96-37 Janget

Page 1 of 4

City of Rockwall (3/87)

APPLICATION AND FINAL PLAT CHECKLIST

a.					D	ate	-22-96
Name of Proposed De	evelopment	STEGER	TOWNE	CROSSIN	G PHAS	SE I.	•
Name of Developer_	STEGER TOWNE C	ROSSING	G, L.P	•			
Address 5025	ARAPAHO ROAD, 407 S, TEXAS 75248	· · · · · · · · · · · · · · · · · · ·		A Company of the Comp	P	hon <u>214</u>	7789-2977
Owner of Record	40/3097, LIMITED P	ARTNERS	SHIP,	A TEXAS	LIMIT	ED PART	NERSHIP
	RUSK, ROCKWALL, T		75087		P	hon@14	/722-3334
Name of Land Plann	er/Surveyor/Engineer_	LAWRI	ENCE A	. CATES	S ASS	OCIATES	, INC.
	O MIDWAY ROAD, 122				P	hone_21	4/385-2272
Total Acreage 32.	AS, TEXAS 75244 4557 AC.	->		Cı	irrent	Zoning	A & C
Number of Lots/Unit	NINE (9)	_					
			Signed	Faige	e G	1.C.T))
and shan be drawn to	enerally conform to the legibly show all data. The Final Plat shall	00 2 521	isfactor	יו בובחם עי	enally	AA6 4.00	llow there are
The following Final of the Rockwall Subs	Plat Checklist is a sur division Ordinance. St. The following check.	nmary o	f the re	quiremen	ts liste	d under	Section VIII
Information						×.	
Provided of Shown on Plat	Not <u>Applicable</u>						
XX		1. Ti graphic	tle or scale,	name of north poir	develont, date	opment, e of plat	written and and key map

XX		2. Location of the development by City, County and State.
Page 2 of 4		
	xx :_	3. Location of development tied to a USGS monument, Texas highway monument or other approved benchmark
_XX	,	4. Accurate boundary survey and property description with tract boundary lined indicated by heavy lines
		5. If no engineering is provided show contours of 5 ft. intervals
_XX		6. Accurate plat dimensions with all engineering information necessary to reproduce plat on the ground
XX		7. Approved name and right-of-way width of each street, both within an adjacent to the development
XX	data-manager -	8. Locations, dimensions and purposes of any easements or other rights-of-way
_XX		9. Identification of each lot or site and block by letter and number and building lines
	:	10. Record owners of contiguous parcels of unsubdivided land, names and lot patterns of contiguous subdivisions, approved Concept Plans, reference recorded subdivision plats or adjoining platted land by record name and by deed record volume and page
_XX		11. Boundary lines, dimensions and descriptions of open spaces to be dedicated for public use of the inhabitants of the development
		12. Certificate of dedication of all streets, alleys, parks and other public uses signed by the owner or owners (see wording)

1			· · · · · · · · · · · · · · · · · · ·
	Page 3 of 4		
	\(\frac{1}{2}\)		13. Designation of the entity responsible for the operation and maintenance of any commonly held property and a waiver releasing the City of such responsibility, a waiver releasing the City for damages in establishment or alteration of graded (see wording)
	*		14. Statement of developer responsibility for storm drainage improvements (see wording)
	<u>_</u> \	·	15. Instrument of dedication or adoption signed by the owner or owners (see wording)
		(16. Space for signatures attesting approval of the plat (see wording)
			17. Seal and signature of the surveyor and/or engineer responsible for surveying the development and/or the preparation of the plat (see wording)
•	<u></u>		18. Compliance with all special requirements developed in preliminary plat review
	_XX		19. Statements indicating that no building permits will be issued until all public improvements are accepted by the City (see wording)
	<u> </u>	1	20. Submit along with plat a calculation sheet indicating the area of each lot
		<u>~</u>	21. Attach copy of any proposed deed restrictions for proposed subdivision

!

City of Rockwall Planning And Zoning Commission

Agenda Date:

June 11,1996

Applicant:

Weber & Company

Agenda Item:

96-37-FP/SP/LP Consider approval of a request from Weber and Company for a final plat, site plan and landscape plan for

Steger Towne Crossing Phase I.

Action Needed:

Discuss and consider approval of the request

Background Information:

PLAT

This property is part of the Steger Towne Crossing development. This plat contains 9 lots as part of the first phase of the development. Cross access and fire lanes are provided to serve this site. The proposed anchor tenants on the property include Target and Albertson's. We are finalizing the 15' R.O.W dedication for FM-740.

LANDSCAPE PLAN

At the work session there was some discussion regarding clustering the trees. Staff reviewed this and was unable to find an acceptable alternative that would brake up this massive parking lot.

Additional screening has been added to the rear of the Albertson's store and along the chain link fence behind the Target on the north side of Steger Towne Drive.

SITE PLAN

The site plan meets the parking and site requirements for the commercial zoning district.

Recommendation:

Staff recommends approval of this request with the following conditions;

- 1. 15' R.O.W. dedication for FM-740 prior to the plat being filed.
- 2. Approval of the engineering plans.

- 3. The existing temporary fire lane and access drives on the Food Lion site be abandon or reconfigured prior to the construction of that portion of Steger Towne Drive.
- 4. The developer of the Steger Towne Crossing allow the property behind Albertson's to petition for cross access to the center.

City of Rockwall City Council

Agenda Date:

June 17, 1996

Applicant:

Weber & Company

Agenda Item:

Action Needed:

96-37-FP/SP/LP Consider approval of a request from Weber and Company for a final plat, site plan and landscape plan for Steger Towne Crossing Phase I.

Discuss and consider approval of the request

Background Information:

PLAT

This property is part of the Steger Towne Crossing development. This plat contains 9 lots as part of the first phase of the development. Cross access and fire lanes are provided to serve this site. The proposed anchor tenants on the property include Target and Albertson's. We are finalizing the 15' R.O.W dedication for FM-740. The right of way for FM-740 will need to be dedicated by separate instrument.

LANDSCAPE PLAN

At the Commission work session there was some discussion regarding clustering the trees instead of the proposed diamond tree islands. Staff reviewed this and was unable to find an acceptable alternative that would brake up this massive parking lot.

Additional screening has been added to the rear of the Albertson's store and along the chain link fence behind the Target on the north side of Steger Towne Drive.

This landscaping plan was revised to match the Boston Market landscaping to achieve a uniform planting along Steger Towne Drive.

SITE PLAN

The site plan meets the parking and site requirements for the commercial zoning district. The anchor stores are still finalizing the plans for exterior materials and colors. Staff has met with

the architects for the anchor stores, and the building plans are still being designed and reviewed by their corporate offices.

Staff Recommendation:

Staff recommends approval of this request with the following conditions;

- 1. 15' R.O.W. dedication by separate instrument for FM-740 prior to the plat being filed.
- Approval of the engineering plans.
- The existing temporary fire lane and access drives on the Food Lion site be abandon or reconfigured prior to the construction of that portion of Steger Towne Drive.
- 4. The developer of the Steger Towne Crossing allow the property behind Albertson's to petition for cross access to the center.

P & Z Recommendation:

Approval with staff conditions.

CITY OF ROCKWALL City Council Agenda

Agenda Date:

June 17, 1996

Agenda No. V.F.

Agenda Item:

PZ-37-FP/SP/LP Hold Public Hearing Regarding Request for Sign Variance Consider Approval of a Request from Weber and Company for a Final Plat, Site Plan, Landscape Plan and Sign Plan for Stegar Towne Crossing Phase I Generally located on the east side of FM-740 South of I-30 and Take Any Necessary Action.

Item Generated By:

Action Needed:

Background Information:



CITY OF ROCKWALL

"THE NEW HORIZON"

Memorandum

TO:

Julie Couch, City Manager

FROM:

Bill Crolley, City Planner

RE:

Steger Towne Crossing Request for a Sign Plan and Sign Height Variance

DATE:

June 12, 1996

The Developer of the Steger Towne Crossing Addition is requesting approval of a sign plan in conjunction with the proposed development. A detailed sign plan map with sign sizes will be sent in the Friday packet.



CITY OF ROCKWALL

"THE NEW HORIZON"

Memorandum

TO:

Julie Couch, City Manager

FROM:

Bill Crolley, City Planner

RE:

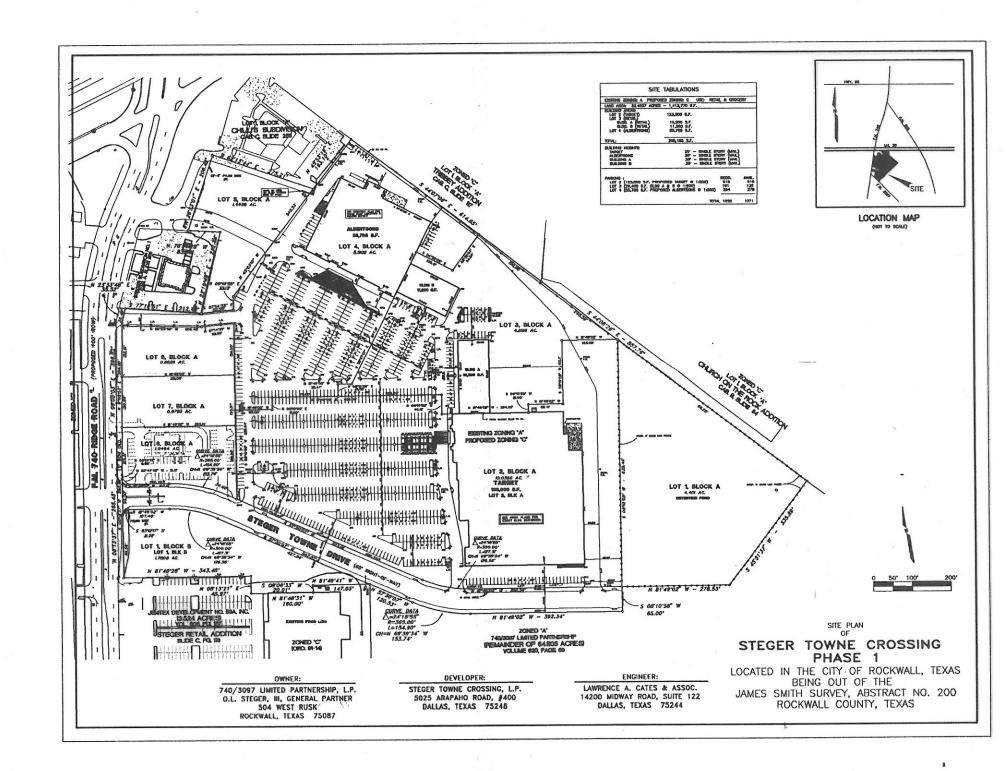
Steger Towne Crossing Request for a Sign Plan and Sign Height

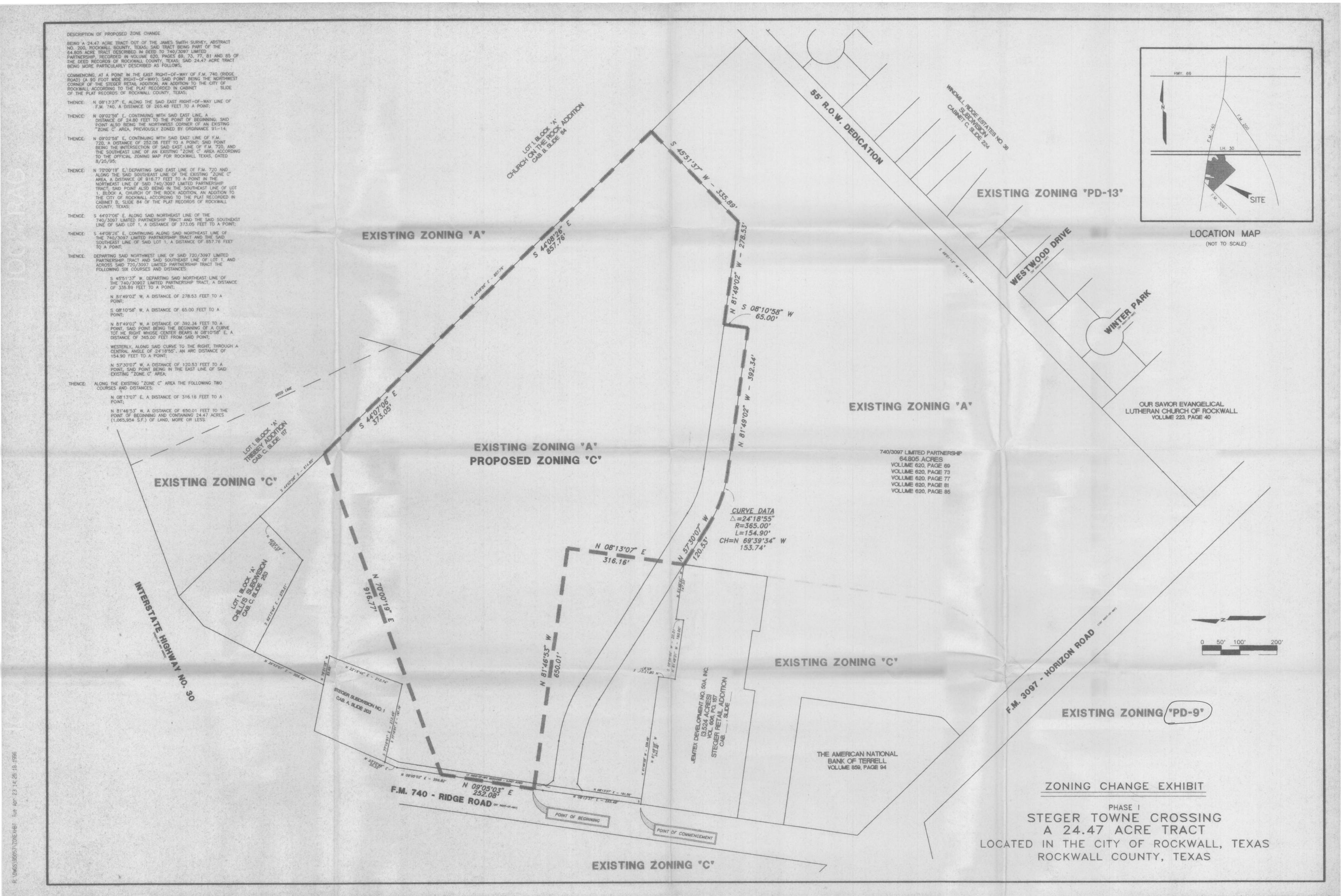
Variance

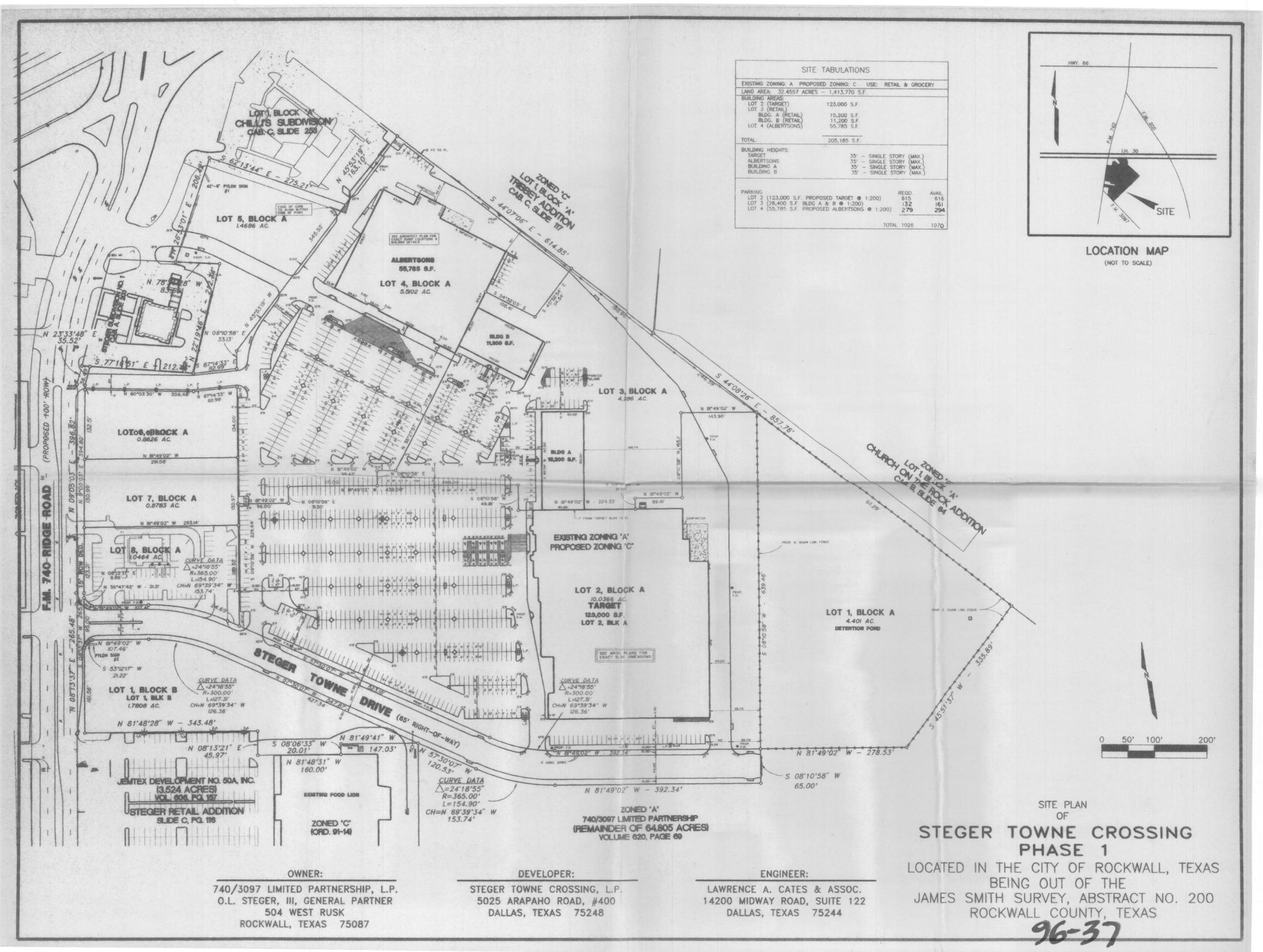
DATE:

