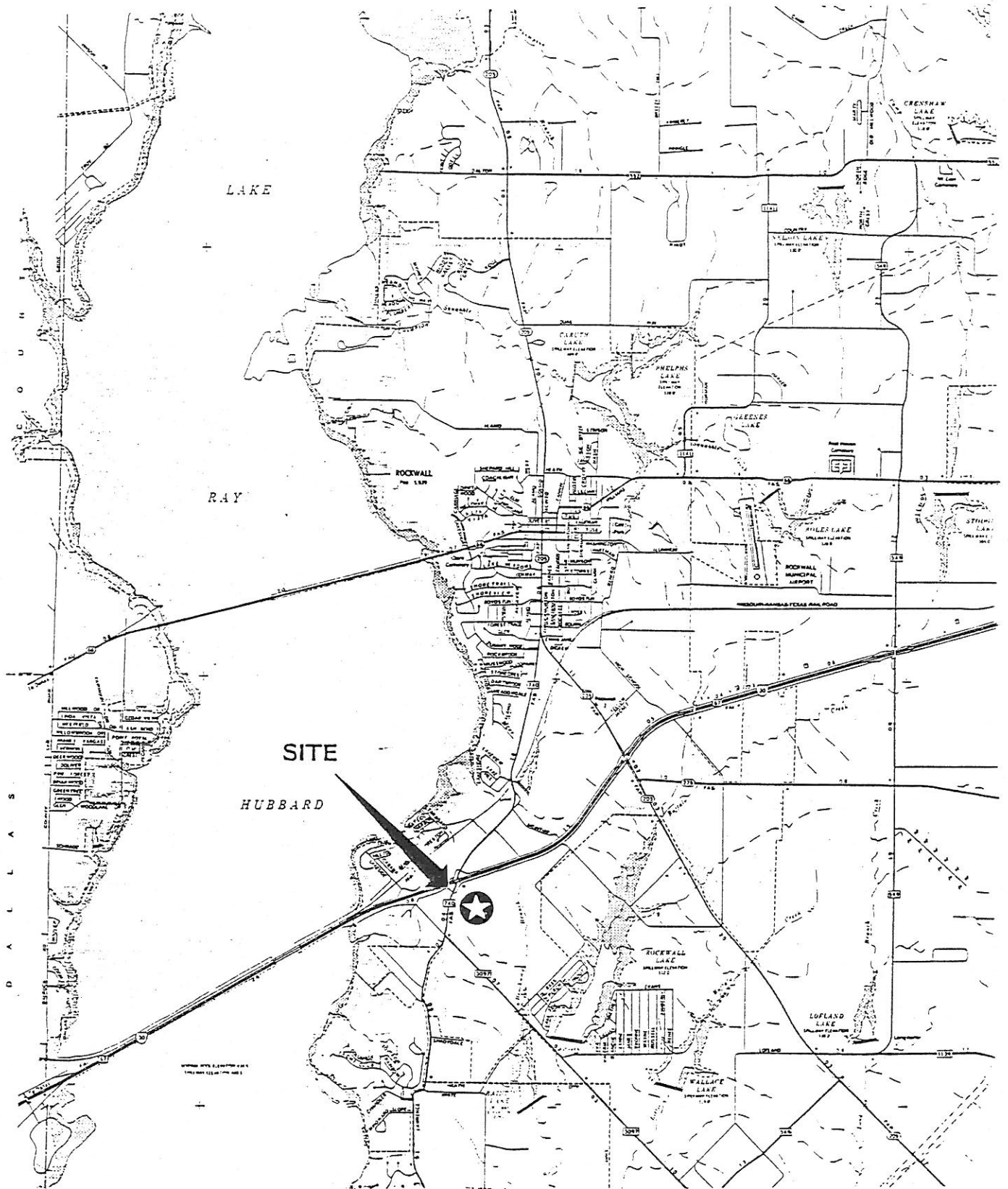
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### INTRODUCTION

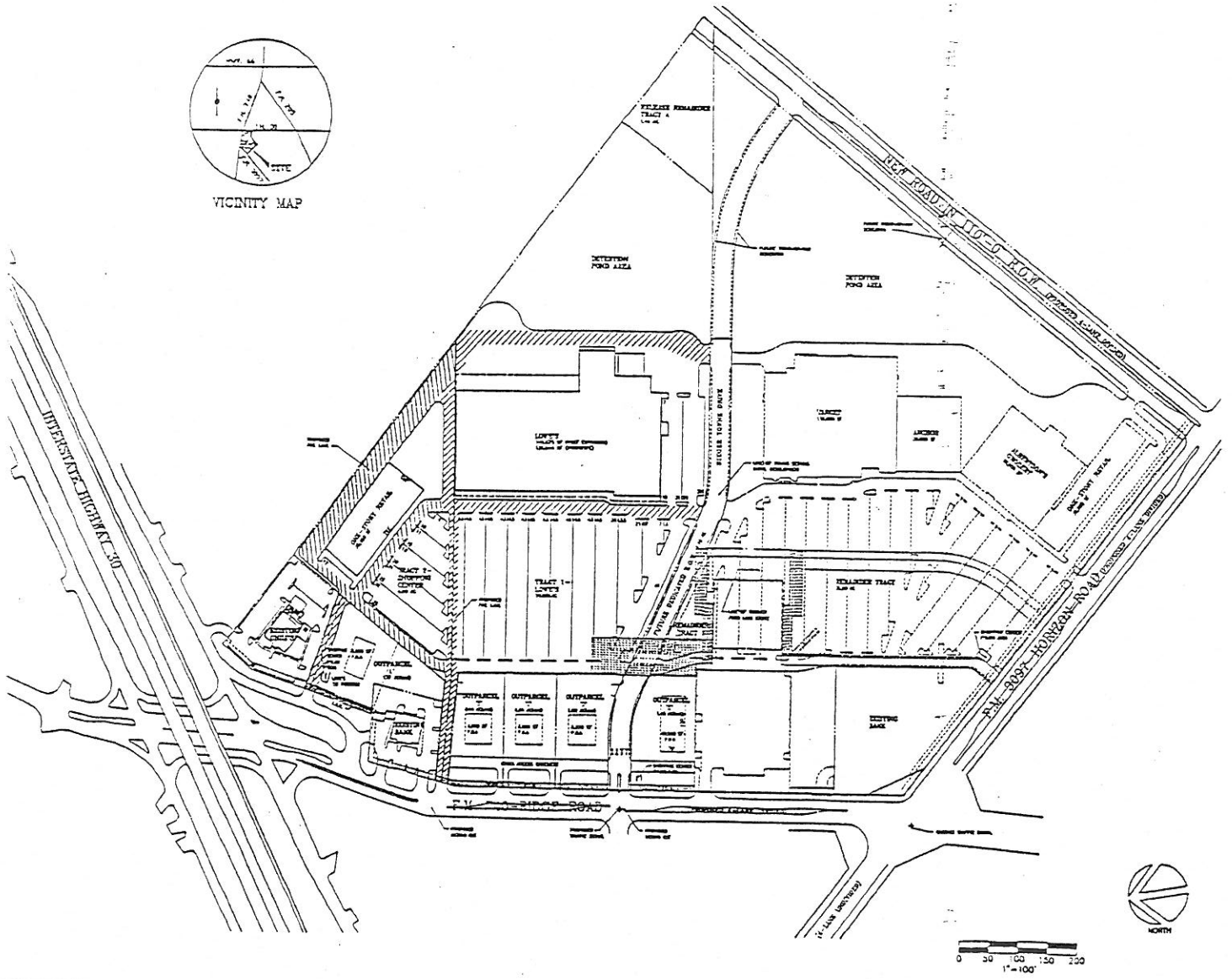
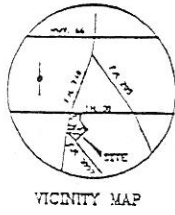
The purpose of this memorandum is to address the City of Rockwall's need to evaluate the traffic-related issues of the proposed Steger Towne Crossing located on the east side of Ridge Road (FM-740 between IH-30 and Horizon Road (FM-3097). Exhibit 1 illustrates the site location. The proposed 64.82 acre site includes 415,000 square feet of retail uses which may include a hardware store, a discount center, a grocery store and supporting retail. Additional commercial sites are planned on five outparcels, ranging in size from 0.94 acres to 1.37 acres. As shown in Exhibit 2, the site plan depicts the dedication of right-of-way for the proposed Steger Towne Drive from Ridge Road to a "New Road" adjacent to the property.

The impact of the site-generated trips on the adjacent roadway network was determined by analyzing the intersection and interchange capacity during the PM peak traffic hour for the following conditions:

- existing background traffic for 1995;
- projected background traffic for 1997;
- projected background 1997 traffic with site-related traffic.



**EXHIBIT 1  
SITE LOCATION**



**Weber & Company**  
REAL ESTATE DEVELOPMENT/INVESTMENT

**GOOD FULTON & FARRELL ARCHITECTS**



**EXHIBIT 2  
SITE PLAN**

## STUDY AREA ROADWAYS

The study area considered in this analysis contains a freeway, arterials and collectors in the vicinity of the site. Exhibit 3 depicts the City of Rockwall Thoroughfare Plan in the study area. Descriptions of these roadways are as follows:

IH-30 - is a grade-separated, east/west freeway providing access from Rockwall to Dallas to the west. Access ramps are provided to and from Ridge Road from the freeway main lanes from the east and west of Ridge Road. Continuous, one-way frontage roads are provided for both eastbound and westbound traffic.

Ridge Road (FM-740) - is a north/south, two-lane, undivided roadway connecting IH-30 and the southern portions of the city to SH-205 in the north, forming a key route in the local street network. The recently reconstructed interchange with IH-30 is signalized. The City's Thoroughfare Plan identifies this facility as a M4D (minor, four-lane, divided roadway) along the existing right-of-way from south of Horizon Road to north of IH-30. Ridge Road is planned for reconstruction by the Texas Department of Transportation (TxDOT) as a four-lane divided roadway from IH-30 to Horizon Road.

Horizon Road (FM-3097) - is currently a two-lane, two-way roadway east of Ridge Road. West of Ridge Road, Horizon Road is one-way northwest-bound, providing access to IH-30 and north of IH-30. Between IH-30 and Ridge Road, Horizon Road is currently being reconstructed and widened to accommodate two traffic flow as a four lane undivided roadway.

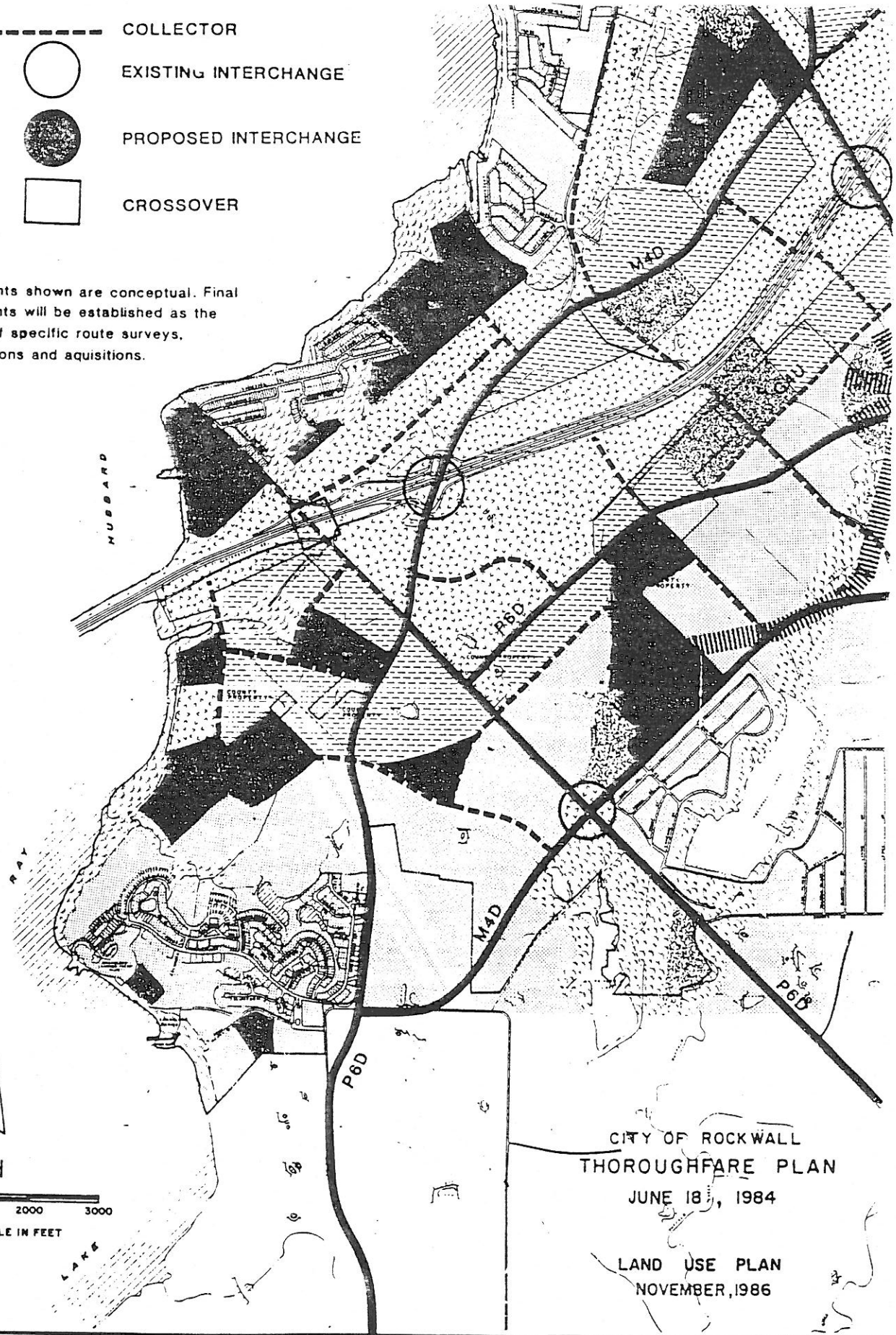
## TRAFFIC ANALYSIS

### Existing Traffic Volumes

From August 18 through November 2, 1995, DeShazo, Tang & Associates conducted 24-hour traffic volume counts on the study area roadways. Exhibit 4 illustrates the existing daily traffic volumes. Guidelines established by the North Central Texas Council of Governments (NCTCOG) state that two-lane arterials can acceptably accommodate approximately 14,500 vehicles per day in suburban areas while four-lane, divided arterials can accommodate up to 32,000 vehicles per day. Exhibit 5A depicts the roadway Levels-of-Service (LOS) for the streets adjacent to the site. LOS refers to the operational conditions within a traffic stream and their perception by motorists. There are six LOS conditions that are designated from "A" to "F", with "A" representing the best operational conditions and "F" the worst conditions. Typically, LOS above "E" are desired.

- COLLECTOR
- EXISTING INTERCHANGE
- PROPOSED INTERCHANGE
- CROSSOVER

NOTE: Alignments shown are conceptual. Final alignments will be established as the result of specific route surveys, dedications and acquisitions.



