ENGINEERING PLANS

HARBOR HILL RESIDENCES

City of Rockwall, Texas

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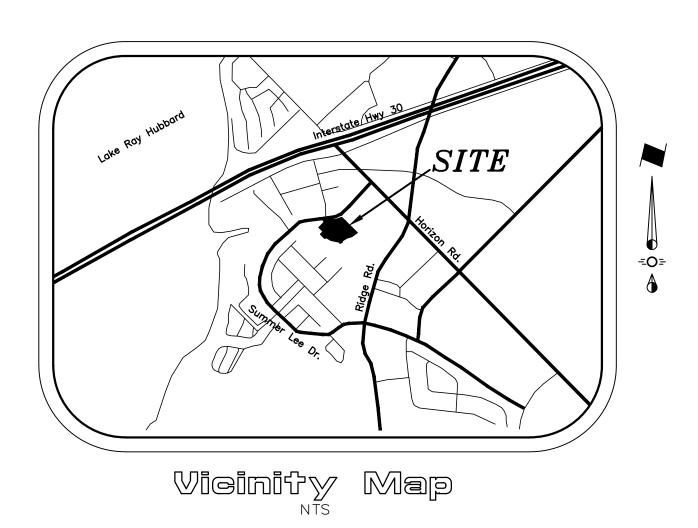
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Prepared For

DBA Architects

111 S. Kentucky Street, Suite 210 McKinney, Texas 75069

Engineer



1720 W. Virginia Street 972.562.4409 McKinney, Texas 75069 Texas P.E. Firm No. F-5935 This Record Drawing is a combination of the sealed engineering contract drawings for this project, modified by revision, change order, field order and information furnished by the contractor. The information shown on the Record Drawings is believed to be accurate based on information furnished by the contractor. The original sealed drawings are on file at the office of:

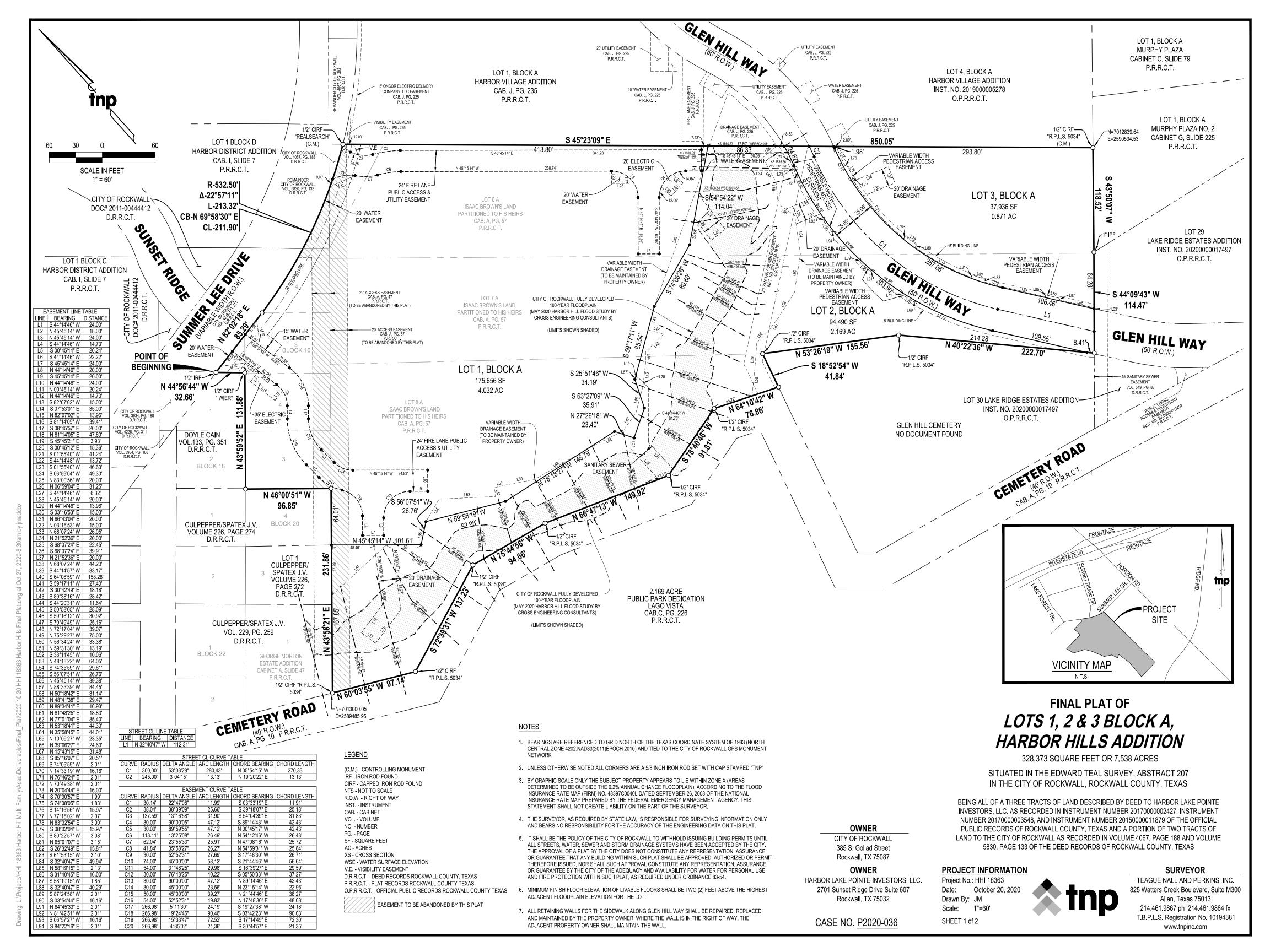
Cross Engineering Consultants
1720 W. Virginia Street
McKinney, Texas 75069

(972) 562—4409
Record Drawings Prepared On:

12/22/2023

The seal that originally appeared on this document was authorized by T. Zachary Grimes, Texas No. 122013 on 5/17/2021. Alteration of a sealed document without proper notification of the responsible Engineer is an offense under the Texas Engineering Practice Act.

ISSUE DATES:	REVISIONS:	DATE	BY
9/17/2020	Issued for Permit Addendum 2		
11/30/2020	Issued for Permit Addendum 3		
12/23/2020	Issued for Permit Addendum 4		
1/22/2021	Issued for Pricing II		
1/29/2021	Issued for Permit Addendum 5		
4/19/2021	Issued for Construction		
5/17/2021	Revision 1		
12/22/2023	Record Drawings		



OWNERS CERTIFICATION

STATE OF TEXAS}
COUNTY OF ROCKWALL}

WHEREAS Harbor Lake Pointe Investors, LLC., and the City of Rockwall are the owners a tract of land situated in the Edward Teal Survey, Abstract 207 and being all of three tracts of land described by deed to Harbor Lake Pointe Investors, LLC. as recorded in Instrument Number 20170000002427, Instrument Number 20170000003548 and Instrument Number 20150000011879 of the Official Public Records of Rockwall County, Texas, and and a portion of two tracts of land to the City of Rockwall as recorded in Volume 4067, Page 188 and Volume 5830, Page 133 of the Deed Records of Rockwall County, Texas, also being all of Lot 3 and 4, Block 18 and a portion of Lot 2 and 4 and all of Lot 3, Block 16 of George Morton Estate as recorded in Cabinet A, slide 47 of the Plat Records of Rockwall County, Texas, also being a portion of Lot 6A, Lot 7A and Lot 8A of Isaac Brown's Land Partitioned to his Heirs as recorded in Cabinet A, Page 57 of the Plat Records of Rockwall County, Texas, and being more particularly described as follows:

BEGINNING at a 1/2 inch iron rod found the northwest corner of said Harbor Heights tract recorded in Instrument Number 2017000002427 also lying on the southerly line of Summer Lee Drive, a variable width right-of-way;

THENCE North 82 degrees 02 minutes 18 seconds East, along the southerly line of said Summer Lee Drive, a distance of 85.29 feet to a 5/8 inch iron rod with cap stamped "TNP" set for the beginning of a curve to the left;

THENCE along the southerly line of said Summer Lee Drive with said curve to the left having a radius of 532.50 feet, a central angle of 22 degrees 57 minutes 11 seconds, an arc length of 213.32 feet, a chord bearing of North 69 degrees 58 minutes 30 seconds East, a distance of 211.90 feet to a 5/8 inch iron rod with cap stamped "REALSEARCH" found for the west corner of Lot 1, Block A, Harbor Village Addition, an addition to the City of Rockwall as recorded in Cabinet J, Page 225 of the Plat Records of Rockwall County,

THENCE South 45 degrees 23 minutes 09 seconds East, along the southwest line of said Lot 1, Block A, a distance of 850.05 feet to a 1/2 inch iron rod with cap stamped "R.P.L.S. 5034" found for the south corner of same, also lying on the northwest line of Lot 1, Block A, Murphy Plaza No. 2, an addition to the City of Rockwall as recorded in Cabinet G, Slide 225 of the Plat Records of Rockwall County, Texas;

THENCE South 43 degrees 50 minutes 07 seconds West, along the northwest line of said Murphy Plaza, a distance of 118.52 feet to a 1 inch iron pipe found for the west corner of same, also for the north corner of Lot 29, Lake Ridge Estates, an addition to the City of Rockwall as recorded in Instrument Number 20200000017497 of the Official Public Records of Rockwall County, Texas;

THENCE South 44 degrees 09 minutes 43 seconds West, along the northwest line of said Lake Ridge Estates, a distance of 114.47 feet to a 5/8 inch iron rod with cap stamped "TNP" set for the southeast corner of the aforementioned Harbor Lake Pointe Investors, LLC. tract as recorded in Instrument Number 20150000011879;

THENCE along the southerly line of said Harbor Lake Point Investors tract recorded in Instrument Number 20150000011879 the following courses and distances:

North 40 degrees 22 minutes 36 seconds West, a distance of 222.70 feet to a 1/2 inch iron rod with cap stamped "R.P.L.S. 5034 found

North 53 degrees 26 minutes 19 seconds West, a distance of 155.56 feet to a 1/2 inch iron rod with cap stamped "R.P.L.S. 5034 found

South 18 degrees 52 minutes 54 seconds West, a distance of 41.84 feet to a 5/8 inch iron rod with cap stamped "TNP" set for corner; North 64 degrees 10 minutes 42 seconds West, a distance of 76.86 feet to a 1/2 inch iron rod with cap stamped "R.P.L.S. 5034 found for a second second

South 78 degrees 40 minutes 46 seconds West, a distance of 91.81 feet to a 1/2 inch iron rod with cap stamped "R.P.L.S. 5034 found

North 66 degrees 47 minutes 13 seconds West, a distance of 149.92 feet to a 5/8 inch iron rod with cap stamped "TNP" set for corner;

North 75 degrees 44 minutes 56 seconds West, a distance of 94.66 feet to a 1/2 inch iron rod with cap stamped "R.P.L.S. 5034 found

for corner;
South 72 degrees 39 minutes 31 seconds West, a distance of 137.23 feet to a 1/2 inch iron rod with cap stamped "R.P.L.S. 5034

found for corner; North 60 degrees 03 minutes 55 seconds West, a distance of 97.14 feet to a 1/2 inch iron rod with cap stamped "R.P.L.S. 5034 found for corner lying on the southeast line of the aforementioned George Morton Estate;

THENCE North 43 degrees 58 minutes 21 seconds East along the southeast lineof said George Morton Estate, a distance of 231.86 feet to 5/8 inch iron rod with cap stamped "TNP" set for corner for the south corner of the aforementioned Harbor Lake Point Investors

tract recorded in Instrument Number 20170000003548;

THENCE North 46 degrees 00 minutes 51 seconds West, along the southwest line of last mentioned Harbor Lake Point Investors tract,

THENCE North 43 degrees 59 minutes 52 seconds East, a distance of 131.88 feet to a 1/2 inch iron rod with cap stamped "WIER" found for ell corner of the aforementioned Harbor Lake Point Investors tract recorded in Instrument Number 20170000002427;

a distance of 96.85 feet to 5/8 inch iron rod with cap stamped "TNP" set for corner at the west corner of same;

THENCE North 44 degrees 56 minutes 44 seconds West, along a southwest line of last mentioned Harbor Lake Point Investors tract, a distance of 32.66 feet to the POINT OF BEGINNING containing 328,373 square feet, or 7.538 acres of land.

NOW, THEREFORE, KNOW ALL MEN BY THESE PRESENTS:

STATE OF TEXAS}
COUNTY OF ROCKWALL}

HARBOR HILLS POINTE INVESTORS, LLC.

My Commission Expires:

We the undersigned owner's of the land shown on this plat, and designated herein as <u>HARBOR HILLS ADDITION</u> subdivision to the City of Rockwall, Texas, and whose name is subscribed hereto, hereby dedicate to the use of the public forever all streets, alleys, parks, water courses, drains, easements and public places thereon shown on the purpose and consideration therein expressed. We further certify that all other parties who have a mortgage or lien interest in the <u>[HARBOR HILLS ADDITION]</u> subdivision have been notified and signed this plat. We understand and do hereby reserve the easement strips shown on this plat for the purposes stated and for the mutual use and accommodation of all utilities desiring to use or using same. I also understand the following:

- 1. No buildings shall be constructed or placed upon, over, or across the utility easements as described herein.
- 2. Any public utility shall have the right to remove and keep removed all or part of any buildings, fences, trees, shrubs, or other growths or improvements which in any way endanger or interfere with construction, maintenance or efficiency of their respective system on any of these easement strips; and any public utility shall at all times have the right of ingress or egress to, from and upon the said easement strips for purpose of construction, reconstruction, inspecting, patrolling, maintaining, and either adding to or removing all or part of their respective system without the necessity of, at any time, procuring the permission of anyone.
- 3. The City of Rockwall will not be responsible for any claims of any nature resulting from or occasioned by the establishment of grade of streets in the subdivision.
- 4. The developer and subdivision engineer shall bear total responsibility for storm drain improvements.
- 5. The developer shall be responsible for the necessary facilities to provide drainage patterns and drainage controls such that properties within the drainage area are not adversely affected by storm drainage from the development.
- 6. No house dwelling unit, or other structure shall be constructed on any lot in this addition by the owner or any other person until the developer and/or owner has complied with all requirements of the Subdivision Regulations of the City of Rockwall regarding improvements with respect to the entire block on the street or streets on which property abuts, including the actual installation of streets with the required base and paving, curb and gutter, water and sewer, drainage structures, storm structures, storm sewers, and alleys, all according to the specifications of the City of Rockwall; or
- 7. Property owner shall be responsible for maintaining, repairing, and replacing all systems within the drainage and detention easements.

Until an escrow deposit, sufficient to pay for the cost of such improvements, as determined by the city's engineer and/or city administrator, computed on a private commercial rate basis, has been made with the city secretary, accompanied by an agreement signed by the developer and/or owner, authorizing the city to make such improvements at prevailing private commercial rates, or have the same made by a contractor and pay for the same out of the escrow deposit, should the developer and/or owner fail or refuse to install the required improvements within the time stated in such written agreement, but in no case shall the City be obligated to make such improvements itself. Such deposit may be used by the owner and/or developer as progress payments as the work progresses in making such improvements by making certified requisitions to the city secretary, supported by evidence of work done; or

Until the developer and/or owner files a corporate surety bond with the city secretary in a sum equal to the cost of such improvements for the designated area, guaranteeing the installation thereof within the time stated in the bond, which time shall be fixed by the city council of the City of Rockwall.

We further acknowledge that the dedications and/or exaction's made herein are proportional to the impact of the Subdivision upon the public services required in order that the development will comport with the present and future growth needs of the City; I (we), my (our) successors and assigns hereby waive any claim, damage, or cause of action that I (we) may have as a result of the dedication of exactions made herein.

epresentative:			
TATE OF TEXAS} OUNTY OF ROCKWALL}			
efore me, the undersigned authority, on this day erson whose name is subscribed to the foregoin and consideration therein stated.			
iven upon my hand and seal of office this	day of	, 2020.	
otary Public in and for the State of Texas			

SURVEYOR'S CERTIFICATE

NOW, THEREFORE KNOW ALL MEN BY THESE PRESENTS:

THAT I, <u>Brian J. Maddox</u>, do hereby certify that I prepared this plat from an actual and accurate survey of the land, and that the corner monuments shown thereon were properly placed under my personal supervision.

GIVEN UNDER MY HAND AND SEAL OF OFFICE THIS THE ____ DAY OF ______, 20

BRIAN J. MADDOX, R.P.L.S. NO. 5430

RECOMMENDED FOR FINAL APPRO	<u>VAL</u>		
Planning and Zoning Commission		Date	
APPROVED I hereby certify that the above and foregoing pla	at of <u>HARBOR HILLS ADDITION</u>	I, an addition to the City of Rockwall, Texas	s, was
approved by the City Council of the City of Rock	kwall on the day of	, 2020.	
This approval shall be invalid unless the approv County, Texas, within one hundred eighty (180)	•	•	ckwall
WITNESS OUR HANDS, this day of	, 2020.		
Mayor, City of Rockwall	City Secretary	City Engineer	

FINAL PLAT OF LOTS 1, 2 & 3 BLOCK A, HARBOR HILLS ADDITION

328,373 SQUARE FEET OR 7.538 ACRES

SITUATED IN THE EDWARD TEAL SURVEY, ABSTRACT 207 IN THE CITY OF ROCKWALL, ROCKWALL COUNTY, TEXAS

BEING ALL OF A THREE TRACTS OF LAND DESCRIBED BY DEED TO HARBOR LAKE POINTE INVESTORS, LLC. AS RECORDED IN INSTRUMENT NUMBER 20170000002427, INSTRUMENT NUMBER 20170000003548, AND INSTRUMENT NUMBER 20150000011879 OF THE OFFICIAL PUBLIC RECORDS OF ROCKWALL COUNTY, TEXAS AND A PORTION OF TWO TRACTS OF LAND TO THE CITY OF ROCKWALL AS RECORDED IN VOLUME 4067, PAGE 188 AND VOLUME 5830, PAGE 133 OF THE DEED RECORDS OF ROCKWALL COUNTY, TEXAS

PROJECT INFORMATION

Project No.: HHI 18363

Date: October 20, 2020

Drawn By: JM

Scale: 1"=60'

SHEET 2 of 2



SURVEYOR

TEAGUE NALL AND PERKINS, INC.
825 Watters Creek Boulevard, Suite M300
Allen, Texas 75013
214.461.9867 ph 214.461.9864 fx
T.B.P.L.S. Registration No. 10194381
www.tnpinc.com

CASE NO. P2020-036

OWNER

CITY OF ROCKWALL

385 S. Goliad Street Rockwall, TX 75087 **OWNER**

HARBOR LAKE POINTE INVESTORS, LLC.

2701 Sunset Ridge Drive Suite 607

Rockwall, TX 75032

GENERAL ITEMS

- 1. All construction shall conform to the requirements set forth in the City of Rockwall's Engineering Department's "Standards of Design and Construction" and the "Standard Specifications for Public Works Construction" by the North Texas Central Council of Governments, 5th edition amended by the City of Rockwall. The CONTRACTOR shall reference the latest City of Rockwall standard details provided in the Rockwall Engineering Departments "Standards of Design and Construction" manual for details not provided in these plans. The CONTRACTOR shall possess one set of the NCTCOG Standard Specifications and Details and the City of Rockwall's "Standards of Design and Construction" manual on the project site at all times
- Where any conflicting notes, details or specifications occur in the plans the City of Rockwall General Construction Notes, Standards, Details and Specifications shall govern unless detail or specification is more strict.
- 3. The City of Rockwall Engineering Departments "Standards of Design and Construction" can be found online at: http://www.rockwall.com/engr.asp
- 4. All communication between the City and the CONTRACTOR shall be through the Engineering Construction Inspector and City Engineer or designated representative only. It is the responsibility of the CONTRACTOR to contact the appropriate department for inspections that do not fall under this approved engineering plan set.
- Prior to construction, CONTRACTOR shall have in their possession all necessary permits, plans, licenses, etc.
- 6. The CONTRACTOR shall have at least one original stamped and signed set of approved engineering plans and specifications on-site and in their possession at all times. A stop work order will be issued if items are not on-site. Copies of the approved plans will not be substituted for the required original "approved plans to be on-site".
- 7. All material submittals, concrete batch designs and shop drawings required for City review and approval shall be submitted by the CONTRACTOR to the City sufficiently in advance of scheduled construction to allow no less than 10 business days for review and response by the City.
- 8. All site dimensions are referenced to the face of curb or edge of pavement unless otherwise noted.
- 9. The City requires ten (10%) percent-two (2) year maintenance bond for paving, paving improvements, water systems, wastewater systems, storm sewer systems including detention systems, and associated fixtures and structures which are located within the right-of-ways or defined easements. The two (2) year maintenance bond is to state "from date of City acceptance" as the starting time.
- 10. A review of the site shall be conducted at twenty (20) months into the two (2) year maintenance period. The design engineer or their designated representative and the CONTRACTOR shall be present to walk the site with the City of Rockwall Engineering Inspection personnel.

