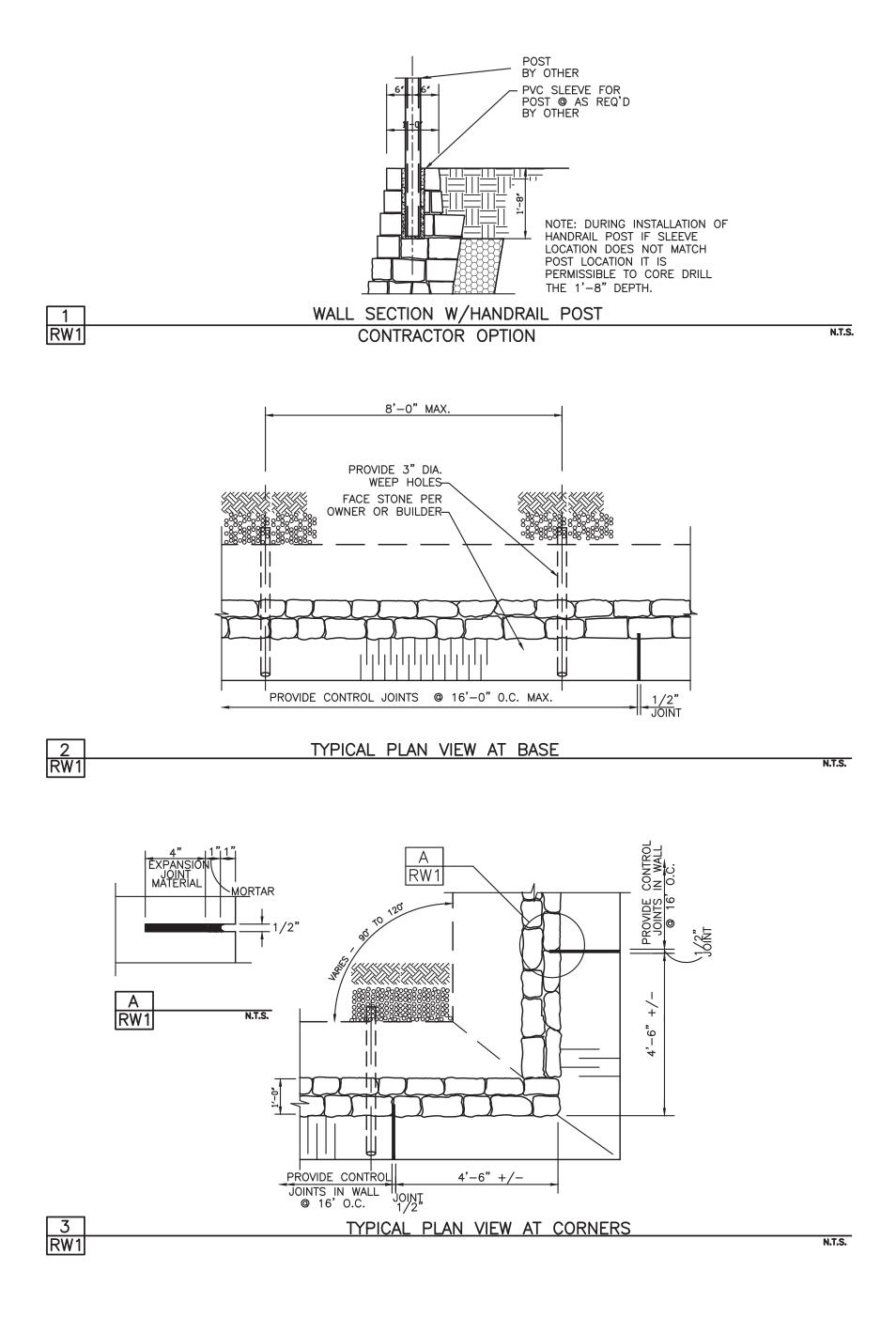


20 0				AMB		SION BY
(IN FEET) 1 inch = 20	ft.			2 10-11-17 UPDATED GRADING PLANS	07-17-17 UPDATED GRADING PLANS AND WALL DETAILS	NO. DATE REVISION
		DATE BY	DES. 05-19-17 MMR	DRN. 05-19-17 MMR	05-19-17	
				172 North Fielder Road	FALKOFSKE	-
		The use of these plans and	specifications shall be restricted to the original site for which they were prepared. Any reproduction or distribution is exorressly limited to such use.	Any other reproduction, reuse, or disclosure by any method, in whole or in port is prohibited	These drawings and specifications contain proprietary information and title remains in FALKOFSKE ENGINEERING.	
			HARBOR URBAN GENTER SWC HORIZON ROAD AND SUMMER LEE DRIVE BOCKWALL TEYAS	~	WALLCO RETAINING WALLS, INC. 4800 S.E. LOOP 820	WORTH, TE)
PROP. DRING.	RECORD DRAWINGS THIS DRAWING HAS BEEN REVISED TO SHOW THOSE CHANGES DURING THE CONSTRUCTION PROCESS REPORTED BY THE CONTRACTOR TO FALKOFSKE ENGINEERING, INC. AND CONSIDERED TO BE SIGNIFICANT. THIS DRAWING IS NOT GUARANTEED TO BE "AS BUILT" BUT IS BASED ON THE INFORMATION MADE AVAILABLE. DATE: 05/20/2020 BY: Aaron Berkes, P.E.	JC		715 NAL		