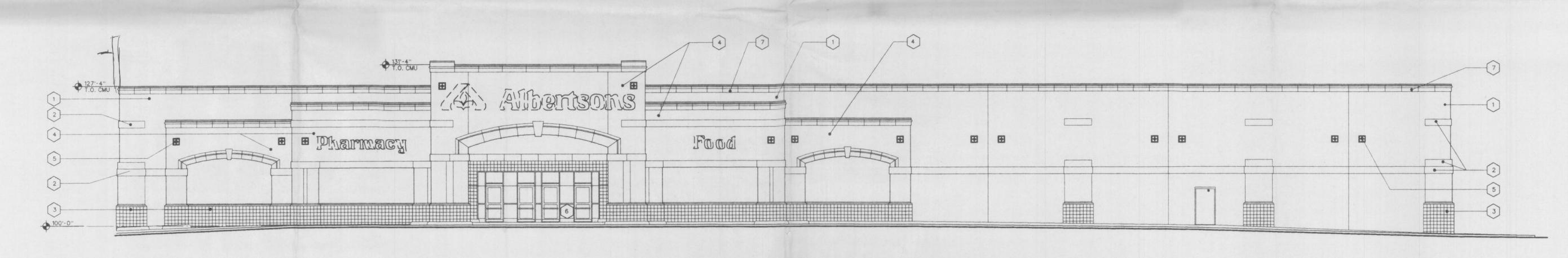
June 14, 1996

The Developer of the Steger Towne Crossing Addition is requesting approval of a sign plan in conjunction with the proposed development. The sign ordinance allows approval of a sign plan for shopping centres. Included for review is the sign plan requested by the applicant. As proposed all individual businesses will have monument signs. There are three pylon signs requested with this plan. These three signs as defined by the sign ordinance are combination identification signs and directory signs. The I-30 pylon sign as proposed is 42' 9" tall. The sign ordinance limits the height of this sign to 30'. The Council may allow height variance as the applicant has requested. The remaining two pylon signs are located at the intersection of Steger Towne Crossing Drive and F.M. 740 and F.M.3097. These two Pylon signs are 25' tall with 260 sq. ft. of sign area. Both of these signs meet the sign ordinance requirements.



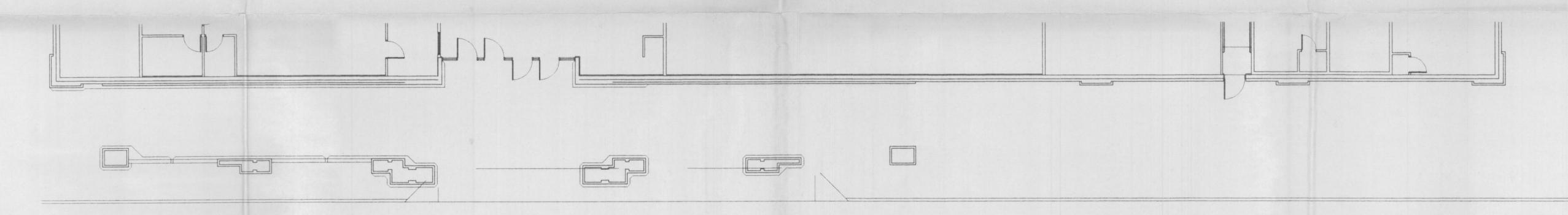






FRONT ELEVATION

SCALE: 3/32" = 1'-0"



PARTIAL PLAN

SCALE: 3/32" = 1'-0"

EXTERIOR COLOR SCHEDULE

GENERAL NOTES

BRICK

- 1. COLORS TO BE SELECTED BY ARCHITECT FROM MANUFACTURER'S STANDARD COLORS.
- 2. FINISHES ARE ALSO REQUIRED AT INSIDE FACE OF ALL EXPOSED PARAPET WALLS.

 3. REFER TO SPECIFICATIONS FOR PAINT MANUFACTURERS & SPECIAL COATING SYSTEMS.
- 4. COLORS TO MATCH PROPOSED ADJACENT RETAIL.

CONCRETE MASONRY UNITS (TYPES)

СМИ

1 SMOOTH FACE CMU

CMU

INTEGRAL COLOR: A

SPLIT FACE CMU
INTEGRAL COLOR: A

3 SPLIT FACE CMU CENTER SCORED

INTEGRAL COLOR: B

BRICK
COLOR:

ACCENT TILE

STOREFRONT/ENTRY DOORS/ SILL FLASHING/TRIM AT STOREFRONT

EXTERIOR FINISH SYSTEM E.I.F.S.

EXTERIOR HOLLOW METAL DOORS & FRAMES, RAILINGS, ROLL-UP DOORS & FRAMES, METAL CAPS

ROOF TOP MECHANICAL EQUIPMENT EXTENDING ABOVE PARAPET

5 QUARRY TILE COLOR: "PACIFIC BLUE"

6 VISTAWALL
COLOR: "MEDIUM BRONZE"

7 THORO SYSTEM PRODUCTS
THOROWALL COLOR: "MOBE PEARL"
FINISH: FINE TEXTURE

8 PAINT TO MATCH THOROWALL COLOR:

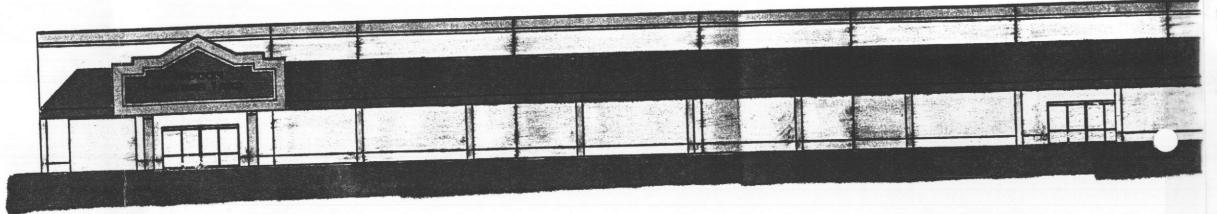
9 PAINT TO MATCH THOROWALL COLOR: "MOBE PEARL"

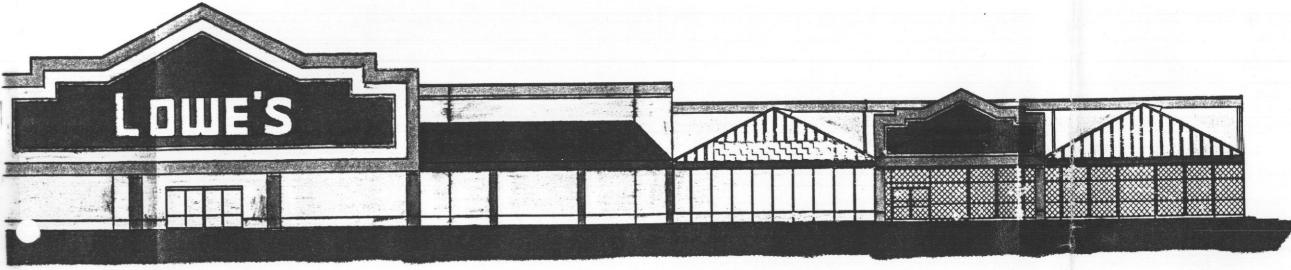
ALBERTSONS S.E.C. 1-30 & RIDGE ROAD

ROCKWALL, TEXAS

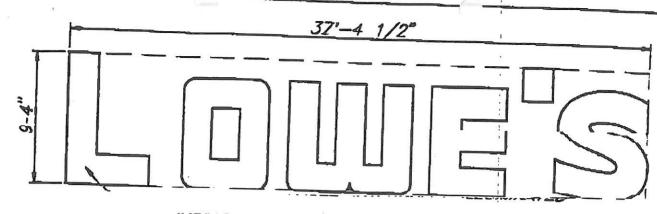


DATE: 5/28/96 PROJECT: 6633 Drawn by: LAH

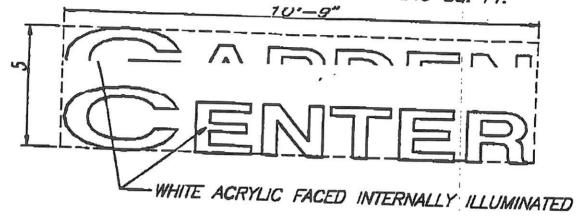




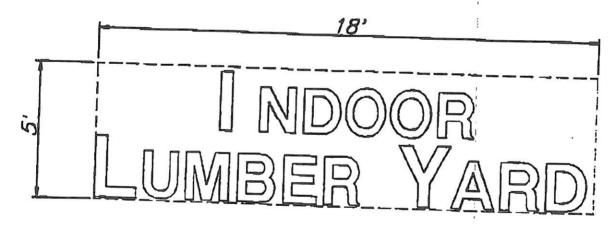
FRONT ELEVATION



INDIVIDUAL SIGN LETTERS = 259 SQ. FT.



SIGN LETTERS = 35 SQ. FT. RECTANGULAR SIGN TOTAL = 50 SQ. FT.

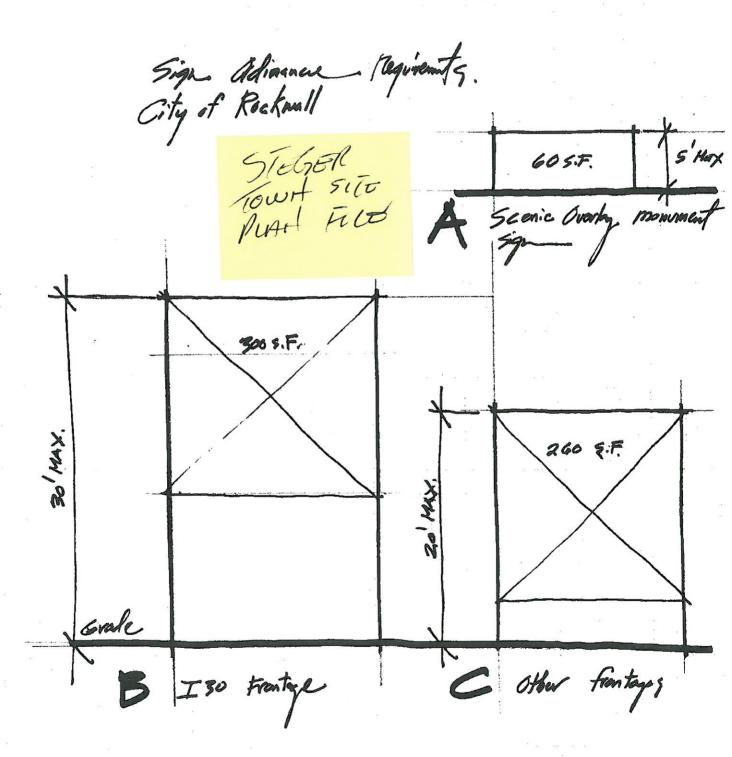


SIGN LETTERS = 46 SQ. FT. RECTANGULAR SIGN TOTAL = 84 SQ. FT.

LOWE'S STANDARD BUILDING SIGNS

GOOD FULTON & FARRELL ARCHITECTS

3102 Oak Lawn Avenue Suite 250 Dallas, Texas 75219 214 / 528-5599 Fax 214 / 521-8672 Sheet No.
Sheet Title
Project No.
Date
Revisions



SECTION III

Sign Standards

A. Structural Standards

- General Sign Provisions: All signs located or to be located within the City of Rockwall shall conform to the general provisions set forth in Table II entitled "Functional Standards". In addition, the following specific standards shall apply.
- 2. Marquee Signs: Marquee signs erected on the face of a marquee shall be built as an integral part of the marquee. Such sign faces shall not have a vertical height of more than four (4) feet nor exceed seventy-five percent (75%) of the width of such building or store frontage, nor the sign standards of Table II. Vertical clearance shall be subject to the requirements of Paragraph 5 of this Section. No sign shall be allowed to overhang public property unless such sign has been approved by the Sign Board of Review.

Wall Signs: Wall signs may be painted directly on a window or door, but no other building surface. All other wall signs shall be attached to, and not painted onto, any building. The sign brackets or supports for wall signs may not project more than two (2) inches from said wall. Such sign faces shall not have a vertical height of more than six (6) feet nor exceed ten percent (10%) of the front face area of the building or store front as established in approved plans submitted to the City or sixty (60) square feet, whichever is greater, nor the sign standards of Table II. Vertical clearance shall be subject to the requirements of Paragraph 5 of this Section. (Ord. 85-65A)

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City of Rockwall

FINAL DRAFT TRAFFIC IMPACT STUDY FOR THE PROPOSED STEGER TOWNE CROSSING IN ROCKWALL, TEXAS

Prepared by:

DeShazo, Tang & Associates, Inc. 400 S. Houston St., Suite 330 Dallas, Texas 75202



City of Rockwall

FINAL DRAFT TRAFFIC IMPACT STUDY FOR THE PROPOSED STEGER TOWNE CROSSING IN ROCKWALL, TEXAS

Prepared by:

DeShazo, Tang & Associates, Inc. 400 S. Houston St., Suite 330 Dallas, Texas 75202



Final Draft

Traffic Impact Study for the Proposed Steger Towne Crossing in Rockwall, Texas

Prepared for:

Mr. Bill Crolley City of Rockwall

Prepared by:

DeShazo, Tang & Associates, Inc. 400 South Houston Street, Suite 330 Dallas, Texas 75202 (214) 748-6740 J95118

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Trip Generation	8
Trip Distribution	9
Projected Traffic Volumes	
ecommendations and Conclusions	

LIST OF EXHIBITS

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2	Site Plan	3
3	City of Rockwall Thoroughfare Plan	5
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5A	Existing 24-Hour Roadway Level of Service	7
5B	Existing PM Peak Hour Intersection Level of Service	8
6	Trip Generation Summary	9
7	Trip Distribution	
8	Proposed Median Opening Locations and Configurations	
9	PM Peak Hour Site-Generated Traffic	
10	PM Peak Hour Base Plus Site-Generated Traffic	
11	Level of Service Summary - PM Peak Hour of the Adjacent Street	



DeShazo, Tang & Associates, Inc.
Engineers ♦ Planners
400 S. Houston St, Suite 330
Dallas, Texas 75202-4802
214/748-6740 ♦ FAX 214/748-7037

TECHNICAL MEMORANDUM

To:

Mr. Bill Crolley

City of Rockwall

From:

DeShazo, Tang & Associates, Inc.

Date:

January 24, 1996

Subject:

Traffic Impact Study for the Proposed Steger Towne Crossing in Rockwall, Texas; J95118

INTRODUCTION

The purpose of this memorandum is to address the City of Rockwall's need to evaluate the traffic-related issues of the proposed Steger Towne Crossing located on the east side of Ridge Road (FM-740 between IH-30 and Horizon Road (FM-3097). Exhibit 1 illustrates the site location. The proposed 64.82 acre site includes 415,000 square feet of retail uses which may include a hardware store, a discount center, a grocery store and supporting retail. Additional commercial sites are planned on five outparcels, ranging in size from 0.94 acres to 1.37 acres. As shown in Exhibit 2, the site plan depicts the dedication of right-of-way for the proposed Steger Towne Drive from Ridge Road to a "New Road" adjacent to the property.

The impact of the site-generated trips on the adjacent roadway network was determined by analyzing the intersection and interchange capacity during the PM peak traffic hour for the following conditions:

- existing background traffic for 1995;
- projected background traffic for 1997;
- projected background 1997 traffic with site-related traffic.