EROSION CONTROL & VEGETATION

- 1. The CONTRACTOR or developer shall be responsible, as the entity exercising operational control, for all permitting as required by the Environmental Protection Agency (EPA) and the Texas Commission on Environmental Quality (TCEQ). This includes, but is not limited to, preparation of the Storm Water Pollution Prevention Plan (SWPPP), the Construction Site Notice (CSN), the Notice of Intent (NOI), the Notice of Termination (NOT) and any Notice of Change (NOC) and is required to pay all associated fees
- 2. Erosion control devices as shown on the erosion control plan for the project shall be installed prior to the start of land disturbing activities.
- 3. All erosion control devices are to be installed in accordance with the approved plans, specifications and Storm Water Pollution Prevention Plan (SWPPP) for the project. Erosion control devices shall be placed and in working order prior to start of construction. Changes are to be reviewed and approved by the design engineer and the City of Rockwall prior to implementation.
- 4. If the Erosion Control Plans and Storm Water Pollution Prevention Plan (SWPPP) as approved cannot appropriately control erosion and off-site sedimentation from the project, the erosion control plan and/or the SWPPP is required to be revised and any changes reported to the Texas Commission on Environmental Quality (TCEQ), when applicable.
- 5. All erosion control devices shall be inspected weekly by the CONTRACTOR and after all major rain events, or more frequently as dictated in the project Storm Water Pollution Prevention Plan (SWPPP). CONTRACTOR shall provide copies of inspection's reports to the engineering inspection after each inspection.
- The CONTRACTOR shall not dispose of waste and any materials into streams, waterways or floodplains. The CONTRACTOR shall secure all excavation at the end of each day and dispose of all excess materials.
- CONTRACTOR shall take all available precautions to control dust. CONTRACTOR shall control dust by sprinkling water or other means as approved by the City Engineer.
- 8. CONTRACTOR shall establish grass and maintain the seeded area, including watering, until a "Permanent Stand of Grass" is obtained at which time the project will be accepted by the City. A "Stand of Grass" (not winter rye or weeds) shall consist of 75% to 80% coverage of all disturbed areas and a minimum of one-inch (1") in height as determined by the City. No bare spots will be allowed. Re-seeding will be required in all washed areas and areas that don't grow.
- All City right-of-ways shall be sodded if disturbed. No artificial grass is allowed in any City right-of-way and/or easements.
- 10. All adjacent streets/alleys shall be kept clean at all times
- 11. CONTRACTOR shall keep construction site clean at all times, immediately contain all debris and trash, all debris and trash shall be removed at the end of each work day, and all vegetation on the construction site 10-inches or taller in height must be cut immediately.
- 12. Suspension of all construction activities for the project will be enforced by the City if any erosion control requirements are not meet. Work may commence after deficiency has been rectified.
- 13. During construction of the project, all soil stockpiles and borrow areas shall be stabilized or protected with sediment trapping measures. The CONTRACTOR is responsible for the temporary protection and permanent stabilization of all soil stockpiles on-site as well as borrow areas and soil intentionally transported from the project site.
- 14. Where construction vehicles access routes intersect paved or public roads/alleys, construction entrances shall be installed to minimize the transport of sediment by vehicular tracking onto paved surfaces. Where sediment is transferred onto paved or public surfaces, the surface shall be immediately cleaned. Sediment shall be

- removed from the surface by shoveling or sweeping and transported to a sediment disposal area. Pavement washing shall be allowed only after sediment is removed in this manner.
- 15. All drainage inlets shall be protected from siltation, ineffective or unmaintained protection devices shall be immediately replaced and the inlet and storm system cleaned. Flushing is not an acceptable method of cleaning.
- 16. During all dewatering operations, water shall be pumped into an approved filtering device prior to discharge into a receiving outlet.

TRAFFIC CONTROL

- 1. All new Detouring or Traffic Control Plans are required to be submitted to the City for review and approval a minimum of 21 calendar days prior to planned day of implementation.
- 2. When the normal function of the roadway is suspended through closure of any portion of the right-of-way, temporary construction work zone traffic control devices shall be installed to effectively guide the motoring public through the area. Consideration for road user safety, worker safety, and the efficiency of road user flow is an integral element of every traffic control zone.
- 3. All traffic control plans shall be prepared and submitted to the Engineering Department in accordance with the standards identified in Part VI of the most recent edition of the TMUTCD. Lane closures will not occur on roadways without an approval from the Rockwall Engineering Department and an approved traffic control plan. Traffic control plans shall be required on all roadways as determined by the City Engineer or the designated representative.
- 4. All traffic control plans must be prepared, signed, and sealed by an individual that is licensed as a professional engineer in the State of Texas. All traffic control plans and copies of work zone certification must be submitted for review and approval a minimum of three (3) weeks prior to the anticipated temporary traffic control.
- 5. The CONTRACTOR executing the traffic control plan shall notify all affected property owners two (2) weeks prior to any the closures in writing and verbally.
- 6. Any deviation from an approved traffic control plan must be reviewed by the City Engineer or the designated representative. If an approved traffic control plan is not adhered to, the CONTRACTOR will first receive a verbal warning and be required to correct the problem immediately. If the deviation is not corrected, all construction work will be suspended, the lane closure will be removed, and the roadway opened to traffic.
- 7. All temporary traffic control devices shall be removed as soon as practical when they are no longer needed. When work is suspended for short periods of time at the end of the workday, all temporary traffic control devices that are no longer appropriate shall be removed or covered. The first violation of this provision will result in a verbal warning to the construction foreman. Subsequent violations will result in suspension of all work at the job site for a minimum of 48 hours. All contractors working on City funded projects will be charged one working day for each 24 hour closure.
- 3. Lane closures on any major or minor arterial will not be permitted between the hours of 6:00 am to 9:00 am and 3:30 pm to 7:00 pm. Where lane closures are needed in a school area, they will not be permitted during peak hours of 7:00 am 9:00 am and 3:00 pm to 5:00 pm. Closures may be adjusted according to the actual start-finish times of the actual school with approval by the City Engineer. The first violation of this provision will result in a verbal warning to the construction foreman. Subsequent violations will result in suspension of all work at the job site for a minimum of 48 hours. All contractors working on City funded projects will be charged one working day for each 24 hour closure of a roadway whether they are working or not.
- 9. No traffic signs shall be taken down without permission from the City.
- 10. No street/roadway will be allowed to be fully closed.

UTILITY LINE LOCATES

- It is the CONTRACTOR's responsibility to notify utility companies to arrange for utility locates at least 48
 hours prior to beginning construction. The completeness and accuracy of the utility data shown on the plans
 is not guaranteed by the design engineer or the City. The CONTRACTOR is responsible for verifying the
 depth and location of existing underground utilities proper to excavating, trenching, or drilling and shall be
 required to take any precautionary measures to protect all lines shown and .or any other underground utilities
 not on record or not shown on the plans.
- 2. The CONTRACTOR shall be responsible for damages to utilities
- 3. CONTRACTOR shall adjust all City of Rockwall utilities to the final grades.
- 4. All utilities shall be placed underground.
- 5. CONTRACTOR shall be responsible for the protection of all existing main lines and service lines crossed or exposed by construction operations. Where existing mains or service lines are cut, broken or damaged, the CONTRACTOR shall immediately make repairs to or replace the entire service line with same type of original construction or better. The City of Rockwall can and will intervene to restore service if deemed necessary and charge the CONTRACTOR for labor, equipment, material and loss of water if repairs aren't made in a timely manner by the CONTRACTOR.
- 6. The City of Rockwall (City utilities) is not part of the Dig Tess or Texas one Call 811 line locate system. All City of Rockwall utility line locates are to be scheduled with the City of Rockwall Service Center. 972-771-7730. A 48-hour advance notice is required for all non-emergency line locates.
- 7. Underground utility lines shall be installed in accordance with the following standards in addition to other applicable criteria:
 - a. No more than 500 linear feet of trench may be opened at one time.
 - b. Material used for backfilling trenches shall be properly compacted to 95% standard density in order to minimize erosion, settlement, and promote stabilization that the geotechnical engineer recommends.
 c. Applicable safety regulations shall be complied with.
- 11. This plan details pipes up to 5 feet from the building. Refer to the building plans for building connections. CONTRACTOR shall supply and install pipe adapters as necessary.
- 12. All underground lines shall be installed, inspected, and approved prior to backfilling.
- 13. All concrete encasement shall have a minimum of 28 days compressive strength at 3,000 psi (min. 5.5 sack mix).

WATER LINE NOTES

- 1. The CONTRACTOR shall maintain existing water service at all times during construction.
- Proposed water lines shall be AWWA C900-16 PVC Pipe (blue in color) for all sizes, DR 14 (PC 305) for
 pipeline sizes 12-inch and smaller, and DR 18 (PC 235) for 14-inch and larger water pipelines unless otherwise
 shown on water plan and profiles sheets. Proposed water lines shall be constructed with minimum cover of 4
 feet for 6-inch through 8-inch, 5 feet for 12-inch through 18-inch and 6 feet for 20-inch and larger.
- 3. Proposed water line embedment shall be NCTCOG Class 'B-3' as amended by the City of Rockwall's engineering standards of design and construction manual.
- 4. CONTRACTOR shall coordinate the shutting down of all water lines with the City of Rockwall Engineering Inspector and Water Department. The City shall operate all water valves. Allow 5 business days from the date of notice to allow City personnel time to schedule a shut down. Two additional days are required for the CONTRACTOR to notify residents in writing of the shut down after the impacted area has been identified. Water shut downs impacting businesses during their normal operation hours is not allowed. CONTRACTOR is required to coordinate with the Rockwall Fire Department regarding any fire watch requirements as well as any costs incurred when the loss of fire protection to a structure occurs.
- CONTRACTOR shall furnish and install gaskets on water lines between all dissimilar metals and at valves (both existing and proposed).
- All fire hydrants and valves removed and salvaged shall be returned to the City of Rockwall Municipal Service Center.
- Blue EMS pads shall be installed at every change in direction, valve, curb stop and service tap on the proposed water line and every 250'.
- 8. All water valve hardware and valve extensions, bolts, nuts and washers shall be 316 stainless steel.
- All fire hydrants bolts, nuts and washers that are buried shall be 316 stainless steel.
 Abandoned water lines to remain in place shall be cut and plugged and all void spaces within the abandoned
- I. Abandoned water lines to remain in place shall be cut and plugged and all void spaces within the abandoned line shall be filled with grout, flowable fill or an expandable permanent foam product. Valves to be abandoned in place shall have any extensions and the valve box removed and shall be capped in concrete.
- 11. All fire hydrants will have a minimum of 5 feet of clearance around the appurtenance including but not limited to parking spaces and landscaping.
- 12. All joints are to be megalug joints with thrust blocking.
- 13. Water and sewer mains shall be kept 10 feet apart (parallel) or when crossing 2 feet vertical clearance.
- 14. CONTRACTOR shall maintain a minimum of 4 feet of cover on all water lines.
- 15. All domestic and irrigation services are required to have a testable backflow device with a double check valve installed per the City of Rockwall regulations at the property line and shown on plans.

WASTEWATER LINE NOTES

- 1. The CONTRACTOR shall maintain existing wastewater service at all times during construction.
- 2. Wastewater line for 4-inch through 15-inch shall be Green PVC SDR 35 (ASTM D3034) [less 10 ft cover] and SDR 26 (ASTM D3034) [10 ft or more cover]. For 18-inch and lager wastewater line shall be Green PVC PS 46 (ASTM F679) [less 10 ft cover] and PS 115 (ASTM F679) [10 ft or more cover]. No services will be allowed on a sanitary sewer line deeper than 10 feet.
- Proposed wastewater line embedment shall be NCTCOG Class 'H' as amended by the City of Rockwall's public works standard design and construction manual.
- 4. Green EMS pads shall be installed at every 250', manhole, clean out and service lateral on proposed wastewater lines.
 5. CONTRACTOR shall CCTV all existing wastewater lines that are to be abandoned to ensure that all laterals
- are accounted for and transferred to proposed wastewater lines prior to abandonment.
- 6. All abandoned wastewater and force main lines shall be cut and plugged and all void spaces within the abandoned line shall be filled with grout, flowable fill or an expandable permanent foam product.
- 7. Existing manholes and cleanouts not specifically called to be relocated shall be adjusted to match final grades.
- 8. All wastewater pipes and public services shall be inspected by photographic means (television and DVD) prior to final acceptance and after franchise utilities are installed. The CONTRACTOR shall furnish a DVD to the Engineering Construction Inspector for review. Any sags, open joints, cracked pipes, etc. shall be repaired or removed by the CONTRACTOR at the CONTRACTOR's expense. A television survey will be performed as part of the final testing in the twentieth (20th) month of the maintenance period.
- All manholes (public or private) shall be fitted with inflow prevention. The inflow prevention shall conform
 to the measures called out in standard detail R-5031.
- 10. All new or existing manholes being modified shall have corrosion protection being Raven Liner 405 epoxy coating, ConShield, or approved equal. Consheild must have terracotta color dye mixed in the precast and cast-in-place concrete. Where connections to existing manholes are made the CONTRACTOR shall rehab manhole as necessary and install a 125 mil thick coating of Raven Liner 405 or approved equal.
- 11. If an existing wastewater main or trunk line is called out to be replaced in place a wastewater bypassing pump plan shall be required and submitted to the Engineering Construction Inspector and City Engineer for approval prior to implementation. Bypass pump shall be fitted with an auto dialer and conform to the City's Noise Ordinance. Plan shall be to the City sufficiently in advance of scheduled construction to allow no less than 10 business days for review and response by the City.
- 12. CONTRACTOR shall maintain a minimum of 4 feet of cover on all wastewater lines.



GENERAL CONSTRUCTION NOTES
Sheet 1 of 2
June 2020

CITY OF ROCKWALL ENGINEERING DEPARTMENT

385 S. Goliad Rockwall. Texas 75087 P (972) 771-7746 F (972) 771-7748

DEMOLITION, REMOVAL, DISPOSAL AND EXCAVATION NOTES

- All pavements to be removed and replaced shall be saw cut to full depth along neat squared lines shown in the plans.
- 2. Proposed concrete pavement shall be constructed with longitudinal butt construction joints at all connections to existing concrete pavement.
- 3. All public concrete pavement to be removed and replaced shall be full panel replacement, 1-inch thicker and on top of 6-inch thick compacted flexbase.
- 4. No excess excavated material shall be deposited in low areas or along natural drainage ways without written permission from the affected property owner and the City of Rockwall. No excess excavation shall be deposited in the City Limits without a permit from the City of Rockwall. If the CONTRACTOR places excess materials in these areas without written permission, the CONTRACTOR will be responsible for all damages resulting from such fill and shall remove the material at their own cost.

PAVING AND GRADING

- All detention systems are to be installed and verified for design compliance along with the associated storm sewer and outflow structures, prior to the start of any paving operations (including building foundations). Erosion protection shall be placed at the pond outflow structures, silt fence along the perimeter of the pond along with any of the associated erosion BMPs noted on the erosion control plan, and the sides and bottom of the detention system shall have either sod or anchored seeded curlex installed prior to any concrete placement.
- 2. All paving roadway, driveways, fire lanes, drive-isles, parking, dumpster pads, etc. sections shall have a minimum thickness, strength, reinforcement, joint type, joint spacing and subgrade treatment shall at a minimum conform to the City standards of Design and Construction and table below.

Stuart/Dayson out Tyma	Minimum Thickness	Streng th 28-	Minimum (sacks /		Steel Ro	einforcement
Street/Pavement Type	(inches)	Day (psi)	Machine placed	Hand Placed	Bar #	Spacing (O.C.E.W.)
Arterial	10"	3,600	6.0	6.5	#4 bars	18"
Collector	8"	3,600	6.0	6.5	#4 bars	18"
Residential	6"	3,600	6.0	6.5	#3 bars	24"
Alley	7"-5"-7"	3,600	6.0	6.5	#3 bars	24"
Fire Lane	6"	3,600	6.0	6.5	#3 bars	24"
Driveways	6"	3,600	6.0	6.5	#3 bars	24"
Barrier Free Ramps	6"	3,600	N/A	6.5	#3 bars	24"
Sidewalks	4"	3,000	N/A	5.5	#3 bars	24"
Parking Lot/Drive Aisles	5"	3,000	5.0	5.5	#3 bars	24"
Dumpster Pads	7"	3,600	6.0	6.5	#3 bars	24"

- 3. Reinforcing steel shall be tied (100%). Reinforcing steel shall be set on plastic chairs. Bar laps shall be minimum 30 diameters. Sawed transverse dummy joints shall be spaced every 15 feet or 1.25 time longitudinal butt joint spacing whichever is less. Sawing shall occur within 5 to 12 hours after the pour, including sealing. Otherwise, the section shall be removed and longitudinal butt joint constructed.
- 4. No sand shall be allowed under any paving.
- 5. All concrete mix design shall be submitted to the City for review and approval prior to placement.
- 6. Fly ash may be used in concrete pavement locations provided that the maximum cement reduction does not exceed 20% by weight per C.Y. of concrete. The fly ash replacement shall be 1.25 lbs. per 1.0 lb. cement reduction
- 7. All curb and gutter shall be integral (monolithic) with the pavement.
- 8. All fill shall be compacted by sheep's foot roller to a minimum 95% standard proctor. Maximum loose lift for compaction shall be 8 inches. All lifts shall be tested for density by an independent laboratory. All laboratory compaction reports shall be submitted to the City Engineering Construction Inspector once results are received. All reports will be required prior to final acceptance.
- All concrete compression tests and soil compaction/density tests are required to be submitted to the City's Engineering Inspector immediately upon results.
- 10. All proposed sidewalks shall include barrier free ramps at intersecting streets, alleys, etc. Barrier free ramps (truncated dome plate in Colonial or brick red color) shall meet current City and ADA requirements and be approved by the Texas Department of Licensing and Regulation (TDLR).
- 11. All public sidewalks shall be doweled into pavement where it abuts curbs and driveways. Expansion joint material shall be used at these locations.
- 12. All connection of proposed concrete pavement to existing concrete pavement shall include a longitudinal butt joint as the load transfer device. All longitudinal butt joints shall be clean, straight and smooth (not jagged in appearance)
- 13. Cracks formed in concrete pavement shall be repaired or removed by the CONTRACTOR at the City's discretion. CONTRACTOR shall replace existing concrete curbs, sidewalk, paving, a gutters as indicated on the plans and as necessary to connect to the existing infrastructure, including any damage caused by the CONTRACTOR.
- 14. All residential lots will require individual grading plans submitted during the building permit process that correspond with the engineered grading and drainage area plans.
- 15. Approval of this plan is not an authorization to grade adjacent properties when the plans or field conditions warrant off-site grading. Written permission must be obtained and signed from the affected property owner(s) and temporary construction easements may be required. The written permission shall be provided to the City as verification of approval by the adjacent property owner(s). Violation of this requirement will result in suspension of all work at the job site until issue has been rectified.
- 16. All cut or fill slopes of non-paved areas shall be a maximum of 4:1 and minimum of 1%.
- 17. CONTRACTOR agrees to repair any damage to property and the public right-of-way in accordance with the City Standards of Design and Construction.
- 18. CONTRACTOR shall protect all monuments, iron pins/rods, and property corners during construction.
- 19. CONTRACTOR shall ensure positive drainage so that runoff will drain by gravity flow to new or existing drainage inlets or sheet flow per these approved plans.