CITY OF ROCKWALL  
 THOROUGHFARE PLAN  
 JUNE 18, 1984  
 LAND USE PLAN  
 NOVEMBER, 1986



EXHIBIT 3  
 CITY OF ROCKWALL THOROUGHFARE PLAN

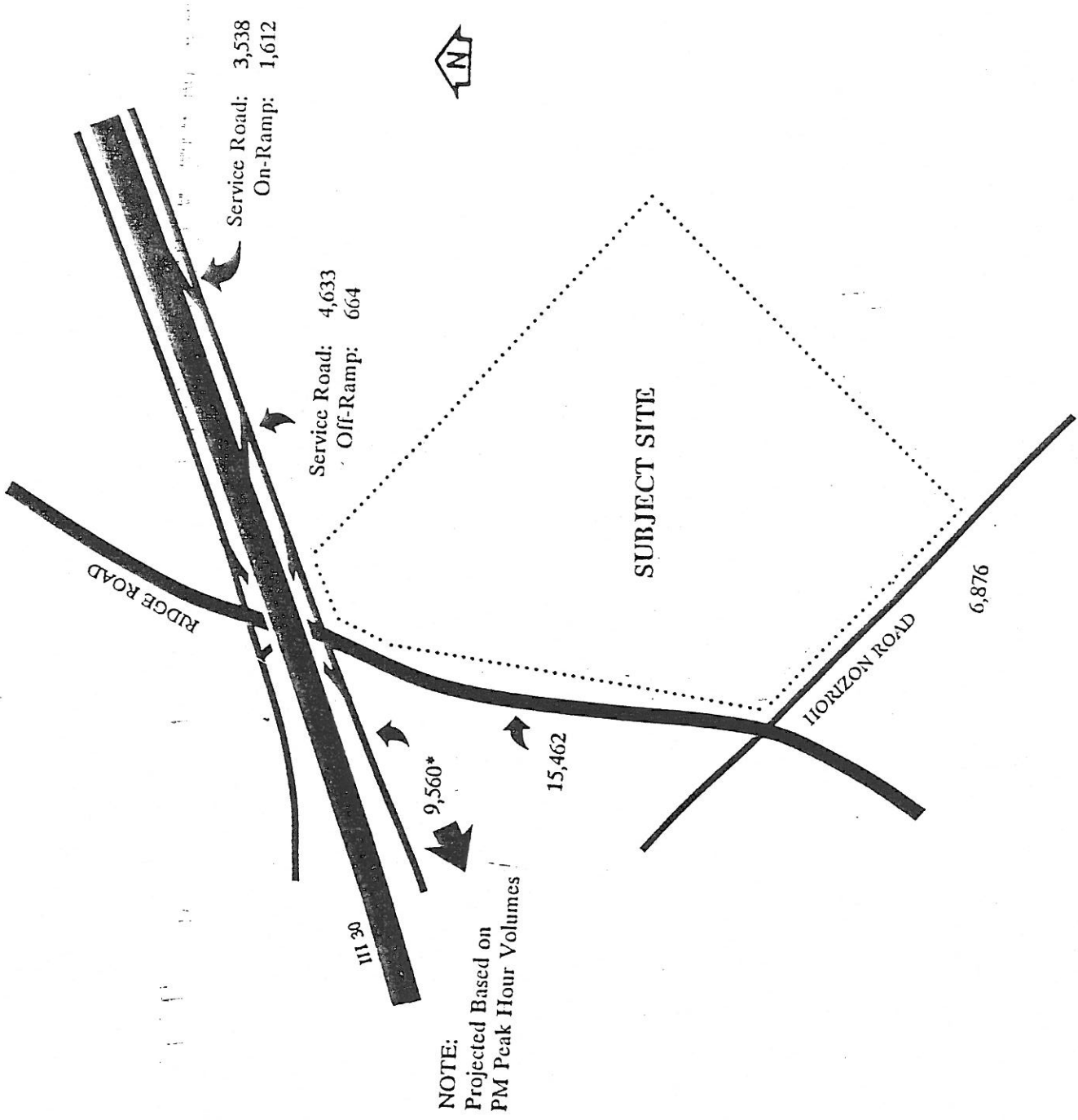


EXHIBIT 4

EXISTING DAILY TRAFFIC VOLUMES



**EXHIBIT 5A  
EXISTING 24-HOUR ROADWAY LEVEL-OF-SERVICE**

Location	Existing Capacity	Existing Daily Traffic Volume	Percent of Existing Capacity Used/ LOS
Ridge Road between IH-30 and Horizon Road	14,000	15,462	110% / F
Horizon Road southeast of Ridge Road	12,500	6,876	55% / A
IH-30 eastbound frontage road between Ridge Road and off-ramp	7,000	4,633	66% / B
IH-30 eastbound frontage road east of Ridge Road after on-ramp	14,000	3,538	25% / A
IH-30 off-ramp east of Ridge Road	14,000	1,612	12% / A
IH-30 on-ramp east of Ridge Road	14,000	664	5% / A

As shown, only Ridge Road experiences a traffic demand which exceeds the existing capacity and operates at an undesirable LOS.

Existing PM peak traffic hour intersection turning movement volumes were also examined to determine the existing traffic operations. The intersection LOS is measured in terms of average delay per vehicle as defined in the following table.

**INTERSECTION LEVEL-OF-SERVICE CRITERIA**

LOS	SIGNALIZED Average Stopped Delay (seconds per vehicle)	UNSIGNALIZED Average Total Delay (seconds per vehicle)
A	≤ 5.0	≤ 5.0
B	5.1 to 15.0	5.0 to 10.0
C	15.1 to 25.0	10.1 to 20.0
D	25.1 to 40.0	20.1 to 30.0
E	40.1 to 60.0	30.1 to 45.0
F	> 60.0	≥ 45.0

LOS results were determined using the 1994 Highway Capacity Software (HCS) for signalized and unsignalized intersections and PASSER-III for the freeway interchanges. Intersections examined included the Ridge Road/IH-30 interchange and Ridge Road at Horizon Road. The current LOS for these intersections during the evening peak hour are shown in Exhibit 5B.

**EXHIBIT 5B  
EXISTING PM PEAK HOUR  
INTERSECTION LEVEL-OF-SERVICE**

Intersection	LOS/Delay (sec/veh)
Ridge Road at IH-30 Westbound Frontage Road	B/8.2
Ridge Road at IH-30 Eastbound Frontage Road	B/11.8
Ridge Road at Carlisle Plaza/Steger Towne Drive	F/(unsignalized)
Ridge Road at Horizon Road	B/14.4

As shown, only the unsignalized intersection of Ridge Road at Carlisle Plaza/Steger Towne Drive operates at an unacceptable LOS.

A summary of the intersection analyses are provided in the Appendix.

**Trip Generation**

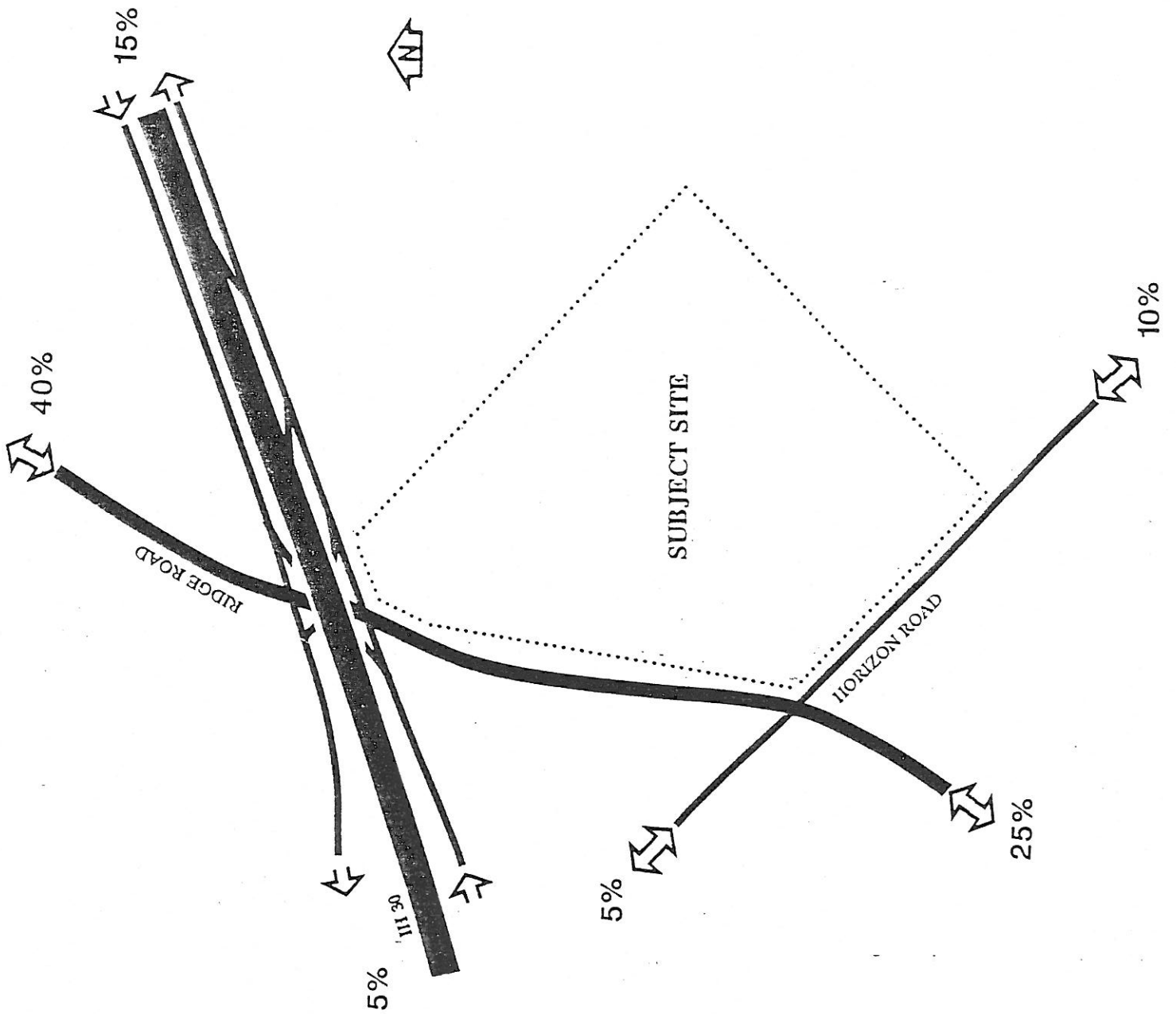
The Institute of Transportation Engineers (ITE) 5th Edition *Trip Generation Manual* was used to project the number of trip ends generated by the proposed development. The manual summarizes field research in trip generation rates for various land uses in the form of graphs and equations. The category for Shopping Center was used to estimate the trip-ends generated by the proposed development. Exhibit 6 presents a summary of the trip-ends generated by the proposed development for typical 24-hour and PM peak hour (of the adjacent street) periods. The evening peak hour represents the highest overall traffic volumes on the adjacent roadways. As shown in Exhibit 6, pass-by trips were applied to the outparcels planned for the development. It was assumed these outparcels would be developed as fast-food restaurants with drive-through lanes. According to the *Trip Generation Manual*, approximately 43 percent of the trips generated by these fast-food restaurants could be composed of existing traffic on the adjacent street system. Therefore, these pass-by trips are not considered newly-generated trips. As shown, the proposed development is expected to generate approximately 29,640 trips on a typical weekday, with about 2,344 of these trips occurring during the evening peak hour. Supplemental information from the *Trip Generation Manual* is provided in the Appendix.

**EXHIBIT 6  
TRIP GENERATION SUMMARY**

Use	Amount (square feet)	Total Daily Trip Ends	PM Peak Hour of Adjacent Street Traffic		
			In	Out	Total
Shopping Center	415,380	17,210	813	813	1,626
Fast-Food w/Drive-through	6,000	3,793	114	105	219
Fast-Food w/Drive-through	6,000	3,793	114	105	219
Fast-Food w/Drive-through	6,000	3,793	114	105	219
Fast-Food w/Drive-through	6,000	3,793	114	105	219
Fast-Food w/Drive-through	10,500	6,637	199	184	384
<b>TOTAL</b>	<b>449,880</b>	<b>39,018</b>	<b>1,468</b>	<b>1,418</b>	<b>2,886</b>
Fast-Food Pass-by	43%	9,378	282	260	542
<b>TOTAL ADDITIONAL TRIPS</b>	<b>449,880</b>	<b>29,641</b>	<b>1,186</b>	<b>1,158</b>	<b>2,344</b>

**Trip Distribution**

Trip distributions for the site-related traffic were determined using demographic information provided by the NCTCOG. Trips related to the proposed Steger Towne Center were distributed throughout the study area based on the relative location of residential land uses. Based on this analysis, the projected additional traffic was assigned to the local roadway network using assumed shortest travel paths. Trip distribution results are summarized in Exhibit 7.



**EXHIBIT 7**  
**TRIP DISTRIBUTION**

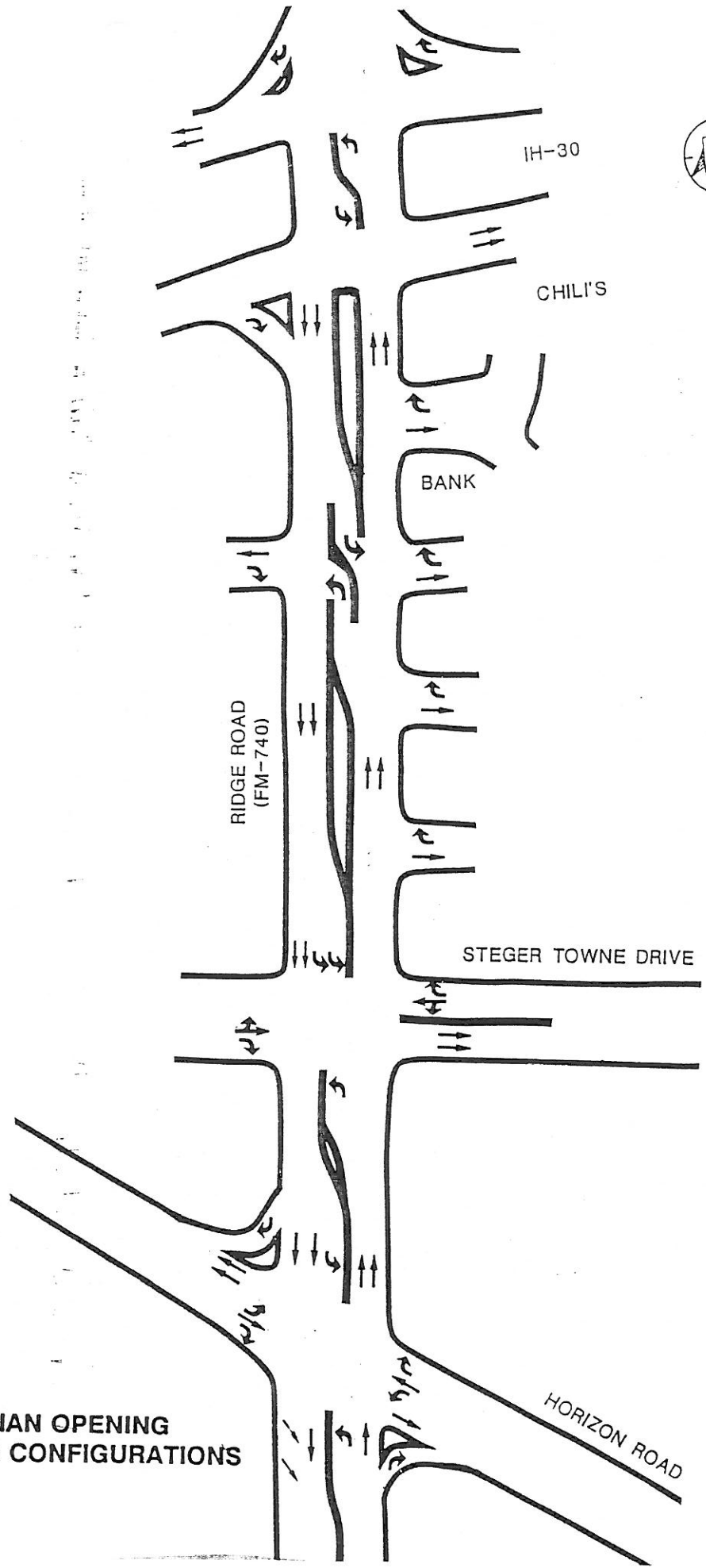
### **Projected Traffic Volumes**

The projected background traffic volumes for the design year 1997 were obtained from the *Traffic Impact Study for the Proposed Wal-Mart Supercenter on IH-30 in Rockwall, Texas*. The projected background with Wal-Mart traffic was used as the base traffic volumes in this study. Ridge Road was assumed to be a four-lane, divided roadway adjacent to the site. Horizon Road was assumed to be a two-lane, undivided roadway adjacent to the site. West of Ridge Road, Horizon Road was assumed to be a two-way roadway. Median openings were assumed to be located on Ridge Road at the proposed Steger Towne Drive and at a driveway between the IH-30 interchange and the proposed Steger Towne Drive. At the north driveway location, left turns exiting Steger Towne Shopping Center and Carlisle Plaza were assumed to be prohibited. Exhibit 8 illustrates the proposed median opening locations and configurations.