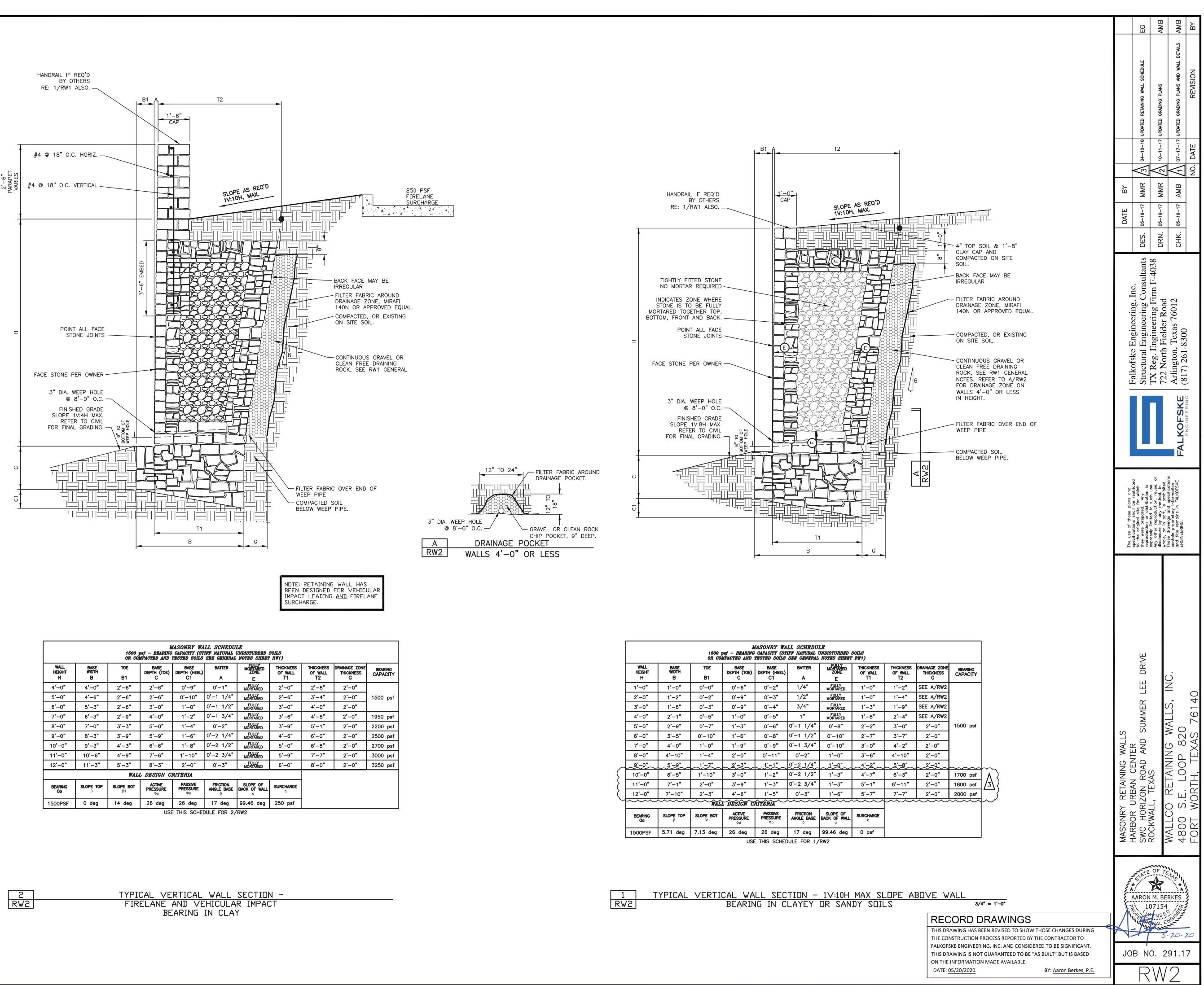




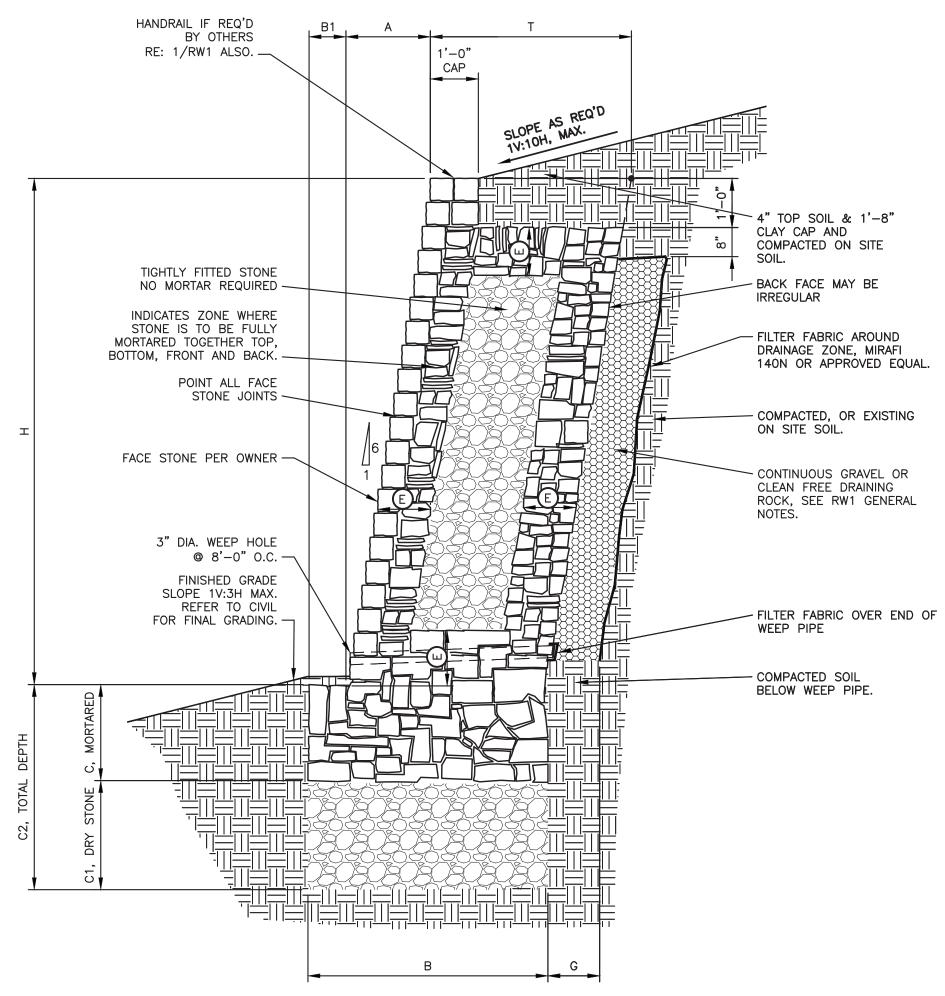
		—	1	В	8	
		\vdash		AMB		BΥ
					LL DETAILS	
					GRADING PLANS AND WALL	REVISION
1. Design Building Code				ig plans	IG PLANS	REVIS
International Building Code, 2009 Edition				d grading		
2. <u>Geotechnical Report</u>				UPDATED	UPDATED	
Firm: ECS Texas, LLP. Report No. 19:6898 Dated: Aug Allowable Bearing Capacity 1500psf, 2500psf	ust 12, 2016			10-11-17	07-17-17	DATE
Note: All of the above noted bearing capacities are anticipated thr				\sum_{i}		- v
design for multiple bearing capacity options. It will be field we the conditions of the soil at the base of the wall during exca the length of the retaining wall it is permitted to change bear	vation. If the bearing capacity changes along	BY	MMR	MMR	AMB	
3. <u>Geotechnical Criteria</u>		μ			-19-17	
Bearing on Stiff Natural Undisturbed Clayey Soils or Co	mpacted and Tested Soils	DATE	05-19-17	. 05-19-17	02	
Allowable Bearing = 1500 psf, min. Friction Angle between Base of Wall and Soil - 17 deg			DES.	DRN.	CHK.	
Backfill Soil Parameters: Backfill Soil - On Site Soils Backfill On Site Soil Angle of Internal Friction PHI = 26 deg			n	038		
Base Soil Parameters:			nc. onsulta	n F-40		
Soil at Toe - Natural, Undisturbed or Fill Soils Angle of Internal Friction PHI = 26 deg			C,F	g Fim oad	0au 012	
Bearing on Remediated Base Allowable Bearing = 2500 psf, min.			Engineering	erin r R	us 76	_
Friction Angle Between Base of Wall and Soil - 28 deg Backfill Soil Parameters:				цü	ΞĔά	2222
Backfill Soil - On Site Soils Backfill Angle of Internal Friction PHI = 26 deg		-	ctural	Reg. E.	- • I	107
Base Soil Parameters: Soil at Toe - Natural, Undisturbed Soils Angle of Internal Friction PHI = 26 deg		;	Falkotske Structural		Arlington	(110)
The use of heavy equipment within 3'-0" of the wall could da	amage the wall and should be avoided.				К	RING
Locate base of walls on undisturbed or properly compacted	-				DFS	GINEE
4. <u>Materials:</u>					LK(Z Ш
Rock for Wall Construction: Average density of masonry stone wall varies from 135 pcf to 145 pcf.	Size of stone within wall varies from 4" to 18". Crushed					
concrete is acceptable to be used in the wall construction in place of na contractor and owner/developer.	atural stone. Face stone to be coordinated between		pe	r or	su Ju	
Drainage Zone Materials: Drainage zone materials may be composed of clean gravel or stone ra	nging from 1" to 5". Crushed concrete is acceptable	ns and	e restricte or which Any oution is	uch use. n, reuse, thod, in	provinced specificati formation FALKOFSK	