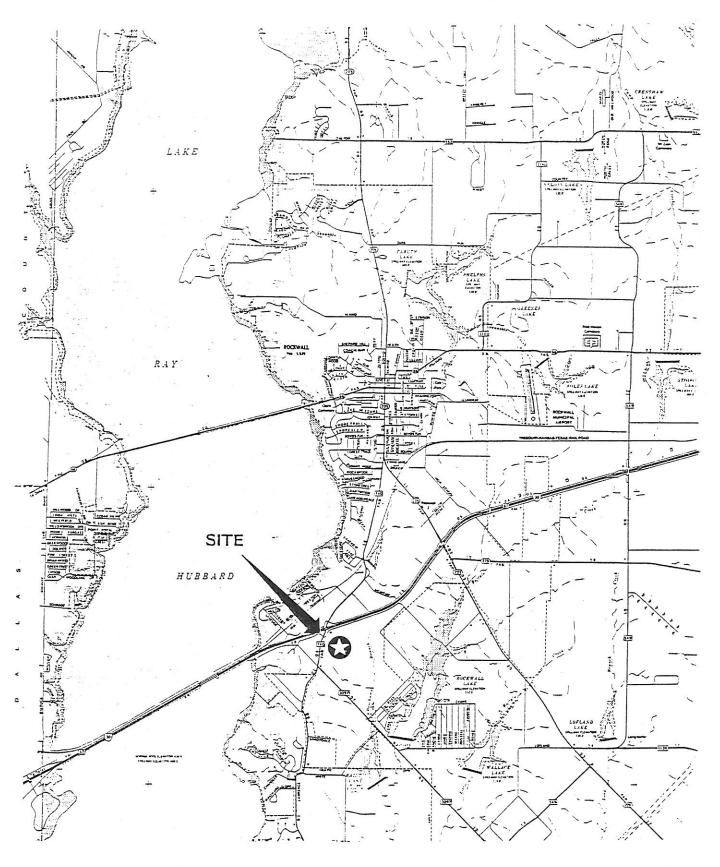
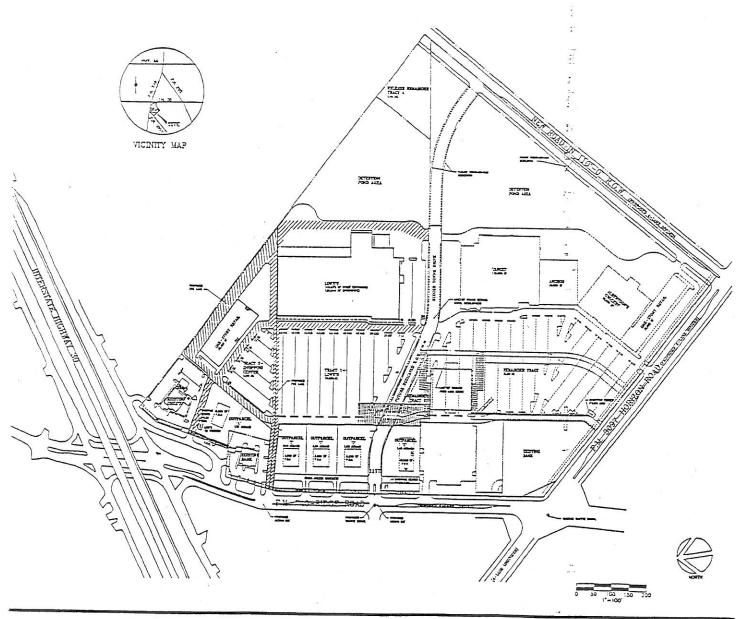




EXHIBIT 1 SITE LOCATION



Werer & Company

GOOD FULTON & FARRELL ARCHITECTS



STUDY AREA ROADWAYS

The study area considered in this analysis contains a freeway, arterials and collectors in the vicinity of the site. Exhibit 3 depicts the City of Rockwall Thoroughfare Plan in the study area. Descriptions of these roadways are as follows:

<u>IH-30</u> - is a grade-separated, east/west freeway providing access from Rockwall to Dallas to the west. Access ramps are provided to and from Ridge Road from the freeway main lanes from the east and west of Ridge Road. Continuous, one-way frontage roads are provided for both eastbound and westbound traffic.

Ridge Road (FM-740) - is a north/south, two-lane, undivided roadway connecting IH-30 and the southern portions of the city to SH-205 in the north, forming a key route in the local street network. The recently reconstructed interchange with IH-30 is signalized. The City's Thoroughfare Plan identifies this facility as a M4D (minor, four-lane, divided roadway) along the existing right-of-way from south of Horizon Road to north of IH-30. Ridge Road is planned for reconstruction by the Texas Department of Transportation (TxDOT) as a four-lane divided roadway from IH-30 to Horizon Road.

<u>Horizon Road (FM-3097)</u> - is currently a two-lane, two-way roadway east of Ridge Road. West of Ridge Road, Horizon Road is one-way northwest-bound, providing access to IH-30 and north of IH-30. Between IH-30 and Ridge Road, Horizon Road is currently being reconstructed and widened to accommodated two traffic flow as a four lane undivided roadway.

TRAFFIC ANALYSIS

Existing Traffic Volumes

From August 18 through November 2, 1995, DeShazo, Tang & Associates conducted 24-hour traffic volume counts on the study area roadways. Exhibit 4 illustrates the existing daily traffic volumes. Guidelines established by the North Central Texas Council of Governments (NCTCOG) state that two-lane arterials can acceptably accommodate approximately 14,500 vehicles per day in suburban areas while four-lane, divided arterials can accommodate up to 32,000 vehicles per day. Exhibit 5A depicts the roadway Levels-of-Service (LOS) for the streets adjacent to the site. LOS refers to the operational conditions within a traffic stream and their perception by motorists. There are six LOS conditions that are designated from "A" to "F", with "A" representing the best operational conditions and "F" the worst conditions. Typically, LOS above "E" are desired.

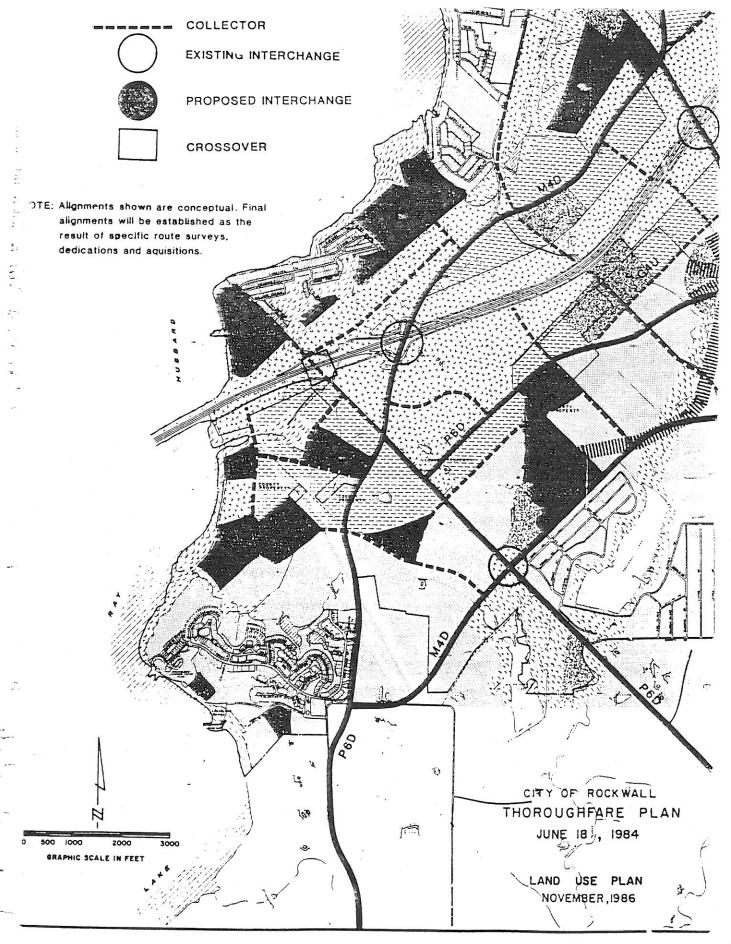




EXHIBIT 3
CITY OF ROCKWALL THOROUGHFARE PLAN

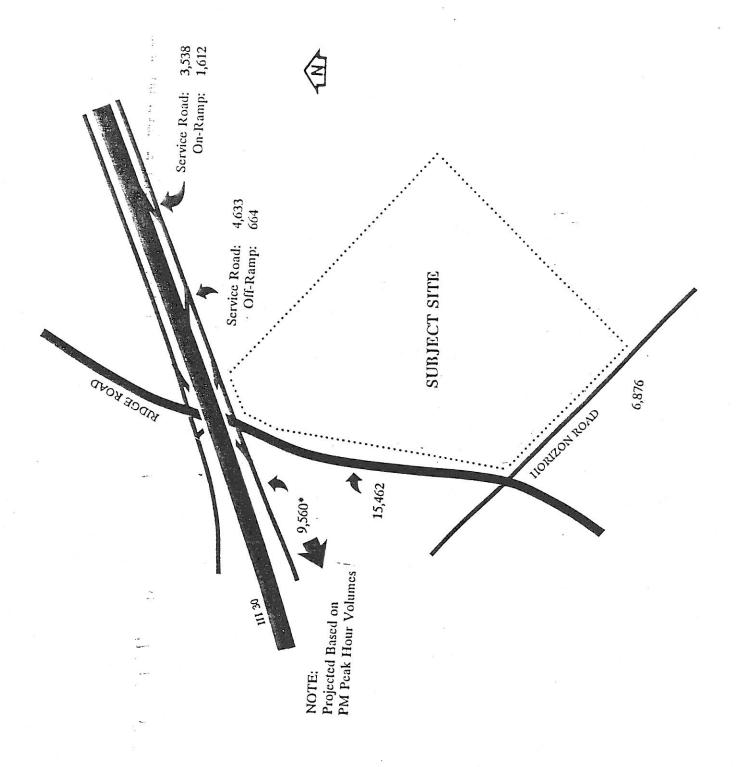




EXHIBIT 4

EXISTING DAILY TRAFFIC VOLUMES

EXHIBIT 5A EXISTING 24-HOUR ROADWAY LEVEL-OF-SERVICE

Location	Existing Capacity	Existing Daily Traffic Volume	Percent of Existing Capacity Used/ LOS
Ridge Road between IH-30 and Horizon Road	14,000	15,462	110% / F
Horizon Road southeast of Ridge Road	12,500	6,876	55% / A
IH-30 eastbound frontage road between Ridge Road and off-ramp	7,000	4,633	66% / B
IH-30 eastbound frontage road east of Ridge Road after on-ramp	14,000	3,538	25% / A
IH-30 off-ramp east of Ridge Road	14,000	1,612	12% / A
IH-30 on-ramp east of Ridge Road	14,000	664	5% / A

As shown, only Ridge Road experiences a traffic demand which exceeds the existing capacity and operates at an undesirable LOS.

Existing PM peak traffic hour intersection turning movement volumes were also examined to determine the existing traffic operations. The intersection LOS is measured in terms of average delay per vehicle as defined in the following table.

INTERSECTION LEVEL-OF-SERVICE CRITERIA

LOS	SIGNALIZED Average Stopped Delay (seconds per vehicle)	UNSIGNALIZED Average Total Delay (seconds per vehicle)
Α	<u><</u> 5.0	<u><</u> 5.0
В	5.1 to 15.0	5.0 to 10.0
C	15.1 to 25.0	10.1 to 20.0
D	25.1 to 40.0	20.1 to 30.0
E	40.1 to 60.0	30.1 to 45.0
F	> 60.0	<u>≥</u> 45.0

LOS results were determined using the 1994 Highway Capacity Software (HCS) for signalized and unsignalized intersections and PASSER-III for the freeway interchanges. Intersections examined included the Ridge Road/IH-30 interchange and Ridge Road at Horizon Road. The current LOS for these intersections during the evening peak hour are shown in Exhibit 5B.

EXHIBIT 5B EXISTING PM PEAK HOUR INTERSECTION LEVEL-OF-SERVICE

Intersection	LOS/Delay (sec/veh)		
Ridge Road at IH-30 Westbound Frontage Road	B/8.2		
Ridge Road at IH-30 Eastbound Frontage Road	B/11.8		
Ridge Road at Carlisle Plaza/Steger Towne Drive	F/(unsignalized)		
Ridge Road at Horizon Road	B/14.4		

As shown, only the unsignalized intersection of Ridge Road at Carlisle Plaza/Steger Towne Drive operates at an unacceptable LOS.

A summary of the intersection analyses are provided in the Appendix.

Trip Generation

The Institute of Transportation Engineers (ITE) 5th Edition *Trip Generation Manual* was used to project the number of trip ends generated by the proposed development. The manual summarizes field research in trip generation rates for various land uses in the form of graphs and equations. The category for Shopping Center was used to estimate the trip-ends generated by the proposed development. Exhibit 6 presents a summary of the trip-ends generated by the proposed development for typical 24-hour and PM peak hour (of the adjacent street) periods. The evening peak hour represents the highest overall traffic volumes on the adjacent roadways. As shown in Exhibit 6, pass-by trips were applied to the outparcels planned for the development. It was assumed these outparcels would be developed as fast-food restaurants with drive-through lanes. According to the *Trip Generation Manual*, approximately 43 percent of the trips generated by these fast-food restaurants could be composed of existing traffic on the adjacent street system. Therefore, these pass-by trips are not considered newly-generated trips. As shown, the proposed development is expected to generate approximately 29,640 trips on a typical weekday, with about 2,344 of these trips occurring during the evening peak hour. Supplemental information from the *Trip Generation Manual* is provided in the Appendix.

EXHIBIT 6
TRIP GENERATION SUMMARY

Use	Amount	Total Daily Trip Ends	PM Peak Hour of Adjacent Street Traffic			
	(square feet)		In	Out	Total	
Shopping Center	415,380	17,210	813	813	1,626	
Fast-Food w/Drive-through	6,000	3,793	114	105	219	
Fast-Food w/Drive-through	6,000	3,793	114	105	219	
Fast-Food w/Drive-through	6,000	3,793	114	105	219	
Fast-Food w/Drive-through	6,000	3,793	114	105	219	
Fast-Food w/Drive-through	10,500	6,637	199	184	384	
TOTAL	449,880	39,018	1,468	1,418	2,886	
Fast-Food Pass-by	43%	9,378	282	260	542	
TOTAL ADDITIONAL TRIPS	449,880	29,641	1,186	1,158	2,344	

Trip Distribution

Trip distributions for the site-related traffic were determined using demographic information provided by the NCTCOG. Trips related to the proposed Steger Towne Center were distributed throughout the study area based on the relative location of residential land uses. Based on this analysis, the projected additional traffic was assigned to the local roadway network using assumed shortest travel paths. Trip distribution results are summarized in Exhibit 7.

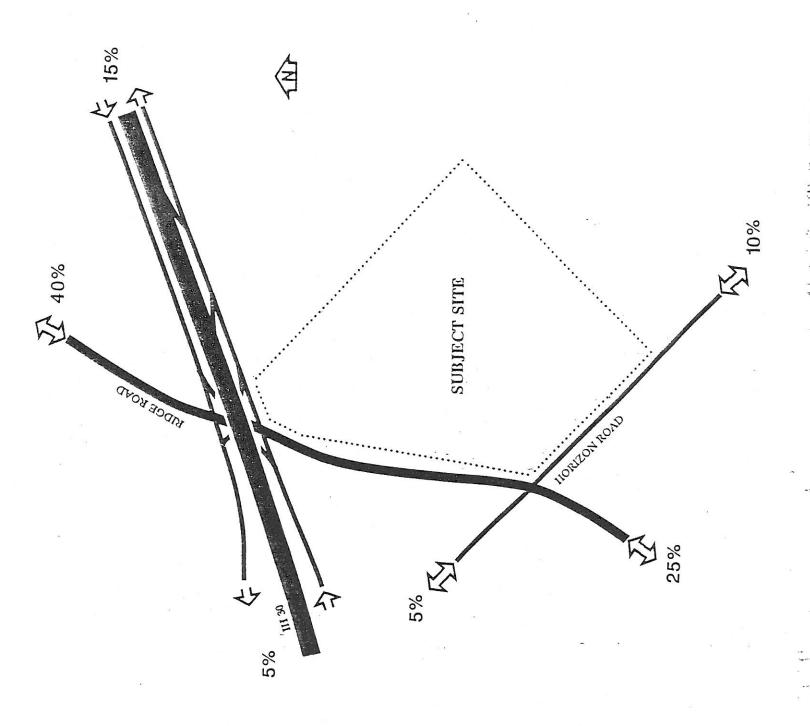




EXHIBIT 7
TRIP DISTRIBUTION

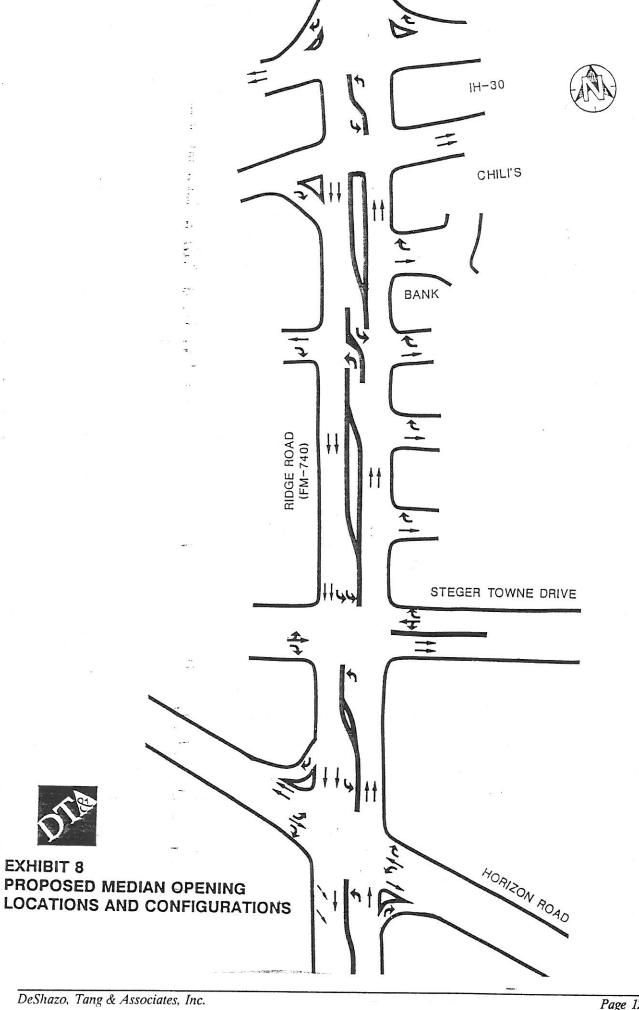
Projected Traffic Volumes

The projected background traffic volumes for the design year 1997 were obtained from the *Traffic Impact Study* for the Proposed Wal-Mart Supercenter on IH-30 in Rockwall, Texas. The projected background with Wal-Mart traffic was used as the base traffic volumes in this study. Ridge Road was assumed to be a four-lane, divided roadway adjacent to the site. Horizon Road was assumed to be a two-lane, undivided roadway adjacent to the site. West of Ridge Road, Horizon Road was assumed to be a two-way roadway. Median openings were assumed to be located on Ridge Road at the proposed Steger Towne Drive and at a driveway between the IH-30 interchange and the proposed Steger Towne Drive. At the north driveway location, left turns exiting Steger Towne Shopping Center and Carlisle Plaza were assumed to be prohibited. Exhibit 8 illustrates the proposed median opening locations and configurations.