DRAINAGE / STORM SEWER NOTES

- 1. The CONTRACTOR shall maintain drainage at all times during construction. Ponding of water in streets, drives, trenches, etc. will not be allowed. Existing drainage ways shall not be blocked or removed unless explicitly stated in the plans or written approval is given by the City.
- . All structural concrete shall be 4200 psi compressive strength at 28 days minimum 7.0 sack mix, air entrained, unless noted otherwise. Fly ash shall not be allowed in any structural concrete.
- 3. Proposed storm sewer embedment shall be NCTCOG Class 'B' as amended by the City of Rockwall's Engineering Department Standards of Design and Construction Manual.
- 4. All public storm pipe shall be a minimum of 18-inch reinforced concrete pipe (RCP), Class III, unless otherwise noted.
- 5. All storm pipe entering structures shall be grouted to assure connection at the structure is watertight.
- 6. All storm structures shall have a smooth uniform poured mortar invert from invert in to invert out.
- 7. All storm sewer manholes in paved areas shall be flush with the paving grade, and shall have traffic bearing ring and covers.

RETAINING WALLS

- 1. All retaining walls, regardless of height, will be reviewed and approved by the City Engineering Department
- All retaining walls (including foundation stem walls), regardless of height, will be constructed of rock/stone/brick or rock/stone/brick faced. No smooth concrete walls are allowed. Wall materials shall be the same for all walls on the project.
- 3. All portions, including footings, tie-backs, and drainage backfill, of the wall shall be on-site and not encroach into any public easements or right-of-way. The entire wall shall be in one lot and shall not be installed along a lot line.
- 4. All walls 3 feet and taller will be designed and signed/sealed by a registered professional engineer in the State of Texas. The wall design engineer is required to inspect the wall construction and supply a signed/sealed letter of wall construction compliance to the City of Rockwall along with wall as-builts prior to City Engineering acceptance.
- 5. No walls are allowed in detention easements. A variance to allow retaining walls in a detention easement will require approval by the Planning and Zoning Commission with appeals being heard by the City Council.

FINAL ACCEPTANCE AND RECORD DRWINGS/AS-BUILTS

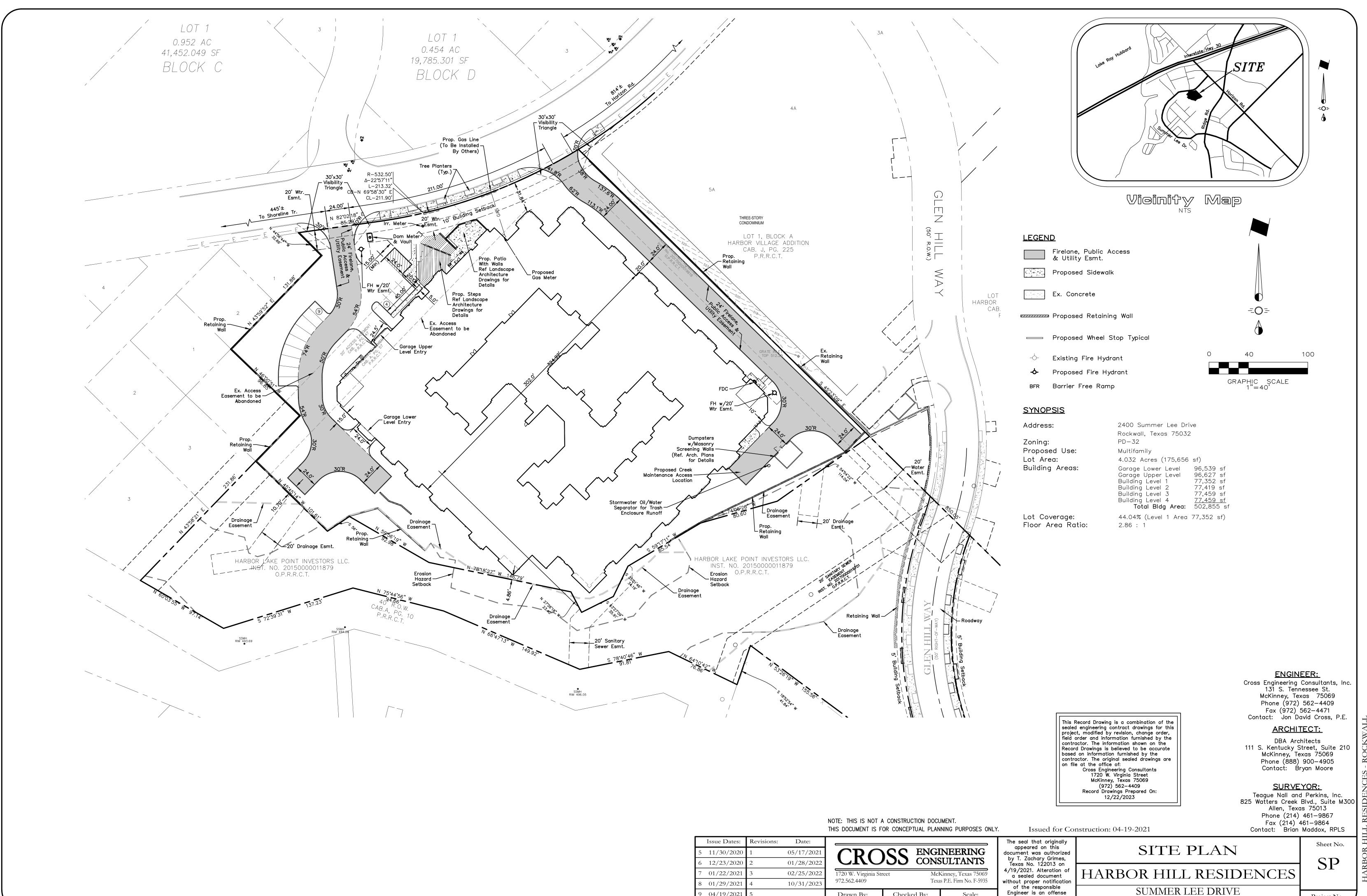
- Final Acceptance shall occur when all the items on the Checklist for Final Acceptance have been completed
 and signed-off by the City. An example of the checklist for final acceptance has been included in the
 Appendix of the Standards of Design and Construction. Items on the checklist for final acceptance will vary
 per project and additional items not shown on the check list may be required.
- 2. After improvements have been constructed, the developer shall be responsible for providing to the City "As Built" or "Record Drawings". The Design Engineer shall furnish all digital files of the project formatted in Auto Cad 14, or 2000 format or newer and Adobe Acrobat (.pdf) format with a CD-ROM disk or flash drive. The disk or drive shall include a full set of plans along with any landscaping, wall plans, and details sheets.
- 3. Submit 1-set of printed drawings of the "Record Drawings" containing copies of all sheets to the Engineering Construction Inspector for the project. The printed sheets will be reviewed by the inspector PRIOR to producing the "Record Drawing" digital files on disk or flash drive. This will allow any revisions to be addressed prior to producing the digital files.
- 4. Record Drawing Disk drawings shall have the Design Engineers seal, signature and must be stamped and dated as "Record Drawings" or "As Built Drawings" on all sheets.
- The City of Rockwall will not accept any Record Drawing disk drawings which include a disclaimer. A disclaimer shall not directly or indirectly state or indicate that the design engineer or the design engineer's surveyor/surveyors did not verify grades after construction, or that the Record Drawings were based solely on information provided by the construction contractor/contractors. Any Record Drawings which include like or similar disclaimer verbiage will not be accepted by the City of Rockwall.
- 6. Example of Acceptable Disclaimer: "To the best of our knowledge ABC Engineering, Inc., hereby states that this plan is As-Built. This information provided is based on surveying at the site and information provided by the contractor."



GENERAL CONSTRUCTION NOTES Sheet 2 of 2 June 2020

CITY OF ROCKWALL ENGINEERING DEPARTMENT

385 S. Goliad Rockwall. Texas 75087 P (972) 771-7746 F (972) 771-7748



04/19/2021

10 12/22/2023

Checked By:

C.E.C.I.

Scale:

1"=40'

under the Texas

Engineering Practice Act.

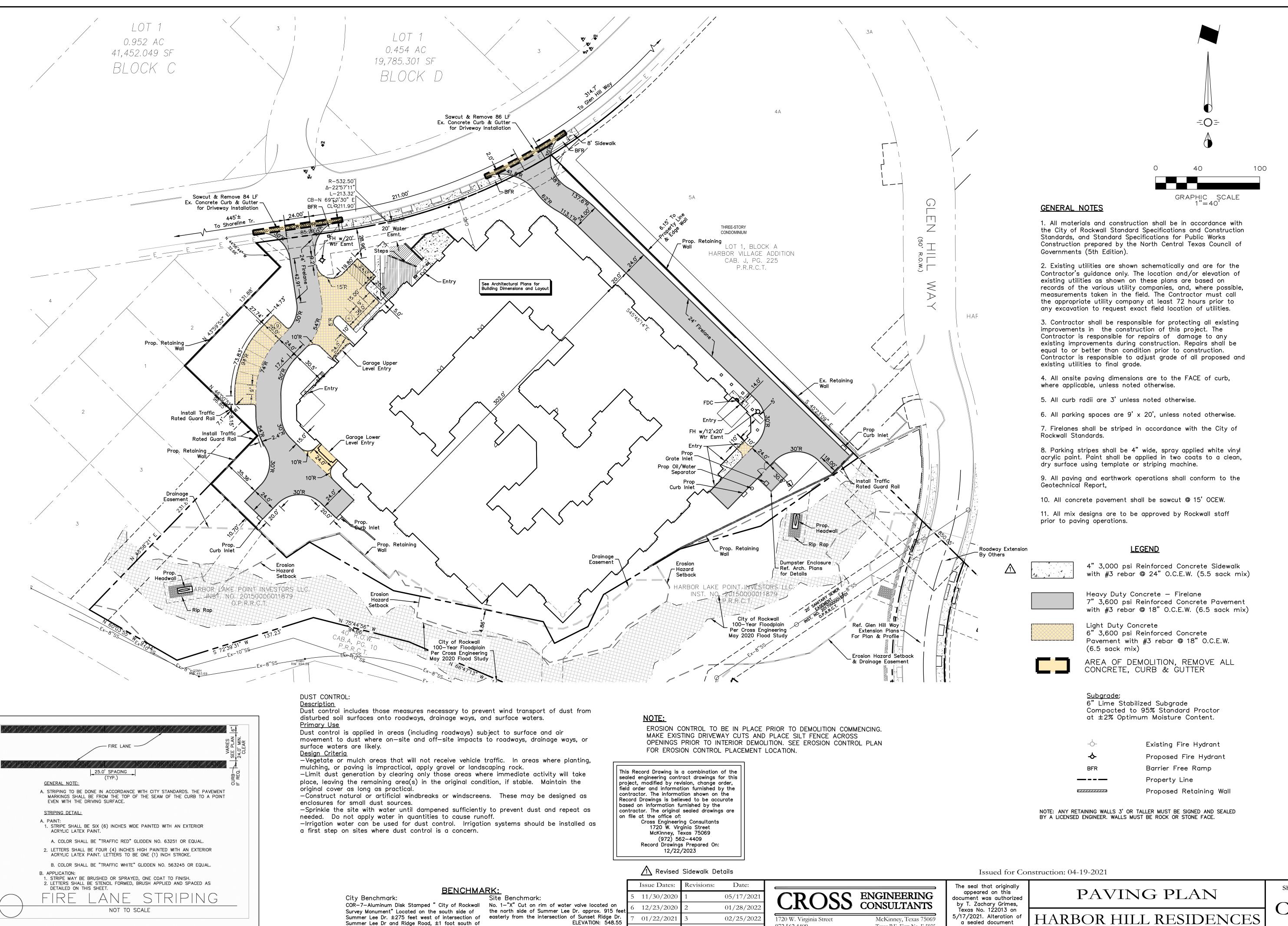
ROCKWALL, TEXAS

Drawn By:

C.E.C.I.

Project No.

18090



01/22/2021

01/29/2021

04/19/2021

10 12/22/2023

ELEVATION: 548.55

ELEVATION: 474.56

No. 2-"X" Cut on southeast corner of curb inlet

located on the east side of Shoreline Tr. approx.

480 feet northerly from the north side of Summer

Summer Lee Dr and Ridge Road, ±1 foot south of

curb line.

ELEVATION: 567.52

02/25/2022

10/31/2023

1720 W. Virginia Street

Checked By:

C.E.C.I.

972.562.4409

Drawn By:

C.E.C.I.

Sheet No.

Project No.

18090

5/17/2021. Alteration of

a sealed document

of the responsible

Engineer is an offense

under the Texas

Engineering Practice Act.

vithout proper notification

SUMMER LEE DRIVE

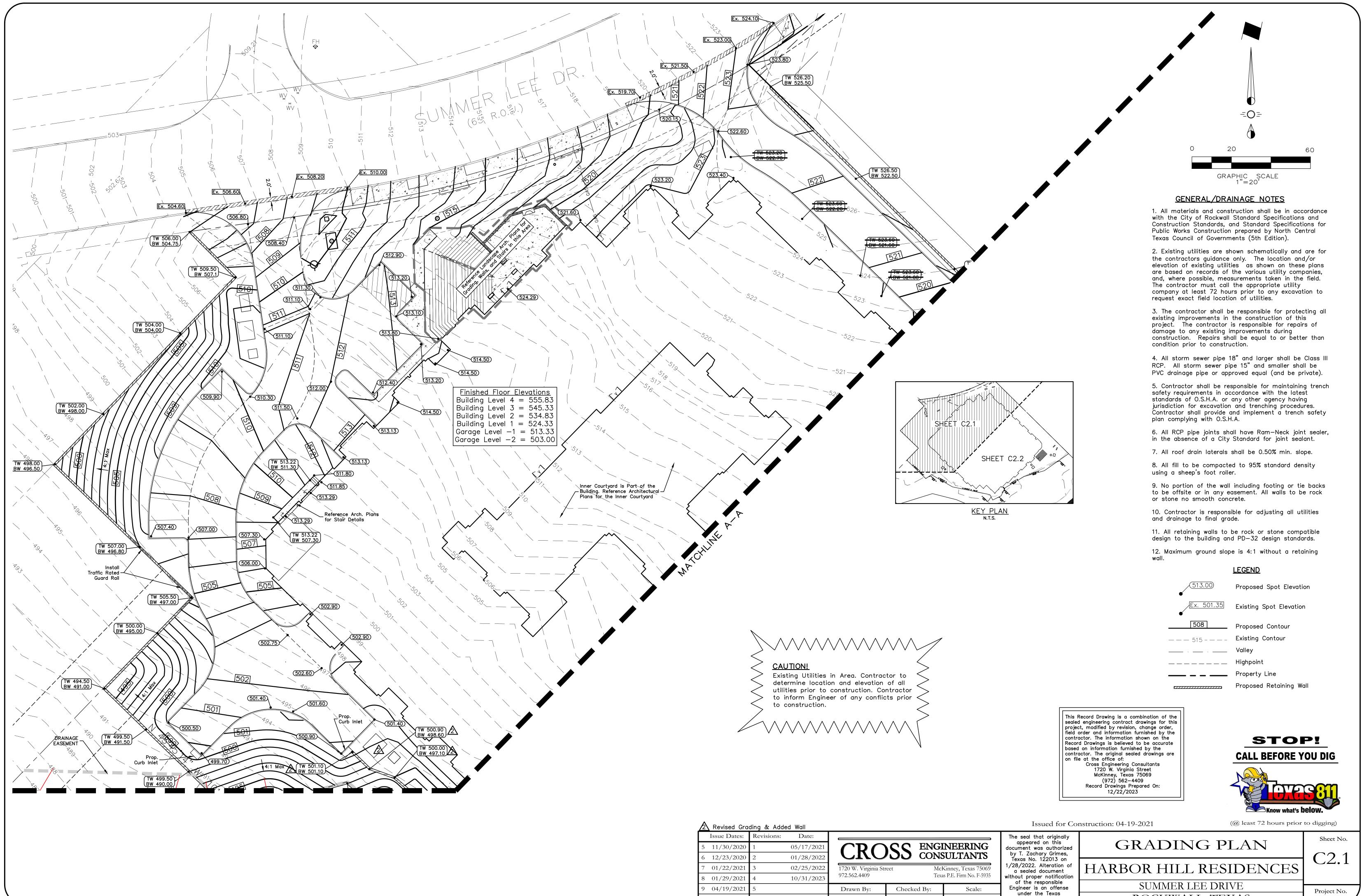
ROCKWALL, TEXAS

McKinney, Texas 75069

Texas P.E. Firm No. F-5935

Scale:

1"=40'



0 12/22/2023

C.E.C.I.

1"=20'

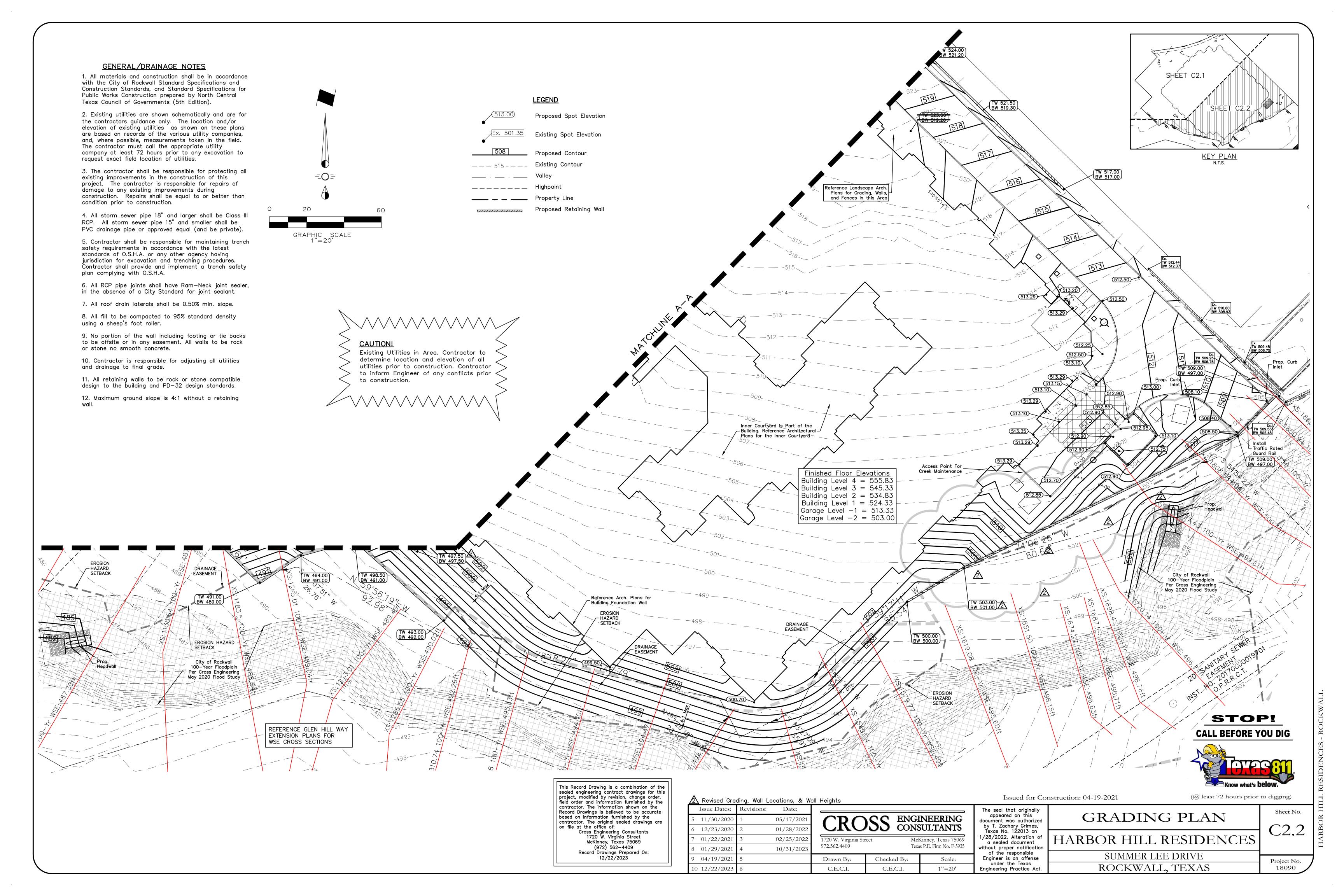
C.E.C.I.