provided it is clean and generally free of dust or other deleterious mate Filter fabric shall be Mirafi 140N or approved equal.		hese pla	shall be al site fo epared. or distrit	nted to s productio any me	purt, is gs and s ietary inf iains in f	
Portland Cement Mortar for Retaining Wall Construction The portland cement mortar used for construction of the ma		use of t	ifications he origin were pr oduction	essly lim other re losure by	e, or m te drawin ain prop title rem INEERING.	
with the following proportions per cubic yard of concrete. T "batch tickets" clearly indicating that the appropriate amoun mixer truck load. The batch tickets shall clearly indicate the	t of materials are provided in each concrete	The	to 1 they repr	exp disc		
and shall be provided to Falkofske Engineering, Inc. for revi Contents Amount per cubic yard						
Type 1 Portland cement: 376 lbs						
Type F Fly Ash94lbsFine Aggregate (sand):3250lbsPotable Water235lbs		DETAILS				
Admixture Eucon 100 48 oz average Concrete retarders such as "Eucon 100 Retarder" may be u	used at the discretion of the masonry wall					
contractor. A greater amount of retarder (about 64 ounces) amount of retarder (about 32 ounces) is typically used durin		STANDARD				
Please note that the above proportions will provide a portlar of about f'c = 2500 psi. Falkofske Engineering, Inc. does no above proportions are verified by way of the "batch tickets".	ot require any concrete testing provided the	STAN	DRIVE			
5. <u>Construction Reviews</u>		න ග	L L L L		N N	
Falkofske Engineering, Inc. shall be called for construction r	review of masonry wall.	NOTES			Ś	140
6. <u>Retaining Wall Design Constraints</u>			SUMMER		MALL 0	76
Retaining walls should not have solid fence placed on top or Retaining walls shall not have additional surcharge placed a Retaining walls shall not have slope at base or top of wall the	bove wall other than that shown on these plans.	WALLS	AND		0 20 20 20 00 00 00 00 00 00 00 00 00 00	, AS
The retaining walls noted above require special design. Minor variations in the construction of the retaining walls fro	m these documents may be accepted at the				AININ(00P	Ц Ц
discretion of the design engineer.		Z Z	N RO	IEXAS	ETAI I O	Ľ.
		RET	URBAN IZON F	Ц — Г		/0R
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		MASC	HARBUR SWC HOF		WAL 480	FOR
			STATE	OF 7	Etach	
			AARON		FRKFS	1
				0715 18	4	111.
	RECORD DRAWINGS THIS DRAWING HAS BEEN REVISED TO SHOW THOSE CHANGES DURING	\mathbf{A}	-	NAL	ENG	- 20
	THE CONSTRUCTION PROCESS REPORTED BY THE CONTRACTOR TO FALKOFSKE ENGINEERING, INC. AND CONSIDERED TO BE SIGNIFICANT. THIS DRAWING IS NOT GUARANTEED TO BE "AS BUILT" BUT IS BASED		B N		3-20- 291.1	
	ON THE INFORMATION MADE AVAILABLE. DATE: <u>05/20/2020</u> BY: <u>Aaron Berkes, P.E.</u>			Λ	/ 1	
			Г	VV	/	