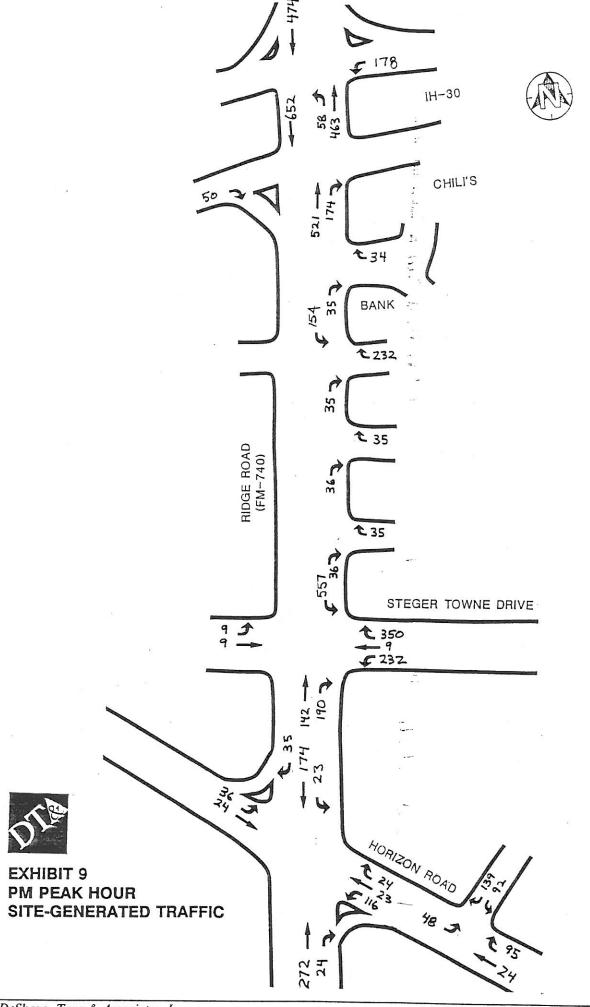
Site-related trips were then assigned to the committed/programmed roadway network based on the trip distribution results. Exhibit 9 depicts the evening peak hour site-related traffic movements in the vicinity of the site. Exhibit 10 summarizes the projected background traffic with the site-related traffic volumes included.

Site Impact Determination

Intersection capacity analyses were conducted for the projected background, and projected background with siterelated traffic conditions to determine the intersection levels of service with and without the development. The results are presented in Exhibit 11. A summary of the intersection analyses are provided in the Appendix.



Page 12



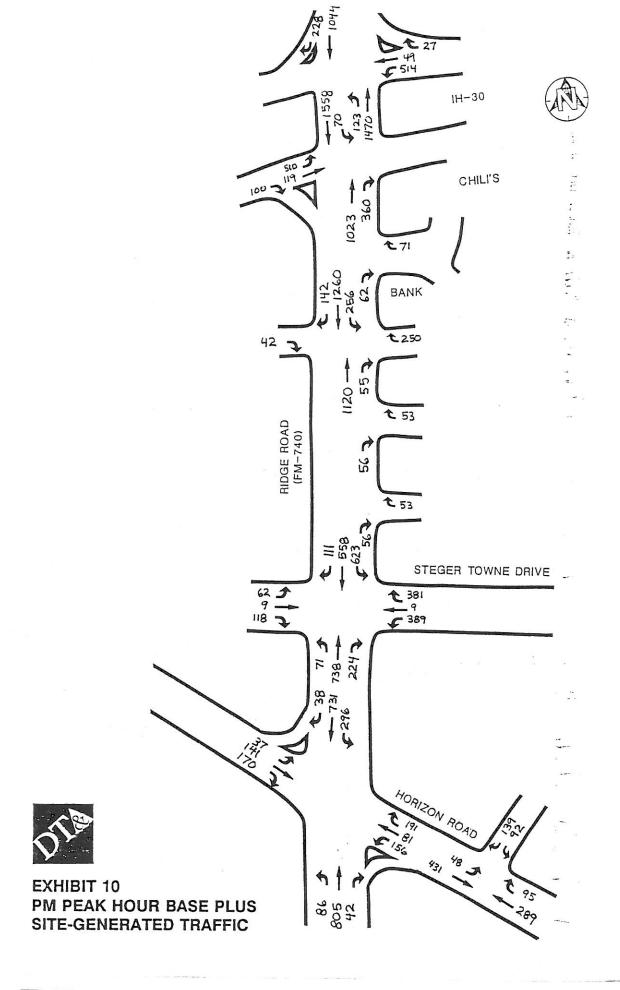


EXHIBIT 11 LEVEL-OF-SERVICE SUMMARY PM PEAK HOUR OF THE ADJACENT STREET

Intersection	Projected Background Traffic LOS/Delay(sec/veh)	Projected Background Traffic with Site Traffic LOS/Delay(sec/veh)
Ridge Road @ IH-30 WBFR	B/8.2	B/9.7
Ridge Road @ IH-30 EBFR	B/11.8	B/12.8
Ridge Road @ North Driveway (unsignalized)	D/1.2	D/3.0
Ridge Road @ Steger Towne Drive/ Carlisle Plaza	F/66.0 (unsignalized)	D/34.7 (signalized) C/21.4 (Dual SB left)
Ridge Road @ Horizon Road	B/14.4	C/21.8
Horizon Road @ South Driveway (unsignalized)	Not Applicable	C/2.9

RECOMMENDATIONS AND CONCLUSIONS

The programmed widening of Ridge Road between IH-30 and Horizon Road with recommended modifications to the intersection of Steger Town Boulevard is projected to accommodate all phases of the proposed Steger Towne Crossing Development. The projected LOS at IH-30 and Ridge Road is expected to remain the same with or without development of the subject site. The intersection of Ridge Road and Horizon Road will be reconstructed as part of the TxDOT Ridge Road widening. With these planned/programmed improvements, the intersection is projected to operate at a LOS C with an average vehicle delay of only 21.8 seconds.

The prevalent direction of site related trips occurs north of IH-30. Traffic traveling southbound on Ridge Road has two primary opportunities to execute left turn maneuvers into the subject site. The site's primary access will be at Steger Towne Drive, which is proposed to be signalized when warranted according to the Texas Manual of Uniform Traffic Control Devices (TxMUTCD). As part of this analysis, a need has been identified to provide for southbound dual left turn lanes on Ridge Road at Steger Towne Drive. This modification may be accommodated as part of the planned/programmed improvements to Ridge Road by TxDOT.

A channelized median providing a left turn for the northern driveway of Steger Town Crossing is also proposed. This type of median would allow all movements into the site. However, departing left turning vehicles (i.e. those desiring to travel southbound on Ridge Road), would do so via the intersection of Ridge Road at Steger Towne Drive/Carlisle Plaza. This recommended median treatment maximizes traffic operations of the public roadway system and enhances safety of the planned facilities.

It is, however, recognized that existing traffic movements in to and out of existing developments will be affected. Other median design options may be explored, however, such efforts are beyond the scope of this study. Generally, a full median opening at the northern driveway would present conflicts of turning movements with existing developments on both sides of Ridge Road. The projections of LOS for a full median opening at the northern driveway is "F". This value is primarily attributed to the projected delay of the left-turn departing vehicles. The delay to other movements, i.e. right-turn entering and departing vehicles and the south bound left-turning vehicles, would not be significantly affected with a full median opening design.

The remaining intersection analyses shown in Exhibit 11 reflect adequate LOS. It is also concluded that the existing and the programmed improvements to Ridge Road can accommodate the proposed development of Steger Towne Crossing. The construction of the "New Road" adjacent to the east boundary of the site and/or the extension of Steger Towne Drive to this road is not necessary from a transportation engineering analysis perspective as part of approval for this development.

Appendix

Street Observations	DeShaz	Tang & Associates, Inc.
Street: IH-30 Access Rd I Location: 600 feet east of		
Ridge Road/ City/State: Rockwall, Texas Project-ID#: 95118 - 162	FM-740	
I. Street Width:		
II. Street Material: Concrete		
III. Curbing/Gutters?: Concrete		
IV. Number of Lanes: 1	(one) each	
V. Divided?: Yes		
VI. Traffic Control Devices	: None	
VII. Pedestrian Crosswalks?	: Not Applicable	
VIII. Pedestrian Pushbutton?:	Not Applicable	
IX. On-street Parking:	Unmarked	
X. Posted Speed Limit:	Unmarked	
XI. Adjacent Land Uses:	Commercial, Agricultural	
XII. Additional Observations/	Comments:	

Automated Traffic (int DeShaz Tang & Associates, Inc. Street: IH-30 Access Road Eastbound Location: 3,100 feet east of Ridge Road/FM-740 City/State: Rockwall Project-ID#: 95118 - 163 Date:: November 1, 1995 Day of Week: Wednesday Data Source: DT&A 5,150 24-Hour Volume: But Ramp Total Vehic Peak Time EB Access Rd IH-30 On-Ramp Time | Peak EB Access Rd IH-30 On-Ramp 113 * Directional Volumes 3,538 1,612 Equipment ID#: 24-Hour Volume 5,150

Street C	bservations		DeShaz	Tang & Associates, Inc.
Street:	IH-30 Access	Road Eastbound		
Location:	3,100 feet east	of		
	Ridge R	oad/FM-740		
City/State:	Rockwall	27		
Project-ID#:	95118 - 163	f		
7	Charle W. Int			
1.	Street Width:	i.		
		=		
		* %		
II	Street Material:	-		
11.		p: Concrete; Acc	ess Rd: Asphalt	
		-		
III.	Curbing/Gutters?:			
	Оп-катр:	Concrete; Access R	d: Concrete Curb on Le	eft, Open Drainage on Right
IV.	Number of Lanes:	On-Ramp: 1 (c	one); Access Rd:	2 (two)
V.	Divided?: Ye	S		
VI.	Traffic Control De	evices: <i>None</i>		
VII.	Pedestrian Crossw	alks?: Not	Applicable	
VIII.	Pedestrian Pushbu	ton?: Not	Applicable	
IX.	On-street Parking:	No		
Х.	Posted Speed Limi	t: Unmarked	i	
XI.	Adjacent Land Use	s: Agricultur	al	
XII.	Additional Observa	tions/Comments:		
		<u>.</u>		
		· ·		
	_ = 2			

Automated Traffic C DeShaze Tang & Associates, Inc. nt Street: FM-740/Ridge Road Location: 1,000 feet south of IH-30 City/State: Rockwall, Texas 0.9 0.6 Project-ID#: 95118 - 164 0.7 Date: November 1, 1995 0.6 Day of Week: Wednesday Data Source: DT&A 0.2 1200 1400 15,462 24-Hour Volume: Time of Day North bound Time | Peak Northbound Southbound Time Peak Northbound Southbound -- 95 - 108 _121 = 114 -127 1,417 --- 69 1,175 450 * Directional Volumes 6,649 8,813 Equipment ID#: 24-Hour Volume 15,462

Street C	bservations			DeShaz	Tang & Associates, Inc.
Street:	FM-740/Ridge	Road			
	1,000 feet sout				
	IH-30				
City/State:	Rockwall, Texa	ıs			\$*************************************
Project-ID#:	95118 - 164				ž
I.	Street Width:		36	3 feet	#
	on out in taut.			— leet	*;
					E,
					:
II.	Street Material:				
	Asphalt				
III.	Curbing/Gutters?:				-
	Drainage	Ditch			*
IV.	Number of Lanes:	2 (two) plus	one shared	left turn lane	
V.	Divided?: No	-			
VI.	Traffic Control De	vices: <i>Inters</i> e	ection traffic	signals at H	Torizon & IH-30 Access Rd
VII.	Pedestrian Crosswa	ılks?:	No		
VIII.	Pedestrian Pushbuti	on?:	No		a a
IX.	On-street Parking:	No			
X. 1	Posted Speed Limit	: 40 ı	mph		
XI.	Adjacent Land Uses	: Сотт	ercial		<u>.</u>
XII.	Additional Observa	ions/Comments:			
					,
					· •

Automated Traffic unt Tang & Associates, Inc. DeSha Street: Horizon Road Location: 310 feet east of FM-740/Ridge Road City/State: Rockwall, Texas Project-ID#: 95118 - 165 Date: November 1, 1995 Day of Week: Wednesday Data Source: DT&A 6,876 24-Hour Volume: Time Peak NW Bound SE Bound Time Peak NW Bound SE Bound

Equipment ID#: 3594

 Directional Volumes
 3,449
 3,427

 24-Hour Volume
 6,876

Street C	bservations			DeShaz	Tang & Associates, Inc.
Street	Horizon Road				5
Location:	310 feet east of	•			
1	FM-740/	Ridge Road		_	
City/State:	Rockwall, Texa	ıs		-	
Project-ID#:	95118 – 165			_	
Ĭ.	Street Width:			25.7 feet	
Ē.					
П.	Street Material: Asphalt				
ш.	Curbing/Gutters?: <i>Drainage</i>	Ditch			
IV.	Number of Lanes:	2 (Two)			
V.	Divided?: No				
VI.	Traffic Control De	vices: <i>Inte</i>	rsection	Traffic Control Si	ignals at FM-740
VII.	Pedestrian Crosswa	ilks?:	None		
VIII.	Pedestrian Pushbut	ton?:	None		
IX.	On-street Parking:	No			
Χ.	Posted Speed Limit	: 45	mph		
XI.	Adjacent Land Use	s: Com	mercial,	Agricultural	
XII.	Additional Observa	tions/Comment	s:		
			•2 990		
-	# # # # # # # # # # # # # # # # # # #				
					*,

	DO TOTHE		[5		DeShazo, Tang & Assoicates, Inc	ang & Asso	oicates, Inc
	Location: City/State:	n: Ridge Road / FM-740 & IH-30 WB Service Road e: Rockwall, Texas County: Rockwall	-30 WB Service Road County: Rockwall	Date/Day:	August 18, 1995 / Thursday		
	Signalization: Project-ID#:			Data Source: Data Collector(s): Comments:	D1&A Dewey Bishop Sunny & Hot		
	Time of	Northbound on Ridge Road / FM-740	Southbound on Ridge Road / FM-740		Westbound on IH-30 Service Road		
	Count Begin End	1 Left Thru	Thru Right		Uturn Læft Thru Right	Fifteen P. Minute H. Subtotals To	Peak Peak Hour H Totals Facur
	-			-			
1.000							
			136 65		72 15	519	
	16:30 16:45 16:45 17:00	16 176	94 50		n E1 °		
			122 70		90 12		1,886
	17:30 17:45 17:45 18:00	12 229 18 234			17 66 10 5	500 1,	1,988
	18:00 18:15 18:15 18:30	16 241 20 218	126 52		63 6		
					CI OC	470 2,	2,043
<u> </u>	PM Peak Hour Total/Direction		479 228		5 288 49	2,050	
	%Turn Peak Hr Factor	6.2% 93.8%	67.8%		68.7% 11.7%		
			0.88 0.81		0.55 0.80 0.82 0.75	u.a	ישווייייי.