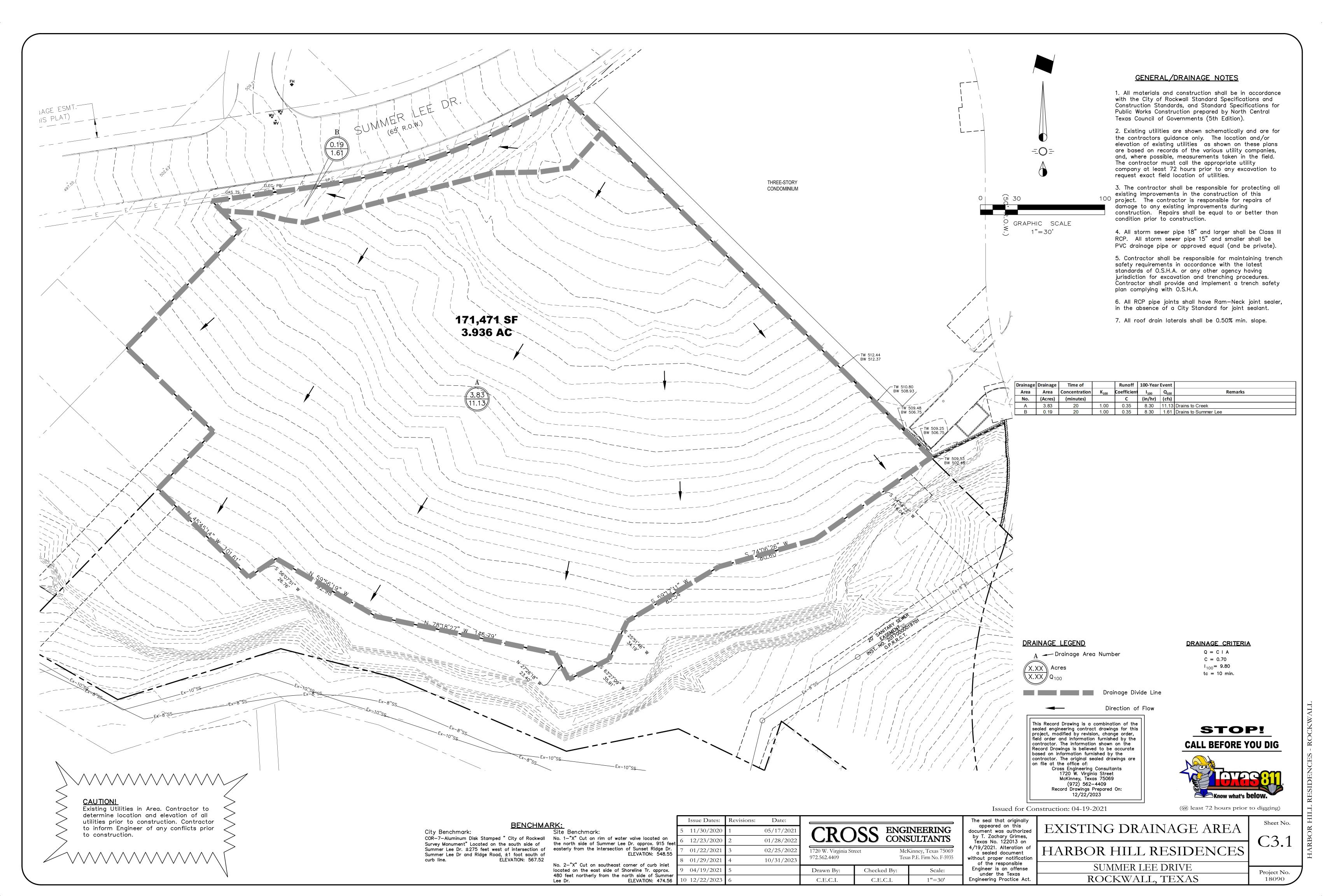
Engineering Practice Act.

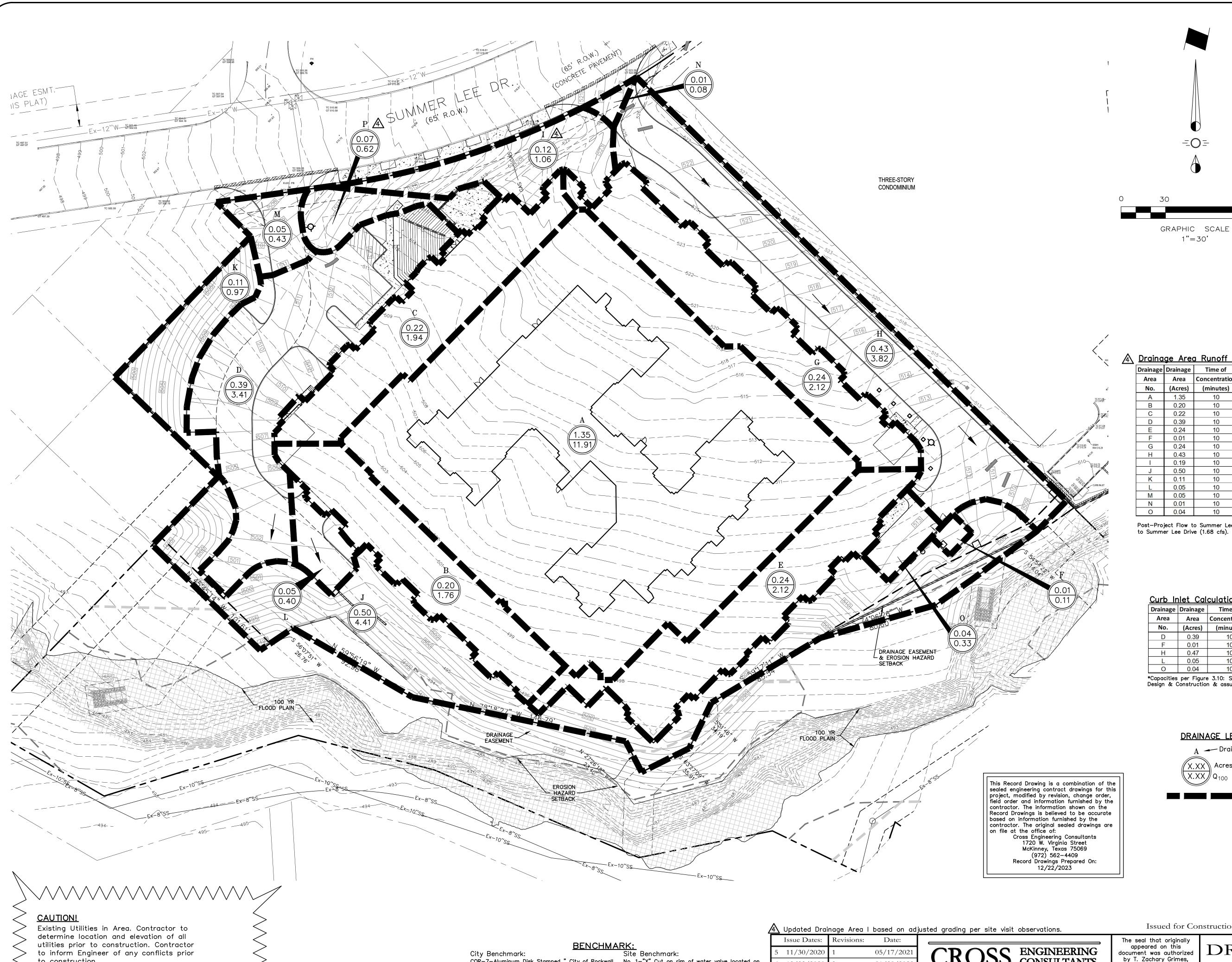
ARBOR HILL RESIDENCES - ROCKWALL

18090

ROCKWALL, TEXAS







COR-7-Aluminum Disk Stamped "City of Rockwall No. 1-"X" Cut on rim of water valve located on

ELEVATION: 567.52

the north side of Summer Lee Dr. approx. 915 feet easterly from the intersection of Sunset Ridge Dr. ELEVATION: 548.55

No. 2-"X" Cut on southeast corner of curb inlet

located on the east side of Shoreline Tr. approx.

480 feet northerly from the north side of Summer

ELEVATION: 474.56

Survey Monument" Located on the south side of

Summer Lee Dr. ±275 feet west of intersection of

Summer Lee Dr and Ridge Road, ±1 foot south of

12/23/2020

01/22/2021

01/29/2021

10 12/22/202

01/28/2022

02/25/2022

10/31/2023

1720 W. Virginia Street

972.562.4409

Drawn By:

C.E.C.I.

to construction.

GENERAL/DRAINAGE NOTES

- 1. All materials and construction shall be in accordance with the City of Rockwall Standard Specifications and Construction Standards, and Standard Specifications for Public Works Construction prepared by North Central Texas Council of Governments (5th Edition).
- 2. Existing utilities are shown schematically and are for the contractors guidance only. The location and/or elevation of existing utilities as shown on these plans are based on records of the various utility companies, and, where possible, measurements taken in the field. The contractor must call the appropriate utility company at least 72 hours prior to any excavation to request exact field location of utilities.
- 3. The contractor shall be responsible for protecting all existing improvements in the construction of this project. The contractor is responsible for repairs of damage to any existing improvements during construction. Repairs shall be equal to or better than condition prior to construction.
- 4. All storm sewer pipe 18" and larger shall be Class III RCP. All storm sewer pipe 15" and smaller shall be PVC drainage pipe or approved equal (and be private).
- 5. Contractor shall be responsible for maintaining trench safety requirements in accordance with the latest standards of O.S.H.A. or any other agency having jurisdiction for excavation and trenching procedures. Contractor shall provide and implement a trench safety plan complying with O.S.H.A.
- 6. All RCP pipe joints shall have Ram-Neck joint sealer, in the absence of a City Standard for joint sealant.
- 7. All roof drain laterals shall be 0.50% min. slope.

A Drainage Area Runoff Calculations

1"=30'

<u> </u>		3 	<u> </u>		••••			
	Drainage	Drainage	Time of		Runoff	100-Year	Event	
	Area	Area	Concentration	K ₁₀₀	Coefficient	I ₁₀₀	Q ₁₀₀	Remarks
	No.	(Acres)	(minutes)		С	(in/hr)	(cfs)	
	Α	1.35	10	1.00	0.90	9.80	11.91	Courtyard & Roof Drainage
	В	0.20	10	1.00	0.90	9.80	1.76	Roof Drains
	С	0.22	10	1.00	0.90	9.80	1.94	Roof Drains
	D	0.39	10	1.00	0.90	9.80	3.41	5' Curb Inlet
	E	0.24	10	1.00	0.90	9.80	2.12	Roof Drains
	F	0.01	10	1.00	0.90	9.80	0.11	Drains to 5' Inlet & Contech CDS1515-3-C separator
	G	0.24	10	1.00	0.90	9.80	2.12	Roof Drains
	Н	0.43	10	1.00	0.90	9.80	3.82	5' Curb Inlet
	1	0.19	10	1.00	0.90	9.80	1.68	Landscape Drains
	J	0.50	10	1.00	0.90	9.80	4.41	Sheet Flow to Creek
	K	0.11	10	1.00	0.90	9.80	0.97	Sheet Flow to the West
	L	0.05	10	1.00	0.90	9.80	0.40	5' Curb Inlet
	M	0.05	10	1.00	0.90	9.80	0.43	Sheet Flow to Summer Lee Drive
	N	0.01	10	1.00	0.90	9.80	0.08	Sheet Flow to Summer Lee Drive
	0	0.04	10	1.00	0.90	9.80	0.33	5' Curb Inlet

Post-Project Flow to Summer Lee Drive from Drainage Areas I, M, & N (1.06+0.43+0.08 = 1.57cfs) is lower than Pre-Project flow to Summer Lee Drive (1.68 cfs).

<u>Curb Inlet Calculations</u>

Drainage	Drainage	Time of	Runoff	100-Yea	r Event	Inlet	Inlet	On-	Inlet
Area	Area	Concentration	Coefficient	I ₁₀₀	I ₁₀₀ Q ₁₀₀		Type	Grad/Sag	Capacity*
No.	(Acres)	(minutes)	С	(in/hr)	(cfs)	(ft)			cfs
D	0.39	10	0.90	9.80	3.41	5.00	Curb	Sag	6.99
F	0.01	10	0.90	9.80	0.11	5.00	Curb	Sag	6.99
Н	0.47	10	0.90	9.80	4.15	5.00	Curb	Sag	6.99
L	0.05	10	0.90	9.80	0.40	5.00	Curb	Sag	6.99
0	0.04	10	0.00	0.80	0.33	5.00	Curb	San	6 00

*Capacities per Figure 3.10: Standard Curb Inlet Capacity Sag/Low Point from City of Rockwall Standards of Design & Construction & assuming a depth of 0.5 feet.

DRAINAGE LEGEND

A — Drainage Area Number $X \times X$ Acres X.XX Q₁₀₀

Drainage Divide Line

Direction of Flow

STOP! **CALL BEFORE YOU DIG**

DRAINAGE CRITERIA Q = C I A

> C = 0.90I₁₀₀= 9.80

tc = 10 min.



Issued for Construction: 04-19-2021

by T. Zachary Grimes,

Texas No. 122013 on

10/31/2023. Alteration of

a sealed document

without proper notification

of the responsible

Engineer is an offense

under the Texas

Engineering Practice Act.

CONSULTANTS

Checked By:

C.E.C.I.

McKinney, Texas 75069

Texas P.E. Firm No. F-5935

Scale:

1"=30'

(@ least 72 hours prior to digging)