MASONRY WALL SCHEDULE 1500 psf - BEARING CAPACITY (STIFF NATURAL UNDISTURBED SOILS OR COMPACTED AND TESTED SOILS SEE GENERAL NOTES SHEET RW1)										
WALL HEIGHT H	BASE WIDTH B	toe B1	BASE DEPTH (TOE) C	BASE DEPTH (HEEL) C1	batter A	FULLY MORTARED ZONE E	THICKNESS OF WALL T1	THICKNESS OF WALL T2	DRAINAGE ZONE THICKNESS G	BEARING CAPACITY
4'-0"	4'-0"	2'-6"	2'-6"	0'-9"	0'-1"	FULLY MORTARED	2'-0"	2'-8"	2'-0"	
5'-0"	4'-6"	2'-6"	2'-6"	0'-10"	0'-1 1/4"	FULLY MORTARED	2'-6"	3'-4"	2'-0"	1500 psf
6'-0"	5'-3"	2'-6"	3'-0"	1'-0"	0'-1 1/2"	FULLY MORTARED	3'-0"	4'-0"	2'-0"	
7'-0"	6'-3"	2'-9"	4'-0"	1'-2"	0'-1 3/4"	FULLY MORTARED	3'-6"	4'-8"	2'-0"	1950 psf
8'-0"	7'-0"	3'-3"	5'-0"	1'-4"	0'-2"	FULLY MORTARED	3'-9"	5'-1"	2'-0"	2200 psf
9'-0"	8'-3"	3'-9"	5'-9"	1'-6"	0'-2 1/4"	FULLY MORTARED	4'-6"	6'-0"	2'-0"	2500 psf
10'-0"	9'-3"	4'-3"	6'-6"	1'-8"	0'-2 1/2"	FULLY MORTARED	5'-0"	6'-8"	2'-0"	2700 psf
11'-0"	10'-6"	4'-9"	7'-6"	1'-10"	0'-2 3/4"	FULLY MORTARED	5'-9"	7'-7"	2'-0"	3000 psf
12'-0"	11'-3"	5'-3"	8'-3"	2'-0"	0'-3"	FULLY MORTARED	6'-0"	8'-0"	2'-0"	3250 psf
	WALL DESIGN CRITERIA									
BEARING Qa	SLOPE TOP β	SLOPE BOT β1	ACTIVE PRESSURE Φ_a	PASSIVE PRESSURE Φp	$\begin{array}{c} \textbf{FRICTION}\\ \textbf{ANGLE} \textbf{BASE}\\ \delta \end{array}$	SLOPE OF BACK OF WALL	SURCHARGE q			
1500PSF	0 deg	14 deg	26 deg	26 deg	17 deg	99.46 deg	250 psf	1		