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Intersecti	Intersection Traffic Movements	S		DeShazo, T	DeShazo, Tang & Assoicates,	ates, Inc
Location: City/State: Signalization: Project-ID#:	Ridge Road / FM-740 & IH-30 EB Service Road Rockwall, Texas County: Rockwall Signalized 95089 - F	30 EB Service Road County: Rockwall	Date/Day: Data Source: Data Collector(s):	August 18, 1995 / Thursday DT&A Buck Woolverton		
			Comments:	Sunny & Hot		
Time of	Northbound on Ridge Road / FM-740	Southbound on Ridge Road / FM-740	Eastbound on IH-30 EB Service Road			
Count Begin End	2 1 Thru Right	1 2 Left Thru	1 1> 1 1 UTom Left Theo Right		Fifteen Feak Minute Hour Subtotals Totals	Her Hac
				1.100		
16:00 16:15 16:15 16:30	· 89 34	12 208	6 101 26 71		547	
16:30 16:45 16:45 17:00			50 5			
51:71 00:71			130 29			
			88 33 136 28		556 2,131 541 2,218	3
-	109 30	16 204	140 29		_	\$ 0.95
18:15 18:30		6 149	7 132 27 63	_	500 2,233	
PM Peak Hour Total/Direction		76 767	494 119		2,295	
Peak Hr Factor	74.0% 26.0%	80:16 %0:6	12.4%			
	23	100	0.64 0.88 0.88 0.89		File: C2	File: C2X31IRS.WK1

	en Peak Peak te Hour als Totals Factor	378 460 388 403 1,629 • 88.53%	File: AZXZIIRS.WKI
October 30, 1995 / Monday DT&A Charles L. DeShazo Cloudy & Dry	Fifteen Minute Subtotals	E 4 E 4	1,629
Date/Day: Data Source: Data Collector(s): Conditions:	Enstbound out of Driveway 20 0> 1 <0 Left Right	2 1 2 2 9 9 5 2 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	9 19 32.1% 67.9% 45.0% 52.8%
FM-740/Ridge Road & Driveway to Bank/Chili's Rockwall, Texas County: Rockwall None 95118 - B	Southbound on FM-740 2U I I Left Thru	2 254 4 341 3 257 1 280	10 1,132 1,142 0.9% 99.1% 62.5% 83.0%
FM-740/Ridge Road Rockwall, Texas None 95118 - B	Northbound on FM-740 20 1 <0	114 2 102 2 124 0 112 3	452 7 459 98.5% 1.5% 91.1% 58.3%
Location: City/State: Signalization: Project-ID#:	Time of Count Begin End	17:00 17:15 17:15 17:30 17:30 17:45 17:45 18:00	PM Peak Hour Total/Direction % Turn Peak Hour Factor Observations:

nc	Peak Hour Factor	98.09%	
es, l	Pe Hc Fac	*	HIRS.WK
DeShazo, Tang & Associates, Inc >mber 2, 1995 / Thursday A laine Rodgers dy & Dry	Peak Hour Totals	1,644	File: AZX211RS.WKI
ang & 4	Fifteen Minute Subtotals	388 425 419 412	48.
hazo, T 2, 1995 / 7 Rodgers Dry	id out of iza Drvwy J <1 Right	34 27 32 25	118 156 75.6% 86.8%
DeShazo, Tang & overwher 2, 1995 / Thursday DT&A C. Blaine Rodgers Cloudy & Dry	Westbound out of Carlisle Plaza Drvwy 20 1> 1 <1 Ueff Right	11 8 6 01	38 1 24.4% 86.4%
المراجعة ال			
Traffic Movements FM-740/Ridge Road & Carlisle Plaza Main Drwy Rockwall, Texas County: Rockwall Dut Co	Southbound on FM-740 2U 1 < 0 Thru Right	182 25 233 41 227 25 226 20	868 111 979 88.7% 11.3% 93.1% 67.7%
Intersection Traffic Movements Location: FM-740/Ridge Road & Car City/State: Rockwall, Texas Signalization: None Project-ID#: 95118 - C	Northbound or FM-740 2U 1> 1 Left Thru	15 121 14 102 14 112 11 120	54 455 509 10.6% 89.4% 90.0% 94.0%
Lersection Location: City/State: Signalization: Project-ID#:	Time of Count jin End	17:15 17:30 17:45 18:00	PM Peak Hour Total/Direction % Turn Peak Hour Factor Observations:
Sis	Tim Cou Begin	17:00 17:15 17:30 17:30	PM Peak Hou Total/Directio % Turn Peak Hour Fa

	Location: City/State: Signalization: Project-ID#:		FM-740/Ridge Road & Carlisle Plaza Driveway Rockwall, Texas County: Rockwall None 95118 - A	vay Date/Day: II Data Source: Data Collector(s): Conditions:	October 30, 1995 / Monday DT&A C. Blaine Rodgers Cloudy & Dry	onday	
1. ct 1. c	e af	rthbound on FM-740 2U	Southbound on FM-740 20 20 1 <0		Carlisle Plaza Drvwy 20 1> 1 <1	Fifteen Minute	
17:15 5 111 225 39 9 398 17:30 17:30 4 18:30 6 121 258 31 6 10 418 1670 4 18:30 17:45 17						Subjorals	
K Hour 17 431 1,009 142 29 42 1,670 1.51 71 71 71 70.8% 89.0% 89.0% 89.0% File: A2X2HRS.WKI						398 418 416 438	
	PM Peak Hour Total/Direction %Turn Peak Hour Fuctor	431 448 96.2% 89.0%	1,009 1,151 87.7% 94.1%		11.	1,670	File: A2X2HRS.WK

DeShazo, Tang & Associates, Inc	игѕдау	Fifteen Peak Peak Minute Hour Hour Subtotals Totals Pactor		TW Company of the com	393 360 419 347 1,519	441 1,567 419 1,626 410 1,617 433 1,703 \$ 98.33%	385 1,647 433 1,661	1,703 File: A2X31IRS.WK1
DeShazo, T	October 5, 1995 / Thursday DT&A C. Blaine Rodgers Fair & Dry	Northwest Bound FM-3097/Horizon Rd	u Northwest Bound.	make a left turn into exted both southbound our minuites directing d up by 16:48.	2 9 43 6 13 32 11 26 74 6 7 47	11 12 35 9 19 41 5 13 43 15 14 48	4 19 40 3 15 29	40 58 167 265 15.1% 21.9% 63.0% 66.7% 55.8% 56.4%
	Date/Day: 'all Data Source: Data Collector(s): Comments:		: The "west" leg of this intersection is a oneway, one-lane street Northwest Bound.	At 16:24 a rear-end collision took place in the southbound lanes approximately 150 feet south of the intersection due to someone stopping to make a left turn into a convenience store located on the southeast corner. This affected both southbound and northbound traffic, but police were on the scene within four minuites directing traffic around the disabled vehicles. The accident was cleaned up by 16:48. There is no left turn lane for the Northbound traffic, yet there is a left turn signal for that movement.				
nents	FM-740/Ridge Rd & FM-3097/Horizon Rd Rockwall, Texas County: Rockwall Traffic Signals	Southbound on FM-740/Ridge Rd (3U)	st" leg of this intersection	At 16:24 a rear-end collision took 150 feet south of the intersection d a convenience store located on the and northbound traffic, but police traffic around the disabled vehicles. There is no left turn lane for the Northat movement.	88 142 3 90 123 0 73 119 5 56 106 3	106 150 0 84 140 1 100 138 0 100 139 3	81 142 1 95 177 3	390 567 4 961 40.6% 59.0% 0.4% 92.0% 80.1% 20.0%
Intersection Traffic Movem	FM-740/Ridge Rd & Rockwall, Texas Traffic Signals 95056 - A	Northbound on FM-740/Ridge Rd (2U) <0 <0 Left Thri Right	Note:	* At 16:24 150 feet a conven and north traffic ar * There is	20 82 4 28 64 4 29 77 5 23 94 5	20 105 2 21 100 4 22 83 6 23 85 6	20 73 5 26 78 7	86 373 18 477 18.0% 78.2% 3.8% 76.8% 88.8% 64.3%
Intersection	Location: City/State: Signalization: Project-ID#:	Time of Count Begin End			16:00 16:15 16:15 16:30 16:30 16:45 16:45 17:00	17:00 17:15 17:15 17:30 17:30 17:45 17:45 18:00	18:15 18:30	PM Peak Hour Total/Direction %Turn Peak Hour Factor

Location:	FM-740/Ridge Rd &	FM-740/Ridge Rd & FM-3097/Horizon Rd		October 11, 1995 / Wednesday	ednesday		
Cuty/state:	TOCKWAII, I CAUS	County: ACCAWAII		DICA			
Signalization: Project-ID#:	1 raffic Signals 95056 – B		Data Collector(s):	C. Blaine Rodgers Fair & Dry			
	Northbound on	Southbound on		Northwest Bound			
Time of	FM-740/Ridge Rd	FM-740/Ridge Rd		FM-3097/Horizon Rd	Liften	Dank	Dast
Count Begin End	Left Thru Right	Left Thri Right		Left Thru Right	***************************************	Hour	Hour Factor
3	00	11					
06:30		9			3		
	43 29 1	7 37 1		1 26 30	175	349	
		34			236	585	
07:15 07:30	59 50 4	9 54 2		5 : 40 38	261	846	
		20.			317	1,075	
	52 85 8	65		33	314	1,153	
		54		29	310		
08:45 09:00	47 65 3	15 65 1		5 25 27	302	1,243	98.03%
- L				1.51	846		
Total/Direction		186		284	2		
%Turn Peak Hour Factor	59.6% 38.8% 1.6% 71.8% 42.0% 18.8%	19.9% 76.9% 3.2% 22.0% 51.1% 75.0%		25.0% 72.7% 52.2%			
-	;						
	Note:						
	The "wes	* The "west" leg of this intersection is a oneway, one-lane street Northwest Bound.	s a oneway, one-lane street	Northwest Bound.		Sa Tasa an dan	į
	* There is n	There is no left turn lane for the Northbound traffic, yet there is a left turn signal	rthbound traffic, yet there i		53		
	for that m	for that movement.					
	* One Vehi Horizon F make a so	One Vehicle was observed going the wrong way on the oneway section of Horizon Road at 08:10. This was not added to the data, but the vehicle did make a southeast bound through movement.	s wrong way on the oneway ot added to the data, but the vement.	section of e vehicle did			
	1						
						File: A2X311RS.WK1	S.WKI

HCM: SIGNALIZED INTERSECTION SUMMARY Version 2.4 01-23-1996

Center For Microcomputers In Transportation

Streets: (E-W) IH 30 WBFR

Analyst: GCL Area Type: Other

(N-S) Ridge Road File Name: WBRB2.HC9

11-1-95 PM Peak

Comment: Base Traffic Volumes (Includes Walmart Study)

	Eastbo	und	We	stbou	 nd	No	rthbou	ind	So	uthboi	==== und
	L T	R 	L	T	R	L	T	R	L	T	R
No. Lanes Volumes Lane Width RTOR Vols Lost Time			336 12.0	12.0	1 27 12.0 0 3.00		12.0	0			1 228 12.0 0 3.00
			Signa	al Ope	eratio	ons					
Phase Combin	nation 1	2	3	4	1		5		6	7	8
Thru					NB	Left			*		
Right						Thru Righ			*		
Peds WB Left	*				a D	Peds					
Thru	*				SB	Left Thru					
Right	*					Righ					
Peds						Peds					
NB Right SB Right					EB	Righ					
Green	26.0A				WB	Righ			E 450		
Yellow/AR	0.0				Gre			A 15.			
Cycle Length		Pha	se co	mbina	tion	order	R 0.0	45 #6			

	Lane	Group:	Intersect Adj Sat	ion Perfo V/c	ormance g/C	Summary		Approa	ch:
	Mvmts	Cap	Flow	Ratio	Ratio	Delay	LOS	Delay	LOS
WB	L	452	1770	0.453	0.256	18.7	Ċ	18.5	С
	${f LT}$	459	1796	0.438	0.256	13.6	C	10.5	C
	R	405	1583	0.069	0.256	16.4	C		
NB	$\mathbf L$	535	1770	0.127	0.333	4.3	A	4.3	А
	${f T}$	2525	3725	0.415	0.678	4.3	A	Ŧ•J	А
SB	\mathbf{T}	2856	5588	0.231	0.511	7.9	В	8.0	В
	R	809	1583	0.297	0.511	8.3	В	0.0	D
_		Inte	ersection I	Delay =	8.2 se	c/veh Ir	tersect	cion LOS	= R
Lost	Time/C	Cycle, L	= 6.0 se		ical v/	c(x)	= 0.425		В
						` '		- Table	

HCM: SIGNALIZED INTERSECTION SUMMARY Version 2.4 01-23-1996 Center For Microcomputers In Transportation

Streets: (E-W) IH 30 WBFR

Analyst: GCL

(N-S) Ridge Road

File Name: WBRBD.HC9

Area Type: Other 11-1-95 PM Peak

Comment: Base Plus Development Traffic Volumes

	Ea	astbou	nd	We	===== stbou	====: nd	NO:	===== rthbou	nd	====== So	===== uthboi	====
	L 	T	R 	L	T	R	L	Т	R	L	Т	R
No. Lanes Volumes				1 514	> 1 49	1 27	1 123	2 1410			3 1044	1 228
Lane Width RTOR Vols Lost Time					12.0	0			0			12.0
rosc lime				3.00	3.00	3.00	3.00	3.00			3.00	3.00
				Sign	al Ope	eratio	ns					
Phase Combin	atior	1 1	2	3		1	J.1.0	5		6	7	8
EB Left Thru		5				NB	Left			*	,	J
Right							Thru			*		
Peds							Righ					
WB Left		*				SB	Peds Left					
Thru		*				SB	Thru					
Right		*					Righ					
Peds							Peds					
NB Right						EB	Righ	ıt				
SB Right						WB	Righ					
Green	26	.0A				Gre			A 15.	OΑ		
Yellow/AR		.0						R 0.0				
Cycle Length	. 90	COCC	Dha			. !			٠.	•		

	T	~	Intersect:	ion Perf	ormance	Summary			
	Lane	Group:	Adj Sat	V/C	q/C			Approac	h.
	Mvmts	Cap	Flow	Ratio	Ratio	Delay	LOS	Delay	LOS

WB	L	452	1770	0.694	0.256	22.7	C ·	21.6	С
	LT	457	1790	0.610	0.256	20.8	C		_
9077-0575-09-60-6	R	405	1583	0.069	0.256	16.4	C		
NB	L	386	1770	0.334	0.333	12.1	В	6.0	В
	${f T}$	2525	3725	0.617	0.678	5.5	В	0.0	ь
SB	\mathbf{T}	2856	5588	0.423	0.511	8.9	В	8.8	В
	R	809	1583	0.297	0.511	8.3	В	0.0	ь
	60 (CON2)	Inte	rsection D	elay =		c/veh Int		ion LOS	= B
Lost	Time/C	Cycle, L	= 6.0 se	c crit	ical w/	(0/3/)	0 750	TON TOD	- D

Cycle Length: 90 secs Phase combination order: #1 #5 #6

me/Cycle, L = 6.0 sec Critical v/c(x) = 0.758

Center For Microcomputers In Transportation

Streets: (E-W) IH 30 EBFR (N-S) Ridge Road Analyst: GCL File Name: EBRB2.HC9

Area Type: Other 11-1-95 PM Peak Comment: Base: Traffic Volumes (Includes Walmart Study) ______,

	Eastbo	und	Wes	tbound		Nor	thbo	 und	So	===== uthbou	==== ind
	L T	R	L	T	R	L	${f T}$	R	L	T	R
No. Lanes Volumes Lane Width RTOR Vols Lost Time	1 > 1 510 119 12.0 12.0 2 3.00 3.00	1 50 12.0 0 3.00					3 502 12.0	1 186 12.0 0 3.00	1 70 12.0	2 906 12.0 3.00	0
			Signa	l Oper	 atio	ns					
Phase Combin EB Left Thru Right Peds	nation 1 * * * *	2	3	4	NB	Left Thru Righ Peds	t *	ŧ	6	7	8
WB Left Thru Right Peds					SB	Left Thru Righ	* t		*		
NB Right SB Right Green Yellow/AR Cycle Length	31.0A 0.0 : 90 secs	s Pha	se com	nbinat	EB WB Gre Yel	low/A	t t 46.0 R 0.0	A 13. 0. #5 #6	0		