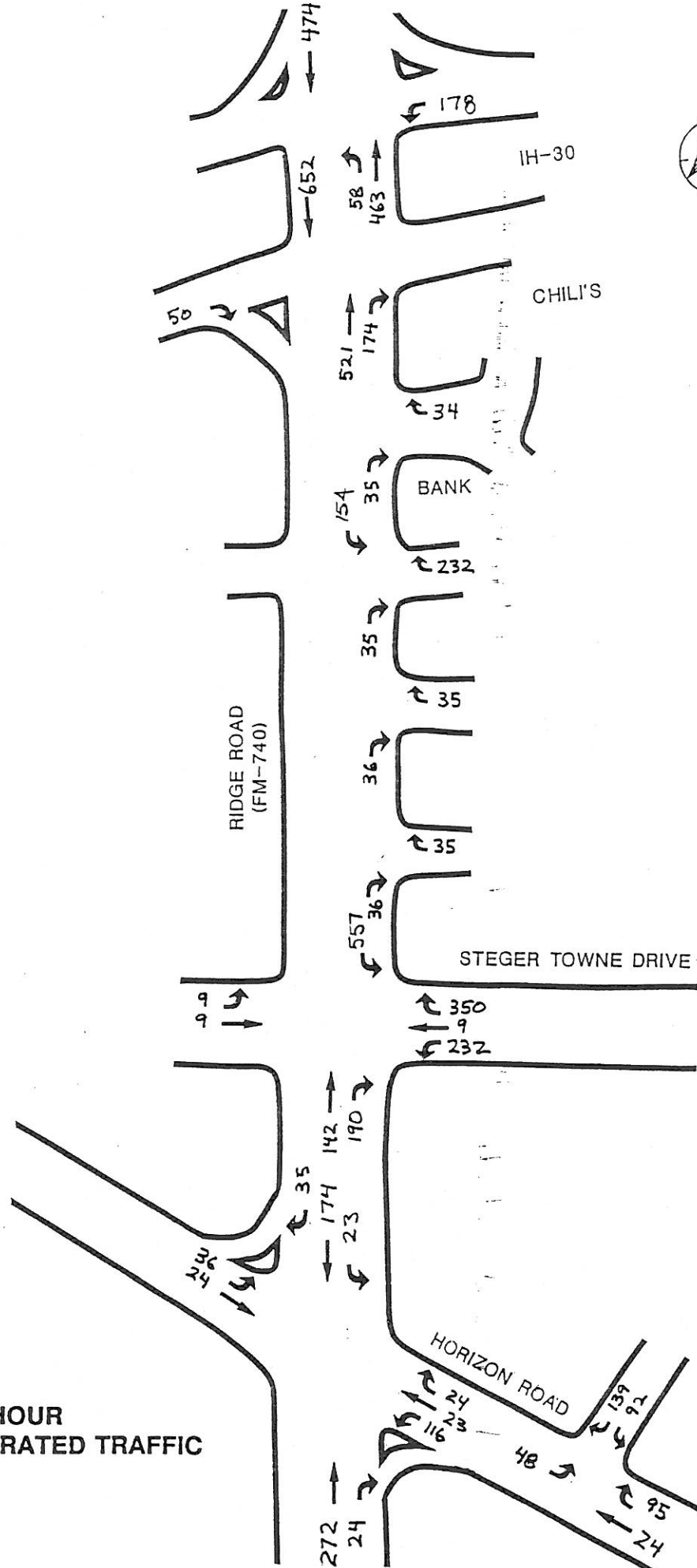
Site-related trips were then assigned to the committed/programmed roadway network based on the trip distribution results. Exhibit 9 depicts the evening peak hour site-related traffic movements in the vicinity of the site. Exhibit 10 summarizes the projected background traffic with the site-related traffic volumes included.

### **Site Impact Determination**

Intersection capacity analyses were conducted for the projected background, and projected background with site-related traffic conditions to determine the intersection levels of service with and without the development. The results are presented in Exhibit 11. A summary of the intersection analyses are provided in the Appendix.



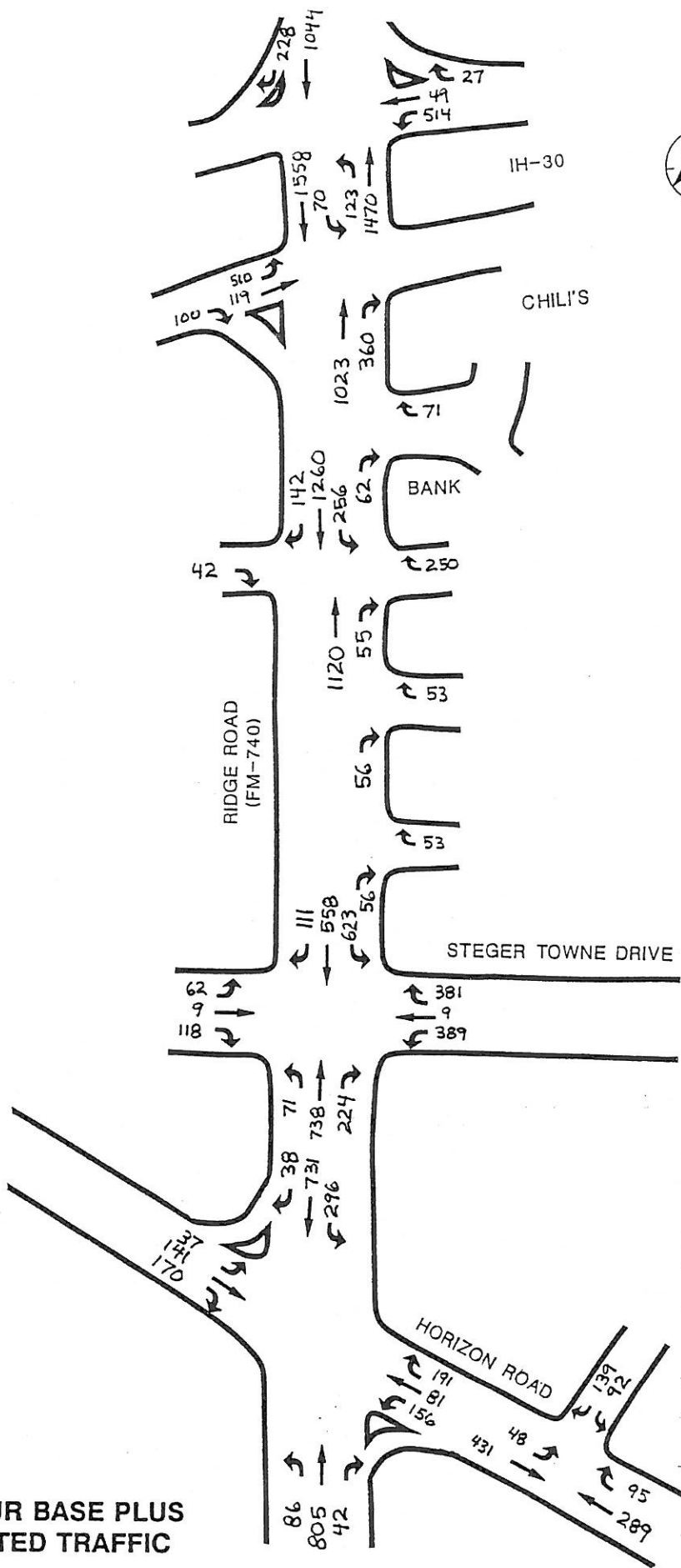
**EXHIBIT 8  
PROPOSED MEDIAN OPENING  
LOCATIONS AND CONFIGURATIONS**



**EXHIBIT 9**  
**PM PEAK HOUR**  
**SITE-GENERATED TRAFFIC**



**EXHIBIT 10  
PM PEAK HOUR BASE PLUS  
SITE-GENERATED TRAFFIC**





**EXHIBIT 11**  
**LEVEL-OF-SERVICE SUMMARY**  
**PM PEAK HOUR OF THE ADJACENT STREET**

Intersection	Projected Background Traffic LOS/Delay(sec/veh)	Projected Background Traffic with Site Traffic LOS/Delay(sec/veh)
Ridge Road @ IH-30 WBFR	B/8.2	B/9.7
Ridge Road @ IH-30 EBFR	B/11.8	B/12.8
Ridge Road @ North Driveway (unsignalized)	D/1.2	D/3.0
Ridge Road @ Steger Towne Drive/ Carlisle Plaza	F/66.0 (unsignalized)	D/34.7 (signalized) C/21.4 (Dual SB left)
Ridge Road @ Horizon Road	B/14.4	C/21.8
Horizon Road @ South Driveway (unsignalized)	Not Applicable	C/2.9

**RECOMMENDATIONS AND CONCLUSIONS**

The programmed widening of Ridge Road between IH-30 and Horizon Road with recommended modifications to the intersection of Steger Town Boulevard is projected to accommodate all phases of the proposed Steger Towne Crossing Development. The projected LOS at IH-30 and Ridge Road is expected to remain the same with or without development of the subject site. The intersection of Ridge Road and Horizon Road will be reconstructed as part of the TxDOT Ridge Road widening. With these planned/programmed improvements, the intersection is projected to operate at a LOS C with an average vehicle delay of only 21.8 seconds.

The prevalent direction of site related trips occurs north of IH-30. Traffic traveling southbound on Ridge Road has two primary opportunities to execute left turn maneuvers into the subject site. The site's primary access will be at Steger Towne Drive, which is proposed to be signalized when warranted according to the Texas Manual of Uniform Traffic Control Devices (TxMUTCD). As part of this analysis, a need has been identified to provide for southbound dual left turn lanes on Ridge Road at Steger Towne Drive. This modification may be accommodated as part of the planned/programmed improvements to Ridge Road by TxDOT.

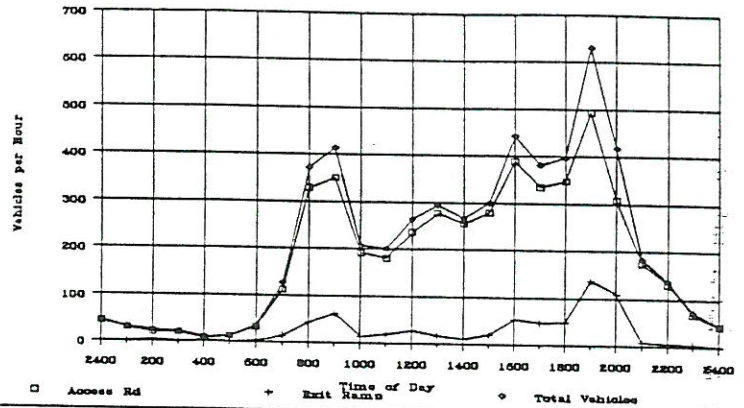
A channelized median providing a left turn for the northern driveway of Steger Town Crossing is also proposed. This type of median would allow all movements into the site. However, departing left turning vehicles (i.e. those desiring to travel southbound on Ridge Road), would do so via the intersection of Ridge Road at Steger Towne Drive/Carlisle Plaza. This recommended median treatment maximizes traffic operations of the public roadway system and enhances safety of the planned facilities.

It is, however, recognized that existing traffic movements in to and out of existing developments will be affected. Other median design options may be explored, however, such efforts are beyond the scope of this study. Generally, a full median opening at the northern driveway would present conflicts of turning movements with existing developments on both sides of Ridge Road. The projections of LOS for a full median opening at the northern driveway is "F". This value is primarily attributed to the projected delay of the left-turn departing vehicles. The delay to other movements, i.e. right-turn entering and departing vehicles and the south bound left-turning vehicles, would not be significantly affected with a full median opening design.

The remaining intersection analyses shown in Exhibit 11 reflect adequate LOS. It is also concluded that the existing and the programmed improvements to Ridge Road can accommodate the proposed development of Steger Towne Crossing. The construction of the "New Road" adjacent to the east boundary of the site and/or the extension of Steger Towne Drive to this road is not necessary from a transportation engineering analysis perspective as part of approval for this development.