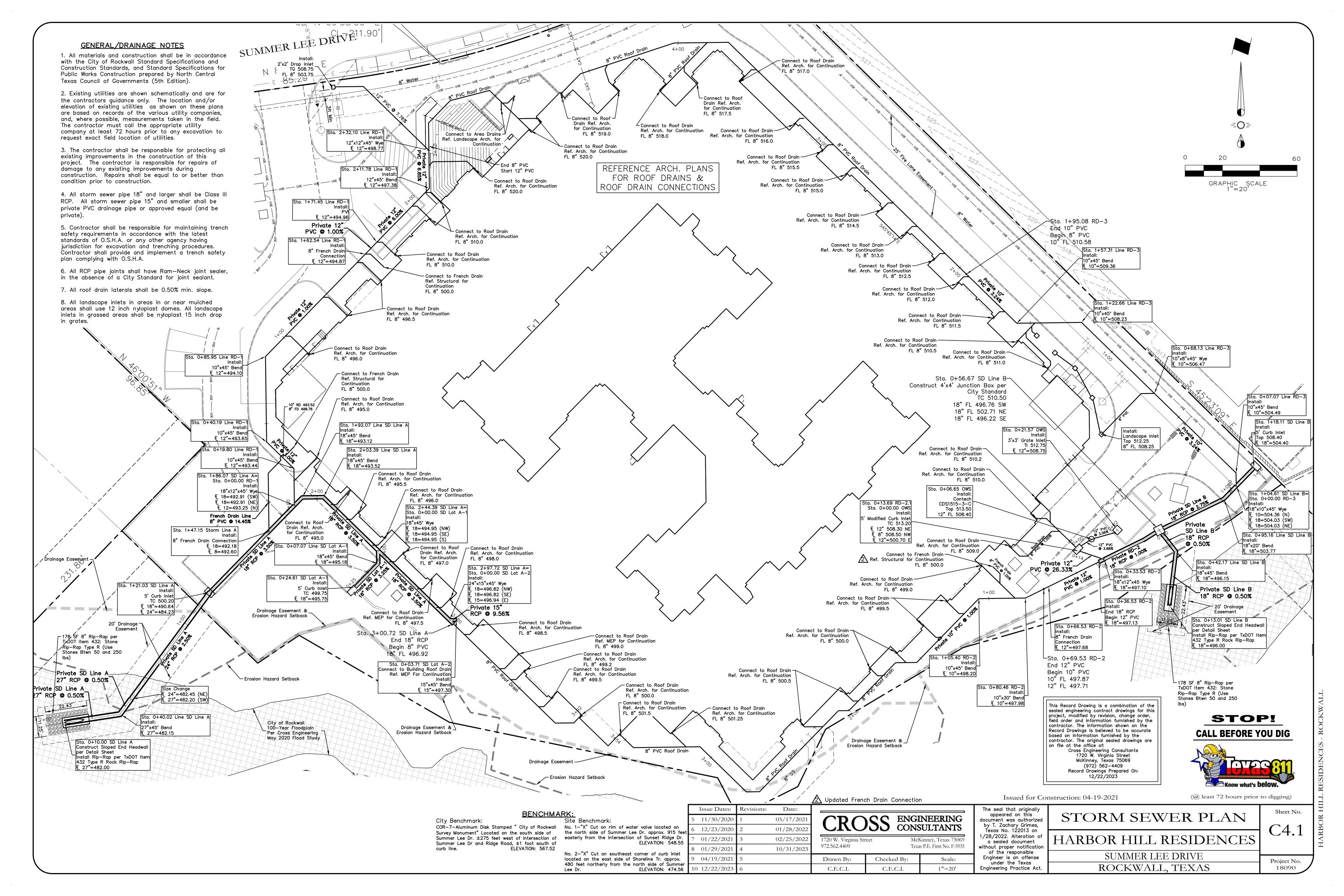
DRAINAGE AREA MAP

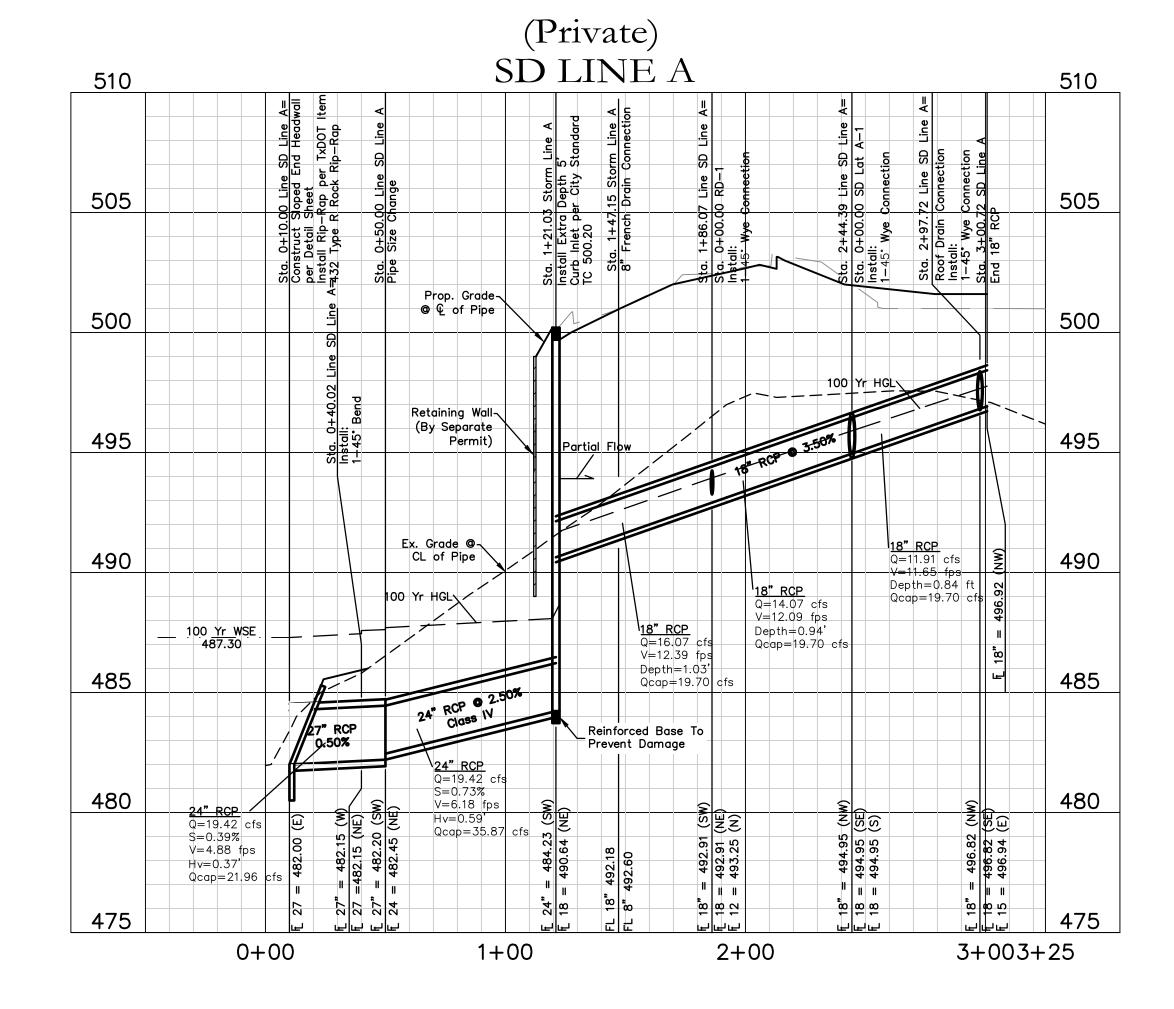
HARBOR HILL RESIDENCES

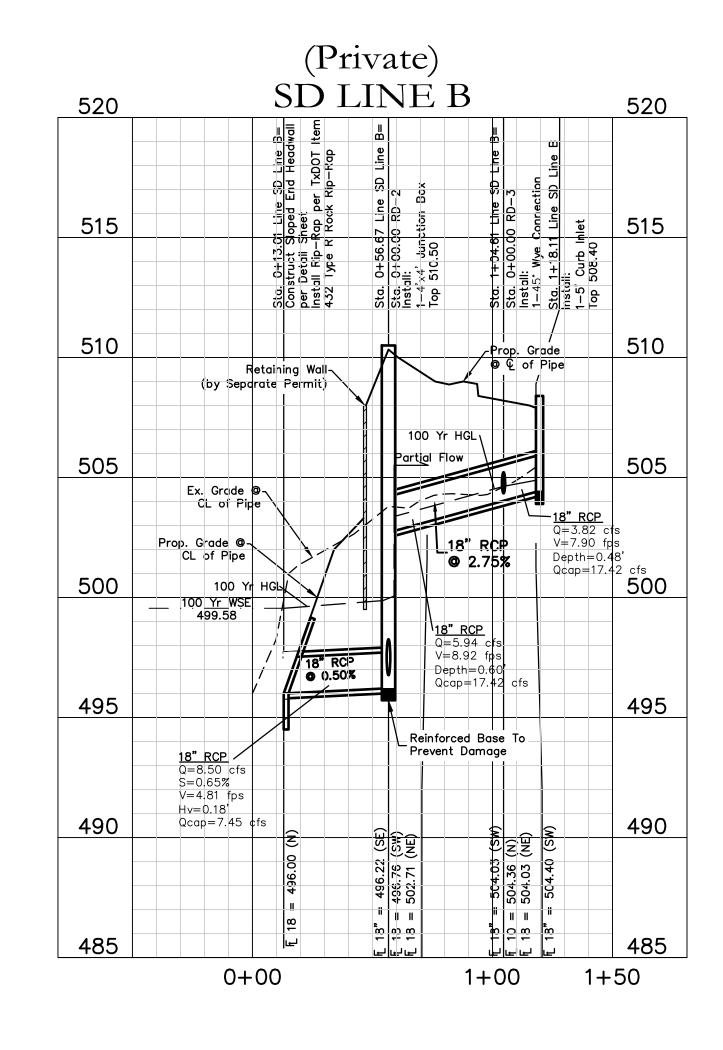
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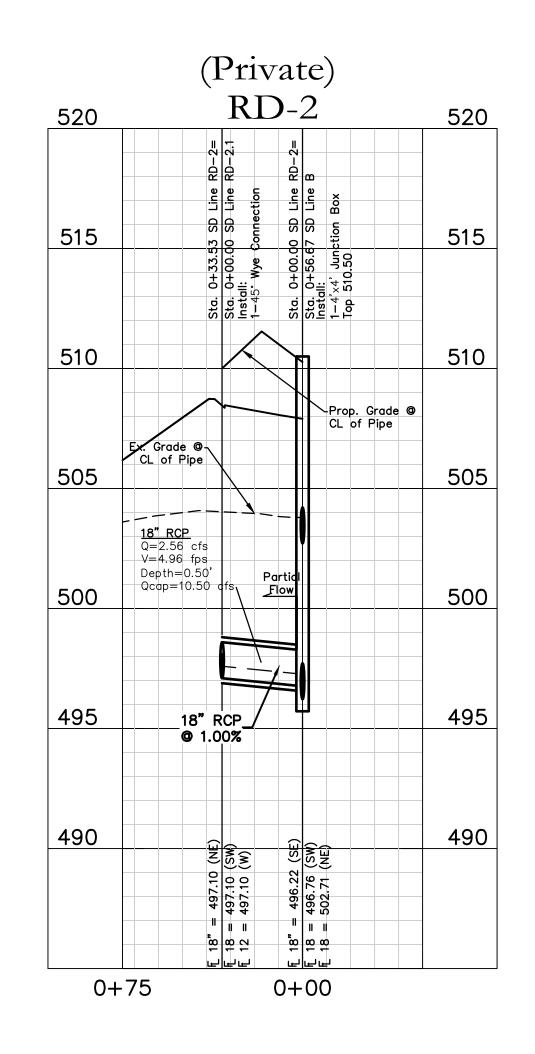
SUMMER LEE DRIVE ROCKWALL, TEXAS

Project No. 18090









Sytem				Pipe		Wetted	Hydraulic		DS	US		Drainage	Runoff			Acumulated		Rainfall		Q	Partial		Friction	Friction			Head			Top of
ID	DS Sta	US Sta	Length	Diameter	Area	Perimeter	Radius	Mannings n	Flowline	Flowline	Slope	Areas	Coeff C	Area	CA	CA	Tc	Intensity	Q 100	Capacity	Flow	Velocity	Slope	Loss	Hv	К	Loss	HGL DS	HGL US	Curb
			ft	in	ft2	ft	ft		ft	ft	ft/ft			Acres			min	(in/hr)	cfs	cfs		ft/s	ft/ft	ft	ft		ft	ft	ft	ft
Α	121.03	121.03	-	-	-	-	-	-	-	-	-	A,B,C,D,L	0.90	2.21	1.98	1.98	10.00	9.80	19.42	-	Yes	-	-		-	-	-	488.78	-	500.20
0	50.00	121.03	71.03	24	3.14	6.28	0.5	0.013	482.45	484.23	0.025	-	0.90	0	0	1.98	10.00	9.80	19.42	35.87	No	6.18	0.0073	0.52	0.59	1.00	0.59	487.67	488.19	-
Α	40.02	50.00	9.98	27	3.98	7.07	0.5625	0.013	482.15	482.2	0.005	-	0.90	0	0	1.98	10.00	9.80	19.42	21.96	No	4.88	0.0039	0.04	0.37	0.21	0.08	487.55	487.59	-
Α	10.00	40.02	30.02	27	3.98	7.07	0.5625	0.013	482	482.15	0.005	-	0.90	0	0	1.98	10.00	9.80	19.42	21.96	No	4.88	0.0039	0.12	0.37	0.37	0.14	487.3	487.42	-
Sytem				Pipe		Wetted	Hydraulic		DS	US		Drainage	Runoff			Acumulated		Rainfall		Q	Partial		Friction	Friction			Head			Top of
ID	DS Sta	US Sta	Length	Diameter	Area	Perimeter	Radius	Mannings n	Flowline	Flowline	Slope	Areas	Coeff C	Area	CA	CA	Tc	Intensity	Q 100	Capacity	Flow	Velocity	Slope	Loss	Hv	К	Loss	HGL DS	HGL US	Curb
			ft	in	ft2	ft	ft		ft	ft	ft/ft			Acres			min	(in/hr)	cfs	cfs		ft/s	ft/ft	ft	ft		ft	ft	ft	ft
В	56.67	56.67	0	18	-	-	-	-	-	-	-	E,F,G,H,0	0.90	0.96	0.87	0.87	10.00	9.80	8.50	-	Yes	-	-	-	-	-	-	500.04	-	-
В	13.01	56.67	43.66	18	1.77	4.71	0.375	0.013	496	496.2	0.005	-	0.90	0	0	0.87	10.00	9.80	8.50	7.45	No	4.81	0.0065	0.28	0.36	0.5	0.18	499.58	499.86	-
Cutom				Dina		\\/o++od	Lludroulio		DC	LIC		Drainaga	Dunoff			A aumoulate d		Rainfall		0	Dortiol		Frietion	Fristian			Hood			Ton of
Sytem	D C C:			Pipe		Wetted	Hydraulic		DS	US	01	Drainage				Acumulated	_		0.400	Q	Partial	,,,				.,	Head			Top of
ID	DS Sta	US Sta	Length	Diameter	Area	Perimeter	Radius	Mannings n	Flowline	Flowline	Slope	Areas	Coeff C	Area	CA	CA	Тс	Intensity	Q 100	Capacity	Flow	Velocity	Slope	Loss	Hv	К		HGL DS	HGL US	Curb
			ft	in	ft2	ft	ft		ft	ft	ft/ft			Acres			min	(in/hr)	cfs	cfs		ft/s	ft/ft	ft	ft		ft	ft	ft	ft
RD-2	0	33.53	33.53	18	1.77	4.71	0.38	0.013	496.76	497.10	0.01	E, F, & O	0.90	0.29	0.26	0.26	10.00	9.80	2.56	10.50	Yes	1.45	0.01	0.22	0.36	0.50	0.18	504.36	504.58	-

10 12/22/2023

This Record Drawing is a combination of the sealed engineering contract drawings for this project, modified by revision, change order, field order and information furnished by the contractor. The information shown on the Record Drawings is believed to be accurate based on information furnished by the contractor. The original sealed drawings are on file at the office of:

Cross Engineering Consultants

1720 W. Virginia Street

McKinney, Texas 75069 (972) 562-4409 Record Drawings Prepared On: 12/22/2023



Issued for Construction: 04-19-2021

(@ least 72 hours prior to digging)

Issue Dates:	Revisions:	Date:				
11/30/2020	1	05/17/2021			INEERING	d
12/23/2020	2	01/28/2022		CONS	SULTANTS	
01/22/2021	3	02/25/2022			Kinney, Texas 75069	4
01/29/2021	4	10/31/2023	972.562.4409	Texa	as P.E. Firm No. F-5935	wi
04/19/2021	5		Drawn By:	Checked By:	Scale:	

C.E.C.I.

C.E.C.I.

	133464 101 601	13 (1 (
NEERING SULTANTS	The seal that originally appeared on this document was authorized by T. Zachary Grimes,	
Kinney, Texas 75069 as P.E. Firm No. F-5935	Texas No. 122013 on 4/19/2021. Alteration of a sealed document without proper notification	E
Scale:	of the responsible Engineer is an offense	
H:1"=40' V:1"=4'	under the Texas Engineering Practice Act.	

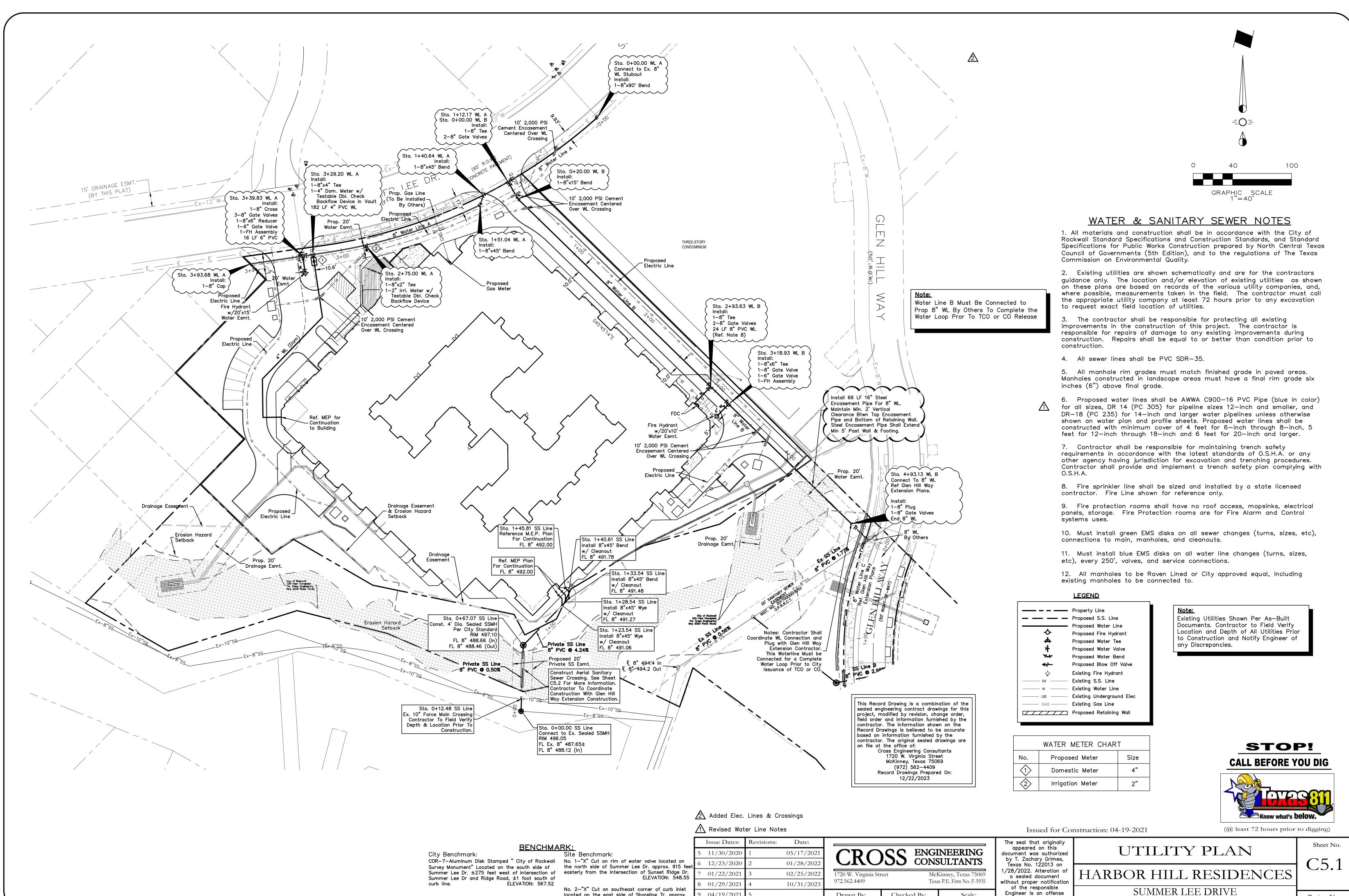
STORM SEWER PROFILES HARBOR HILL RESIDENCES

ROCKWALL, TEXAS

SUMMER LEE DRIVE

Project No. 18090

Sheet No. C4.2



located on the east side of Shoreline Tr. approx.

480 feet northerly from the north side of Summer

ELEVATION: 474.56

04/19/2021

10 12/22/2023

Drawn By:

C.E.C.I.

Checked By:

C.E.C.I.

Scale:

1"=40'

Project No.