	Lane	Group:	Intersect: Adj Sat	ion Perf V/c	ormance g/C	Summary		Approac	
	Mvmts	Cap	Flow	Ratio	Ratio	Delay	LOS	Delay	LOS
==	_								
EB	L	551	1770	0.596	0.311	21.2	С	20.9	C
	\mathtt{LT}	562	1806	0.594	0.311	21.2	С		Ū
	R	492	1583	0.108	0.311	16.8	C		
NB	T	2670	5588	0.218	0.478	10.4	В	10.5	В
	R	756	1583	0.259	0.478	10.7	В		D
SB	L	. 516	1770	0.143	0.289	6.5	В	6.7	В
	T	2318	3725	0.432	0.622	6.8	В	0.7	Б
Lost	Time/C	Inte Cycle, L	ersection D		11.8 se	c/veh Int	ersect	cion LOS	= B
T03 C	TTIME/ C	Acre, P	= 6.0 se	c Cri	tical v/	C(X) =	0 195	7	

st Time/Cycle, L = 6.0 sec Critical v/c(x) = 0.487

HCM: SIGNALIZED INTERSECTION SUMMARY Version 2.4 01-23-1996

Center For Microcomputers In Transportation

Streets: (E-W) IH 30 EBFR

Analyst: GCL

(N-S) Ridge Road File Name: EBRBD.HC9

Analyst: GCL
Area Type: Other 11-1-95 PM Peak

Comment: Base + Development Traffic Volumes

		astbo		Wes		ound	No	rthbo	ind	Sou	===== ıthboı	==== ind		
	L	T	R	L	; T	R	L	${f T}$	R	L	T	R		
No. Lanes Volumes Lane Width RTOR Vols Lost Time	510 12.0	> 1 119 12.0 3.00	0						0	1 70 12.0		0		
Phase Combin	Signal Operations Phase Combination 1 2 3 4 5 6 7 0													

				Si	gnal	Oper	atio	ns				
		se Combination	n 1	2	3	4	1		5	6	7	8
	EB	Left	*				NB	Left			•	Ü
		Thru	*		· ·			Thru	*			
		Right	*		-			Right	*			
		Peds			12.6			Peds				
	WB	Left					SB	Left	*	*		
		Thru						Thru	*	*		
		Right						Right				
		Peds					_ =	Peds				
	NB	Right					EB	Right				
	SB	Right					WB	Right				
	Gree		1.0A				Gree		5.0A	13.0P		
Yellow/AR 0.0							Yel	low/AR (0.0		
Cycle Length: 90 secs				Phase	comb	inati	ion o	order:	1 #5	(A) 11 (A)		

						그 전 1의 역사 전기 등에 대기를 모르는 사는 취			
	T a w a	C	Intersec	tion Perf	formance	Summary			
	Lane	Group:	Adj Sat	V/C	g/C			Approac	rh:
	Mvmts	Cap	Flow	Ratio	Ratio	Delay	Los	Delay	Los
EB	L	551	1770	0.596	0.311	21.2	С	20.7	С
	LT	562	1806	0.594	0.311	21.2	Ċ	20.7	C
AN CONTRACTOR	R	492	1583	0.213	0.311	17.4	C		
NB	T	2670	5588	0.444	0.478	11.9	В	12.1	В
	R	756	1583	0.501	0.478	12.7	В		D
SB	L	339	1770	0.218	0.289	12.2	В	10.1	В
	\mathbf{T}	2318	3725	0.743	0.622	10.0	В	10.1	D
	200	Inte	ersection	Delay =	12.8 se	c/veh Int	0.000	ion LOS	= B

Lost Time/Cycle, L = 6.0 sec Critical v/c(x) = 0.694

"BASE <GID01> TEXAS DEPARTMENT OF HIGHWAYS AND PUBLIC TRANSPORTATION DIAMOND INTERCHANGE SIGNALIZATION - 145105 PASSER3 PASSER III-90 **VER 1.0** OCT 90 PPPP SSS EEEEE RRRR AAA SSS IIIIIIIIIII PAASSSE RR PAAS SE RR P I Ĩ I I SSS EEEE RRRR S E R R AAAAA SSS PPPP I I S P A A Ī I A A S S S S E P R R Ī. I A A SSS SSS EEEEE R R P IIIIIIIIIIII FREEWAY NAME - - - IH 30 CITY NAME - - - - - - - ROCKWALL DISTRICT NUMBER - - - - - - - 02

DATE - - - - - - - - - 11/01/95

RUN NUMBER - - -

*** PARAMETERS ***

NUMBER OF INTERCHANGES - - - 1

LOWER CYCLE LIMIT (SEC) - - - 90

UPPER CYCLE LIMIT (SEC) - - - - 100

CYCLE INCREMENT (SEC) - - - - 10 ..

*** OPTIONS ***

OPTIMIZE INTERNAL OFFSETS ? - - YES

EVALUATE INTERNAL OFFSETS ? - - NO

*** LEFT-SIDE MOVEMENT DATA ***

TRAFFIC VOLUME SATURATION MINIMUM MOVEMENT (VPH) FLOW (VPHG) PHASE (SEC) * * * * * * * * * * * * * * * * * * *	*	*	*	*	*	*	*	*	*	*	* :	* *	*	*	*	*	*	: ;	* *	*	*	*	*	*	: 4	,	k .	*	*	*	*	*	*	*	*	*	*	*	*
MOVEMENT (VPH) FLOW (VPHG) PHASE (SEC) * * * * * * * * * * * * * * * * * * *	5.5						-	rr/	AFI	FIC								V	OLU	ME				SA	JT	JRA	T	ΙO	N			N	IIN	IIM	IUN	1			•
ARTERIAL RIGHT-TURN 129 1900 STRAIGHT-THROUGH 327 3800 10 STRAIGHT-THEN-LEFT 55 1900 - FRONTAGE ROAD RIGHT-TURN 14 1900 STRAIGHT-THROUGH 119 719 10 LEFT-THEN-STRAIGHT 510 3081 - LEFT-THEN-LEFT 0 0 1900 5 INTERIOR LEFT-TURN 70 1900 5		- 21		0.277	1020		ľ	7ON	E	IEN	T							(7	/PH	I)				FI	OV	7	(V	PН	G)		F								1
RIGHT-TURN 129 1900 - STRAIGHT-THROUGH 327 3800 10 STRAIGHT-THEN-LEFT 55 1900 - FRONTAGE ROAD RIGHT-TURN 14 1900 - STRAIGHT-THROUGH 119 719 10 LEFT-THEN-STRAIGHT 510 3081 - LEFT-THEN-LEFT 0 0 0 - INTERIOR LEFT-TURN 70 1900 5	*	*	*	*	*	*	*	*	*	*	* ;	* *	*	*	*	*	*	: :	k *	*	*	*	*	*	* *	. 7	k ·	*	*	*				*	*	*	*	*	, *
RIGHT-TURN 129 1900 - STRAIGHT-THROUGH 327 3800 10 STRAIGHT-THEN-LEFT 55 1900 - FRONTAGE ROAD RIGHT-TURN 14 1900 - STRAIGHT-THROUGH 119 719 10 LEFT-THEN-STRAIGHT 510 3081 - LEFT-THEN-LEFT 0 0 0 - INTERIOR LEFT-TURN 70 1900 5									•																														-
STRAIGHT-THROUGH 327 3800 10 STRAIGHT-THEN-LEFT 55 1900 - FRONTAGE ROAD 14 1900 - RIGHT-TURN 14 1900 - STRAIGHT-THROUGH 119 719 10 LEFT-THEN-STRAIGHT 510 3081 - LEFT-THEN-LEFT 0 0 - INTERIOR 1900 5					AL																																		1
STRAIGHT-THROUGH 327 3800 10 STRAIGHT-THEN-LEFT 55 1900 - FRONTAGE ROAD 14 1900 - STRAIGHT-THROUGH 119 719 10 LEFT-THEN-STRAIGHT 510 3081 - LEFT-THEN-LEFT 0 0 - INTERIOR LEFT-TURN 70 1900 5						R]	[GI	IT-	JT-	JRN								1	129	1					19	00)							_					
STRAIGHT-THEN-LEFT 55 1900 - FRONTAGE ROAD 14 1900 - RIGHT-TURN 14 1900 - STRAIGHT-THROUGH 119 719 10 LEFT-THEN-STRAIGHT 510 3081 - LEFT-THEN-LEFT 0 0 - INTERIOR 1900 5						SI	CRA	IC	CH	-T	HRO	OUG	H					-	327	•														10	r.			5	
FRONTAGE ROAD RIGHT-TURN 14 1900 - STRAIGHT-THROUGH 119 719 10 LEFT-THEN-STRAIGHT 510 3081 - LEFT-THEN-LEFT 0 0 - INTERIOR LEFT-TURN 70 1900 5						SI	CRA	AIC	HI	т - т	HEN	J-T	म.म	T																				10				-	
RIGHT-TURN 14 1900 - STRAIGHT-THROUGH 119 719 10 LEFT-THEN-STRAIGHT 510 3081 - LEFT-THEN-LEFT 0 0 - INTERIOR LEFT-TURN 70 1900 5						2017-2005						_		_					55	16					1,5	, 0 (,							_	•			-	-
RIGHT-TURN 14 1900 - STRAIGHT-THROUGH 119 719 10 LEFT-THEN-STRAIGHT 510 3081 - LEFT-THEN-LEFT 0 0 - INTERIOR LEFT-TURN 70 1900 5					FF	SON	J Tr Z	CF	י ד	2 O A																												- 2	-
STRAIGHT-THROUGH 119 719 10 LEFT-THEN-STRAIGHT 510 3081 - LEFT-THEN-LEFT 0 0 - INTERIOR LEFT-TURN 70 1900 5																											500											-	~.
LEFT-THEN-STRAIGHT 510 3081																																		-					
LEFT-THEN-LEFT 0 0 - INTERIOR LEFT-TURN 70 1900 5																		- 3	19						7	19)							10	ĺ,				
LEFT-THEN-LEFT 0 0 - INTERIOR LEFT-TURN 70 1900 5													GH	${f T}$				5	510						30	81								_					
INTERIOR LEFT-TURN 70 1900 5						LE	FI	-T	HE	N-	LEI	$^{\mathrm{r}}$							0															_	6				6
LEFT-TURN 70 1900 5																																						7	1.7
LEFT-TURN 70 1900 5					IN	TE	RI	OF	2																														-
CEDATCHE ENDONOR																			70						10	00								_				8	10
																		_								500 M								5					
STRAIGHT-THROUGH 737 3800 -						O I	. 17.	110	TI					/	3/						38	00)							-									

The state of the s

*** RIGHT-SIDE MOVEMENT DATA ***

* * * * * * * * * * * * * * * * * * *
RIGHT-TURN 228 1900 - STRAIGHT-THROUGH 439 3800 10
STRAIGHT-THROUGH 439 3800 10
STRAIGHT-THEN-LEFT 70 1900 -
FRONTAGE ROAD
RIGHT-TURN 27 1900 -
STRAIGHT-THROUGH 49 537 10
LEFT-THEN-STRAIGHT 298 3263 -
LEFT-THEN-LEFT 0 0
INTERIOR
LEFT-TURN 55 1900 5
STRAIGHT-THROUGH 837 3800 -

		i in In.	LEKNAL DI	ELAY-OFFSET INFORMATION ***
*	* * * * * * PHASING	* * * * * * OPTIMIZE?	* * * *	* * * * * * * * * * * * * * * * * * *
*	* * * * *		* * * *	INTERIOR QUEUE STORAGE * * * * * * * * * * * * * * * * * * *
	LEAD-LEAD	Ţ. Y	-	THROUGH MOVEMENT AT LEFT SIDE (VEH) 16
	LAG -LEAD	I Y	-	LEFT-TURN MOVEMENT AT LEFT SIDE (VEH) 8
	LEAD-LAG	Y	: -	THROUGH MOVEMENT AT RIGHT SIDE (VEH) 16
	LAG -LAG	Y	-	LEFT-TURN MOVEMENT AT RIGHT SIDE (VEH) 8
	TTI -LEAD	~ Y	_	
*	* * * * * * *	* * * * * * D LEFT TURNS?	* * * *	* * * * * * * * * * * * * * * * * * * *
*	* * * * * *	* * * * * *		INTERIOR TRAVEL TIMES * * * * * * * * * * * * * * * * * * *
	LEFT-SIDE IN RIGHT-SIDE I	TERSECTION NTERSECTION	YES YES	LEFT TO RIGHT (SEC) 10 RIGHT TO LEFT (SEC) 10

	*** GEN	ERAL SIGN	ALIZATION	INFORMATIO	N ***		
* * * * * * * * * * * * * * * * * * *	A	В	ERSECTION	* * * * *	В	* * * * * * * * * * * * * * * * * * *	*
PHASE TIME (SE	C) 33.6	40.1 1	6.3 49.		26.9 1	.4.3 63.1	
V/C RATIO LEVEL OF SERV		.41 A		38 * .24	.36 A	.25 .34 A A	
DELAY (SEC/VEH LEVEL OF SERV) 22.21 ICE C	20.08 1 C	1.27 6. B B		28.45 C	5.48 2.98 A A	
STORAGE RATIO LEVEL OF SERV * * * * * * * * PHASE ORDER INTERNAL OFF	* * * * * LEAD-LAG		B C	26 *		.14 .17 C C * * * * * * * * * * * * * * * * * * *	*
<spi01></spi01>				€8F₹ ₩3+ ₹	J		
* * * INTERCHANG			HASING IN	EBFR' WBFR=	LUS 8/8.0	RUN 01 PAGE	4B
* * * * * * * *	* * * * *	* * * *	* * * * *	* * * * * *	* * * * *	* * * * * * *	t
PHASE INTERVAL NUMBER	* LEFT-SID * A * < * *>	В	C * A * <		B * ^ * P	HASE INTERVAI LENGTH (SEC)	i.
* * * * * * * *	* * * * *	* * * *	* * * * * *	* * * * * *	* * * * *	* * * * * * *	r
1 2 3 4 5	* * * * * * * *	A 33.6 A B 40.1 B C 16.3	* * * * * * * *	B A C B B	* * * * * * * * *	4.00 29.60 19.20 14.30 6.60 16.30	
* * * * * * * * INTERNAL OFFS	* * * * * * ET 4 S	* * * * ; EC	*_* * * *		* * * * CLE LENGT ASE ORDER		

*** GENERAL SIGNALIZATION INFORMATION ***

						1			
* * * * * * * * *	* * * * * * *	* * * *	* * *	* *	* * *	* * * *	* * * *	* * *	*
MEASURES OF	LEFT-SIDE								
EFFECTIVENESS	A B						-11111111111111111111111111111111111111		
TITECITATIVEDS	. А Б	С	A+C	*	Α	[±] B	C	A+C	
* * * * * * * * * *	* * * * * * *	* * * *	* * *	* *	* * *	*: * * *	* * *	* * *	*
				*		1.			
PHASE TIME (SEC)	37.4 44.8	17.8	55.2	* "	54.5	29.9	15 6	70.1	
				*	33	23.5	13.0	70.1	
V/C RATIO	26 41	27	2.0		0.4				
TEVEL OF CHRISTON					.24		.25	.33	
LEVEL OF SERVICE	A A	A	Α	*	A	- A	A	A	
				*					
DELAY (SEC/VEH)	24.30 21.68	14.14	8.03	* 1	13.95	31.06	5.03	1.80	
LEVEL OF SERVICE	C C	В		*	В				
	<u> </u>	ъ	Б	^	D	- C	A	A	
				*					
STORAGE RATIO		.18	.55	*			.08	.10	
LEVEL OF SERVICE		C		*		4.	В		
* * * * * * * * * *	ياساساس ملت ملت					2	В	В	
DILLOW CORPORT					* * *	* * * *	* * *	* * *	*
	G -LEAD	TOTAL IN	TERCHA	NGE	DELAY	15.57	VEH-HE	RS/HR	
INTERNAL OFFSET	65 SEC				SEC	_0.0,		(2) 1110	
		0-0-11	1110 111	100	2110				

<SPI01>
* * * INTERCHANGE 1 RIDGE ROAD

RUN 01 PAGE 5B

CYCLE LENGTH 100 SEC PHASE ORDER LAG -LEAD

*	** SIGNAL PHAS:	NG INFORMATION ***	
* * * * * * * * * * *	* * * * * * * *	* * * * * * * * * * * * *	* * * * * * * * * *
	T-SIDE SEQUENCE	* RIGHT-SIDE SEQUENCE	*
THE TAXBERTY :	СВ		*
PHASE INTERVAL * <	<	* <	T111/1/11
*	-> V	*	* LENGTH (SEC)
*	V	*	*
* * * * * * * * * * *	* * * * * * * *	* * * * * * * * * * *	* * * * * * * * *
*		*	*
1 *	A	* A	* 35.10
2 *	A	* B	* 2.30
4 *	B	* B	* 17.80
5 *	В	* B	* 9.80 * 15.60
6 *	В	* A	* 15.60 * 19.40
*		*	*
* * * * * * * * * * *	* * * * * * * *	* * * * * * * * * * *	* * * * * * * * *
INTERNAL OFFSET	65 SEC	CYCLE L	ENGTH 100 SEC