Appendix

Street: *IH-30 Access Rd Eastbound*  
 Location: *600 feet east of Ridge Road/FM-740*  
 City/State: *Rockwall, Texas*  
 Project-ID#: *95118 - 162*  
 Date: *November 1, 1995*  
 Day of Week: *Wednesday*  
 Data Source: *DT&A*



24-Hour Volume: 5,297

Time	Peak	EB Access Rd	IH-30 Exit Ramp	Time	Peak	EB Access Rd	IH-30 Exit Ramp
2400				1200			
15		9	1	1215		60	7
30		7	0	1230		62	2
45		3	0	1245		78	6
100		11	0	1300		78	2
		30	1			278	17
115		5	4	1315		74	4
130		9	1	1330		52	2
145		4	0	1345		62	3
200		2	0	1400		68	3
		20	5			256	12
215		7	0	1415		66	3
230		9	0	1430		75	4
245		2	0	1445		72	5
300		2	1	1500		67	9
		20	1			280	21
315		4	1	1515		89	8
330		1	0	1530		100	14
345		0	2	1545		114	16
400		3	0	1600	443	86	16
		8	3			389	54
415		1	0	1615		81	12
430		1	0	1630		89	8
445		3	0	1645		79	14
500		8	0	1700		86	13
		13	0			335	47
515		4	0	1715		94	10
530		9	0	1730		97	17
545		5	0	1745		85	12
600		15	3	1800		72	10
		33	3			348	49
615		17	0	1815		123	11
630		27	4	1830		101	18
645		28	2	1845		130	46
700		40	9	1900		140	62
		112	15			494	137
715		66	5	1915	686	126	63
730		66	5	1930		78	28
745		84	14	1945		49	11
800		113	19	2000		56	7
		329	43			309	109
815		98	29	2015		52	3
830	490	111	22	2030		42	2
845		83	6	2045		37	3
900		59	6	2100		44	1
		351	63			175	9
915		50	3	2115		37	2
930		46	3	2130		40	2
945		48	7	2145		27	0
1000		48	2	2200		27	2
		192	15			131	6
1015		48	3	2215		16	0
1030		42	4	2230		17	1
1045		49	6	2245		19	3
1100		42	7	2300		16	1
		181	20			68	5
1115		51	6	2315		20	0
1130		70	12	2330		9	1
1145		48	7	2345		9	0
1200		68	3	2400		6	0
		237	28			44	1

Directional Volumes 4,633 664

Equipment ID#: 3595

24-Hour Volume 5,297

Street Observations

DeShaz Tang & Associates, Inc.

Street: IH-30 Access Rd Eastbound

Location: 600 feet east of  
Ridge Road/FM-740

City/State: Rockwall, Texas

Project-ID#: 95118 - 162

I. Street Width:

II. Street Material:  
*Concrete*

III. Curbing/Gutters?:  
*Concrete*

IV. Number of Lanes: *1 (one) each*

V. Divided?: *Yes*

VI. Traffic Control Devices: *None*

VII. Pedestrian Crosswalks?: *Not Applicable*

VIII. Pedestrian Pushbutton?: *Not Applicable*

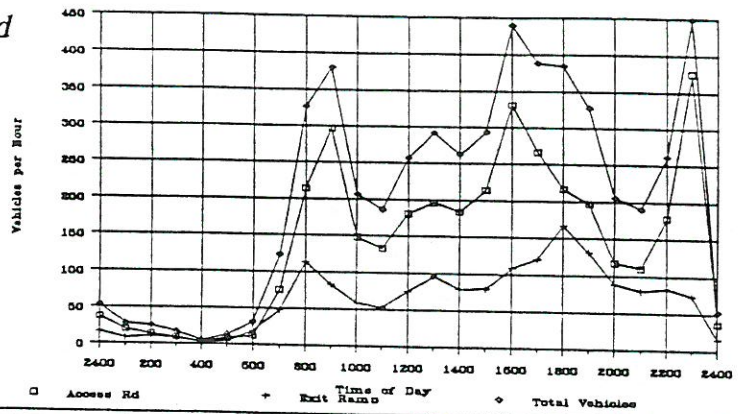
IX. On-street Parking: *Unmarked*

X. Posted Speed Limit: *Unmarked*

XI. Adjacent Land Uses: *Commercial, Agricultural*

XII. Additional Observations/Comments:

Street: *IH-30 Access Road Eastbound*  
 Location: *3,100 feet east of Ridge Road/FM-740*  
 City/State: *Rockwall*  
 Project-ID#: *95118 - 163*  
 Date: *November 1, 1995*  
 Day of Week: *Wednesday*  
 Data Source: *DT&A*



24-Hour Volume: **5,150**

Time	Peak	EB Access Rd	IH-30 On-Ramp	Time	Peak	EB Access Rd	IH-30 On-Ramp
2400				1200			
15		7	0	1215		42	21
30		6	1	1230		43	21
45		3	1	1245		64	19
100		4	6	1300		47	35
		20	8			196	96
115		2	6	1315		57	23
130		8	3	1330		41	13
145		2	2	1345		43	17
200		2	0	1400		43	26
		14	11			184	79
215		2	1	1415		48	15
230		4	6	1430		56	24
245		1	1	1445		49	26
300		2	0	1500		61	15
		9	8			214	80
315		1	1	1515		61	26
330		1	0	1530		73	21
345		0	1	1545		110	41
400		1	1	1600		87	21
		3	3			331	109
415		1	1	1615	449	66	30
430		0	1	1630	449	66	28
445		2	1	1645		64	30
500		6	2	1700		71	33
		9	5			267	121
515		3	1	1715		59	40
530		2	2	1730		69	48
545		1	7	1745	416	52	44
600		6	9	1800		37	35
		12	19			217	167
615		8	8	1815		73	46
630		16	11	1830		37	33
645		17	15	1845		46	32
700		34	14	1900		41	20
		75	48			197	131
715		28	30	1915		28	29
730		40	36	1930		34	22
745		54	25	1945		23	15
800		92	22	2000		32	23
		214	113			117	89
815		86	25	2015		32	26
830		95	29	2030		27	14
845	436	71	16	2045		21	19
900		45	13	2100		30	21
		297	83			110	80
915		30	22	2115		26	27
930		29	19	2130		36	23
945		43	8	2145		53	19
1000		46	10	2200		63	15
		148	59			178	84
1015		33	12	2215		217	38
1030		29	12	2230	526	101	20
1045		41	14	2245		40	6
1100		30	15	2300		17	10
		133	53			375	74
1115		31	23	2315		12	9
1130		49	24	2330		10	3
1145		44	9	2345		11	0
1200		57	20	2400		4	4
		181	76			37	16

Directional Volumes 3,538 1,612

Equipment ID#: **3592**

24-Hour Volume **5,150**

Street Observations

DeShaz Tang & Associates, Inc.

Street: IH-30 Access Road Eastbound

Location: 3,100 feet east of  
Ridge Road/FM-740

City/State: Rockwall

Project-ID#: 95118 - 163

I. Street Width:

II. Street Material:

*On-Ramp: Concrete; Access Rd: Asphalt*

III. Curbing/Gutters?:

*On-Ramp: Concrete; Access Rd: Concrete Curb on Left, Open Drainage on Right*

IV. Number of Lanes: *On-Ramp: 1 (one); Access Rd: 2 (two)*

V. Divided?: *Yes*

VI. Traffic Control Devices: *None*

VII. Pedestrian Crosswalks?: *Not Applicable*

VIII. Pedestrian Pushbutton?: *Not Applicable*

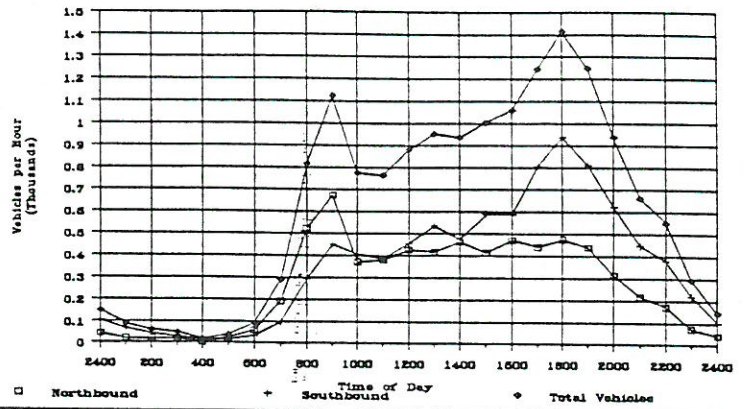
IX. On-street Parking: *No*

X. Posted Speed Limit: *Unmarked*

XI. Adjacent Land Uses: *Agricultural*

XII. Additional Observations/Comments:

Street: *FM-740/Ridge Road*  
 Location: *1,000 feet south of IH-30*  
 City/State: *Rockwall, Texas*  
 Project-ID#: *95118 - 164*  
 Date: *November 1, 1995*  
 Day of Week: *Wednesday*  
 Data Source: *DT&A*



24-Hour Volume: **15,462**

Time	Peak	Northbound	Southbound	Time	Peak	Northbound	Southbound
2400				1200			
15		11	18	1215		95	145
30		4	13	1230		108	152
45		3	24	1245		121	125
100		2	20	1260		93	417
			12	1275			115
			67	1290			537
115		7	15	1315		114	123
130		8	8	1330		117	122
145		3	10	1345		104	112
200		2	8	1360		127	462
			41	1375			117
215		4	5	1390			474
230		4	9	1415		116	129
245		4	10	1430		110	156
300		8	4	1445		113	148
			28	1460		75	414
315		1	2	1475			158
330		0	5	1490			591
345		0	4	1515		95	136
400		2	4	1530		126	125
			15	1545		117	150
415		3	4	1560		133	180
430		6	5	1575			591
445		6	2	1615		101	257
500		9	4	1630		121	169
			15	1645		100	168
515		5	12	1660		119	441
530		15	6	1675			211
545		15	6	1690			805
600		24	10	1715		113	208
			34	1730		129	233
615		27	19	1745		116	477
630		47	22	1760		117	255
645		46	21	1775			940
700		72	36	1800		475	218
			98	1815	1,417	105	467
715		79	50	1830		105	950
730		108	75	1845		107	202
745		141	76	1860		107	207
800		195	93	1875		121	184
			294	1890		438	811
815		188	110	1915		99	191
830		210	102	1930		89	170
845	1,175	162	115	1945		60	130
900		115	123	1960		66	133
			450	1975		314	624
915		100	98	2000			115
930		67	90	2015		65	118
945		86	108	2030		69	128
1000		119	109	2045		36	86
			405	2100		46	115
1015		119	92	2115		52	113
1030		76	99	2130		53	113
1045		97	95	2145		34	85
1100		87	100	2160		33	69
			386	2175		172	380
1115		99	114	2215		23	54
1130		103	103	2230		15	62
1145		109	117	2245		17	64
1200		115	122	2260		18	41
			456	2275		73	221
				2315		10	33
				2330		15	25
				2345		11	19
				2400		7	26
						43	103

Directional Volumes **6,649** **8,813**

Equipment ID#: **3593**

24-Hour Volume **15,462**



Street Observations

DeShaz Tang & Associates, Inc.

Street: FM-740/Ridge Road

Location: 1,000 feet south of

IH-30

City/State: Rockwall, Texas

Project-ID#: 95118 - 164

I. Street Width: 36.3 feet

II. Street Material:  
Asphalt

III. Curbing/Gutters?:  
Drainage Ditch

IV. Number of Lanes: 2 (two) plus one shared left turn lane.

V. Divided?: No

VI. Traffic Control Devices: Intersection traffic signals at Horizon & IH-30 Access Rd

VII. Pedestrian Crosswalks?: No

VIII. Pedestrian Pushbutton?: No

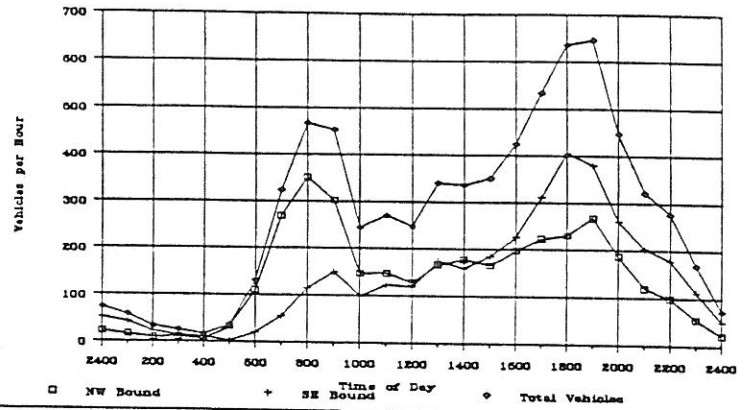
IX. On-street Parking: No

X. Posted Speed Limit: 40 mph

XI. Adjacent Land Uses: Commercial

XII. Additional Observations/Comments:

Street: *Horizon Road*  
 Location: *310 feet east of FM-740/Ridge Road*  
 City/State: *Rockwall, Texas*  
 Project-ID#: *95118 - 165*  
 Date: *November 1, 1995*  
 Day of Week: *Wednesday*  
 Data Source: *DT&A*



24-Hour Volume: **6,876**

Time	Peak	NW Bound	SE Bound	Time	Peak	NW Bound	SE Bound
2400				1200			
15		4	10	1215		39	40
30		5	8	1230		44	46
45		4	16	1245		43	40
100		3	8	1300		40	49
		16	42			166	175
115		2	8	1315		44	39
130		5	7	1330		39	34
145		1	6	1345		42	42
200		2	2	1400		53	44
		10	23			178	159
215		3	3	1415		43	41
230		4	7	1430		46	45
245		0	5	1445		39	43
300		4	0	1500		38	57
		11	15			166	186
315		1	1	1515		45	53
330		1	4	1530		54	47
345		0	4	1545		56	63
400		4	2	1600		43	64
		6	11			198	227
415		6	0	1615		48	95
430		6	2	1630		68	72
445		12	1	1645		48	74
500		10	0	1700		59	71
		34	3			223	312
515		8	7	1715		56	85
530		23	6	1730		49	100
545		32	3	1745		53	119
600		47	5	1800		73	102
		110	21			231	406
615		49	8	1815		75	97
630		68	14	1830		49	108
645		76	18	1845	679	76	99
700		76	16	1900		68	76
		269	56			273	380
715		70	17	1915		55	69
730		81	19	1930		53	76
745		110	34	1945		39	64
800		91	45	2000		40	52
		352	115			187	261
815		89	41	2015		41	50
830	535	84	41	2030		36	58
845		83	30	2045		24	52
900		47	38	2100		19	43
		303	150			120	203
915		44	26	2115		27	48
930		34	24	2130		32	48
945		28	25	2145		23	45
1000		41	23	2200		17	37
		147	98			99	178
1015		41	34	2215		23	28
1030		27	29	2230		14	33
1045		42	30	2245		13	31
1100		38	30	2300		6	21
		148	123			56	113
1115		28	40	2315		8	22
1130		31	28	2330		9	13
1145		35	26	2345		5	4
1200		34	25	2400		1	12
		128	119			23	51

Directional Volumes **3,449** **3,427**

Equipment ID#: **3594**

24-Hour Volume **6,876**

Street Observations

DeShaz Tang & Associates, Inc.