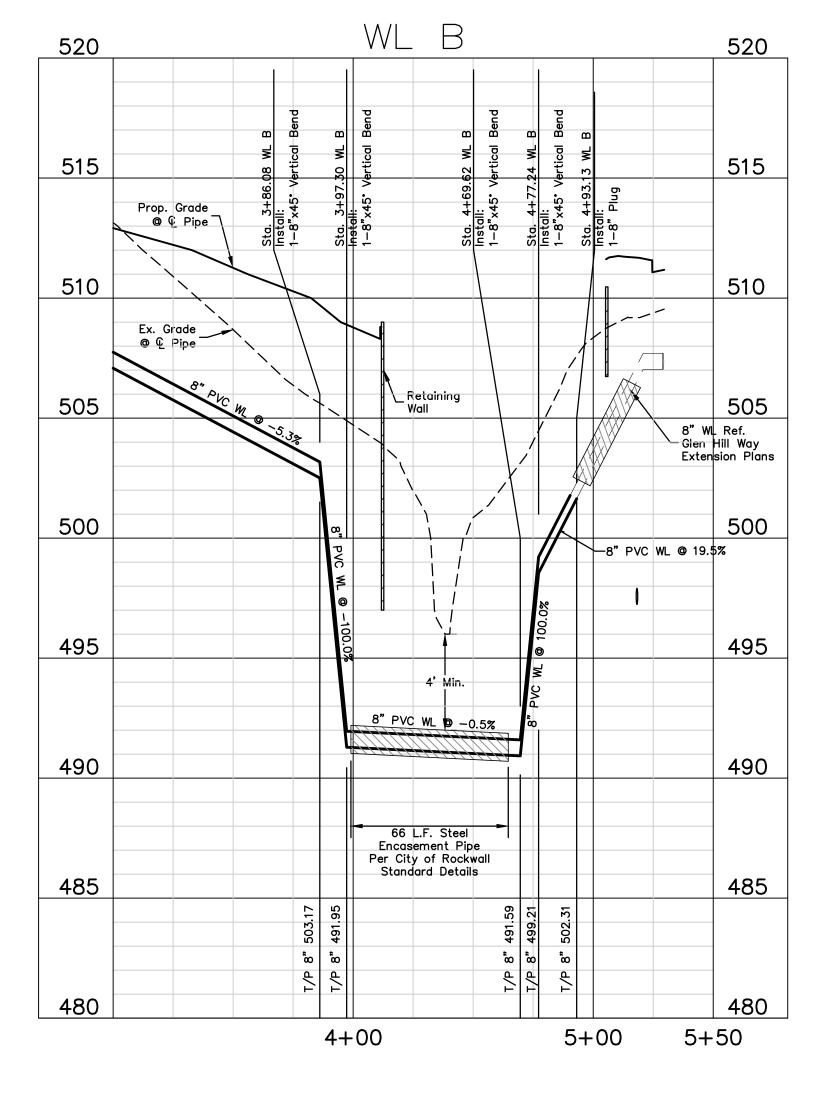
18090

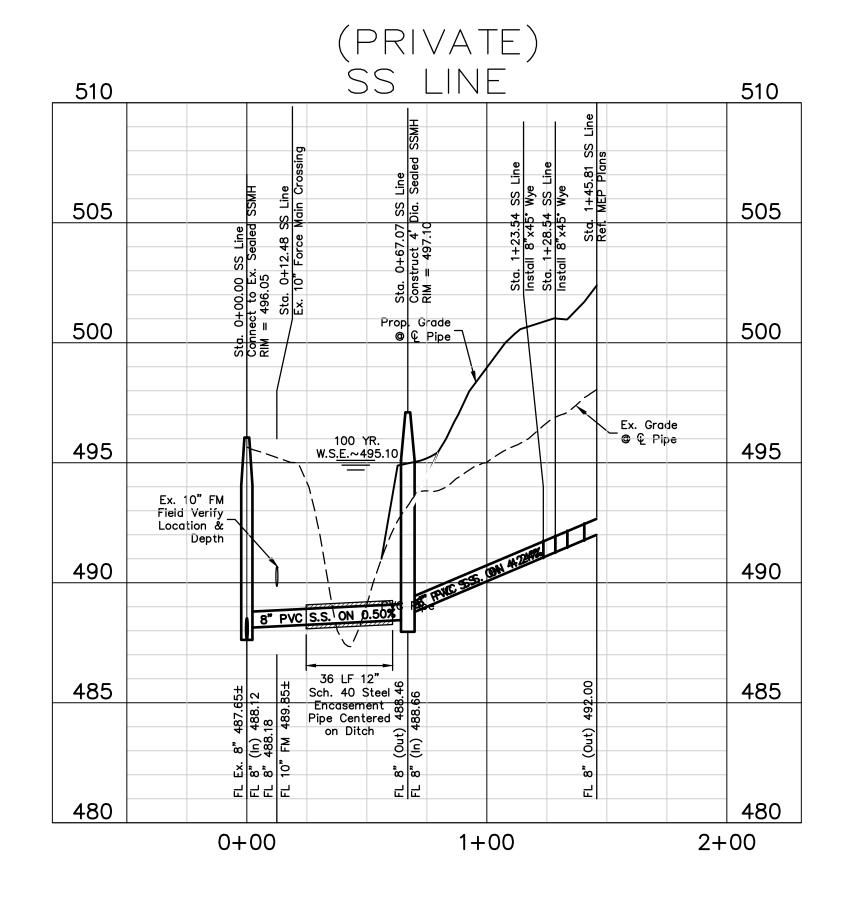
ROCKWALL, TEXAS

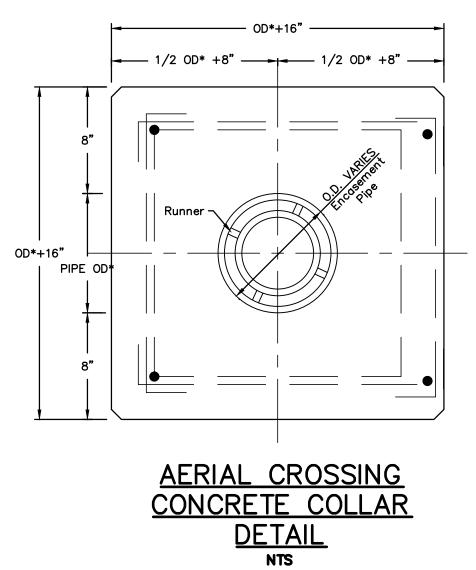
Engineer is an offense

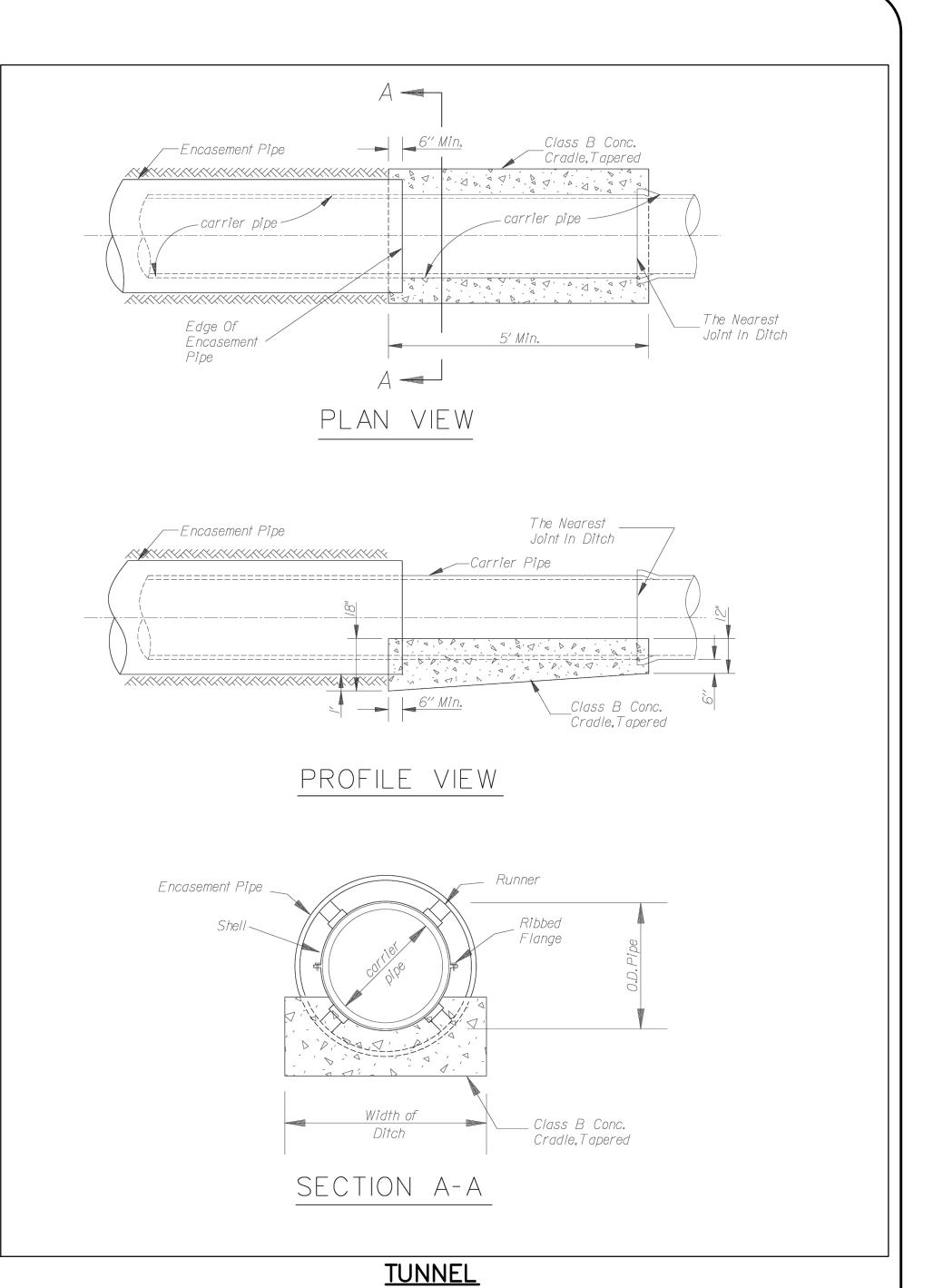
under the Texas

Engineering Practice Act.









APPROACHES WITH CASING SPACERS

SANITARY SEWER AERIAL CROSSING DETAILS ON THIS SHEET PROVIDED FOR REFERENCE AND BIDDING PURPOSES ONLY. CONTRACTOR TO PROVIDE PLANS FOR THE AERIAL CROSSING SIGNED AND SEALED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF TEXAS PRIOR TO CONSTRUCTION. STRUCTURAL ENGINEER AND CONTRACTOR TO COORDINATE DESIGN AND CONSTRUCTION OF AERIAL CROSSING WITH PROPOSED CREEK IMPROVEMENTS INCLUDED IN THE NEARBY GLEN HILL WAY EXTENSION PROJECT.

> This Record Drawing is a combination of the sealed engineering contract drawings for this project, modified by revision, change order, field order and information furnished by the contractor. The information shown on the Record Drawings is believed to be accurate based on information furnished by the contractor. The original sealed drawings are on file at the office of:
> Cross Engineering Consultants
> 1720 W. Virginia Street
> McKinney, Texas 75069 (972) 562-4409 Record Drawings Prepared On: 12/22/2023

STOP!



Issued for Construction: 04-19-2021

(@ least 72 hours prior to digging)

BENCHMARK: City Benchmark:

COR-7-Aluminum Disk Stamped "City of Rockwall No. 1-"X" Cut on rim of water valve located on Survey Monument" Located on the south side of Summer Lee Dr. ±275 feet west of intersection of Summer Lee Dr and Ridge Road, ±1 foot south of curb line. ELEVATION: 567.52

Site Benchmark: the north side of Summer Lee Dr. approx. 915 feet easterly from the intersection of Sunset Ridge Dr. ELEVATION: 548.55 No. 2—"X" Cut on southeast corner of curb inlet located on the east side of Shoreline Tr. approx.

Issue Dates: Revisions: Date: 11/30/2020 05/17/202 12/23/2020 01/28/2022 01/22/2021 02/25/2022 10/31/2023 01/29/2021 04/19/202 480 feet northerly from the north side of Summer 10 12/22/202 ELEVATION: 474.56

CROSS ENGINEERING CONSULTANTS 1720 W. Virginia Street McKinney, Texas 75069 Texas P.E. Firm No. F-5935 972.562.4409 Drawn By: Checked By: Scale:

C.E.C.I.

: 1"=40' V: 1"=

C.E.C.I.

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under the Texas

Engineering Practice Act.

HARBOR HILL RESIDENCES

UTILITY PROFILES

SUMMER LEE DRIVE

ROCKWALL, TEXAS

Project No. 18090

Sheet No.

972.562.4409

Drawn By:

C.E.C.I.

10/31/2023

01/29/2021

04/19/2021

12/22/2023

Texas P.E. Firm No. F-5935

Scale:

1"=40'

Checked By:

C.E.C.I.

without proper notification

of the responsible

Engineer is an offense

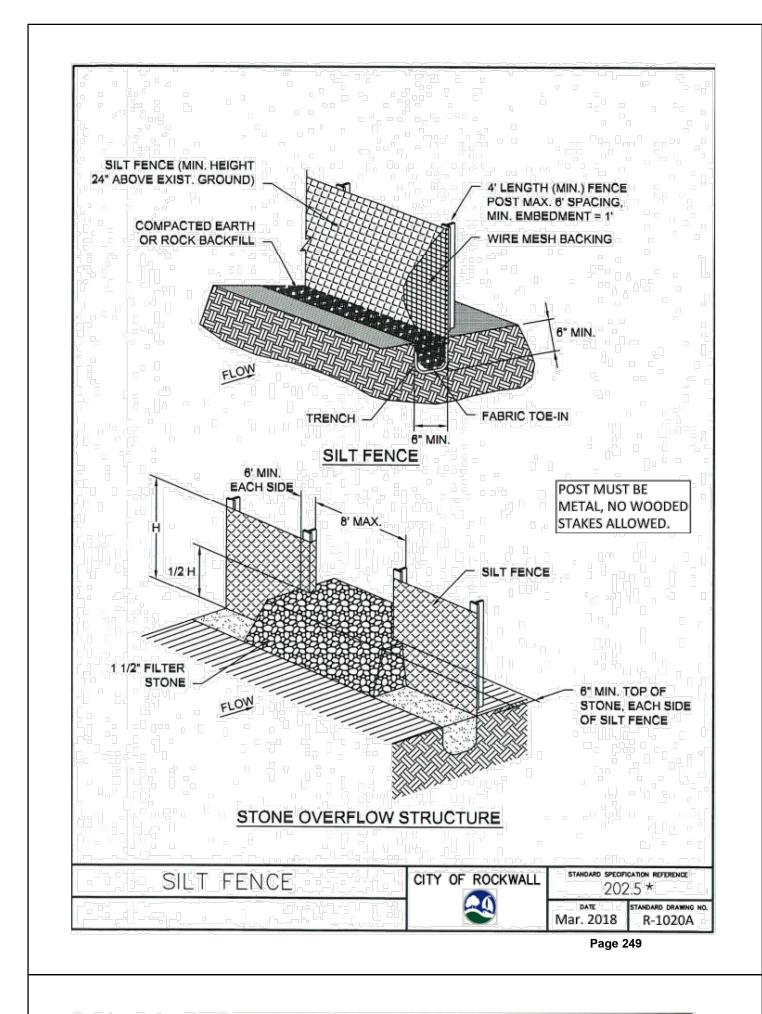
under the Texas

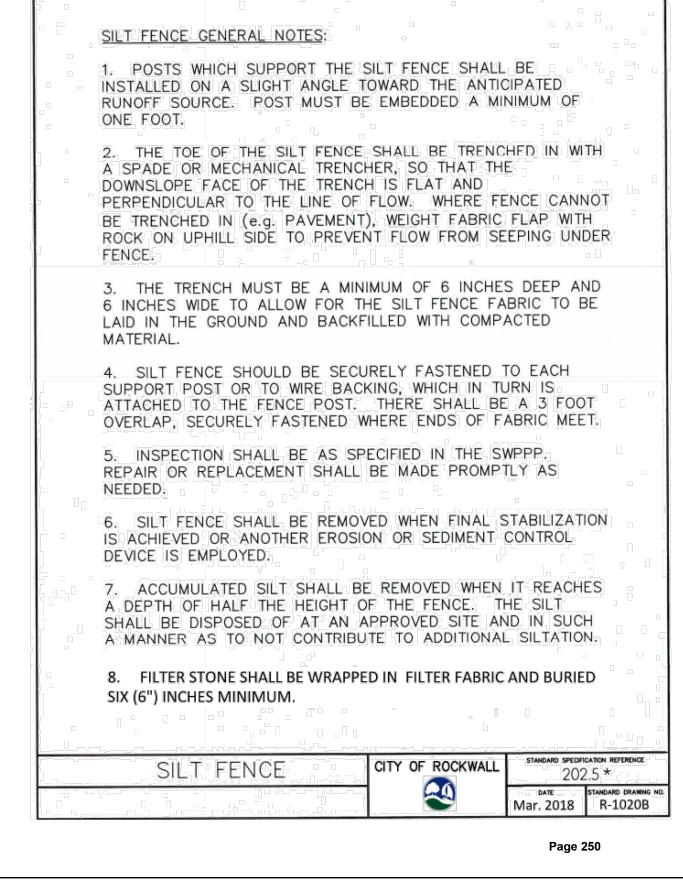
Engineering Practice Act.

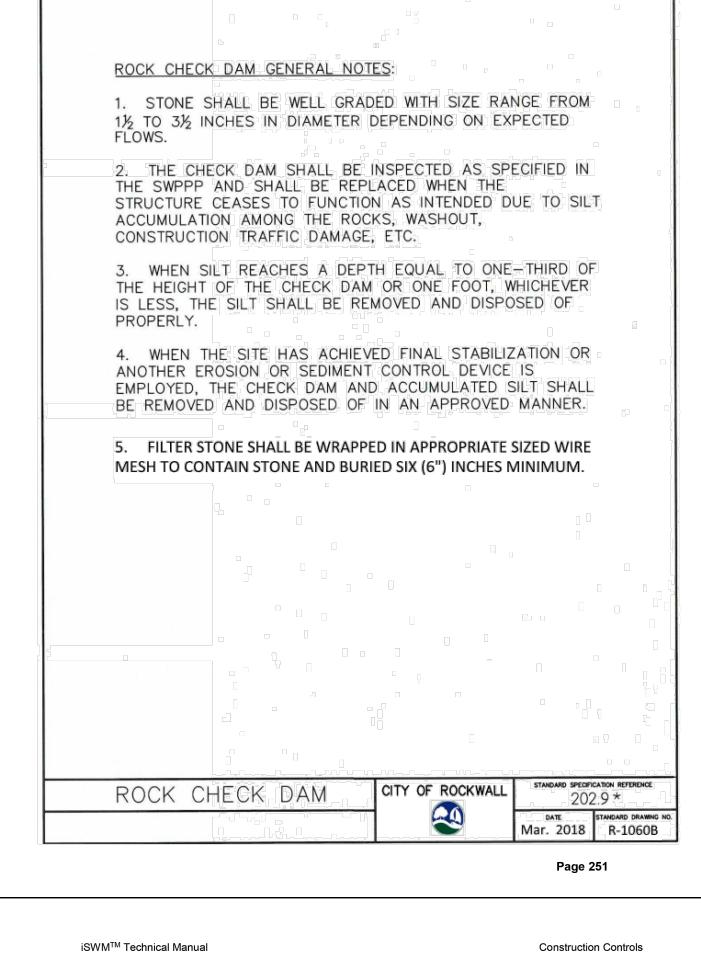
HARBOR HILL RESIDENCES

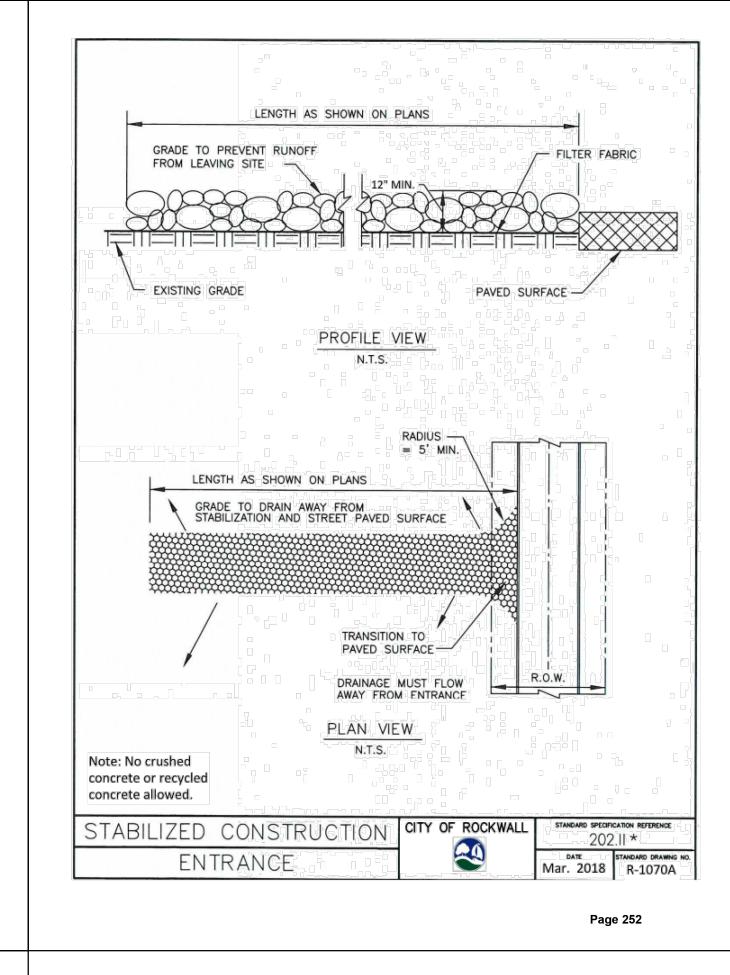
SUMMER LEE DRIVE ROCKWALL, TEXAS

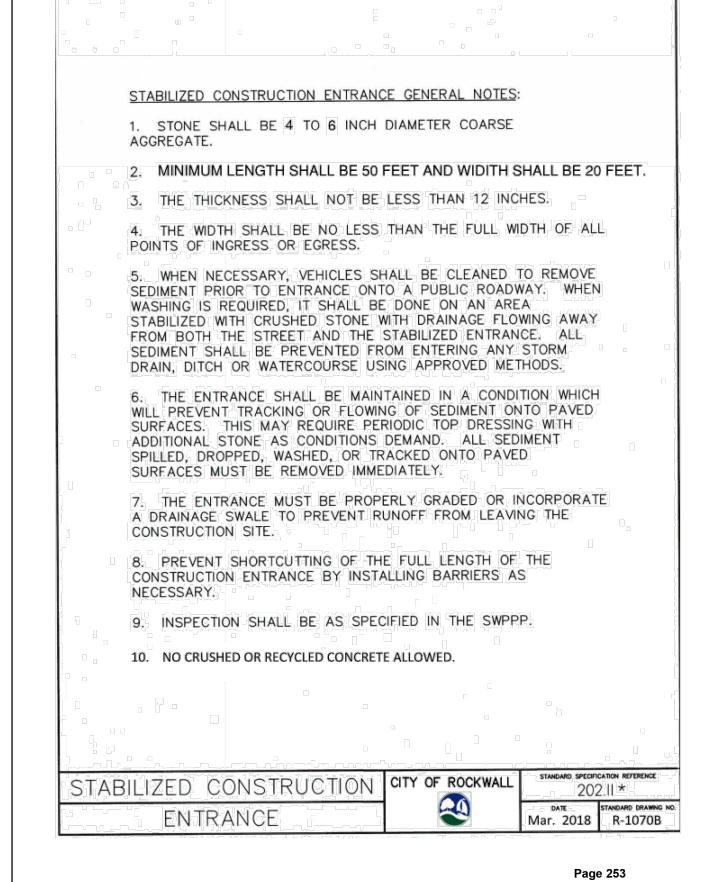
Project No. 18090

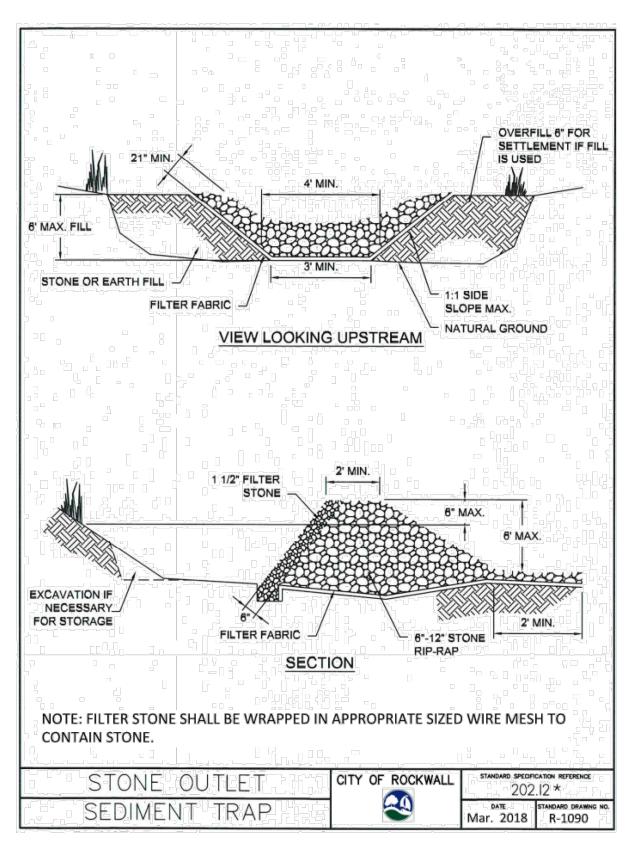












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CONCRETE WASHOUT PLAN VIEW CONCRETE WASHOUT SECTION A-A NOTE: SANDBAGS MAY BE REPLACED BY A SOIL BERM TO ANCHOR THE PLASTIC LINING. Figure 4.1 Schematics of Concrete Washout Containment Concrete Waste Management CC-190 April 2010, Revised 9/2014

PLASTIC LINING -

0 12/22/2023

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STOP! **CALL BEFORE YOU DIG**

Issued for Construction: 04-19-2021

(@ least 72 hours prior to digging)

Issue Dates: Revisions: Date: TO CC ENGINEERING 11/30/2020 05/17/2021 12/23/2020 01/28/2022 01/22/2021 02/25/2022 1720 W. Virginia Street McKinney, Texas 75069 Texas P.E. Firm No. F-5935 972.562.4409 10/31/2023 01/29/20204/19/2021 Drawn By: Checked By: Scale:

C.E.C.I.