MAS 1500 psf - BEARING CA OR COMPACTED AND TES							
	WALL HEIGHT H	BASE WIDTH B	toe B1	BASE DEPTH (TOE) C	BASE DEPTH (HEEL) C1		
	1'-0"	1'-0"	0'-0"	0'-6"	0'-2"		
	2'-0"	1'-2"	0'-2"	0'-9"	0'-3"		
	3'-0"	1'-6"	0'-3"	0'-9"	0'-4"		
	4'-0"	2'-1"	0'-5"	1'-0"	0'-5"		
	5'-0"	2'-9"	0'-7"	1'-3"	0'-6"		
	6'-0"	3'-5"	0'-10"	1'-6"	0'-8"		
	7'-0"	4'-0"	1'-0"	1'—9"	0'-9"		
	8'-0"	4'-10"	1'-4"	2'-0"	0'-11"		
\sim	9'-0"	5'-9"	1'-7"	2'-3"	1'-1"		
(10'-0"	6'-5"	1'-10"	3'-0"	1'-2"		
8	11'-0"	7'-1"	2'-0"	3'-9"	1'-3"		
(12'-0"	7'-10"	2'–3"	4'-6"	1'-5"		
\sim			WAL	L DESIGN C	RITERIA		
	BEARING Qa	SLOPE TOP β	SLOPE BOT β1	ACTIVE PRESSURE Фа	PASSIVE PRESSURE Φp		
	1500PSF	5.71 deg	7.13 deg	26 deg	26 deg		
				LICE			



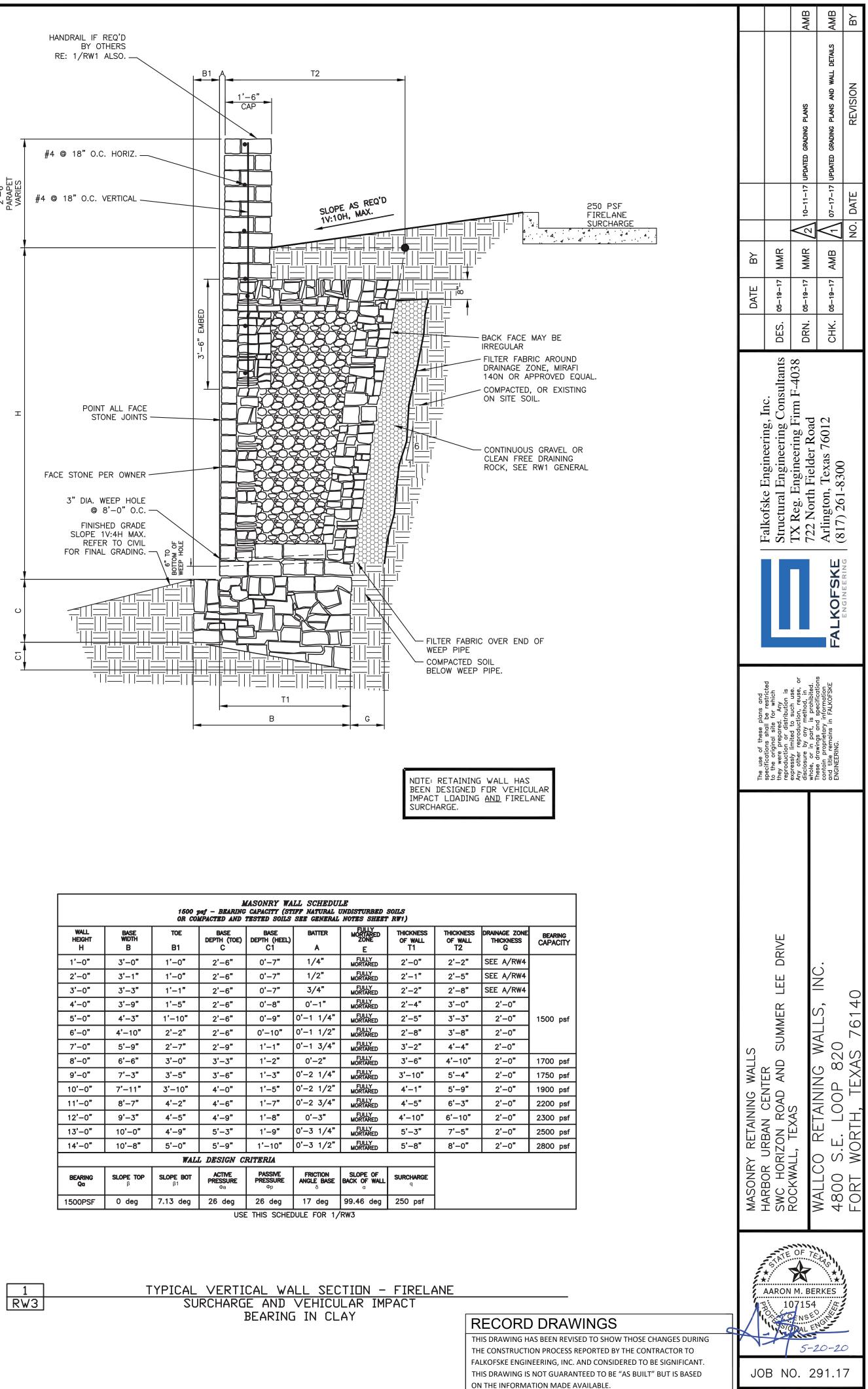
			psf – BEARING		IFF NATURAL	LE UNDISTURBED . NOTES SHEET			
WALL HEIGHT H	BASE WIDTH B	TOE B1	MORTARED DEPTH C	DRY STONE DEPTH C1	TOTAL DEPTH C2	BATTER	FULLY MORTARED ZONE E	THICKNESS OF WALL T	DRAINAC
8'-0"	4'-11"	0'-10"	8'-0"	4'-0"	6'-0"	1'-4"	1'-0"	4'-1"	1'-
9'-0"	6'-0"	1'-1"	8'-0"	4'-0"	7'–0"	1'-6"	1'-0"	4'-11"	1'-
10'-0"	6'-7"	1'-1"	8'-0"	4'-0"	8'-0"	1'-8"	1'-2"	5'-6"	1'
11'-0"	7'-3"	1'-2"	8'-0"	4'-0"	8'-6"	1'-10"	1'-2"	6'-1"	1'.
			WALL DESIG	N CRITERIA					
BEARING Qa	SLOPE TOP β	SLOPE BOT	ACTIVE PRESSURE Φ_a	PASSIVE PRESSURE Φp	FRICTION ANGLE BASE õ	SLOPE OF BACK OF WALL	SURCHARGE q		
2500PSF	5.71 deg	18.43 deg	26 deg	26 deg	28 deg	99.46 deg	0 psf	1	