Street: Horizon Road

Location: 310 feet east of

FM-740/Ridge Road

City/State: Rockwall, Texas

Project-ID#: 95118 - 165

I. Street Width: 25.7 feet

II. Street Material:  
Asphalt

III. Curbing/Gutters?:  
Drainage Ditch

IV. Number of Lanes: 2 (Two)

V. Divided?: No

VI. Traffic Control Devices: Intersection Traffic Control Signals at FM-740

VII. Pedestrian Crosswalks?: None

VIII. Pedestrian Pushbutton?: None

IX. On-street Parking: No

X. Posted Speed Limit: 45 mph

XI. Adjacent Land Uses: Commercial, Agricultural

XII. Additional Observations/Comments:

# Intersection Traffic Movements

DeShazo, Tang & Associates, Inc

Location: Ridge Road / FM-740 & IH-30 WB Service Road  
 City/State: Rockwall, Texas County: Rockwall  
 Signalization: Signalized  
 Project-ID#: 95089 - E

Date/Day: August 18, 1995 / Thursday  
 Data Source: DT&A  
 Data Collector(s): Dewey Bishop  
 Comments: Sunny & Hot

Time of Count	Northbound on Ridge Road / FM-740		Southbound on Ridge Road / FM-740		Westbound on IH-30 Service Road			Fifteen Minute Subtotals	Peak Hour Totals	Peak Hour Factor	
	Begin	End	Left	Thru	Thru	Right	Uturn				Left
16:00											
16:15											
16:30											
16:45											
17:00											
17:15											
17:30											
17:45											
18:00											
18:15											
18:30											
PM Peak Hour Total/Direction	57		867		479			228			2,050
% Turn	6.2%		93.8%		67.8%			32.2%			13.1%
Peak Hr Factor	0.71		0.90		0.88			0.81			0.95

# Intersection Traffic Movements

DeShazo, Tang & Associates, Inc

Location: Ridge Road / FM-740 & IH-30 EB Service Road  
 City/State: Rockwall, Texas County: Rockwall  
 Signalization: Signalized  
 Project-ID#: 95089 - F

Date/Day: August 18, 1995 / Thursday  
 Data Source: DT&A  
 Data Collector(s): Buck Woolverton  
 Comments: Sunny & Hot

Time of Count	Northbound on Ridge Road / FM-740			Southbound on Ridge Road / FM-740			Eastbound on IH-30 EB Service Road			Fifteen Minute Subtotals	Peak Hour Totals	Peak Hour Factor	
	Begin	End	Count	1	2	Thru	Left	Thru	Right				UTurn
16:00													
16:15													
16:30													
16:45													
17:00													
17:15													
17:30													
17:45													
18:00													
18:15													
18:30													
PM Peak Hour Total/Direction													
% Turn													
Peak Hr Factor													

# Intersection Traffic Movements

DeShazo, Tang & Associates, Inc

Location: **FM-740/Ridge Road & Driveway to Bank/Chili's** Date/Day: **October 30, 1995 / Monday**  
 City/State: **Rockwall, Texas** County: **Rockwall** Data Source: **DT&A**  
 Signalization: **None** Data Collector(s): **Charles L. DeShazo**  
 Project-ID#: **95118 - B** Conditions: **Cloudy & Dry**

Time of Count	Northbound on		Southbound on		Eastbound out of		Fifteen Minute Subtotals	Peak Hour Totals	Peak Hour Factor
	FM-740	2U	FM-740	2U	Driveway	2U			
Begin	Thru	Right	Left	Thru	Left	Right			
17:00	114	2	2	254	1	5	378		
17:15	102	2	4	341	2	9	460		
17:30	124	0	3	257	1	3	388		
17:45	112	3	1	280	5	2	403	1,629 *	88.53%
PM Peak Hour	452	7	10	1,132	9	19	1,629		
Total/Direction	459	---	---	1,142	---	28			
% Turn	98.5%	1.5%	0.9%	99.1%	32.1%	67.9%			
Peak Hour Factor	91.1%	58.3%	62.5%	83.0%	45.0%	52.8%			

File: A2X2HRS.WK1

Observations:

# Intersection Traffic Movements

DeShazo, Tang & Associates, Inc

Location: *Access from State Ave* FM-740/Ridge Road & Carlisle Plaza Main Drvwy  
 Date/Day: November 2, 1995 / Thursday  
 City/State: Rockwall, Texas County: Rockwall  
 Signalization: None Data Source: DT&A  
 Project-ID#: 95118 - C Data Collector(s): C. Blaine Rodgers  
 Conditions: Cloudy & Dry

Time of Count	Northbound on		Southbound on		Westbound out of			Fifteen Minute Subtotals	Peak Hour Totals	Peak Hour Factor
	FM-740 2U	1 > 1	FM-740 2U	1 < 0	Carlisle Plaza Drvwy 2U	1 > 1	< 1			
Begin	Left	Thru	Left	Thru	Right	Left	Right			
17:00	15	121	182	25		11	34	388		
17:15	14	102	233	41		8	27	425		
17:30	14	112	227	25		9	32	419		
17:45	11	120	226	20		10	25	412	1,644 *	98.09%
PM Peak Hour	54	455	868	111		38	118	1,644		
Total/Direction	---	509	---	---		---	156	---		
% Turn	10.6%	89.4%	88.7%	11.3%		24.4%	75.6%			
Peak Hour Factor	90.0%	94.0%	93.1%	67.7%		86.4%	86.8%			

File: A2X2HRS.WK1

Observations:

### Intersection Traffic Movements

DeShazo, Tang & Associates, Inc

Location: **FM-740/Ridge Road & Carlisle Plaza Driveway**  
 City/State: **Rockwall, Texas** County: **Rockwall**  
 Signalization: **None**  
 Project-ID#: **95118 - A**

Date/Day: **October 30, 1995 / Monday**  
 Data Source: **DT&A**  
 Data Collector(s): **C. Blaine Rodgers**  
 Conditions: **Cloudy & Dry**

Time of Count	Northbound on		Southbound on		Westbound out of Carlisle Plaza Drvwy			Fifteen Minute Subtotals	Peak Hour Totals	Peak Hour Factor
	FM-740 2U	Left Thru Right	FM-740 2U	Left Thru Right	Left	Thru	Right			
17:00	5	111	225	39	9	9	398			
17:15	2	101	268	31	6	10	418			
17:30	4	98	258	35	8	13	416			
17:45	6	121	258	37	6	10	438	1,670 *	95.32%	
PM Peak Hour Total/Direction	17	431	1,009	142	29	42	1,670			
% Turn	3.8%	96.2%	87.7%	12.3%	40.8%	59.2%				
Peak Hour Factor	70.8%	89.0%	94.1%	95.9%	80.6%	105.0%				

File: A2X2HRS.WK1

Observations:



# Intersection Traffic Movements

DeShazo, Tang & Associates, Inc

Location: **FM-740/Ridge Rd & FM-3097/Horizon Rd** Date/Day: **October 5, 1995 / Thursday**  
 City/State: **Rockwall, Texas** County: **Rockwall** Data Source: **DT&A**  
 Signalization: **Traffic Signals** Data Collector(s): **C. Blaine Rodgers**  
 Project ID#: **95056 - A** Comments: **Fair & Dry**

Time of Count	Northbound on FM-740/Ridge Rd (2U)			Southbound on FM-740/Ridge Rd (3U)			Northwest Bound FM-3097/Horizon Rd (2U)			Fifteen Minute Subtotals	Peak Hour Totals	Peak Hour Factor
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right			
0>	1	1	<0	1	1	<0	1	1	<0			

## Note:

- \* The "west" leg of this intersection is a one-way, one-lane street Northwest Bound.
- \* At 16:24 a rear-end collision took place in the southbound lanes approximately 150 feet south of the intersection due to someone stopping to make a left turn into a convenience store located on the southeast corner. This affected both southbound and northbound traffic, but police were on the scene within four minutes directing traffic around the disabled vehicles. The accident was cleaned up by 16:48.
- \* There is no left turn lane for the Northbound traffic, yet there is a left turn signal for that movement.

16:00	20	82	4	88	142	3	2	9	43	393		
16:15	28	64	4	90	123	0	6	13	32	360		
16:30	29	77	5	73	119	5	11	26	74	419		
16:45	23	94	5	56	106	3	6	7	47	347		1,519
17:00	20	105	2	106	150	0	11	12	35	441		1,567
17:15	21	100	4	84	140	1	9	19	41	419		1,626
17:30	22	83	6	100	138	0	5	13	43	410		1,617
17:45	23	85	6	100	139	3	15	14	48	433		1,703 *
18:00	20	73	5	81	142	1	4	19	40	385		1,647
18:15	26	78	7	95	177	3	3	15	29	433		1,661
PM Peak Hour	86	373	18	390	567	4	40	58	167	1,703		
Total/Direction	18.0%	47.7%	3.8%	40.6%	96.1%	0.4%	15.1%	21.9%	63.0%			
% Turn	76.8%	88.8%	64.3%	92.0%	80.1%	20.0%	66.7%	55.8%	56.4%			
Peak Hour Factor												

# Intersection Traffic Movements

DeShazo, Tang & Associates, Inc

Location: FM-740/Ridge Rd & FM-3097/Horizon Rd  
 City/State: Rockwall, Texas County: Rockwall  
 Signalization: Traffic Signals  
 Project-ID#: 95056 - B

Date/Day: October 11, 1995 / Wednesday  
 Data Source: DT&A  
 Data Collector(s): C. Blaine Rodgers  
 Comments: Fair & Dry

Time of Count	Northbound on FM-740/Ridge Rd			Southbound on FM-740/Ridge Rd			Northwest Bound FM-3097/Horizon Rd			Fifteen Minute Subtotals	Peak Hour Totals	Peak Hour Factor
	Begin	End	Count	Left	Thru	Right	Left	Thru	Right			
06:30	06:45		44	29	0	10	18	2	2	37	32	174
06:45	07:00		43	29	1	7	37	1	1	26	30	175
07:00	07:15		78	38	1	11	34	1	0	54	19	236
07:15	07:30		59	50	4	9	54	2	5	40	38	261
07:30	07:45		61	53	4	16	62	1	4	27	33	261
07:45	08:00		68	70	7	16	70	0	8	33	45	317
08:00	08:15		52	85	8	26	65	1	6	33	38	314
08:15	08:30		51	83	3	27	54	2	4	29	57	310
08:30	08:45		49	87	5	42	62	0	5	25	27	302
08:45	09:00		47	65	3	15	65	1	5	21	26	248
AM Peak Hour			224	146	6	37	143	6	8	157	119	846
Total/Direction			59.6%	38.8%	1.6%	19.9%	76.9%	3.2%	2.8%	55.3%	41.9%	
% Turn			71.8%	42.0%	18.8%	22.0%	51.1%	75.0%	25.0%	72.7%	52.2%	
Peak Hour Factor												98.03%

## Note:

- \* The "west" leg of this intersection is a one-way, one-lane street Northwest Bound.
- \* There is no left turn lane for the Northbound traffic, yet there is a left turn signal for that movement.
- \* One Vehicle was observed going the wrong way on the oneway section of Horizon Road at 08:10. This was not added to the data, but the vehicle did make a southeast bound through movement.

Streets: (E-W) IH 30 WBFR (N-S) Ridge Road  
 Analyst: GCL File Name: WBRB2.HC9  
 Area Type: Other 11-1-95 PM Peak  
 Comment: Base Traffic Volumes (Includes Walmart Study)

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes				1	> 1	1	1	2			3	1
Volumes				336	49	27	65	947			570	228
Lane Width				12.0	12.0	12.0	12.0	12.0			12.0	12.0
RTOR Vols						0						0
Lost Time				3.00	3.00	3.00	3.00	3.00			3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left					*	*		
Thru					*	*		
Right								
Peds								
WB Left		*						
Thru		*						
Right		*						
Peds								
NB Right								
SB Right								
Green	26.0A				49.0A	15.0A		
Yellow/AR	0.0				0.0	0.0		
Cycle Length:	90 secs							

Phase combination order: #1 #5 #6

Intersection Performance Summary

Lane	Group:	Mvmts	Cap	Adj Sat	Flow	v/c	Ratio	g/C	Ratio	Delay	LOS	Approach:	
												Delay	LOS
WB	L	452		1770		0.453		0.256		18.7	C	18.5	C
	LT	459		1796		0.438		0.256		18.6	C		
	R	405		1583		0.069		0.256		16.4	C		
NB	L	535		1770		0.127		0.333		4.3	A	4.3	A
	T	2525		3725		0.415		0.678		4.3	A		
SB	T	2856		5588		0.231		0.511		7.9	B	8.0	B
	R	809		1583		0.297		0.511		8.3	B		

Intersection Delay = 8.2 sec/veh Intersection LOS = B  
 Lost Time/Cycle, L = 6.0 sec Critical v/c(x) = 0.425

Streets: (E-W) IH 30 WBFR (N-S) Ridge Road  
 Analyst: GCL File Name: WBRBD.HC9  
 Area Type: Other 11-1-95 PM Peak  
 Comment: Base Plus Development Traffic Volumes

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes				1	> 1	1	1	2			3	1
Volumes				514	49	27	123	1410			1044	228
Lane Width				12.0	12.0	12.0	12.0	12.0			12.0	12.0
RTOR Vols						0						0
Lost Time				3.00	3.00	3.00	3.00	3.00		0	3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left								
Thru								
Right								
Peds								
WB Left		*						
Thru		*						
Right		*						
Peds								
NB Right								
SB Right								
Green	26.0A				49.0A	15.0A		
Yellow/AR	0.0				0.0	0.0		
Cycle Length:	90 secs	Phase combination order: #1 #5 #6						

Intersection Performance Summary

	Lane Mvmts	Group: Cap	Adj Sat Flow	v/c Ratio	g/C Ratio	Delay	LOS	Approach:	
								Delay	LOS
WB	L	452	1770	0.694	0.256	22.7	C	21.6	C
	LT	457	1790	0.610	0.256	20.8	C		
	R	405	1583	0.069	0.256	16.4	C		
NB	L	386	1770	0.334	0.333	12.1	B	6.0	B
	T	2525	3725	0.617	0.678	5.5	B		
SB	T	2856	5588	0.423	0.511	8.9	B	8.8	B
	R	809	1583	0.297	0.511	8.3	B		

Intersection Delay = 9.7 sec/veh Intersection LOS = B  
 Lost Time/Cycle, L = 6.0 sec Critical v/c(x) = 0.758

Streets: (E-W) IH 30 EBFR (N-S) Ridge Road  
 Analyst: GCL File Name: EBRB2.HC9  
 Area Type: Other 11-1-95 PM Peak  
 Comment: Base Traffic Volumes (Includes Walmart Study)

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	> 1	1					3	1		1	2
Volumes	510	119	50					502	186		70	906
Lane Width	12.0	12.0	12.0					12.0	12.0		12.0	12.0
RTOR Vols			0						0			0
Lost Time	3.00	3.00	3.00					3.00	3.00		3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left	*							
Thru	*							
Right	*							
Peds								
WB Left								
Thru								
Right								
Peds								
NB Right								
SB Right								
Green	31.0A				46.0A	13.0P		
Yellow/AR	0.0				0.0	0.0		
Cycle Length:	90 secs							

Phase combination order: #1 #5 #6

Intersection Performance Summary

Lane	Group:	Mvmts	Adj Sat	v/c	g/c	Delay	LOS	Approach:	
								Cap	Flow
EB	L	551	1770	0.596	0.311	21.2	C		
	LT	562	1806	0.594	0.311	21.2	C	20.9	C
	R	492	1583	0.108	0.311	16.8	C		
NB	T	2670	5588	0.218	0.478	10.4	B	10.5	B
	R	756	1583	0.259	0.478	10.7	B		
SB	L	516	1770	0.143	0.289	6.5	B	6.7	B
	T	2318	3725	0.432	0.622	6.8	B		

Intersection Delay = 11.8 sec/veh Intersection LOS = B  
 Lost Time/Cycle, L = 6.0 sec Critical v/c(x) = 0.487

