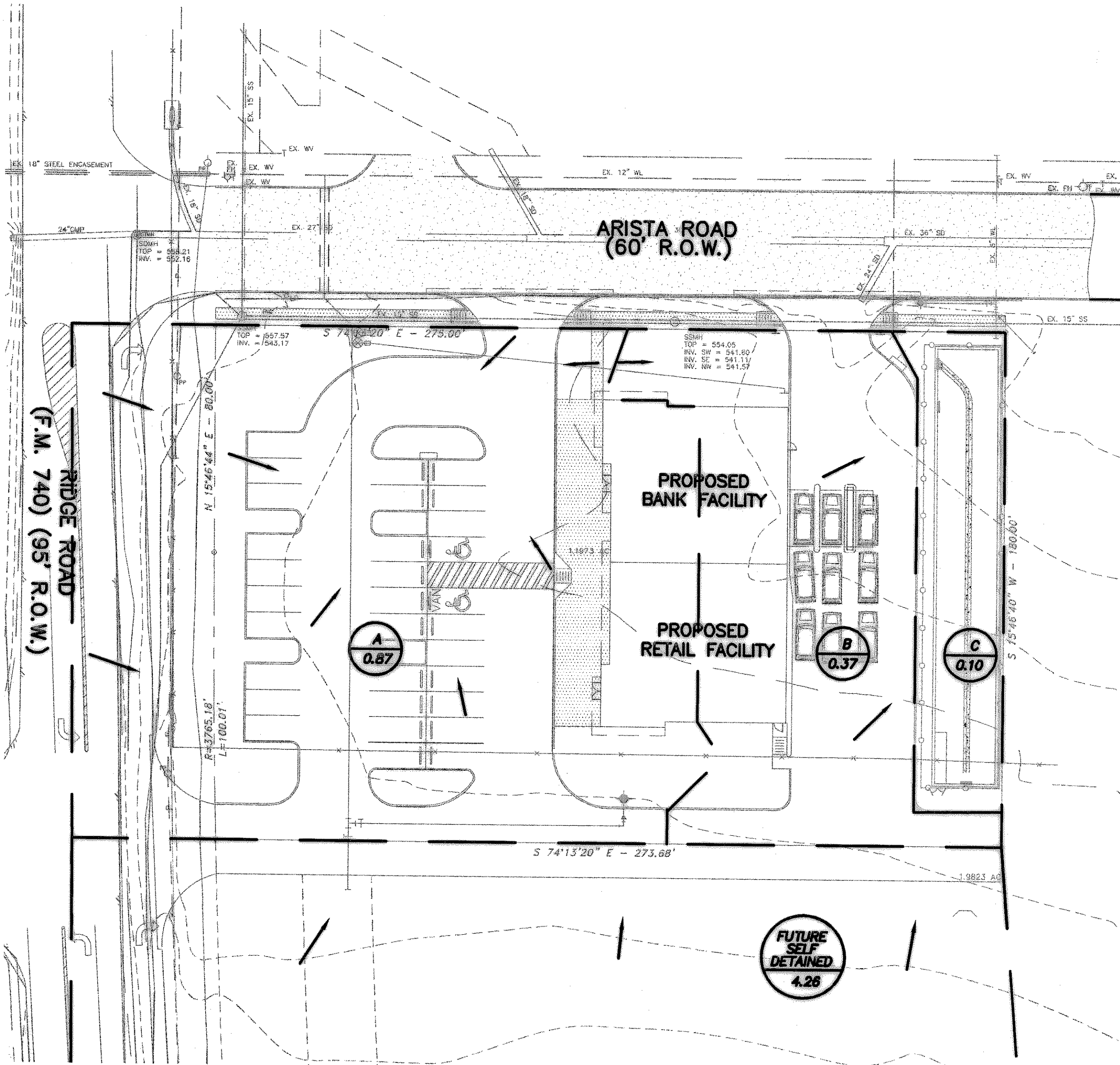


| DRAINAGE AREA CALCULATIONS |                    |      |           |           |         |
|----------------------------|--------------------|------|-----------|-----------|---------|
| AREA NO.                   | DRAINAGE AREA (AC) | C    | Tc (min.) | I (in/hr) | Q (100) |
| A                          | 0.87               | 0.90 | 10        | 9.8       | 7.67    |
| B                          | 0.37               | 0.90 | 10        | 9.8       | 3.26    |
| C                          | 0.10               | 0.90 | 10        | 9.8       | 0.88    |