1"=50'

C.E.C.I.

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EROSION CONTROL DETAILS

HARBOR HILL RESIDENCES

SUMMER LEE DRIVE ROCKWALL, TEXAS

Project No. 18090

Sheet No.



Note: Storm sewer headwalls, wingwalls, box culverts and safety pipe runners shall be per Texas Department of Transportation Standard Details and made part of the City of Rockwall Standard Details.

TxDOT Standard Drawings

Drawings shall be modified as follows:

- 1. All concrete for structures shall be Class F (4200 psi, minimum 7.0 sack cement). 2. No fly ash is allowed in concrete for structures.

Downspout (SEE ARCH. PLANS) Transition Boot Wire screen for rodents " Maximum ▶ Proposed Grade 8" Schedule 40 -8" PVC on 1.00% Min. Slope Refer to drainage plan for size of storm sewer LL DOWNSPOUT COLLECTOR PIPE SHALL BE SCHEDULE 40 PVC.

> **DOWNSPOUT DETAIL** NOT TO SCALE

SAWCUT FULL DEPTH & CURB & GUTTER
9"-15" 1'-3" MIN. 24" LUBRICATED SMOOTH NO. 6 DOWEL BAR TOP 1/4" NO SEALING COMPOUND VERTICAL SAW CUT 3/8" MIN. HOT POURED RUBBER
JOINT SEALING COMPOUND PROPOSED PAVEMENT EXISTING PAVEMENT - DOWEL SPACED ON ONE (1) FOOT CENTER TO CENTER, 6 INCHES OFF TIE BARS. DOWELS AND REINFORCING BARS SHALL BE SUPPORTED BY AN APPROVED DEVICE. LONGITUDINAL BUTT JOINT

N.T.S. 1. NO. 5 SMOOTH DOWEL BAR MAY BE USED IN 5 INCH AND 6 INCH PAVEMENT THICKNESS. 2. LONGITUDINAL BUTT CONSTRUCTION MAY BE UTILIZED IN PLACE OF LONGITUDINAL HINGED (KEYWAY) JOINT AT CONTRACTORS OPTION.
3. DOWEL BARS SHALL BE DRILLED INTO PAVEMENT HORIZONTALLY

BY USE OF A MECHANICAL RIG.

4. DRILLED BY HAND IS NOT ACCEPTABLE, PUSHING DOWEL BARS INTO GREEN CONCRETE NOT ACCEPTABLE.

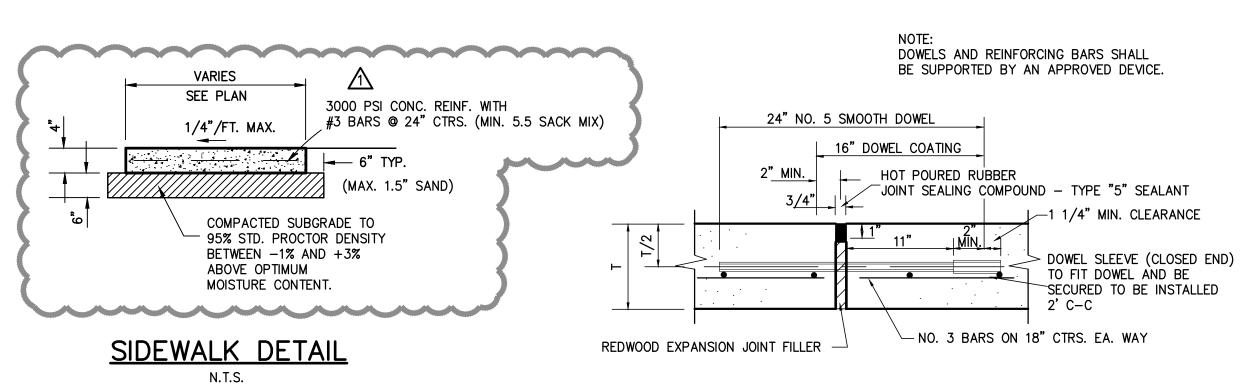
T= PAVEMENT THICKNESS VERTICAL SAW CUT HOT POURED RUBBER 3/8" WIDE MIN. JOINT SEALING COMPOUND TOP 1/4" NO TYPE "5" SEALANT SEALING COMPOUND 1 1/2"‡

MAXIMUM SPACING IS 15' CTRS. (TYP.)

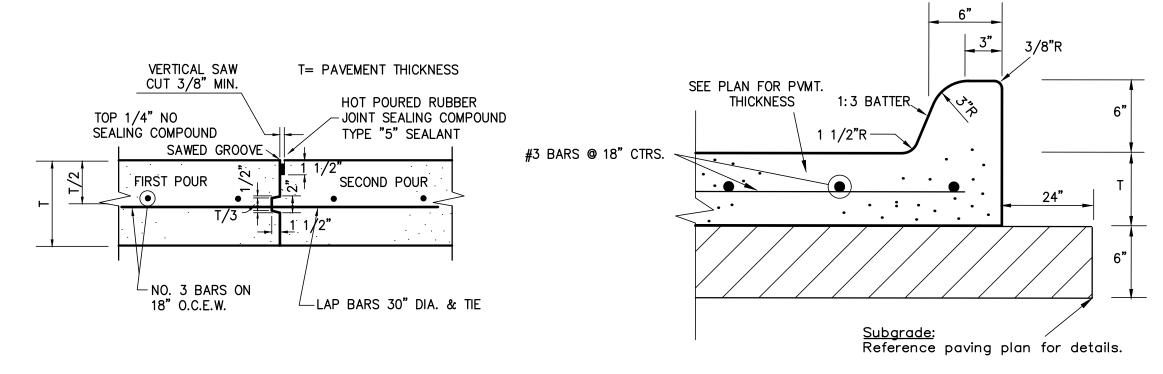
 lack NO. 3 BARS ON

18" CTRS. EA. WAY

SAWED DUMMY (CONTROL) JOINT



EXPANSION JOINT



CONSTRUCTION JOINT

PAVEMENT SECTION WITH CURB

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Cross Engineering Consultants
1720 W. Virginia Street
McKinney, Texas 75069 (972) 562-4409 Record Drawings Prepared On:

12/22/2023

Revised Sidewalk Details

11/30/2020

12/23/2020

01/22/2021

01/29/2021

04/19/2021

0 12/22/2023

Issued for Construction: 04-19-2021

Date: Issue Dates: Revisions: CROSS ENGINEERING CONSULTANTS 05/17/2021 01/28/2022 McKinney, Texas 75069 02/25/2022 1720 W. Virginia Street 972.562.4409 Texas P.E. Firm No. F-5935 10/31/2023 Checked By: Scale: Drawn By:

C.E.C.I.

C.E.C.I.

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CONSTRUCTION DETAILS HARBOR HILL RESIDENCES

SUMMER LEE DRIVE

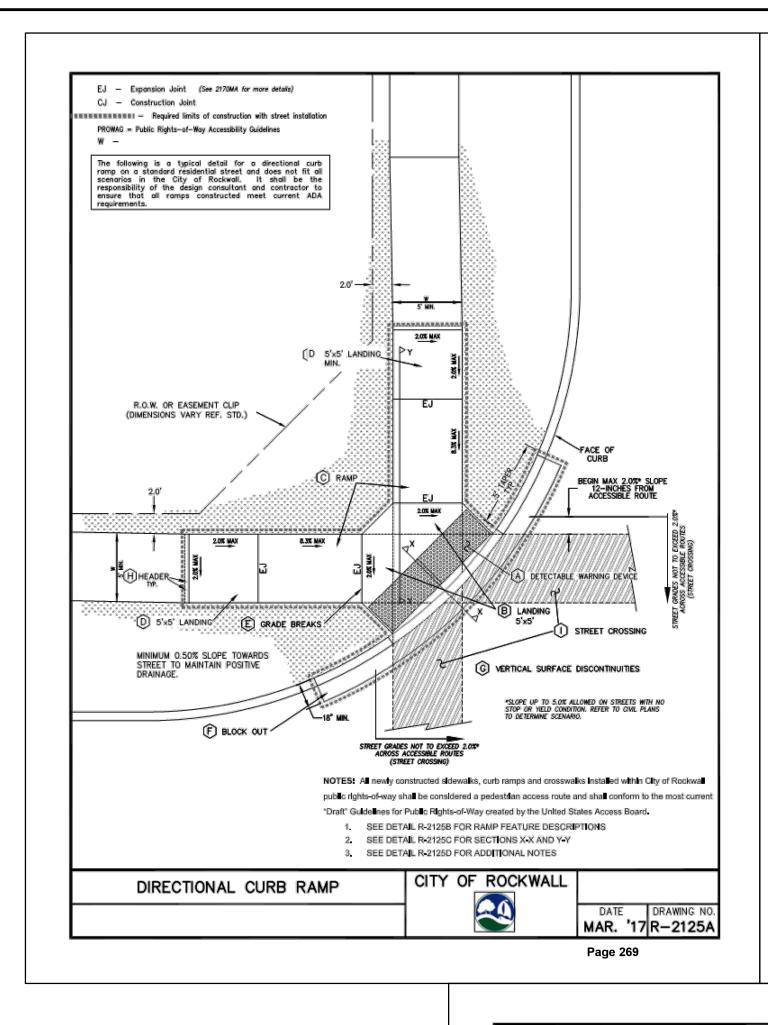
ROCKWALL, TEXAS

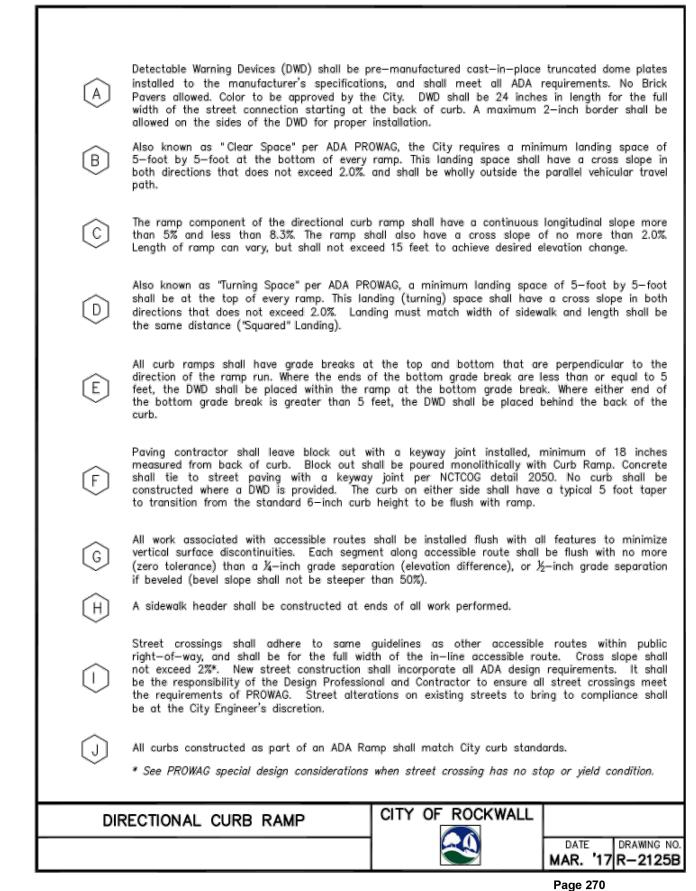
Project No. 18090

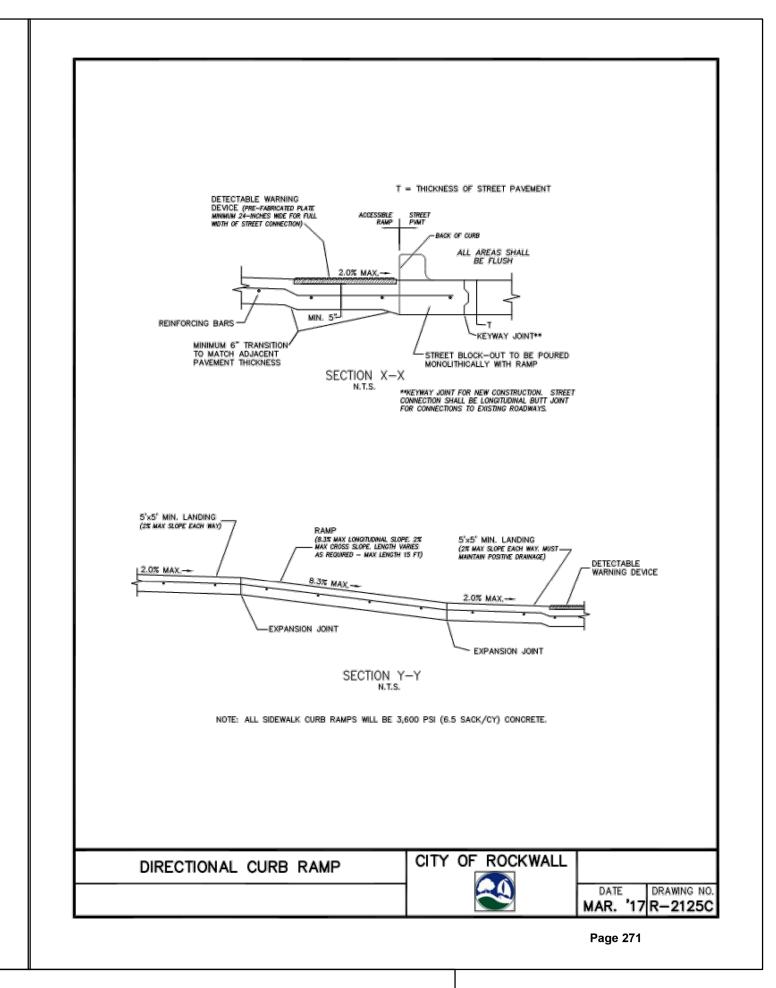
Sheet No.

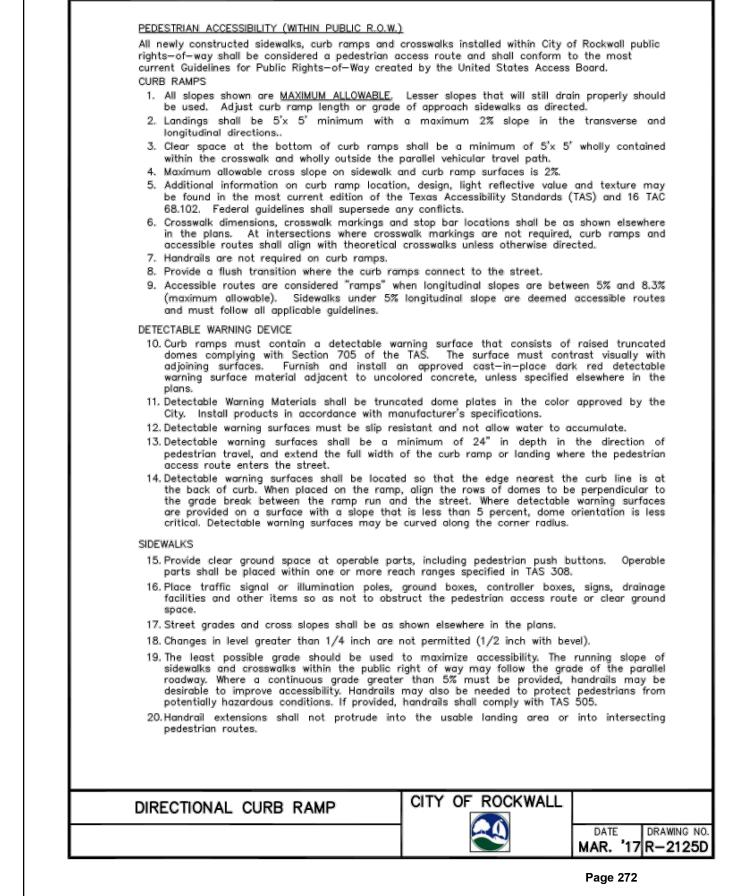
Note: All materials and construction shall be in accordance with the City of Rockwall Standard Specifications and Construction Standards, and Standard Specifications for Public Works Construction prepared by North Central Texas Council of Governments (Latest Revision), and to the regulations of The Texas Commission on Environmental Quality.

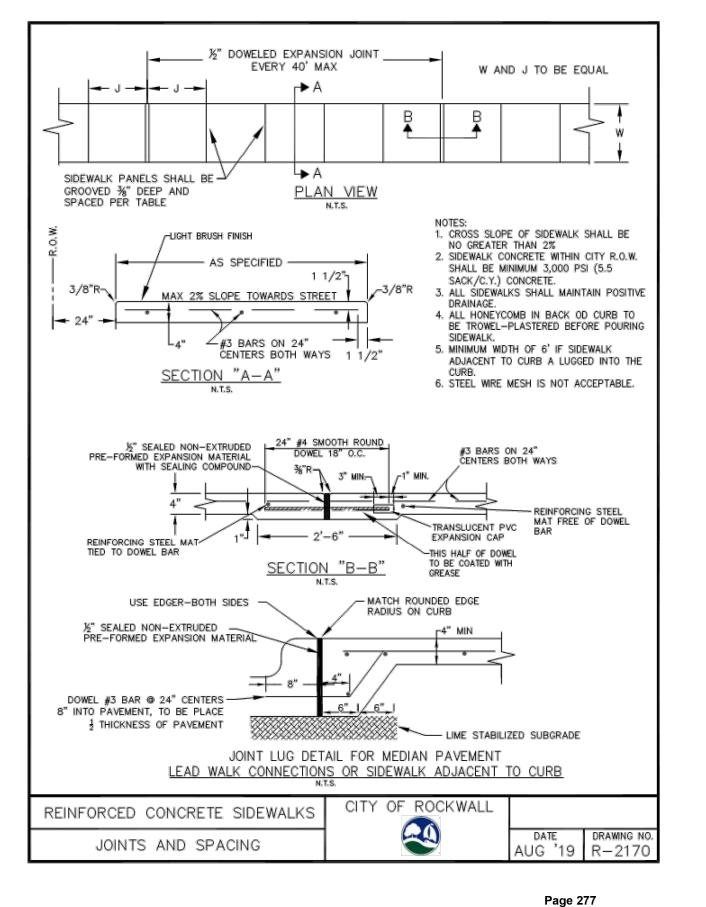
Page 343

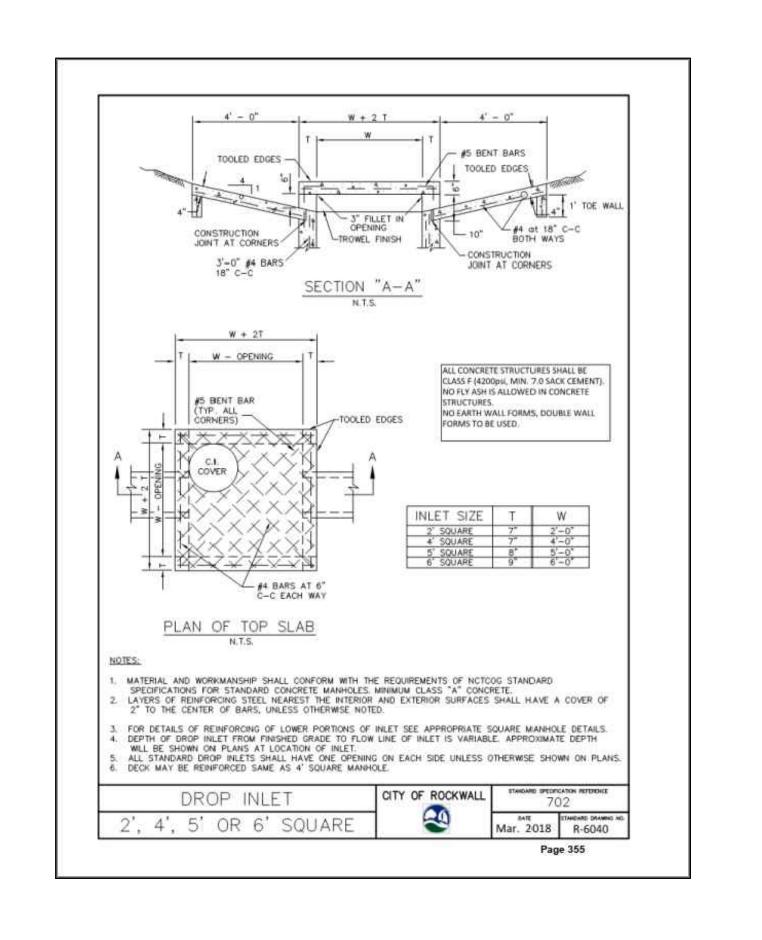












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Record Drawings Prepared On:

12/22/2023

	Issue Dates:	Revisions:	Date:				
5	11/30/2020	1	05/17/2021	CPC			NEERING
6	12/23/2020	2	01/28/2022		CO CO	NS	SULTANTS
7	01/22/2021	3	02/25/2022	1720 W. Virginia Str	reet		Kinney, Texas 75069
8	01/29/2021	4	10/31/2023	972.562.4409		Texa	s P.E. Firm No. F-5935
9	04/19/2021	5		Drawn By:	Checked By:		Scale:
10	12/22/2023	6		C.E.C.I.	C.E.C.I.		