Streets: (E-W) IH 30 EBFR (N-S) Ridge Road  
 Analyst: GCL File Name: EBRBD.HC9  
 Area Type: Other 11-1-95 PM Peak  
 Comment: Base + Development Traffic Volumes

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	> 1	1					3	1		1	2
Volumes	510	119	100					1023	360		70	1558
Lane Width	12.0	12.0	12.0					12.0	12.0		12.0	12.0
RTOR Vols			0						0			0
Lost Time	3.00	3.00	3.00					3.00	3.00		3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left	*							
Thru	*							
Right	*							
Peds								
WB Left								
Thru								
Right								
Peds								
NB Right								
SB Right								
Green	31.0A				46.0A	13.0P		
Yellow/AR	0.0				0.0	0.0		
Cycle Length:	90 secs							

Phase combination order: #1 #5 #6

Intersection Performance Summary

Lane	Group:	Adj Sat	v/c	g/C	Delay	LOS	Approach:	Delay	LOS
Mvmts	Cap	Flow	Ratio	Ratio					
EB	L	551	1770	0.596	0.311	21.2	C	20.7	C
	LT	562	1806	0.594	0.311	21.2	C		
	R	492	1583	0.213	0.311	17.4	C		
NB	T	2670	5588	0.444	0.478	11.9	B	12.1	B
	R	756	1583	0.501	0.478	12.7	B		
SB	L	339	1770	0.218	0.289	12.2	B	10.1	B
	T	2318	3725	0.743	0.622	10.0	B		

Intersection Delay = 12.8 sec/veh Intersection LOS = B  
 Lost Time/Cycle, L = 6.0 sec Critical v/c(x) = 0.694

<GID01>

"Base"

TEXAS DEPARTMENT OF HIGHWAYS AND PUBLIC TRANSPORTATION  
DIAMOND INTERCHANGE SIGNALIZATION - 145105

PASSER3

PASSER III-90

VER 1.0  
OCT 90

PPPP	AAA	SSS	SSS	EEEE	RRRR	IIIIIIIIIIIIIIIIII
P P	A A	S S	S S	E	R R	I I I
P P	A A	S	S	E	R R	I I I
PPPP	AAAAA	SSS	SSS	EEEE	RRRR	I I I
P	A A	S	S	E	R R	I I I
P	A A	S S	S S	E	R R	I I I
P	A A	SSS	SSS	EEEE	R R	IIIIIIIIIIIIIIIIII

\* \* \* \* \* GENERAL IDENTIFICATION DATA \* \* \* \* \*

FREEWAY NAME - - - IH 30

CITY NAME - - - - - ROCKWALL

DISTRICT NUMBER - - - - - 02

DATE - - - - - 11/01/95

RUN NUMBER - - - - - 01

<GID02>

\* \* \* \* \* ISOLATED INTERCHANGE OPERATION \* \* \* \* \*

\*\*\* PARAMETERS \*\*\*

NUMBER OF INTERCHANGES - - - - 1

LOWER CYCLE LIMIT (SEC) - - - - 90

UPPER CYCLE LIMIT (SEC) - - - - 100

CYCLE INCREMENT (SEC) - - - - 10

\*\*\* OPTIONS \*\*\*

OPTIMIZE INTERNAL OFFSETS ? - - YES

EVALUATE INTERNAL OFFSETS ? - - NO

\*\*\* LEFT-SIDE MOVEMENT DATA \*\*\*

```

* * * * *
TRAFFIC          VOLUME          SATURATION          MINIMUM
MOVEMENT        (VPH)            FLOW (VPHG)        PHASE (SEC)
* * * * *

```

ARTERIAL

RIGHT-TURN	129	1900	-
STRAIGHT-THROUGH	327	3800	10
STRAIGHT-THEN-LEFT	55	1900	-

FRONTAGE ROAD

RIGHT-TURN	14	1900	-
STRAIGHT-THROUGH	119	719	10
LEFT-THEN-STRAIGHT	510	3081	-
LEFT-THEN-LEFT	0	0	-

INTERIOR

LEFT-TURN	70	1900	5
STRAIGHT-THROUGH	737	3800	-



\*\*\* RIGHT-SIDE MOVEMENT DATA \*\*\*

```

* * * * *
      TRAFFIC          VOLUME          SATURATION          MINIMUM
      MOVEMENT        (VPH)            FLOW (VPHG)        PHASE (SEC)
* * * * *

```

ARTERIAL

RIGHT-TURN	228	1900	-
STRAIGHT-THROUGH	439	3800	10
STRAIGHT-THEN-LEFT	70	1900	-

FRONTAGE ROAD

RIGHT-TURN	27	1900	-
STRAIGHT-THROUGH	49	537	10
LEFT-THEN-STRAIGHT	298	3263	-
LEFT-THEN-LEFT	0	0	-

INTERIOR

LEFT-TURN	55	1900	5
STRAIGHT-THROUGH	837	3800	-

<DOI01>

\*\*\* INTERNAL DELAY-OFFSET INFORMATION \*\*\*

PHASING	OPTIMIZE?	FORCE?	INTERIOR QUEUE STORAGE	
LEAD-LEAD	Y	-	THROUGH MOVEMENT AT LEFT SIDE (VEH)	16
LAG -LEAD	Y	-	LEFT-TURN MOVEMENT AT LEFT SIDE (VEH)	8
LEAD-LAG	Y	-	THROUGH MOVEMENT AT RIGHT SIDE (VEH)	16
LAG -LAG	Y	-	LEFT-TURN MOVEMENT AT RIGHT SIDE (VEH)	8
TTI -LEAD	Y	-		

PERMITTED LEFT TURNS?	INTERIOR TRAVEL TIMES
LEFT-SIDE INTERSECTION YES	LEFT TO RIGHT (SEC) - - - - - 10
RIGHT-SIDE INTERSECTION YES	RIGHT TO LEFT (SEC) - - - - - 10

<GSI01>

\*\*\* INTERCHANGE 1 RIDGE ROAD

RUN 01 PAGE 4A

\*\*\* GENERAL SIGNALIZATION INFORMATION \*\*\*

MEASURES OF EFFECTIVENESS	LEFT-SIDE INTERSECTION				*	RIGHT-SIDE INTERSECTION			
	A	B	C	A+C		A	B	C	A+C
PHASE TIME (SEC)	33.6	40.1	16.3	49.9	*	48.8	26.9	14.3	63.1
V/C RATIO	.26	.41	.27	.38	*	.24	.36	.25	.34
LEVEL OF SERVICE	A	A	A	A	*	A	A	A	A
DELAY (SEC/VEH)	22.21	20.08	11.27	6.60	*	12.93	28.45	5.48	2.98
LEVEL OF SERVICE	C	C	B	B	*	B	C	A	A
STORAGE RATIO			.08	.26	*			.14	.17
LEVEL OF SERVICE			B	C	*			C	C
PHASE ORDER	LEAD-LAG			TOTAL INTERCHANGE DELAY		14.44			VEH-HRS/HR
INTERNAL OFFSET	4 SEC			CYCLE LENGTH		90 SEC			

*HCS → LOS B = 13.3 sec/veh*  
*EBFR*  
*WSPR*

<SPI01>

\*\*\* INTERCHANGE 1 RIDGE ROAD

*w/out development w devel. timing*  
*EBFR = LOS B / 12.0*  
*WBFR = LOS B / 8.0*

RUN 01 PAGE 4B

\*\*\* SIGNAL PHASING INFORMATION \*\*\*

PHASE INTERVAL NUMBER	LEFT-SIDE SEQUENCE			*	RIGHT-SIDE SEQUENCE			PHASE INTERVAL LENGTH (SEC)
	A	B	C		A	C	B	
1	<----		<----	*	<----	^	^	4.00
2				*				29.60
3		V	V	*				19.20
4	---->			*	---->	----	----	14.30
5				*				6.60
6				*				16.30

INTERNAL OFFSET 4 SEC      CYCLE LENGTH 90 SEC  
 PHASE ORDER LEAD-LAG

<GSI01>

\*\*\* INTERCHANGE 1 RIDGE ROAD

RUN 01 PAGE 5A

\*\*\* GENERAL SIGNALIZATION INFORMATION \*\*\*

MEASURES OF EFFECTIVENESS	LEFT-SIDE INTERSECTION				*	RIGHT-SIDE INTERSECTION			
	A	B	C	A+C		A	B	C	A+C
PHASE TIME (SEC)	37.4	44.8	17.8	55.2	*	54.5	29.9	15.6	70.1
V/C RATIO	.26	.41	.27	.38	*	.24	.35	.25	.33
LEVEL OF SERVICE	A	A	A	A	*	A	A	A	A
DELAY (SEC/VEH)	24.30	21.68	14.14	8.03	*	13.95	31.06	5.03	1.80
LEVEL OF SERVICE	C	C	B	B	*	B	C	A	A
STORAGE RATIO			.18	.55	*			.08	.10
LEVEL OF SERVICE			C	E	*			B	B
PHASE ORDER	LAG -LEAD			TOTAL INTERCHANGE DELAY			15.57 VEH-HRS/HR		
INTERNAL OFFSET	65 SEC			CYCLE LENGTH			100 SEC		

<SPI01>

\*\*\* INTERCHANGE 1 RIDGE ROAD

RUN 01 PAGE 5B

\*\*\* SIGNAL PHASING INFORMATION \*\*\*

PHASE INTERVAL NUMBER	LEFT-SIDE SEQUENCE			*	RIGHT-SIDE SEQUENCE			PHASE INTERVAL LENGTH (SEC)
	A	C	B		A	B	C	
1		A			A		35.10	
2		A			B		2.30	
3		C			B		17.80	
4		B			B		9.80	
5		B			C		15.60	
6		B			A		19.40	
INTERNAL OFFSET	65 SEC				CYCLE LENGTH 100 SEC			
					PHASE ORDER LAG -LEAD			

<GID01> " Base Plus Development"

TEXAS DEPARTMENT OF HIGHWAYS AND PUBLIC TRANSPORTATION  
DIAMOND INTERCHANGE SIGNALIZATION - 145105

PASSER3

PASSER III-90

VER 1.0  
OCT 1990

PPPP	AAA	SSS	SSS	EEEE	RRRR	IIIIIIIIIIIIIIII
P P	A A	S S	S S	E	R R	I I I
P P	A A	S	S	E	R R	I I I
PPPP	AAAAA	SSS	SSS	EEEE	RRRR	I I I
P	A A	S	S	E	R R	I I I
P	A A	S S	S S	E	R R	I I I
P	A A	SSS	SSS	EEEE	R R	IIIIIIIIIIIIIIII

\* \* \* \* \* GENERAL IDENTIFICATION DATA \* \* \* \* \*

FREEWAY NAME - - - IH 30

CITY NAME - - - - - ROCKWALL

DISTRICT NUMBER - - - - - 00

DATE - - - - - 11/01/95

RUN NUMBER - - - - - 02

<GID02>

\* \* \* \* \* ISOLATED INTERCHANGE OPERATION \* \* \* \* \*

\*\*\* PARAMETERS \*\*\*

NUMBER OF INTERCHANGES - - - - 1

LOWER CYCLE LIMIT (SEC) - - - - 90

UPPER CYCLE LIMIT (SEC) - - - - 100

CYCLE INCREMENT (SEC) - - - - 10

\*\*\* OPTIONS \*\*\*

OPTIMIZE INTERNAL OFFSETS ? - - YES

EVALUATE INTERNAL OFFSETS ? - - NO

<IMD01A>

\* \* \* INTERCHANGE 1 RIDGE ROAD

RUN 02 PAGE 2A

\*\*\* LEFT-SIDE MOVEMENT DATA \*\*\*

TRAFFIC MOVEMENT	VOLUME (VPH)	SATURATION FLOW (VPHG)	MINIMUM PHASE (SEC)
ARTERIAL			
RIGHT-TURN	251	1900	-
STRAIGHT-THROUGH	649	3800	10
STRAIGHT-THEN-LEFT	95	1900	-
FRONTAGE ROAD			
RIGHT-TURN	56	1900	-
STRAIGHT-THROUGH	119	719	10
LEFT-THEN-STRAIGHT	510	3081	-
LEFT-THEN-LEFT	0	0	-
INTERIOR			
LEFT-TURN	70	1900	5
STRAIGHT-THROUGH	1196	3800	-

<IMD01B>

\* \* \* INTERCHANGE 1 RIDGE ROAD

RUN 02 PAGE 2B

\*\*\* RIGHT-SIDE MOVEMENT DATA \*\*\*

TRAFFIC MOVEMENT	VOLUME (VPH)	SATURATION FLOW (VPHG)	MINIMUM PHASE (SEC)
ARTERIAL			
RIGHT-TURN	228	1900	-
STRAIGHT-THROUGH	773	3800	10
STRAIGHT-THEN-LEFT	70	1900	-
FRONTAGE ROAD			
RIGHT-TURN	27	1900	-
STRAIGHT-THROUGH	49	394	10
LEFT-THEN-STRAIGHT	423	3406	-
LEFT-THEN-LEFT	0	0	-
INTERIOR			
LEFT-TURN	95	1900	5
STRAIGHT-THROUGH	1159	3800	-

<DOI01>

\* \* \* INTERCHANGE 1 RIDGE ROAD

RUN 02 PAGE 3

\*\*\* INTERNAL DELAY-OFFSET INFORMATION \*\*\*

* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *
PHASING	OPTIMIZE?	FORCE?													INTERIOR QUEUE STORAGE				

LEAD-LEAD	Y	-	THROUGH MOVEMENT AT LEFT SIDE (VEH)												16
-----------	---	---	-------------------------------------	--	--	--	--	--	--	--	--	--	--	--	----

LAG -LEAD	Y	-	LEFT-TURN MOVEMENT AT LEFT SIDE (VEH)												8
-----------	---	---	---------------------------------------	--	--	--	--	--	--	--	--	--	--	--	---

LEAD-LAG	Y	-	THROUGH MOVEMENT AT RIGHT SIDE (VEH)												16
----------	---	---	--------------------------------------	--	--	--	--	--	--	--	--	--	--	--	----

LAG -LAG	Y	-	LEFT-TURN MOVEMENT AT RIGHT SIDE (VEH)												8
----------	---	---	--	--	--	--	--	--	--	--	--	--	--	--	---

TTI -LEAD	Y	-													
-----------	---	---	--	--	--	--	--	--	--	--	--	--	--	--	--

* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *	* * * * *
PERMITTED LEFT TURNS?															INTERIOR TRAVEL TIMES				

LEFT-SIDE INTERSECTION	YES		LEFT TO RIGHT (SEC)												- - - - - 10
------------------------	-----	--	---------------------	--	--	--	--	--	--	--	--	--	--	--	--------------

RIGHT-SIDE INTERSECTION	YES		RIGHT TO LEFT (SEC)												- - - - - 10
-------------------------	-----	--	---------------------	--	--	--	--	--	--	--	--	--	--	--	--------------