DRAINAGE CALCULATIONS

| PRESENT CONDITIONS | FUTURE CONDITIONS (DEVELOPED) | OFF-SITE CONDITIONS (UNDEVELOPED) |
|--------------------|-------------------------------|-----------------------------------|
| Q=CIA              |                               |                                   |
| A(AC.) = 5.6       | A(AC.) = 1.34                 | A(AC.) = 4.26                     |
| C = 0.35           | C = 0.90                      | C = 0.35                          |
| Tc(min.) = 20      | Tc(min.) = 10                 | Tc(min.) = 10                     |
| I100(in/hr) = 8.3  | I100(in/hr) = 9.8             | I100(in/hr) = 9.8                 |
| Q100(cfs) = 16.27  | Q100(cfs) = 11.82             | Q100(cfs) = 14.61                 |

| Storm Duration | I (in/hr) | Q (cfs) | Inflow (cu.ft.) | Outflow (cu.ft.) | Storage (cu.ft.) |
|----------------|-----------|---------|-----------------|------------------|------------------|
| 10.0           | 9.8       | 26.4    | 15860.1         | 9760.8           | 6099.3           |
| 15.0           | 9.0       | 24.3    | 21848.1         | 12201.0          | 9647.1           |
| 20.0           | 8.3       | 22.4    | 28865.1         | 14641.2          | 12223.9          |
| 30.0           | 6.9       | 18.6    | 33500.5         | 19521.6          | 13978.9          |
| 40.0           | 5.8       | 15.6    | 37546.4         | 24402.0          | 13144.4          |
| 50.0           | 5.0       | 13.5    | 40459.5         | 29282.4          | 11177.1          |
| 60.0           | 4.5       | 12.1    | 43696.3         | 34162.8          | 9533.5           |
| 120.0          | 2.7       | 7.3     | 52435.5         | 63445.2          | -11009.7         |
| 180.0          | 2.0       | 5.4     | 58261.7         | 92727.6          | -34465.9         |
| 360.0          | 1.3       | 3.5     | 75740.2         | 180574.8         | -104834.6        |

TOTAL STORAGE VOLUME REQUIRED: 13,979 C.F.  
 TOTAL STORAGE VOLUME PROVIDED:  
 POND LENGTH: 152 L.F.  
 POND WIDTH: 21 FT.  
 AVG. WATER SURFACE DEPTH: 4.4 FT.  
 TOTAL STORAGE VOLUME PROVIDED: 14,045 C.F.  
 PER APPROVED PLANS, "HORIZON RIDGE ADDITION" BY LAWRENCE A. CATES & ASSOC., INC.  
 TOTAL ALLOWABLE FLOW (100-YR) FROM SITE AND FUTURE SELF-DETAINED AREA = 16.27 cfs