USE THIS SCHEDULE FOR 2/RW3

TYPICAL WALL SECTION - 1V:10H MAX SLOPE ABOVE WALL 1V:3H MAX SLOPE BELOW WALL WITH REMEDIATED BASE BEARING IN CLAYEY SOILS

2 RW3

1500 psf - BEARING CAF OR COMPACTED AND TEST							
WALL HEIGHT	BASE WIDTH	TOE	BASE	DEP			
Н	B	B1	depth (toe) C	DEP			
1'-0"	3'-0"	1'-0"	2'-6"				
2'-0"	3'-1"	1'-0"	2'-6"				
3'-0"	3'-3"	1'-1"	2'-6"				
4'-0"	3'-9"	1'-5"	2'-6"				
5'-0"	4'-3"	1'-10"	2'-6"				
6'-0"	4'-10"	2'-2"	2'-6"	C			
7'-0"	5'-9"	2'-7"	2'-9"				
8'-0"	6'-6"	3'-0"	3'-3"				
9'-0"	7'-3"	3'-5"	3'-6"				
10'-0"	7'-11"	3'-10"	4'-0"				
11'-0"	8'-7"	4'-2"	4'-6"				
12'-0"	9'-3"	4'-5"	4'-9"				
13'-0"	10'-0"	4'-9"	5'-3"				
14'-0"	10'-8"	5'-0"	5'-9"	1			
		WAL	L DESIGN C	RITZ			
BEARING Qa	SLOPE TOP β	SLOPE BOT β1	ACTIVE PRESSURE Фа	F Pf			
4500005	0 dea	717 dag					



DATE: <u>05/20/2020</u>

RW3

BY: Aaron Berkes, P.E.

- 4" TOP SOIL & 1'-8" CLAY CAP AND COMPACTED ON SITE SOIL.

- BACK FACE MAY BE IRREGULAR

- FILTER FABRIC AROUND DRAINAGE ZONE, MIRAFI 140N OR APPROVED EQUAL.

- COMPACTED, OR EXISTING ON SITE SOIL.