<GSI01>

\*\*\* INTERCHANGE 1 RIDGE ROAD

RUN 02 PAGE 4A

\*\*\* GENERAL SIGNALIZATION INFORMATION \*\*\*

MEASURES OF EFFECTIVENESS	LEFT-SIDE INTERSECTION				*	RIGHT-SIDE INTERSECTION			
	A	B	C	A+C		A	B	C	A+C
PHASE TIME (SEC)	46.3	30.9	12.8	59.1	*	48.7	26.5	14.8	63.5
V/C RATIO	.36	.55	.38	.51	*	.41	.50	.42	.46
LEVEL OF SERVICE	A	A	A	A	*	A	A	A	A
DELAY (SEC/VEH)	15.32	29.54	13.77	4.23	*	14.42	33.07	7.94	3.28
LEVEL OF SERVICE	B	C	B	A	*	B	D	B	A
STORAGE RATIO			.17	.33	*			.14	.27
LEVEL OF SERVICE			C	D	*			C	C
PHASE ORDER	LAG -LEAD			TOTAL INTERCHANGE DELAY		20.51		VEH-HRS/HR	
INTERNAL OFFSET	64 SEC			CYCLE LENGTH		90 SEC			

EBFR ⇒ LOS B - 11.9 sec/veh  
 WBFR ⇒ LOS B / 8.8

<SPI01>

\*\*\* INTERCHANGE 1 RIDGE ROAD

RUN 02 PAGE 4B

\*\*\* SIGNAL PHASING INFORMATION \*\*\*

PHASE INTERVAL NUMBER	LEFT-SIDE SEQUENCE			*	RIGHT-SIDE SEQUENCE			PHASE INTERVAL LENGTH (SEC)
	A	C	B		A	B	C	
1	<-----	<-----		<-----	^	^	37.50	
2							8.80	
3		----->	V			----->	12.80	
4			V				4.90	
5				----->			14.80	
6							11.20	

Handwritten calculations:  
 A > 46.3 = 46  
 C - 12.8 = 13  
 B > 30.9 = 31  
 A - 48.7 = 49  
 B > 26.5 = 26  
 C - 14.8 = 15

INTERNAL OFFSET 64 SEC

CYCLE LENGTH 90 SEC  
PHASE ORDER LAG -LEAD



<GSI01>

\* \* \* INTERCHANGE 1 RIDGE ROAD

RUN 02 PAGE 5A

\*\*\* GENERAL SIGNALIZATION INFORMATION \*\*\*

MEASURES OF EFFECTIVENESS	LEFT-SIDE INTERSECTION				*	RIGHT-SIDE INTERSECTION			
	A	B	C	A+C		A	B	C	A+C
PHASE TIME (SEC)	51.7	34.4	13.9	65.6	*	54.4	29.4	16.2	70.6
V/C RATIO	.36	.54	.37	.51	*	.40	.49	.41	.46
LEVEL OF SERVICE	A	A	A	A	*	A	A	A	A
DELAY (SEC/VEH)	16.56	31.81	13.97	4.42	*	15.55	35.64	7.77	3.15
LEVEL OF SERVICE	B	C	B	A	*	B	D	B	A
STORAGE RATIO			.17	.34	*			.14	.26
LEVEL OF SERVICE			C	D	*			C	C
PHASE ORDER	LAG -LEAD			TOTAL INTERCHANGE DELAY			22.03 VEH-HRS/HR		
INTERNAL OFFSET	73 SEC			CYCLE LENGTH			100 SEC		

<SPI01>

\* \* \* INTERCHANGE 1 RIDGE ROAD

RUN 02 PAGE 5B

\*\*\* SIGNAL PHASING INFORMATION \*\*\*

PHASE INTERVAL NUMBER	LEFT-SIDE SEQUENCE			*	RIGHT-SIDE SEQUENCE			PHASE INTERVAL LENGTH (SEC)
	A	C	B		A	B	C	
1		A		*	A		43.60	
2		A		*	B		8.10	
3		C		*	B		13.90	
4		B		*	B		7.40	
5		B		*	C		16.20	
6		B		*	A		10.80	
INTERNAL OFFSET	73 SEC				CYCLE LENGTH 100 SEC			
					PHASE ORDER LAG -LEAD			

Center For Microcomputers In Transportation

HCS: Unsignalized Intersection Release 2.1

\*\*\*\*\*

File Name ..... RCB.HC0  
 Streets: (N-S) Ridge Road (E-W) Chili's Driveway  
 Major Street Direction.... NS  
 Length of Time Analyzed... 60 (min)  
 Analyst..... GCL  
 Date of Analysis..... 11/1/95  
 Other Information..... Base Traffic Volumes

Two-way Stop-controlled Intersection

	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	0	3<	0	1	2	0	0	0	0	0>	1<	0
Stop/Yield			N			N						
Volumes		482	7	10	741					9	0	19
PHF		.95	.95	.95	.95					.95	.95	.95
Grade		0			0			0			0	
MC's (%)		0	0	0	0					0	0	0
SU/RV's (%)		0	0	0	0					0	0	0
CV's (%)		0	0	0	0					0	0	0
PCE's		1.1	1.1	1.1	1.1					1.1	1.1	1.1

Adjustment Factors

Vehicle Maneuver	Critical Gap (tg)	Follow-up Time (tf)
Left Turn Major Road	5.50	2.10
Right Turn Minor Road	5.50	2.60
Through Traffic Minor Road	6.50	3.30
Left Turn Minor Road	7.00	3.40

\*\*\*\*\*

WorkSheet for TWSC Intersection

-----		
Step 1: RT from Minor Street	WB	EB
-----		
Conflicting Flows: (vph)	164	
Potential Capacity: (pcph)	1143	
Movement Capacity: (pcph)	1143	
Prob. of Queue-free State:	0.98	
-----		
Step 2: LT from Major Street	SB	NB
-----		
Conflicting Flows: (vph)	489	
Potential Capacity: (pcph)	937	
Movement Capacity: (pcph)	937	
Prob. of Queue-free State:	0.99	
-----		
Step 3: TH from Minor Street	WB	EB
-----		
Conflicting Flows: (vph)	1236	
Potential Capacity: (pcph)	206	
Capacity Adjustment Factor due to Impeding Movements	0.99	
Movement Capacity: (pcph)	203	
Prob. of Queue-free State:	1.00	
-----		
Step 4: LT from Minor Street	WB	EB
-----		
Conflicting Flows: (vph)	1236	
Potential Capacity: (pcph)	172	
Major LT, Minor TH Impedance Factor:	0.99	
Adjusted Impedance Factor:	0.99	
Capacity Adjustment Factor due to Impeding Movements	0.99	
Movement Capacity: (pcph)	170	
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Intersection Performance Summary

Movement	FlowRate v (pcph)	MoveCap Cm (pcph)	SharedCap Csh (pcph)	Avg.Total Delay	LOS	Delay By App
WB L	10	170 >	410	> 9.5	> B	9.5
WB R	22	1143 >		>	>	
SB L	12	937		3.9	A	0.1

Intersection Delay = 0.2

Center For Microcomputers In Transportation

File Name ..... RCBD.HCO  
 Streets: (N-S) Ridge Road (E-W) Chili's Driveway  
 Major Street Direction.... NS  
 Length of Time Analyzed... 60 (min)  
 Analyst..... GCL  
 Date of Analysis..... 11/1/95  
 Other Information..... Base Plus Development Traffic Volumes

Two-way Stop-controlled Intersection

	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	0	3<	0	0	2	0	0	0	0	0	0	1
Stop/Yield			N			N						
Volumes		462	67		1410							77
PHF		.95	.95		.95							.95
Grade		0			0			0			0	
MC's (%)		0	0		0							0
SU/RV's (%)		0	0		0							0
CV's (%)		0	0		0							0
PCE's		1.1	1.1		1.1							1.1

Adjustment Factors

Vehicle Maneuver	Critical Gap (tg)	Follow-up Time (tf)
Left Turn Major Road	5.50	2.10
Right Turn Minor Road	5.50	2.60
Through Traffic Minor Road	6.50	3.30
Left Turn Minor Road	7.00	3.40

\*\*\*\*\*

WorkSheet for TWSC Intersection

Step 1: RT from Minor Street	WB	EB
Conflicting Flows: (vph)	188	
Potential Capacity: (pcph)	1112	
Movement Capacity: (pcph)	1112	
Prob. of Queue-free State:	0.92	

Intersection Performance Summary

Movement	FlowRate v(pcph)	MoveCap Cm(pcph)	SharedCap Csh(pcph)	Avg.Total Delay	LOS	Delay By App
WB R	89	1112		3.5	A	

Intersection Delay = 0.1

Center For Microcomputers In Transportation

HCS: Unsignalized Intersection Release 2.1

Page 1

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File Name ..... RNDBD.HCO  
 Streets: (N-S) Ridge Road (E-W) North Driveway  
 Major Street Direction.... NS  
 Length of Time Analyzed... 60 (min)  
 Analyst..... GCL  
 Date of Analysis..... 11/6/95  
 Other Information..... Base + Development Traffic Volumes

Two-way Stop-controlled Intersection

	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	2<	0	1	2<	0	0	0	1	0	0	1
Stop/Yield			N			N						
Volumes	17	1120	55	256	1260	142			42			250
PHF	.95	.95	.95	.95	.95	.95			.95			.95
Grade		0			0			0			0	
MC's (%)	0	0	0	0	0	0			0			0
SU/RV's (%)	0	0	0	0	0	0			0			0
CV's (%)	0	0	0	0	0	0			0			0
PCE's	1.1	1.1	1	1	1.1	1			1			1

Adjustment Factors

Vehicle Maneuver	Critical Gap (tg)	Follow-up Time (tf)
Left Turn Major Road	5.50	2.10
Right Turn Minor Road	5.50	2.60
Through Traffic Minor Road	6.50	3.30
Left Turn Minor Road	7.00	3.40



\*\*\*\*\*

WorkSheet for TWSC Intersection

Step 1: RT from Minor Street	WB	EB
Conflicting Flows: (vph)	588	701
Potential Capacity: (pcph)	697	611
Movement Capacity: (pcph)	697	611
Prob. of Queue-free State:	0.62	0.93
Step 2: LT from Major Street	SB	NB
Conflicting Flows: (vph)	1175	1402
Potential Capacity: (pcph)	401	303
Movement Capacity: (pcph)	401	303
Prob. of Queue-free State:	0.33	0.93

Intersection Performance Summary

Movement	FlowRate v(pcph)	MoveCap Cm(pcph)	SharedCap Csh(pcph)	Avg.Total Delay	LOS	Delay By App
EB R	44	611		6.3	B	
WB R	263	697		8.3	B	
NB L	20	303		12.7	C	0.2
SB L	269	401		26.7	D	4.1

Intersection Delay = 3.0

Center For Microcomputers In Transportation

HCS: Unsignalized Intersection Release 2.1

Page 1

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File Name ..... RNDE.HCO  
 Streets: (N-S) Ridge Road (E-W) Carlisle Plaza North  
 Major Street Direction.... NS  
 Length of Time Analyzed... 60 (min)  
 Analyst..... GCL  
 Date of Analysis..... 11/7/95  
 Other Information..... Existing Traffic Volumes - PM Peak Hour

Two-way Stop-controlled Intersection

	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	1	0	0	1<	0	0>	1<	0	0	0	0
Stop/Yield			N			N						
Volumes	17	431		1009	142		29	0	42			
PHF	.95	.95		.95	.95		.95	.95	.95			
Grade		0		0			0				0	
MC's (%)	0	0		0	0		0	0	0			
SU/RV's (%)	0	0		0	0		0	0	0			
CV's (%)	0	0		0	0		0	0	0			
PCE's	1.1	1.1		1.1	1.1		1.1	1.1	1.1			

Adjustment Factors

Vehicle Maneuver	Critical Gap (tg)	Follow-up Time (tf)
Left Turn Major Road	5.00	2.10
Right Turn Minor Road	5.50	2.60
Through Traffic Minor Road	6.00	3.30
Left Turn Minor Road	6.50	3.40

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WorkSheet for TWSC Intersection

Step 1: RT from Minor Street	WB	EB
Conflicting Flows: (vph)		1080
Potential Capacity: (pcph)		393
Movement Capacity: (pcph)		393
Prob. of Queue-free State:		0.88
Step 2: LT from Major Street	SB	NB
Conflicting Flows: (vph)		1151
Potential Capacity: (pcph)		485
Movement Capacity: (pcph)		485
Prob. of Queue-free State:		0.96
Step 3: TH from Minor Street	WB	EB
Conflicting Flows: (vph)		1528
Potential Capacity: (pcph)		172
Capacity Adjustment Factor due to Impeding Movements		0.96
Movement Capacity: (pcph)		165
Prob. of Queue-free State:		1.00
Step 4: LT from Minor Street	WB	EB
Conflicting Flows: (vph)		1528
Potential Capacity: (pcph)		138
Major LT, Minor TH Impedance Factor:		0.96
Adjusted Impedance Factor:		0.96
Capacity Adjustment Factor due to Impeding Movements		0.96
Movement Capacity: (pcph)		132

Intersection Performance Summary

Movement	FlowRate v (pcph)	MoveCap Cm (pcph)	SharedCap Csh (pcph)	Avg.Total Delay	LOS	Delay By App
EB L	34	132	>	>	>	
EB R	48	393	>	>	>	
			216	26.8	D	26.8
NB L	20	485		7.7	B	0.3

Intersection Delay = 1.2

Center For Microcomputers In Transportation

HCS: Unsignalized Intersection Release 2.1

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File Name ..... RSTB.HC0  
 Streets: (N-S) Ridge Road (E-W) Carlisle Plaza Drive  
 Major Street Direction.... NS  
 Length of Time Analyzed... 60 (min)  
 Analyst..... GCL  
 Date of Analysis..... 11/6/95  
 Other Information..... Existing Traffic w/ Walmart & Widened Ridge Road

Two-way Stop-controlled Intersection

	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	2	0	0	2<	0	0>	1<	0	0	0	0
Stop/Yield			N			N						
Volumes	71	630		763	111		53	0	118			
PHF	.95	.95		.95	.95		.95	.95	.95			
Grade		0		0			0			0		
MC's (%)	0	0		0	0		0	0	0			
SU/RV's (%)	0	0		0	0		0	0	0			
CV's (%)	0	0		0	0		0	0	0			
PCE's	1.1	1.1		1.1	1.1		1.1	1.1	1.1			

Adjustment Factors

Vehicle Maneuver	Critical Gap (tg)	Follow-up Time (tf)
Left Turn Major Road	5.50	2.10
Right Turn Minor Road	5.50	2.60
Through Traffic Minor Road	6.50	3.30
Left Turn Minor Road	7.00	3.40

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## WorkSheet for TWSC Intersection

Step 1: RT from Minor Street	WB	EB
Conflicting Flows: (vph)		437
Potential Capacity: (pcph)		832
Movement Capacity: (pcph)		832
Prob. of Queue-free State:		0.84
Step 2: LT from Major Street	SB	NB
Conflicting Flows: (vph)		874
Potential Capacity: (pcph)		582
Movement Capacity: (pcph)		582
Prob. of Queue-free State:		0.86
Step 3: TH from Minor Street	WB	EB
Conflicting Flows: (vph)		1520
Potential Capacity: (pcph)		141
Capacity Adjustment Factor due to Impeding Movements		0.86
Movement Capacity: (pcph)		121
Prob. of Queue-free State:		1.00
Step 4: LT from Minor Street	WB	EB
Conflicting Flows: (vph)		1520
Potential Capacity: (pcph)		113
Major LT, Minor TH Impedance Factor:		0.86
Adjusted Impedance Factor:		0.86
Capacity Adjustment Factor due to Impeding Movements		0.86
Movement Capacity: (pcph)		97