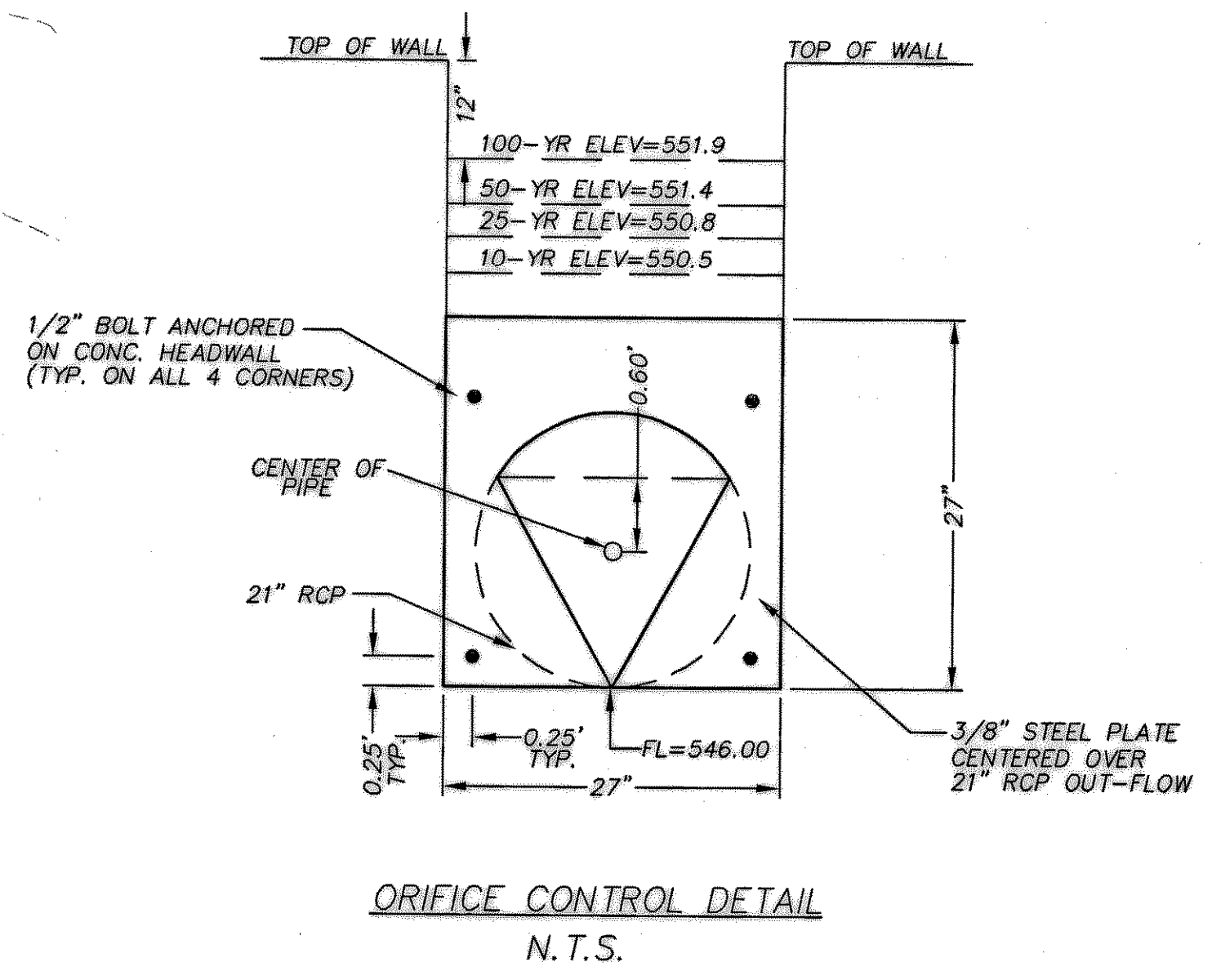
ORIFICE EQUATION:  $Q = 0.62A(2gH)^{1/2}$

**10-YR:**  
 Q10(PRE-DEV.)=11.56 cfs  
 Q10(POST-DEV.)=19.69 cfs  
 V10(REQ.)=9,436 C.F.  
 10 YR. WS = 550.5  
 OUTFALL ELEV. = 546.0  
 ASSUME 21" DIAMETER W/ ORIFICE CONTROL:  
 A = 1.18 S.F.  
 $H = 4.5' - \frac{21"}{24} = 3.62 \text{ FT.}$   
 $Q = (0.64) (1.18) (2g \times 3.62)^{1/2} = 11.5 \text{ cfs}$