- CONTINUOUS GRAVEL OR CLEAN FREE DRAINING ROCK, SEE RW1 GENERAL

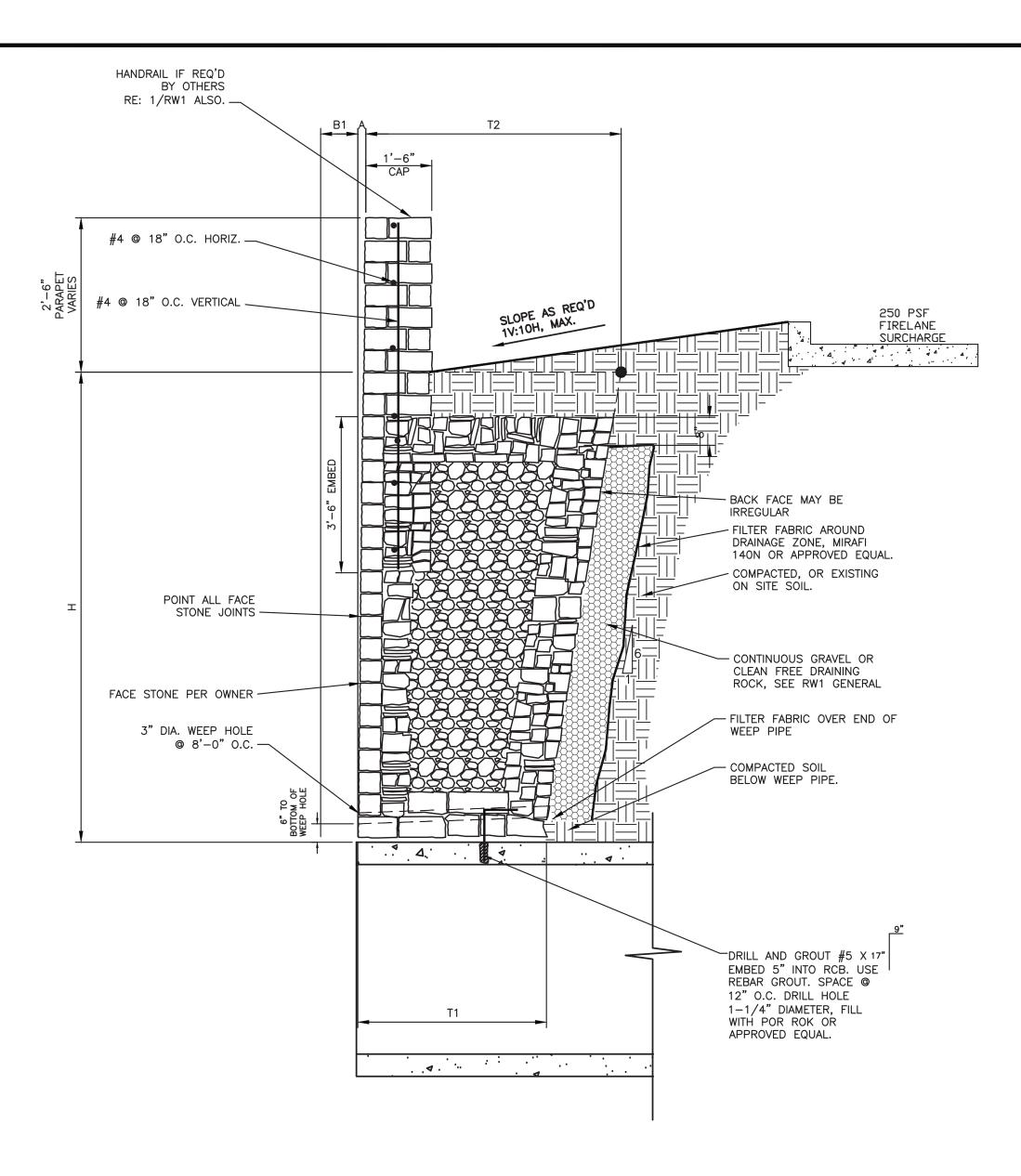
- COMPACTED SOIL

BELOW WEEP PIPE.

INAGE ZONE THICKNESS G BEARING CAPACITY
 1'-0"
 2500 psf

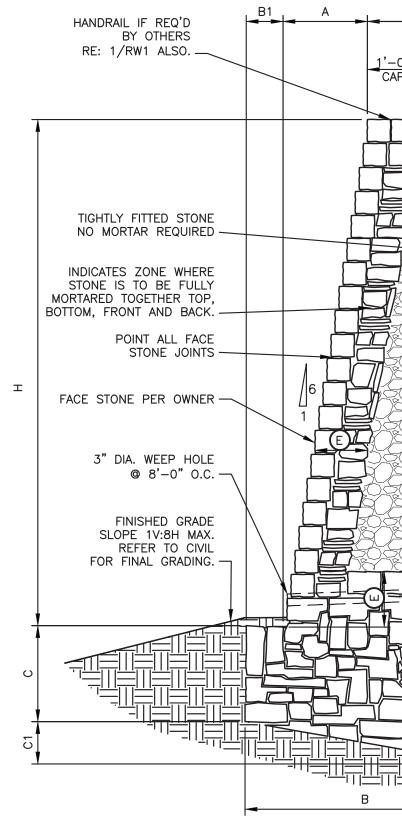
 1'-0"
 2750 psf

1/2" = 1'-0"



MASONRY WALL SCHEDULE 1500 psf – BEARING CAPACITY (BEARING ON CONCRETE SEE GENERAL NOTES SHEET RW1)								
WALL HEIGHT H	BATTER A	FULLY MORTARED ZONE E	THICKNESS OF WALL T1	THICKNESS OF WALL T2	DRAINAGE ZONE THICKNESS G	BEARING CAPACITY		
6'-0"	0'-1 1/2"	FULLY MORTARED	2'-8"	3'-8"	2'-0"	1500	psf	
7'-0"	0'-1 3/4"	FULLY MORTARED	3'-2"	4'-4"	2'-0"			
8'-0"	0'-2"	FULLY MORTARED	3'-6"	4' -10"	2'-0"	1700	psf	
		WAL	L DESIGN C	R <i>ITERIA</i>				
BEARING Qa	SLOPE TOP β	SLOPE BOT β1	ACTIVE PRESSURE Φ_a	PASSIVE PRESSURE Φp	FRICTION ANGLE BASE ō	SLOPE OF BACK OF WALL a	SURCHARGE	
1500PSF	0 deg	7.13 deg	26 deg	26 deg	17 deg	99.46 deg	250 psf	
USE THIS SCHEDULE FOR 2/RW4								