<gid01> " Base F</gid01>	Plus Development"	
PASSER3	AS DEPARTMENT OF HIGHWAYS AND PUBLIC TRANSPORTATION DIAMOND INTERCHANGE SIGNALIZATION - 145105 PASSER III-90	VER 1.0
PPPP P P PPPP P P P P P P P	AAA SSS SSS EEEEE RRRR IIIIIIIIIIIIIIIIIIII	OCT 90
	FREEWAY NAME IH 30	E ₂ -1
	CITY NAME ROCKWALL	<u>.</u>
	DISTRICT NUMBER 00	*
	DATE 11/01/95	
	RUN NUMBER 03	

LOWER CYCLE LIMIT (SEC) - - - 90

UPPER CYCLE LIMIT (SEC) - - - - 100

CYCLE INCREMENT (SEC) - - - - 10

*** OPTIONS ***

OPTIMIZE INTERNAL OFFSETS ? - - YES

EVALUATE INTERNAL OFFSETS ? - - NO

* * * INTERCHANGE 1 RIDGE ROAD

*** LEFT-SIDE MOVEMENT DATA ***

*	*	*		1000	*		TR MO	AF VE	ME	* * IC ENT			***	*	*		_	VO (V	LUI PH	ME)				SA FL	TU:	RA!	ric VPI	HG	*	*	1	* MIN ASE	IIM			*	*	*
*	×	ж	*	ж	ж	*	*	*	*	* *	*	*	*	*	*	*	×	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
				AI	S	IG:	HT AI	-T GH	Т-	N TH:				r				6	51 49 95						190 380	00							10	-)				
				FF	RI SI LI	[G] [R [F]	HT AI I-	-T GH TH	UR T- EN	AD N THI	ROI I'R.	AI		r				1	56 19 10 0			ti			190 71 308	L9							10					
				IN		F	r-	TU:		THI	ROI	JGI	H					11	70 96						190								5					

<IMD01B>
 * * * INTERCHANGE 1 RIDGE ROAD

RUN 02 PAGE 2B

*** RIGHT-SIDE MOVEMENT DATA ***

*	*	*	*	TRA	* * * AFFIC VEMENT * * *	*	*	*	*	*	*	,	(V	LUI PH	ME)		S	ATI IOL'	UR. W	AT (V	IO:	N	P.	* * MI: HAS: * *	NI: E	MUN (SE	1	*	*	*
				ARTERIAI	L																									
				RIGHT-	-TURN								2	28				19	900	0						_				
				STRAIG	GHT-THI	ROT	JGE	I					7	73				38	300	0					1	0				
				STRAIG	SHT-THI	EN-	LE	FT	1					70				19	900	0					-	_				
				FRONTAGE	E ROAD																									
				RIGHT-	-TURN								- 3	27				19	900)						_				
					SHT-THE									49				3	394	1					10)				
				LEFT-I	THEN-SI	RA	ΙG	HT					4	23				34	106	5						_				
				LEFT-I	THEN-LE	FT								0					()					-	-				
				INTERIOR	2																									
				LEFT-T	URN								(95				10	900)					5	=				
				STRAIG		OU	GH					1	115	0.000000					300	2.5					-	-				

*** INTERNAL DELAY-OFFSET INFORMATION ***

	į.		
*		* * * *	* * * * * * * * * * * * * * * * * * * *
*	PHASING OPTIMIZE?	FORCE?	INTERIOR QUEUE STORAGE * * * * * * * * * * * * * * * * * * *
	LEAD-LEAD Y	_	THROUGH MOVEMENT AT LEFT SIDE (VEH) 16
	LAG -LEAD 1 Y	-	LEFT-TURN MOVEMENT AT LEFT SIDE (VEH) 8
	LEAD-LAG Y	- ,	THROUGH MOVEMENT AT RIGHT SIDE (VEH) 16
	LAG -LAG Y	_	LEFT-TURN MOVEMENT AT RIGHT SIDE (VEH) 8
	TTI -LEAD Y	_	
*	* * * * * * * * * * * * * * * * PERMITTED LEFT TURNS	* * * *	* * * * * * * * * * * * * * * * * * *
*	* * * * * * * * * * * * *	* * * *	* * * * * * * * * * * * * * * * * * * *
	LEFT-SIDE INTERSECTION RIGHT-SIDE INTERSECTION	YES YES	LEFT TO RIGHT (SEC) 10 RIGHT TO LEFT (SEC) 10

* * * INTERCHANGE 1 RIDGE ROAD

*** GENERAL SIGNALIZATION INFORMATION ***

LEFT-SIDE INTERSECTION * RIGHT-SIDE INTERSECTION MEASURES OF EFFECTIVENESS C ... A+C Α PHASE TIME (SEC) 46.3 30.9 12.8 59.1 * 48.7 26.5 14.8 63.5 V/C RATIO .36 .55 .38 A A A .51 * .41 .50 .42 .46 LEVEL OF SERVICE A A * A A Α DELAY (SEC/VEH) 15.32 29.54 13.77 4.23 * 14.42 33.07 7.94 3.28 LEVEL OF SERVICE B C A * В -В D STORAGE RATIO .33 * .17 .14 .27 LEVEL OF SERVICE D PHASE ORDER LAG -LEAD TOTAL INTERCHANGE DELAY 20.51 VEH-HRS/HR INTERNAL OFFSET 64 SEC CYCLE LENGTH 90 SEC EBFR => LOS B = 11.9 sec/reh WBFR => L93/8.8

<SPI01> * * * INTERCHANGE 1 RIDGE ROAD

RUN 02 PAGE 4B

*** SIGNAL PHASING INFORMATION ***

* LEFT-SIDE SEQUENCE * RIGHT-SIDE SEQUENCE * СВ Α B C * PHASE INTERVAL <----^ * PHASE INTERVAL NUMBER LENGTH (SEC) A-48.7: +9* 1 A>46.3 46* 37.50 2 8.80 12.80 4.90 ·c-14-6 14.80 INTERNAL OFFSET 64 SEC CYCLE LENGTH 90 SEC

PHASE ORDER LAG -LEAD

	*** GENERAL S	SIGNALIZATION I	NFORMATION	***	
MEASURES OF EFFECTIVENESS	* * * * * * * * * * * * * * * * * * *	INTERSECTION C A+C	* RIGH	T-SIDE INT	ERSECTION
PHASE TIME (SE	C) 51.7 34.4	13.9 65.6	* * 54.4 *	29.4 16	70.6
V/C RATIO LEVEL OF SERV	.36 .54 ICE A A	.37 .51 A A	* .40 * A		.41 .46 A A
DELAY (SEC/VEH LEVEL OF SERV) 16.56 31.81 ICE B C	13.97 4.42 B A	* 15.55 * B		.77 3.15 B A
STORAGE RATIO LEVEL OF SERV * * * * * * * * PHASE ORDER INTERNAL OFF	* * * * * * * * * * * * * * * * * * *	_	* * * * * *	* * * * *	.14 .26 C C * * * * * * EH-HRS/HR

<SPI01> * * * INTERCHANGE 1 RIDGE ROAD

RUN 02 PAGE 5B

*	** SIGNAL PHAS	NG INFORMATION ***	
* * * * * * * * * * * *	* * * * * * * *	* * * * * * * * * * * * * *	* * * * * * * * * *
* LEF	T-SIDE SEQUENCE	* RIGHT-SIDE SEQUENCE	*
* A	СВ	* A B C	*
PHASE INTERVAL * <	<	* < ^ ^	* PHASE INTERVAL
NUMBER *		*	* LENGTH (SEC)
*	-> A A	*>	*
* * * * * * * * * * * *		*	*
*	****	* * * * * * * * * * * *	* * * * * * * * *
1 *	A	* *	*
2		Δ.	* 43.60
2 *	A	* B	* 8.10
3 *	C	* B	* 13.90
4 *	В	* B	* 7.40
5 *	В	* C	* 16.20
6 *	В	* · A	* 10.80
*		*	*
* * * * * * * * * *	* * * * * * * *	* * * * * * * * * *	* * * * * * * * *
INTERNAL OFFSET	73 SEC	CVCLE	ENCTH 100 SEC

CYCLE LENGTH 100 SEC PHASE ORDER LAG -LEAD

Center For Microcomputers In Transportation

HCS: Unsignalized Intersection Release 2.1 Page 1 *******************

File Name RCB.HCO

Streets: (N-S) Ridge Road

(E-W) Chili's Driveway

Major Street Direction.... NS

Length of Time Analyzed... 60 (min)

Analyst..... GCL Date of Analysis..... 11/1/95

Other Information..... Base Traffic Volumes

Two-way Stop-controlled Intersection

	Nor	thbou	nd	Sou	thbou	nd	Eas	stbou	nd	Wes	tboun	===== d
	L	T 	R	L	T	R	L	T	R	L	\mathbf{T}^{-1}	R
No. Lanes Stop/Yield	0	3<	0 N	1	2	0 N	0	0	0	0>	1<	0
Volumes PHF Grade		482 .95	7 .95	10 .95	741 .95					9 .95	0 · 95	.95
MC's (%) SU/RV's (%)		0	0	0	0			0		0	0	0
CV's (%) PCE's		0	0	0	0					0	0 0 1.1	0 0 1.1

Adjustment Factors

Vehicle	Critical	Follow-up
Maneuver	Gap (tg)	Time (tf)
Left Turn Major Road	5.50	2.10
Right Turn Minor Road	5.50	2.60
Through Traffic Minor Road	6.50	3.30
Left Turn Minor Road	7.00	3.40

WorkSheet for TWSC Intersection

z incersection	
WB	EB
164 1143 1143 0.98	
SB	NB
489 937 937 0.99	
WB	EB
1236 206	
0.99 203 1.00	
WB	EB
1236 172	
0.99 0.99	
0.99 170	
	164 1143 1143 0.98 SB 489 937 937 0.99 WB 1236 206 0.99 203 1.00 WB 1236 172 0.99 0.99 0.99

	Center For Microcomputers In Transportation	
HCS:	Unsignalized Intersection Release 2.1	Page 3
****	************************	******

Mov	ement		lowRate (pcph)	MoveCa		SharedCap Csh(pcph)	Avg.Tot Delay	al	LOS	Delay By App
WB	L	1.1	10	170	>		>		>	
WB	R	: . -;~·	22	1143	>	410	>	9.5	> B	9.5
SB	L		12	937			3.9		A	0.1
			Int	ersect	io	n Delav =	0.2			

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Center For Microcomputers In Transportation

HCS: Unsignalized Intersection Release 2.1 Page 1 *****************

File Name RCBD.HC0

Streets: (N-S) Ridge Road

(E-W) Chili's Driveway

Major Street Direction.... NS

Length of Time Analyzed... 60 (min)

Analyst..... GCL

Date of Analysis..... 11/1/95

Other Information..... Base Plus Development Traffic Volumes

Two-way Stop-controlled Intersection

	Northbound			Southbound			Eastbound			Westbound		
	L	T 	R	L .	T	R	L	T	R	L	${f T}$	R
No. Lanes Stop/Yield	0	3<	0 N	0]	2	0 N	0	0	0	0	0	1
Volumes PHF Grade		462 .95	67 .95		95			0	0			77 •95
MC's (%) SU/RV's (%)		0	0		0			0			0	0
CV's (%) PCE's		0 1.1	1.1	1	0		3					0 1.1

Vehicle Maneuver	Critic Gap (t	1011011	
Left Turn Major Road Right Turn Minor Road Through Traffic Minor Road Left Turn Minor Road	5.50 5.50 6.50 7.00	2.60 3.30	

Step 1: RT from Minor Street	WB	1.	EB
	188	Ĭź	
Potential Capacity: (pcph) 11	112	Ξ.	
Movement Capacity: (pcph) 11	112	•	
Prob. of Queue-free State: 0.	92		

Center For	Microcomputers In Transport	ation
HCS: Unsignalized Inters	section Release 2.1	Page 3
*************	*********	******

	ement	FlowRate v(pcph)	MoveCap Cm(pcph)	SharedCap Csh(pcph)	Avg.Total Delay	Los	Delay By App
WB	R	89	1112		3.5	A	

Intersection Delay = 0.1

Center For Microcomputers In Transportation

File Name RNDBD.HC0

Streets: (N-S) Ridge Road

(E-W) North Driveway

Major Street Direction... NS

Length of Time Analyzed... 60 (min)

Analyst..... GCL

Date of Analysis..... 11/6/95

Other Information..... Base + Development Traffic Volumes

Two-way Stop-controlled Intersection

	Northbound L T R			Southbound			===== Ea	===== stbou		Westbound		
				L	T	R	L	${f T}$	R	$\mathbf{L}_{_{_{_{_{_{_{_{_{_{_{_{_{_{_{_{_{_{_$	\mathbf{T}	R
No. Lanes Stop/Yield	1	2<	O N	1	2<	0 N	0	0	1	0	0	1
Volumes PHF Grade MC's (%) SU/RV's (%) CV's (%) PCE's	17 .95 0 0 0	1120 .95 0 0 0 0	55 .95 0 0 0	256 .95 0 0 0	1260 .95 0 0 0 0	142 .95 0 0 0		0	42 .95 0 0 0		0	250 .95 0 0 0

Vehicle	Critical	Follow-up
Maneuver	Gap (tg)	Time (tf)
Left Turn Major Road	5.50	2.10
Right Turn Minor Road	5.50	2.60
Through Traffic Minor Road	6.50	3.30
Left Turn Minor Road	7.00	3.40

Step 1: RT from Minor Street	WB	EB
Conflicting Flows: (vph) Potential Capacity: (pcph) Movement Capacity: (pcph) Prob. of Queue-free State:	588 697 697 0.62	701 611 611 0.93
Step 2: LT from Major Street	SB	NB
Conflicting Flows: (vph) Potential Capacity: (pcph) Movement Capacity: (pcph) Prob. of Queue-free State:	1175 401 401 0.33	1402 303 303 0.93

Intersection Performance Summary

Mov	ement	FlowRate v(pcph)		SharedCap Csh(pcph)	Avg.Total Delay	LOS	Delay By App
EB	R	44	611	£ ·	6.3	В	
WB	R	263	697		8.3	В	
NB SB	L	20 269	303 401	-	12.7 26.7	C D	0.2 4.1

Intersection Delay = 3.0

Center For Microcomputers In Transportation HCS: Unsignalized Intersection Release 2.1 Page 1

File Name RNDE.HCO

Streets: (N-S) Ridge Road

(E-W) Carlisle Plaza North

Major Street Direction.... NS

Length of Time Analyzed... 60 (min) Analyst..... GCL

Date of Analysis..... 11/7/95

Other Information..... Existing Traffic Volumes - PM Peak Hour

Two-way Stop-controlled Intersection

	Northbound			Southbound			===== Ea	==== stbou	==== nd	========== Westbound		
	L 	T	R	L	T	R	L	T	R	L	Т	R
No. Lanes Stop/Yield	1	1	0 N	0	1<	0 N	0>	1<	0	0	0	0
Volumes PHF	17 •95	431 .95	14		1009	142 .95	29	0	42			
Grade MC's (%)	0	0			0	0	.95	.95	.95		0	
SU/RV's (%) CV's (%)	0	0			0	0	0	0	0			
PCE's	1.1	1.1			1.1	1.1	1.1	1.1	1.1			

Vehicle	Critical	Follow-up
Maneuver	Gap (tg)	Time (tf)
Left Turn Major Road	5.00	2.10
Right Turn Minor Road	5.50	2.60
Through Traffic Minor Road	6.00	3.30
Left Turn Minor Road	6.50	3.40

Step 1: RT from Minor Street	WB	EB
Conflicting Flows: (vph) Potential Capacity: (pcph) Movement Capacity: (pcph) Prob. of Queue-free State:		1080 393 393 0.88
Step 2: LT from Major Street	SB	NB
Conflicting Flows: (vph) Potential Capacity: (pcph) Movement Capacity: (pcph) Prob. of Queue-free State:		1151 485 485 0.96
Step 3: TH from Minor Street	WB	EB
Conflicting Flows: (vph) Potential Capacity: (pcph) Capacity Adjustment Factor		1528 172
due to Impeding Movements Movement Capacity: (pcph) Prob. of Queue-free State:		0.96 165 1.00
Step 4: LT from Minor Street	WB	EB
Conflicting Flows: (vph) Potential Capacity: (pcph) Major LT, Minor TH		1528 138
Impedance Factor: Adjusted Impedance Factor: Capacity Adjustment Factor		0.96 0.96
due to Impeding Movements Movement Capacity: (pcph)		0.96 132

	Center For Microcomputers In Transportation	
HCS:	Unsignalized Intersection Release 2.1	Page 3
****	******************	******

Mov	ement	FlowRate V(pcph)	te MoveCap) Cm(pcph)		SharedCap Csh(pcph)	Avg.Total Delay		LOS	Delay By App
EB	L	34	132	>		>		>	
EB	R	48	393	>	216	26.8		D >	26.8
NB	L	20	485			7.7		В	0.3

Intersection Delay = 1.2

Center For Microcomputers In Transportation

HCS: Unsignalized Intersection Release 2.1 Page 1 ******************

File Name RSTB.HC0

Streets: (N-S) Ridge Road

(E-W) Carlisle Plaza Drive

Major Street Direction.... NS

Length of Time Analyzed... 60 (min)