**25-YR:**  
 Q25(PRE-DEV.)=13.13 cfs  
 Q25(POST-DEV.)=22.39 cfs  
 V25(REQ.)=10,945 C.F.  
 25 YR. WS = 550.8  
 OUTFALL ELEV. = 546.0  
 ASSUME 21" DIAMETER W/ ORIFICE CONTROL:  
 A = 1.18 S.F.  
 $H = 4.8' - \frac{21"}{24} = 3.92 \text{ FT.}$   
 $Q = (0.64) (1.18) (2g \times 3.92)^{1/2} = 12.0 \text{ cfs}$

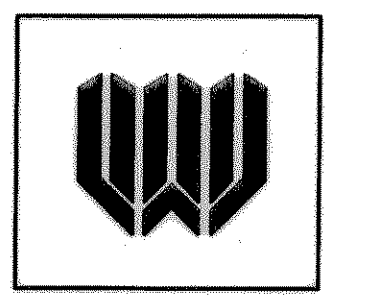
**50-YR:**  
 Q50(PRE-DEV.)=14.70 cfs  
 Q50(POST-DEV.)=24.27 cfs  
 V50(REQ.)=12,455 C.F.  
 50 YR. WS = 551.4  
 OUTFALL ELEV. = 546.0  
 ASSUME 21" DIAMETER W/ ORIFICE CONTROL:  
 A = 1.18 S.F.  
 $H = 5.4' - \frac{21"}{24} = 4.52 \text{ FT.}$   
 $Q = (0.64) (1.18) (2g \times 4.52)^{1/2} = 12.9 \text{ cfs}$

**100-YR:**  
 Q100(PRE-DEV.)=16.27 cfs  
 Q100(POST-DEV.)=26.43 cfs  
 V100(REQ.)=13,979 C.F.  
 100 YR. WS = 551.9  
 OUTFALL ELEV. = 546.0  
 ASSUME 21" DIAMETER W/ ORIFICE CONTROL:  
 A = 1.18 S.F.  
 $H = 5.9' - \frac{21"}{24} = 5.02 \text{ FT.}$   
 $Q = (0.64) (1.18) (2g \times 5.02)^{1/2} = 13.6 \text{ cfs}$



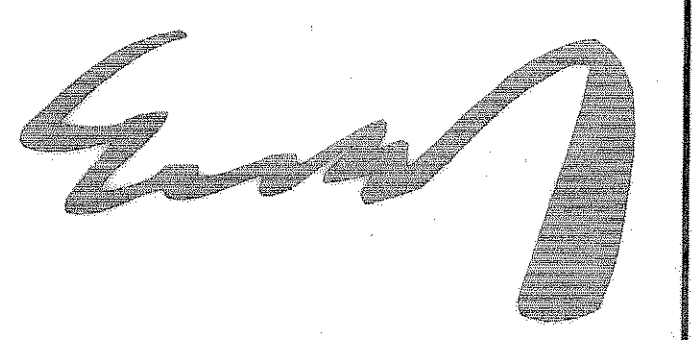
| No. | Date | Description |
|-----|------|-------------|
|-----|------|-------------|

WASHINGTON MUTUAL



ROCKWALL, TEXAS

Yancey-Hausman Development



Philip Ewald Architecture Incorporated  
 Architecture Interior Design Planning  
 4203 Yocum Blvd Suite 100 Houston Texas 77006  
 tel 713.522.9777 fax 713.522.2507

|                    |
|--------------------|
| Drawn: MTL         |
| Checked: J.M.      |
| Proj No: 001702600 |
| Date: 12-04-03     |
| Scale: 1"=30'      |

DRAINAGE AREA MAP & CALCULATIONS

C3

RECORD DRAWING  
 REVISED TO REFLECT KNOWN CHANGES MADE DURING CONSTRUCTION.

PATE ENGINEERS

8150 BROOKRIVER DRIVE  
 SUITE S-700  
 DALLAS, TEXAS, 75247  
 PH. (214) 357-2981  
 FAX (214) 357-2985  
 JOB NO. 001702600

| NO. | DATE | DESCRIPTION |
|-----|------|-------------|
|     |      |             |
|     |      |             |

REF. SHEET C1 FOR GENERAL NOTES