Intersection Performance Summary

Movement	FlowRate v (pcph)	MoveCap Cm (pcph)	SharedCap Csh (pcph)	Avg.Total Delay	LOS	Delay By App
EB L	62	97 >	247	> 66.0	> F	66.0
EB R	136	832 >		>	>	
NB L	83	582		7.2	B	0.7
Intersection Delay =				6.8		



Streets: (E-W) Steger Towne Drive (N-S) Ridge Road  
 Analyst: GCL File Name: RSTBD.HC9  
 Area Type: Other 11-1-95 PM Peak  
 Comment: Base + Development Traffic Volumes

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	> 1		1	> 1		1	1	2	1	1	2	<
Volumes	62	9	118	381	9	389	71	738	224	633	820	111
Lane Width		12.0	12.0		12.0	12.0	12.0	12.0	12.0	12.0	12.0	
RTOR Vols			22			78			0			0
Lost Time	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left	*							
EB Thru	*							
EB Right	*							
EB Peds								
WB Left		*						
WB Thru		*						
WB Right		*						
WB Peds								
NB Right								
SB Right								
Green	9.0A	21.0A			5.0A	29.0P	20.0A	
Yellow/AR	0.0	3.0			0.0	0.0	3.0	
Cycle Length:	90 secs							

Phase combination order: #1 #2 #5 #6 #7

Intersection Performance Summary

Lane	Group:	Adj Sat	v/c	g/C	Delay	LOS	Approach:	Delay	LOS
Mvmts	Cap	Flow	Ratio	Ratio					
EB	LT	119	1784	0.622	0.067	37.6	D	65.4	F
	R	106	1583	0.957	0.067	85.8	F		
WB	LT	414	1776	0.989	0.233	57.5	E	50.0	E
	R	369	1583	0.885	0.233	40.6	E		
NB	L	135	1770	0.556	0.044	24.5	C	41.8	E
	T	828	3725	0.986	0.222	47.4	E		
	R	352	1583	0.671	0.222	27.7	D		
SB	L	692	1770	0.962	0.722	35.6	D	20.1	C
	TR	1992	3659	0.517	0.544	10.1	B		

Intersection Delay = 34.7 sec/veh Intersection LOS = D  
 Lost Time/Cycle, L = 12.0 sec Critical v/c(x) = 0.990

Streets: (E-W) Steger Towne Drive (N-S) Ridge Road  
 Analyst: GCL File Name: RSTBDI.HC9  
 Area Type: Other 11-1-95 PM Peak  
 Comment: Base + Development Traffic Volumes

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	> 1		1	> 1		1	1	2	1	2	2	<
Volumes	62	9	118	381	9	389	71	738	224	633	820	111
Lane Width		12.0	12.0		12.0	12.0	12.0	12.0	12.0	12.0	12.0	
RTOR Vols			0			0			0			0
Lost Time	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left	*							
Thru	*							
Right	*							
Peds								
WB Left		*						
Thru		*						
Right		*						
Peds								
NB Right		*						
SB Right								
Green	9.0A	23.0A			6.0A	22.0P	24.0A	
Yellow/AR	0.0	3.0			0.0	0.0	3.0	
Cycle Length:	90 secs Phase combination order: #1 #2 #5 #6 #7							

Intersection Performance Summary

Lane	Group:	Adj Sat	v/c	g/C	Delay	LOS	Approach:	Delay	LOS
Mvmts	Cap	Flow	Ratio	Ratio					
EB	LT	119	1784	0.622	0.067	37.6	D	42.1	E
	R	158	1583	0.783	0.100	44.8	E		
WB	LT	454	1776	0.903	0.256	39.6	D	24.3	C
	R	897	1583	0.456	0.567	8.9	B		
NB	L	152	1770	0.493	0.067	19.6	C	23.1	C
	T	993	3725	0.821	0.267	27.5	D		
	R	827	1583	0.285	0.522	9.2	B		
SB	L	983	3539	0.698	0.278	23.7	C	16.5	C
	TR	1870	3659	0.550	0.511	11.6	B		

Intersection Delay = 21.4 sec/veh Intersection LOS = C  
 Lost Time/Cycle, L = 12.0 sec Critical v/c(x) = 0.791

Streets: (E-W) Horizon Road  
 Analyst: GCL  
 Area Type: Other  
 Comment: Base Traffic Volumes

(N-S) Ridge Road  
 File Name: RHB.HC9  
 11-1-95 PM Peak

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	1	1	1	2	<	1	1	1	1	2	<
Volumes	1	117	170	40	58	167	86	533	18	273	557	3
Lane Width	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0	12.0	12.0	
RTOR Vols			0			0			0			0
Lost Time	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00

Signal Operations											
Phase Combination	1	2	3	4	5	6	7	8			
EB Left	*										
EB Thru	*										
EB Right	*										
EB Peds											
WB Left		*									
WB Thru		*									
WB Right		*									
WB Peds											
NB Right											
SB Right											
Green	25.0A				5.0A	16.0P	44.0A				
Yellow/AR	0.0				0.0	0.0	0.0				
Cycle Length:	90 secs	Phase combination order: #1 #5 #6 #7									

Intersection Performance Summary									
Lane	Group:	Adj Sat	v/c	g/C	Delay	LOS	Approach:	Delay	LOS
Mvmts	Cap	Flow	Ratio	Ratio					
EB	L	220	902	0.005	0.244	19.5	C	22.0	C
	T	455	1863	0.270	0.244	21.0	C		
	R	387	1583	0.463	0.244	22.7	C		
WB	L	274	1121	0.153	0.244	20.3	C	21.1	C
	TR	809	3310	0.308	0.244	21.2	C		
NB	L	278	1770	0.327	0.044	10.2	B	14.9	B
	T	849	1863	0.661	0.456	15.9	C		
	R	721	1583	0.026	0.456	10.3	B		
SB	L	437	1770	0.657	0.433	17.6	C	9.4	B
	TR	2358	3723	0.262	0.633	5.5	B		
Intersection Delay =					14.4 sec/veh	Intersection LOS =		B	
Lost Time/Cycle, L =					9.0 sec	Critical v/c(x) =		0.640	

Streets: (E-W) Horizon Road (N-S) Ridge Road  
 Analyst: GCL File Name: RHBD.HC9  
 Area Type: Other 11-1-95 PM Peak  
 Comment: Base +Development Traffic Volumes

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	1	1	1	2	<	1	1	1	1	2	<
Volumes	37	141	170	156	81	191	86	805	42	296	731	38
Lane Width	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0	12.0	12.0	
RTOR Vols			0			0			0			0
Lost Time	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left	*							
EB Thru	*							
EB Right	*							
EB Peds								
WB Left	*							
WB Thru	*							
WB Right	*							
WB Peds								
NB Right								
SB Right								
Green	22.0A							
Yellow/AR	0.0				5.0A	18.0P	45.0A	
Cycle Length:	90 secs				0.0	0.0	0.0	

Phase combination order: #1 #5 #6 #7

Intersection Performance Summary

Lane	Group:	Adj Sat	v/c	g/C	Delay	LOS	Approach:	
							Flow	Ratio
EB	L	157	0.249	0.211	22.6	C	24.3	C
	T	393	0.376	0.211	23.4	C		
	R	334	0.536	0.211	25.3	D		
WB	L	202	0.813	0.211	40.4	E	29.6	D
	TR	704	0.426	0.211	23.7	C		
NB	L	183	0.497	0.044	11.3	B	32.5	D
	T	869	0.974	0.467	36.0	D		
	R	739	0.060	0.467	10.0	B		
SB	L	476	0.655	0.478	19.4	C	8.9	B
	TR	2465	0.344	0.667	5.0	A		

Intersection Delay = 21.8 sec/veh Intersection LOS = C  
 Lost Time/Cycle, L = 9.0 sec Critical v/c(x) = 0.892

File Name ..... HSDBD.HCO  
 Streets: (N-S) South Driveway (E-W) Horizon Road  
 Major Street Direction.... EW  
 Length of Time Analyzed... 60 (min)  
 Analyst..... GCL  
 Date of Analysis..... 11/7/95  
 Other Information..... Base + Development PM Peak Hour

Two-way Stop-controlled Intersection

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	0>	1	0	0	1<	0	0	0	0	0>	1<	0
Stop/Yield			N			N						
Volumes	48	431			289	95				92	0	139
PHF	.95	.95			.95	.95				.95	.95	.95
Grade		0			0			0			0	
MC's (%)	0	0			0	0				0	0	0
SU/RV's (%)	0	0			0	0				0	0	0
CV's (%)	0	0			0	0				0	0	0
PCE's	1.1	1.1			1.1	1.1				1.1	1.1	1.1

Adjustment Factors

Vehicle Maneuver	Critical Gap (tg)	Follow-up Time (tf)
Left Turn Major Road	5.00	2.10
Right Turn Minor Road	5.50	2.60
Through Traffic Minor Road	6.00	3.30
Left Turn Minor Road	6.50	3.40

WorkSheet for TWSC Intersection

-----		
Step 1: RT from Minor Street	NB	SB
-----		
Conflicting Flows: (vph)		336
Potential Capacity: (pcph)		936
Movement Capacity: (pcph)		936
Prob. of Queue-free State:		0.83
-----		
Step 2: LT from Major Street	WB	EB
-----		
Conflicting Flows: (vph)		384
Potential Capacity: (pcph)		1125
Movement Capacity: (pcph)		1125
Prob. of Queue-free State:		0.95
TH Saturation Flow Rate: (pcphpl)		1700
RT Saturation Flow Rate: (pcphpl)		
Major LT Shared Lane Prob. of Queue-free State:		0.93
-----		
Step 3: TH from Minor Street	NB	SB
-----		
Conflicting Flows: (vph)		816
Potential Capacity: (pcph)		407
Capacity Adjustment Factor due to Impeding Movements		0.93
Movement Capacity: (pcph)		378
Prob. of Queue-free State:		1.00
-----		
Step 4: LT from Minor Street	NB	SB
-----		
Conflicting Flows: (vph)		816
Potential Capacity: (pcph)		357
Major LT, Minor TH Impedance Factor:		0.93
Adjusted Impedance Factor:		0.93
Capacity Adjustment Factor due to Impeding Movements		0.93
Movement Capacity: (pcph)		332
-----		

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Intersection Performance Summary

Movement	FlowRate v (pcph)	MoveCap Cm (pcph)	SharedCap Csh (pcph)	Avg.Total Delay	LOS	Delay By App
SB L	107	332 >		>	>	
SB R	161	936 >	542	>	C	13.1
EB L	56	1125		3.4	A	0.3

Intersection Delay = 2.9



**PARAGON PROJECT RESOURCES, INC.**  
**ENGINEERING AND MANAGEMENT CONSULTANTS**

May 14, 1996

Mr. Tony R. Tramel, P.E.  
DeShazo Tang & Associates  
Dallas, TX 75202

Via Fax: 741-1937

Dear Tony:

There is no issue with the proposed zoning or platting for Steger Towne crossing, nor do I have any problems with your projected traffic volumes, etc. My personal problem is with the number of drives on the primary arterial through that area.

Your understanding of City guidance on driveway spacing is not complete. An extract of the Commercial District section of the Rockwall Zoning Ordinance is attached for your future use. The standard is 1 drive per 200 feet of street frontage *per site* for arterial streets, or as approved by the City Council. If a "site" is an outparcel, the concept plan is in compliance. If a "site" is the shopping center, the plan requires City Council approval.

As you know, neither PARAGON nor I are traffic engineers, and our reference library is not as extensive as yours. My opinions expressed at the work session were based on my understanding of the Zoning Ordinance and the brief guidance provided in the AASHTO green book (an extract attached). As you know, most of the AASHTO discussion is very general, and Figure II-29 is the only numerical data I found (and I gather from the information you provided, even its conclusions are in dispute). If we believe Figure II-29, then additional drives cause additional accidents. The density of proposed and existing drives onto FM 740 from Steger Towne Crossing and the bank would amount to 28 intersections per kilometer (if it were extended for a kilometer), which I think we would all agree is excessive. For these reasons I asked you to relook your previous recommendations.

Bill Crolley tells me that the concept plan has been previously approved by the Planning and Zoning Commission and the City Council. In that case, the appropriate thing for me to do is abstain on both the Steger Towne and the Boston Market agenda items.

Sincerely,

G. William Quinby, P.E.  
Planning and Zoning Commissioner

cc: Bill Crolley, via Fax 771-7727



Some degree of access control or access management should be included in the development of any street or highway, particularly a new facility where the likelihood of commercial development exists. The type of street or highway to be built should be coordinated with the local land use plan to ensure that the desired control of access can be maintained through local zoning ordinances or subdivision regulations. The control of access may range from minimum driveway regulations to full control. Thus the extent and degree of access control

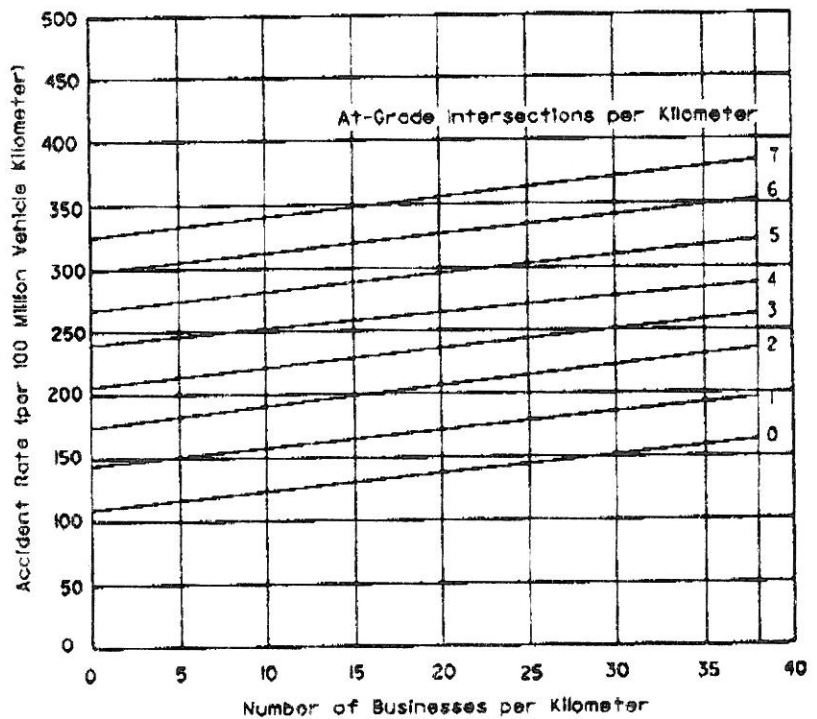


Figure II-29. Accident rate on 4-lane divided non-Interstate highways by number of at-grade intersections per kilometer and number of businesses per kilometer.