2 VERTICAL FACE WALL SECTION WITH IMPACT PARAPET AND RW4 FIRELANE - ON TOP OF CULVERT 1V:10H MAX SLOPE ABOVE WALL



		AMB BY BY
		wall details
HANDRAIL IF REQ'D BY OTHERS B1 A T		: PLANS PLANS AND W REVISION
RE: 1/RW1 ALSO.		GRADING PI GRADING PI R[
		UPDATED G UPDATED G
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		-11-17 -17-17 ATE
TIGHTLY FITTED STONE	<	2 10- 10. D/
	BY MMR	AMB
INDICATES ZONE WHERE STORE IS TO BE FULLY WORTARED TOGETHER TOP, OTTOM, FRONT AND BACK	DATE	05-19-17 05-19-17
POINT ALL FACE STONE JOINTS COMPACTED, OR EXISTING		
FACE STONE PER OWNER		
3" DIA WEED HOLE	ultants -4038	
3" DIA. WEEP HOLE @ 8'-0" O.C. WALLS 4'-0" OR LESS IN HEIGHT.	, Inc. Consu	p 7
FINISHED GRADE SLOPE 1V:8H MAX. REFER TO CIVIL	eering sering]	r Road 76012
REFER TO CIVIL FOR FINAL GRADING	e Engineering, Inc. I Engineering Const Engineering Firm F	Fielder Texas 8300
	kofske I uctural I Reg. Ej	722 North Fielder Road Arlington, Texas 76012 (817) 261-8300
	Falkofske Structural TX Reg. F	722 North Arlington, (817) 261-
B G		
12" TO 24" FILTER FABRIC AROUND DRAINAGE POCKET.		LE .
	ind tricted hich use. or	i in ibited. fications oFSKE
3" DIA. WEEP HOLE © 8'-0" O.C GRAVEL OR CLEAN ROCK	se plans c hall be res site for wi distributio	ny method rt, is proh and speci any inform is in FALK is
CHIP POCKET, 9" DEEP.	se of the ications sl a original were preparative duction or ther rentor	sure by a or in pa drawings in propriet itle remair EERING.
RW4 WALLS 4'-0" OR LESS	The they they are they they are the the the the the the the the the th	disclost whole, These contain and titl ENGINEI
MASONRY WALL SCHEDULE 1500 psf – BEARING CAPACITY (STIFF NATURAL UNDISTURBED SOILS SEE GENERAL NOTES SHEET RW1)		
WALL HEIGHTBASE WIDTH HTOEBASE DEPTH (TOE)BASE DEPTH (TOE)BASE DEPTH (HEEL)BATTERFULLY MORTARED ZONETHICKNESS OF WALL TDRAINAGE ZONE THICKNESS GBEARING CAPACITY	DRIVE	
1'-0" 1'-0" 0'-0" 0'-6" 0'-2" 0'-2" FULLY MORTARED 1'-0" SEE A/RW4 2'-0" 1'-2" 0'-2" 0'-3" 0'-4" FULLY MORTARED 1'-0" SEE A/RW4	LEE	INC.
3'-0" 1'-7" 0'-3" 0'-9" 0'-4" 0'-6" FULLY MORTARED 1'-4" SEE A/RW4 4'-0" 2'-3" 0'-4" 1'-0" 0'-5" 0'-8" FULLY MORTARED 1'-11" SEE A/RW4		4 0
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	SUMMER	ALL 76
WALL DESIGN CRITERIA	WALLS TER AND S	\sim
BEARING QoSLOPE β TOPSLOPE β TOPBOT β_1 ACTIVE PRESSURE ϕ_a PASSIVE PRESSURE ϕ_p FRICTION ANGLE β_b SLOPE OF BACK OF WALL α SURCHARGE q1500PSF5.71 deg7.13 deg26 deg26 deg17 deg99.46 deg0 psf		TAINING LOOP 82 H, TEXA
USE THIS SCHEDULE FOR 1/RW4	IRY RETAINING R URBAN CEN HORIZON ROAD	O RETAI S.E. LO WORTH,
	RY RE Orizo	S.E Nol
	MASONF HARBOF SWC HO ROCKW	WALL(4800 FORT
	× H SN SN	≥ 4 ⊓
	ATEO	FTET
TYPICAL WALL SECTION - 1V:10H MAX SLOPE ABOVE WALL 1V:8H MAX SLOPE BELOW WALL 1/2" = 1'-0"	AARON M	BERKES
BEARING IN CLAYEY OR SANDY SOILS	107	154 June P
RECORD DRAWINGS THIS DRAWING HAS BEEN REVISED TO SHOW THE CONSTRUCTION PROCESS REPORTED BY	V THOSE CHANGES DURING	AL ENG.
FALKOFSKE ENGINEERING, INC. AND CONSID THIS DRAWING IS NOT GUARANTEED TO BE	ERED TO BE SIGNIFICANT.	291.17
ON THE INFORMATION MADE AVAILABLE. DATE: <u>05/20/2020</u>	BY: <u>Aaron Berkes, P.E.</u>	$\mathbb{N}4$

1	TYPICAL WALL SECTION	ON
RW4	1V:8H	MA
	BEARING	IN