Analyst..... GCL

Date of Analysis.......... 11/6/95
Other Information...... Existing Traffic w/ Walmart & Widened Ridge Road

Two-way Stop-controlled Intersection

	Nor	thbou	nd	Sou	thboui	nd	Ea:	stbou	nd	West	bound	====
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes Stop/Yield Volumes PHF Grade MC's (%) SU/RV's (%) CV's (%) PCE's	71 .95 0 0 0	630 .95 0 0 0	ON	0	763 .95 0 0 0	0 N 111 .95	0> 53 .95 0 0 1.1	1< 0.95 0 0 0	0 118 .95 0 0 0	0	0	0

Vehicle Maneuver 	Critical Gap (tg)	Follow-up Time (tf)
Left Turn Major Road	5.50	2.10
Right Turn Minor Road	5.50	2.60
Through Traffic Minor Road	6.50	3.30
Left Turn Minor Road	7.00	3.40

Step 1: RT from Minor Street	₩B	EB
Conflicting Flows: (vph) Potential Capacity: (pcph) Movement Capacity: (pcph) Prob. of Queue-free State:		437 832 832 0.84
Step 2: LT from Major Street	SB	NB
Conflicting Flows: (vph) Potential Capacity: (pcph) Movement Capacity: (pcph) Prob. of Queue-free State:	76. 76.	874 582 582 0.86
Step 3: TH from Minor Street	WB	EB
Conflicting Flows: (vph) Potential Capacity: (pcph) Capacity Adjustment Factor due to Impeding Movements Movement Capacity: (pcph) Prob. of Queue-free State:		1520 141 0.86 121 1.00
Step 4: LT from Minor Street	WB	EB
Conflicting Flows: (vph) Potential Capacity: (pcph) Major LT, Minor TH		1520 113
Impedance Factor: Adjusted Impedance Factor: Capacity Adjustment Factor	a,	0.86 0.86
due to Impeding Movements Movement Capacity: (pcph)	;- 	0.86 97

	Center For Microcomputers In Transportation	
HCS:	Unsignalized Intersection Release 2.1	Dago 3
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Movement		FlowRate V(pcph)	-		SharedCap Csh(pcph)	Avg.Total Delay	LOS		Delay By App	
EB	L	62	97	>		>	>	= -		_
EB	R	136	832	>	247	66.0 >) F		66.0	
NB	L	83	582			7.2	В		0.7	
		Int	ersecti	lor	n Delav =	6.8				

HCM: SIGNALIZED INTERSECTION SUMMARY Version 2.4 01-23-1996

Center For Microcomputers In Transportation

Streets: (E-W) Steger Towne Drive (N-S) Ridge Road

352 1583

R

SB

Are	alyst: GC ea Type:	L Other se + De		nt Traffic	Fi 11 Volum	-1-95 es	me: R PM P	STBD.			
		Eas	stbound T R	Westbo	und	No	rthbo		So	uthbo	und
				L T	R 	L	T	R	L	${f T}$	R
Vol Lan RTC	Lanes umes e Width R Vols		9 118 12.0 12.0 22	12.0	1 9 389 0 12.0 78	12.0		Ω	12.0	820 12.0	0
Los	t Time	3.00 3	3.00 3.00	3.00 3.00	3.00	3.00	3.00	3.00	3.00	3.00	0 3.00
				Signal Op							
Pha	se Combir	nation	1 2	3	4	JIIS	ı	5	6	7	8
EB	Left		*		NB	Left		k	0	*	8
	Thru Right		*			Thru				*	
	Peds		*			Righ				*	
WB	Left		*		25	Peds					
	Thru		*		SB			•	*	*	
	Right		*			Thru			*	*	
	Peds					Righ Peds			*	*	
NB	Right				EB						
SB	Right				WB	_					
Gree			0A 21.0A		Gre			A 29.	OP 20	. 0A	
CVC	low/AR	0.	0 3.0		Yel	low/A	D 0 0	_	_		
			secs Ph	ase combin	ation	order	: #1	#2 #5	#6 #	7	
			Interse	ction Perf	ormanc						
		roup:	Adj Sa	t v/c	g/C	C Dum	шагу		λn	nwood	h.
	Mvmts	Cap	Flow	Ratio	Rati		elay	Los		proac lay	LOS
EB	LT	119	1704	_							
	R	106	1784 1583		0.06		37.6	D	65	. 4	F
WB	LT	414	1776		0.06		85.8 57.5	F			
	R	369	1583		0.23		40.6	E E	50	. 0	E
NB	L	135	1770	0.556	0.04		24.5	C	41	Ω	E
	T	828	3725	0.986	0.22		17.4	E	-7.	. 0	<u>C</u>

828 3725 0.986 0.222 47.4 E 352 1583 0.671 0.222 27.7 D 692 1770 0.962 0.722 35.6 D 20.1 C 1992 3659 0.517 0.544 10.1 B Intersection Delay = 34.7 sec/veh Intersection LOS = D Lost Time/Cycle, L = 12.0 sec Critical v/c(x) = 0.990

Center For Microcomputers In Transportation

Streets: (E-W) Steger Towne Drive (N-S) Ridge Road File Name: RSTBDI.HC9

Area Type: Other 11-1-95 PM Peak

Comment: Base + Development Traffic Volumes

				We	Westbound			Northbound			======================================		
- -	L	T	R	L	T	R	L	${f T}$	R	L	T	R	
No. Lanes Volumes Lane Width RTOR Vols Lost Time	62		1 118 12.0 0	381		12.0	1 71 12.0		1 224 12.0 0	8	820 12.0	111	
												3.00	
Phase Combir	ation	1	2	Signa 3	al Ope	- 1	ons	<u></u>	_				
EB Left		*	2	3	4	NB	T _ £L	5		6	7	8	
Thru		*				ND	Left Thru		•		*		
Right		*					Righ				*		
Peds							Peds				*		
WB Left			*			SB	Left			*			
Thru			*				Thru			*	*		
Right			*				Righ	t		*	*		
Peds NB Right							Peds						
SB Right			*			EB	Righ						
Green	0	03 2	2 03			WB	Righ			*			
Yellow/AR		.0A 2	3.0A				en	6.0	A 22.		.0A		
Cycle Length				se co	mbina	Yel	low/A	R 0.0	0.		.0		
cycle Length	: 90	secs	Pha	se co	mbina	tion	order	: #1	#2 #5	#6 #			

	т	A valente i seconomico con cinco	Intersec	tion Perf	ormance	Summary			
	Lane	Group:	Adj Sat	V/C	g/C			Approa	ch:
	Mvmts	Cap	Flow	Ratio	Ratio	Delay	LOS	Delay	LOS
EB	LT								
LD	100000	119	1784	0.622	0.067	37.6	D	42.1	E
tan "	R	158	1583	0.783	0.100	44.8	E		
WB -	LT	454	1776	0.903	0.256	39.6	D	24.3	С
	R	897	1583	0.456	0.567	8.9	В	24.5	C
NB	L	152	1770	0.493	0.067	19.6	C	23.1	
	T	993	3725	0.821	0.267	27.5	D	23.1	С
	R	827	1583	0.285	0.522	9.2			
SB	L	983	3539	0.698	0.278	23.7	В	3.C =	<u> 2</u>
	TR	1870	3659	0.550	0.511		С	16.5	C
			rsection			11.6	В		
	112 March 2010 - 100 March 2010 - 100 March 2010 March		TOCCCIOII	Detay =	21.4 Se	c/veh Int	PREPCT	ion Toc	- 0

Lost Time/Cycle, L = 12.0 sec Critical v/c(x) = 0.79121.4 sec/veh Intersection LOS = C HCM: SIGNALIZED INTEL ACTION SUMMARY Version 2.4 01-23-1996 Center For Microcomputers In Transportation

Streets: (E-W) Horizon Road Analyst: GCL

Area Type: Other

(N-S) Ridge Road File Name: RHB.HC9 11-1-95 PM Peak

Comment: Base Traffic Volumes

========	========	· voru	mes =====								
	Eastbo	2.0	Westbound			Northbound			Southbound		
	L T	R	L 	T 	R 	L	T 	R	L	T	R
No. Lanes	1 1	Ī1	1	_	<	1	1	1	1	2	<
Volumes Lane Width	1 117	A		58 12.0	167	1007	533	18	273	557	3
RTOR Vols		- 0	=		0			12.0	1,000,000,000,000	12.0	0
Lost Time	3.00 3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00
70	\$ 5 mg	-:-	Signa	al Ope	eratio	ons					
Phase Combin	nation 1 *	2	3	4	1		5		6	7	8
Thru	*				NB	Left		•		*	
Right Peds	*	-				Righ	t			*	
WB Left	*				SB	Peds Left			*	1010	
Thru Right	*					Thru			*	*	
Peds	*					Righ Peds			*	*	
NB Right					EB	Righ					
SB Right Green	25.0A				WB	Righ	t	_			
Yellow/AR	0.0				Gre Yel	en low/A	5.0 R 0.0	A 16. 0.		.0A	
Cycle Length	: 90 secs	Pha	se co	mbina	tion	order	: #1		#7	• 0	

	Lane	Group:	Intersect Adj Sat	ion Perf	ormance	Summary			
	Mvmts	Cap	Flow	V/c Ratio	g/C Ratio	Delay	LOS	Approad Delay	
V <u>ar. or Second</u>						1		Delay	LOS
EB	L	220	902	0.005	0.244	19.5	C	22.0	C
	T	455	1863	0.270	0.244	21.0	Č	22.0	C
	R	387	1583	0.463	0.244	22.7	C		
WB	L	274	1121	0.153	0.244	20.3	C	21.1	0
w	TR	809	3310	0.308	0.244	21.2	C	21.1	С
NB	L	278	1770	0.327	0.044	10.2	В	14.9	В
	T	849	1863	0.661	0.456	15.9	č	±4.7	Б
a D	R	721	1583	0.026	0.456	10.3	В		
SB	L	437	1770	0.657	0.433	17.6	C	9.4	В
	TR	2358	3723	0.262	0.633	5.5	В	J.4	Б
		Inter	section D	elay =		c/veh Int		ion Tos	- D
Lost	Time/C	ycle, L =	9.0 se	c Crit	tical v/	c(v) -	0 640		– Б

Critical V/C(X) = 0.640

HCM: SIGNALIZED INTERSECTION SUMMARY Version 2.4 01-23-1996

Center For Microcomputers In Transportation

Streets: (E-W) Horizon Road Analyst: GCL (N-S) Ridge Road

File Name: RHBD.HC9 Area Type: Other

Are Com	ea Type: nment: Ba ======	Other ase +De	evelo	pment	Traf	fic V	11 olume	-1-05	PM P		CJ		
		L L	T	und R	We:	stbou T	===== nd R	No:	rthbo T	===== und R	So	uthbo T	===== und R
Vol Lan RTO	Lanes umes e Width			1 170 12.0 0	12.0	81 12.0	0	12-0	12.0	^	12.0		38
	t Time	3.00	3.00	3.00				3.00	3.00	3.00	3.00	3.00	3.00
WB NB SB Gree Yell	se Combi Left Thru Right Peds Left Thru Right Peds Right Right Right en Low/AR Le Length	22	* * * * * *	2 Phas	3		eration of the series of the s	Left Thru Righ Peds Left Thru Righ Peds Righ en	t t t t 5.0	A 18.	6 * * * * *	7 * * * * * *	8
EB		Cap 157 393 334	Inte	ersect j Sat Flow 743 L863	tion : V, Rat	Perfo /c tio 249 376		e Sumi		Los C C	 Ap _]	proac lay 	h: Los

		Group:	Intersect	ion Perf V/c	ormance g/C	Summary			
	Mvmts	Cap	Flow	Ratio	Ratio	Delay	Los	Approac Delay	LOS
EB	L T	157 393	743	0.249	0.211	22.6		24.3	 C
	R	334	1863 1583	0.376 0.536	0.211 0.211	23.4 25.3	C		Ü
WB	L TR	202 704	955 3333	0.813	0.211	40.4	D E	29.6	D
NB	L	183	1770	0.426 0.497	0.211 0.044	23.7	В	32.5	Ъ
	T R	869 739	1863 1583	0.974 0.060	0.467	36.0	D	52.5	D
SB	L TR	476	1770	0.655	0.467 0.478	10.0 19.4	B C	8.9	В
	ıĸ	2465 Inte	3698 ersection D	0.344 elav =	0.667	5.0	7		
Lost	Time/C	VClo I	- 0 0		21.0 Se	c/ven int	ersect	ion Los	= C

Lost Time/Cycle, L = 9.0 sec Critical v/c(x) = 0.892

Center For Microcomputers In Transportation HCS: Unsignalized Intersection Release 2.1

File Name HSDBD.HC0

Streets: (N-S) South Driveway

(E-W) Horizon Road

Major Street Direction.... EW

Length of Time Analyzed... 60 (min)

Analyst..... GCL Date of Analysis..... 11/7/95

Other Information..... Base + Development PM Peak Hour

Two-way Stop-controlled Intersection

	Eas	tboun		Westbound			Northbound			Southbound		
		T	R	L	Т	R	L	${f T}$	R	L	\mathbf{T}	R
No. Lanes Stop/Yield Volumes PHF Grade MC's (%) SU/RV's (%) CV's (%)	0> 48 .95 0	1 431 .95 0 0 0	0 N	0	1< 289 .95 0 0	0 N 95 .95	0	0	0	0> 92 .95	1< 0- .95 0 0	139 •95
PCE's	1.1	1.1			1.1	1.1				1.1	1.1	1.1

Vehicle	Critical	Follow-up
Maneuver	Gap (tg)	Time (tf)
Left Turn Major Road	5.00	2.10
Right Turn Minor Road	5.50	2.60
Through Traffic Minor Road	6.00	3.30
Left Turn Minor Road	6.50	3.40

Step 1: RT from Minor Street	NB	SB
Conflicting Flows: (vph) Potential Capacity: (pcph) Movement Capacity: (pcph) Prob. of Queue-free State:		336 936 936 0.83
Step 2: LT from Major Street	WB	EB
Conflicting Flows: (vph) Potential Capacity: (pcph) Movement Capacity: (pcph) Prob. of Queue-free State: TH Saturation Flow Rate: (pcphpl) RT Saturation Flow Rate: (pcphpl) Major LT Shared Lane Prob. of Queue-free State:		384 1125 1125 0.95 1700
Step 3: TH from Minor Street	NB	SB
Conflicting Flows: (vph) Potential Capacity: (pcph) Capacity Adjustment Factor due to Impeding Movements Movement Capacity: (pcph) Prob. of Queue-free State:		816 407 0.93 378 1.00
Step 4: LT from Minor Street	NB	SB
Conflicting Flows: (vph) Potential Capacity: (pcph) Major LT, Minor TH		816 357
Impedance Factor: Adjusted Impedance Factor: Capacity Adjustment Factor		0.93 0.93
due to Impeding Movements Movement Capacity: (pcph)		0.93 332

	Center For Mic	crocomputers In Tra	nsportation
HCS .	Unsignalized Intersect	ion Dele 0 a	
neb.	onsignatized intersect	Lion Release 2.1	Page 3
***	*************	*******	***********
- 15			***********

Movement	FlowRate v(pcph)			SharedCap Csh(pcph)	Avg.To Delay	tal	LOS	Delay By App
SB_ L	107	332	>		>		>	
SB R	161	936	>	542	>	13.1	C >	13.1
EB L	56	1125			3.4		Α	0.3
	Int	ersect	ior	n Delav =	2 0			

May 14, 1996

Mr. Tony R. Tramel, P.E. DeShazo Tang & Associates Dallas, TX 75202

VIA FAX: 741-1937

Dear Tony:

There is no issue with the proposed zoning or platting for Steger Towne crossing, nor do I have any problems with your projected traffic volumes, etc. My personal problem is with the number of drives on the primary arterial through that area.

Your understanding of City guidance on driveway spacing is not complete. An extract of the Commercial District section of the Rockwall Zoning Ordinance is attached for your future use. The standard is 1 drive per 200 feet of street frontage per site for arterial streets, or as approved by the City Council. If a "site" is an outparcel, the concept plan is in compliance. If a "site" is the shopping center, the plan requires City Council approval.

As you know, neither PARAGON nor I are traffic engineers, and our reference library is not as extensive as yours. My opinions expressed at the work session were based on my understanding of the Zoning Ordinance and the brief guidance provided in the AASHTO green book (an extract attached). As you know, most of the AASHTO discussion is very general, and Figure II-29 is the only numerical data I found (and I gather from the information you provided, even its conclusions are in dispute). If we believe Figure II-29, then additional drives cause additional accidents. The density of proposed and existing drives onto FM 740 from Steger Towne Crossing and the bank would amount to 28 intersections per kilometer (if it were extended for a kilometer), which I think we would all agree is excessive. For these reasons I asked you to relook your previous recommendations.

Bill Crolley tells me that the concept plan has been previously approved by the Planning and Zoning Commission and the City Council. In that case, the appropriate thing for me to do is abstain on both the Steger Towne and the Boston Market agenda items.

Sincerely

G. William Quinby, P.E.

Planning and Zoning Commissioner

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