Issued for Construction: 04-19-2021

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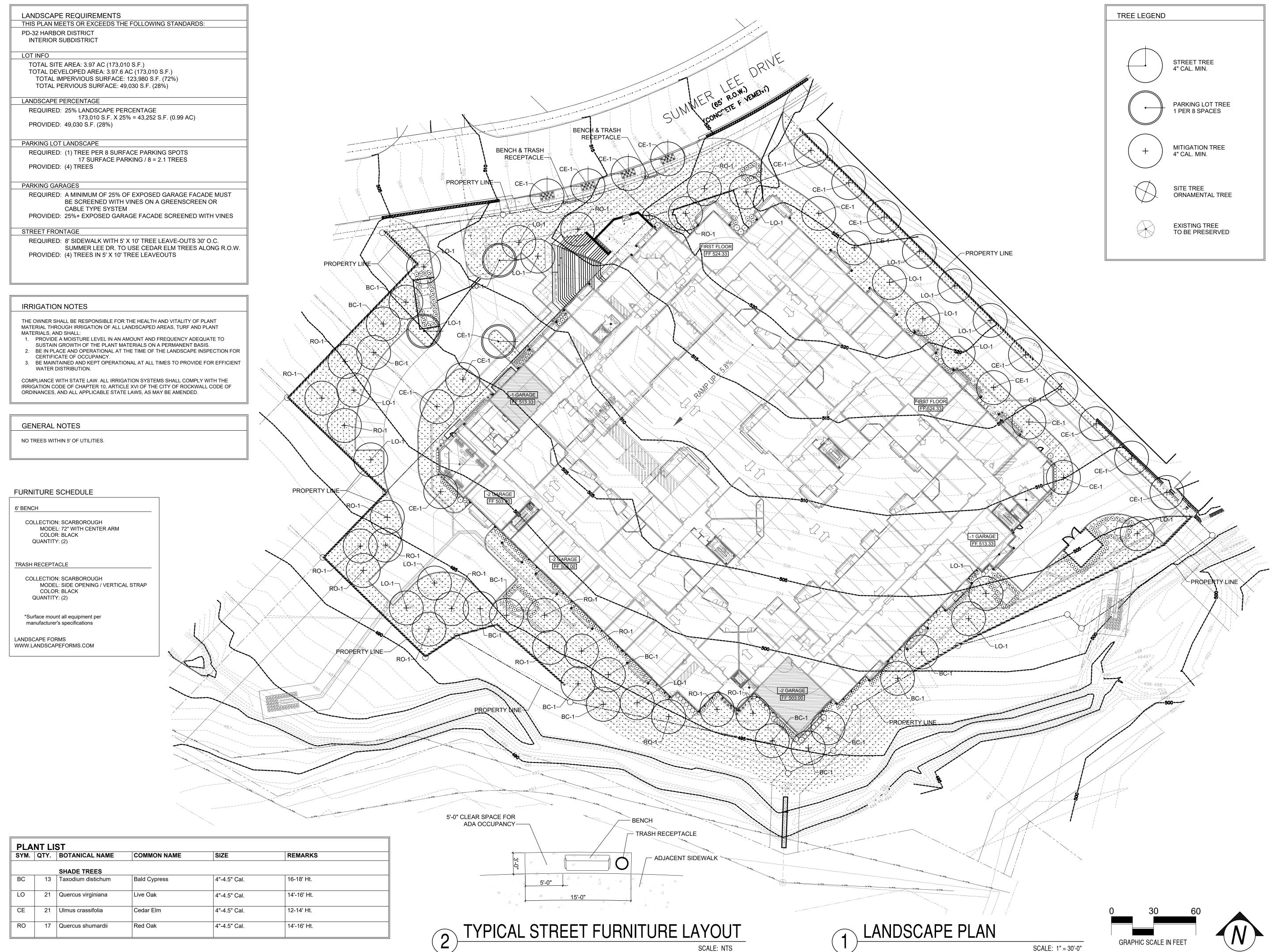
CONSTRUCTION DETAILS HARBOR HILL RESIDENCES

ROCKWALL, TEXAS

SUMMER LEE DRIVE

Project No. 18090

Sheet No.



ISSUES: 11-15-18

1 11-15-18 ISSUE FOR PERMIT
2 11-22-19 ISSUE FOR PERMIT REVIEW
3 12-22-23 AS-BUILT

REVISIONS:

12-04-18 PER CITY COMMENTS
2 05-27-20 PER CITY COMMENTS
2 10-02-20 GLENN HILL WAY REVISION

CLIENT:

DBA Architects 111 S. KENTUCKY SUITE 210 MCKINNEY, TX 75069

CONTACT: REUBEN MENDIOLA (800) 900-4905 reuben@dba-architects.com

HARBOR HILL

landscape architects

MEEKS DESIGN GROUP, INC. 1755 N. COLLINS BLVD., SUITE 300 RICHARDSON, TX 75080 PH (972) 690-7474 F (972) 690-7878



AS-BUILT

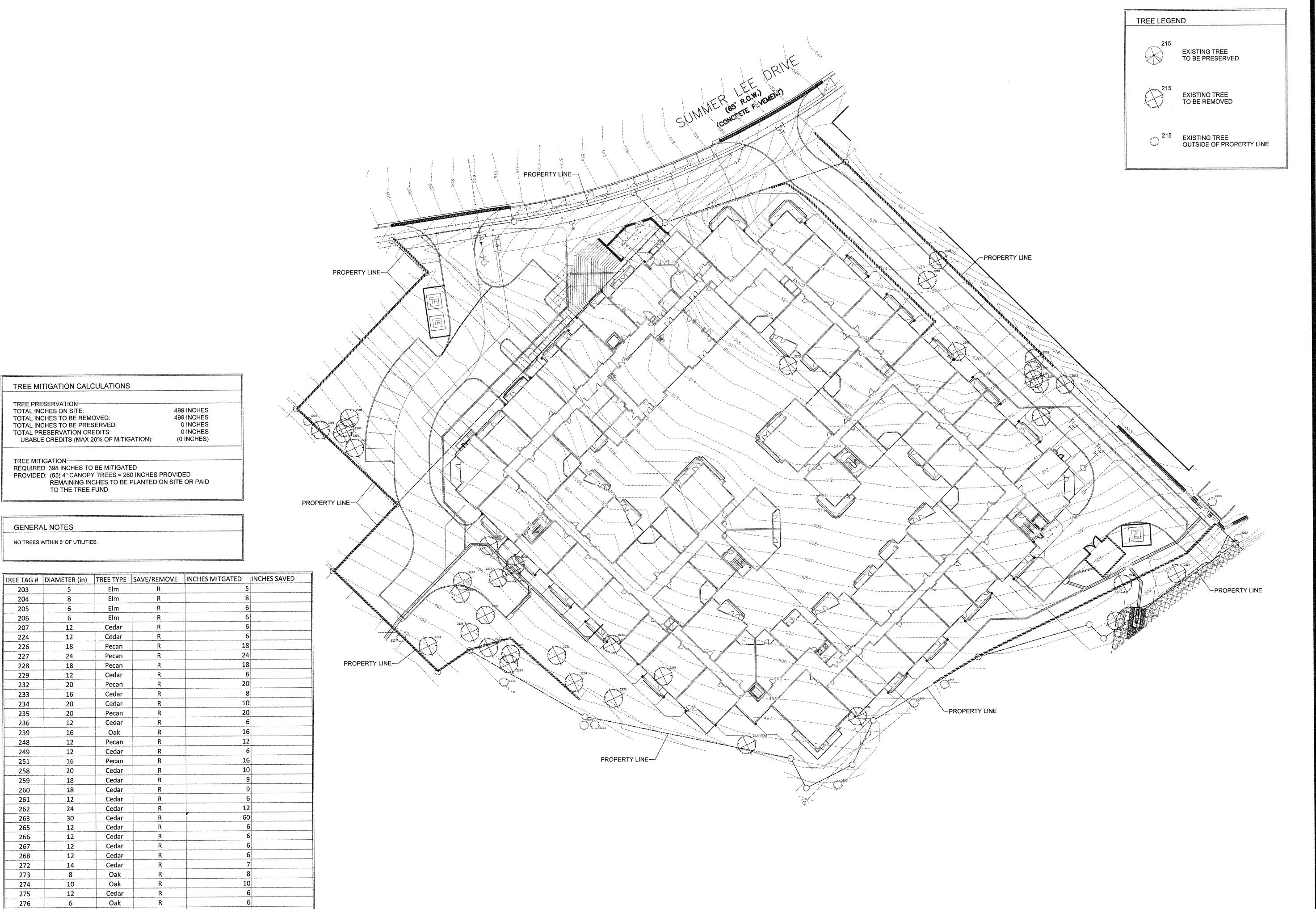
HARBOR HILL LUXURY
RESIDENCES

ROCKWALL, TEXAS

JOB NUMBER: DBA-1601

LANDSCAPE PLAN

LP0.01



TREE SURVEY/MITIGATION

TREE MITIGATION CALCULATIONS

499 INCHES 499 INCHES

0 INCHES

0 INCHES

(0 INCHES)

TREE PRESERVATION—

TOTAL INCHES ON SITE:

TREE MITIGATION-

GENERAL NOTES

203

204

205 206

207

224

226

227

228

229

232

233

234

235

236

239

248

251

258

259

260

262 263

265

267

268

272

274

275

277

278

279

NO TREES WITHIN 5' OF UTILITIES.

12

12

18

24

18

12

20

20

20

12

12

12

20

18

12

24

12

12

12

14

10

12

TOTAL INCHES TO BE REMOVED:

TOTAL PRESERVATION CREDITS:

TOTAL INCHES TO BE PRESERVED:

USABLE CREDITS (MAX 20% OF MITIGATION):

PROVIDED: (65) 4" CANOPY TREES = 260 INCHES PROVIDED

Elm Elm

Elm

Cedar

Cedar

Pecan

Pecan

Pecan Cedar

Pecan

Cedar

Cedar

Pecan

Cedar

Oak

Pecan

Cedar

Pecan

Cedar

Oak

Cedar

Oak

Oak

Oak

Oak

R

R

R

R

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398

TOTAL:

REMAINING INCHES TO BE PLANTED ON SITE OR PAID

REQUIRED: 398 INCHES TO BE MITIGATED

TO THE TREE FUND

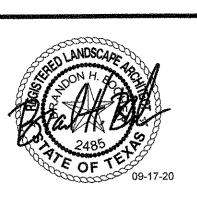
01-09-20 ISSUE FOR PERMIT 1 02-14-20

CLIENT:

DBA Architects 111 S. KENTUCKY SUITE 210 MCKINNEY, TX 75069

REUBEN MENDIOLA (800) 900-4905 reuben@dba-architects.com

architects MEEKS DESIGN GROUP, INC. 1755 N. COLLINS BLVD., SUITE 300 RICHARDSON, TX 75080 PH (972) 690-7474 F (972) 690-7878



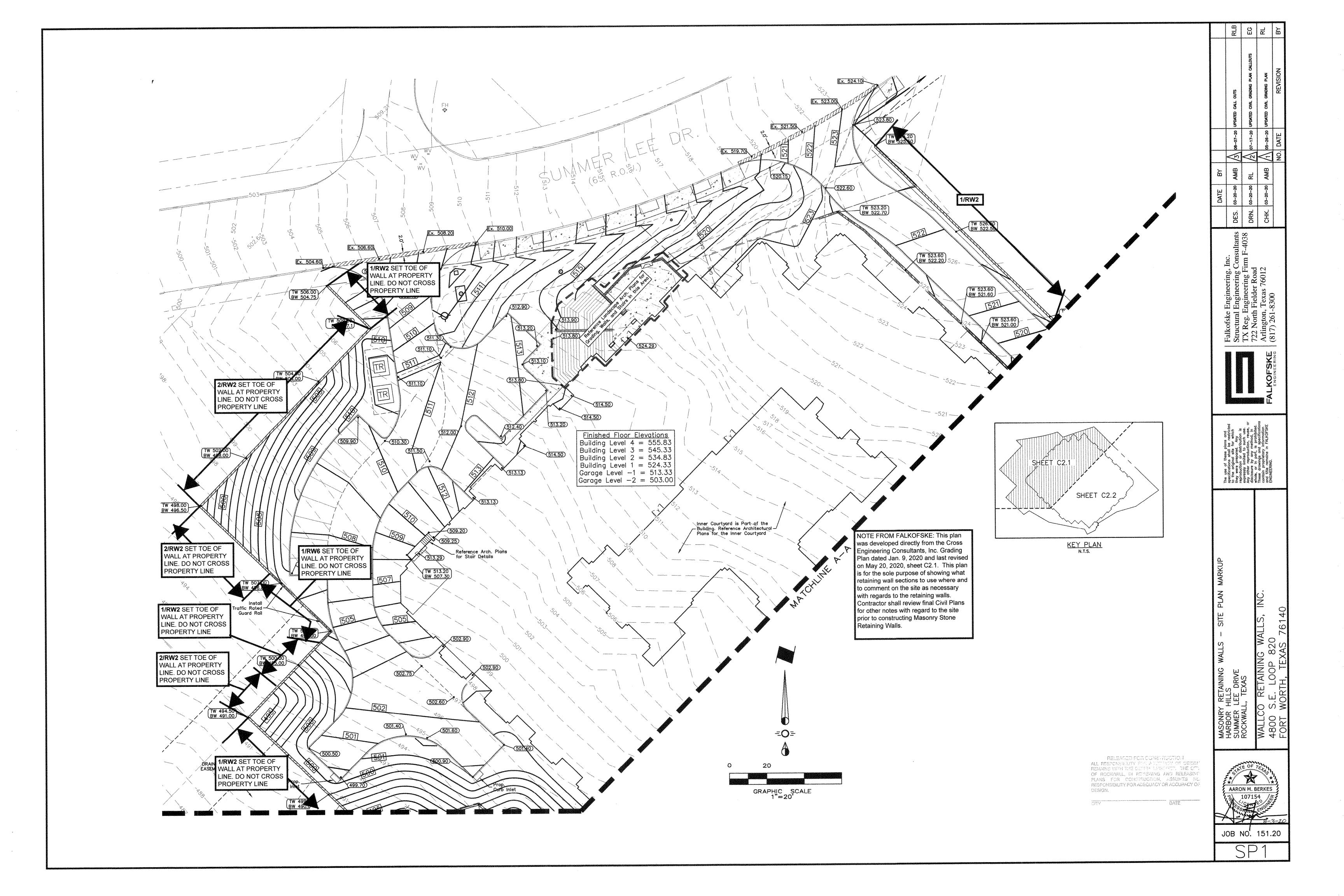
ISSUE FOR PERMIT

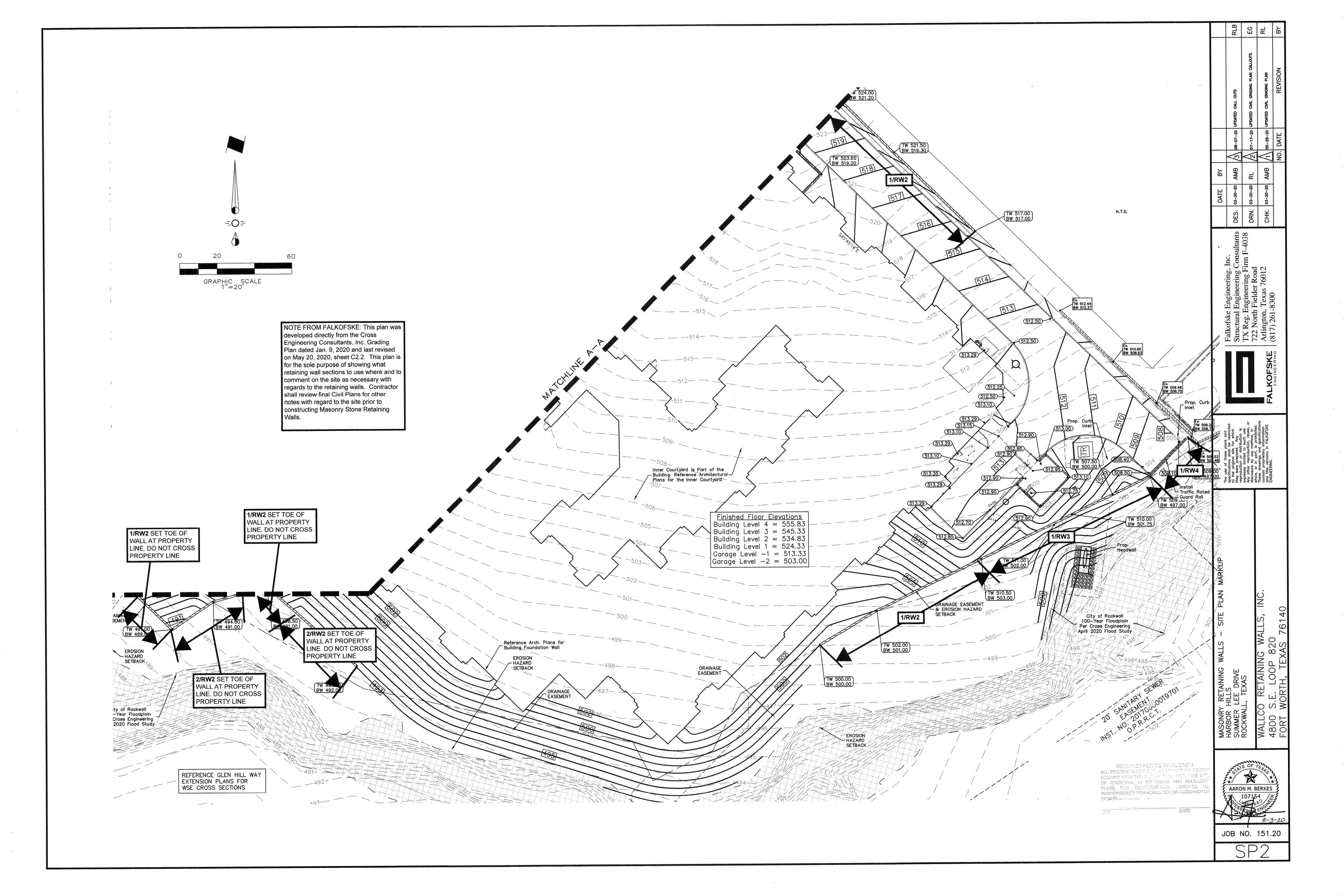
HARBOR HILL LUXURY RESIDENCES

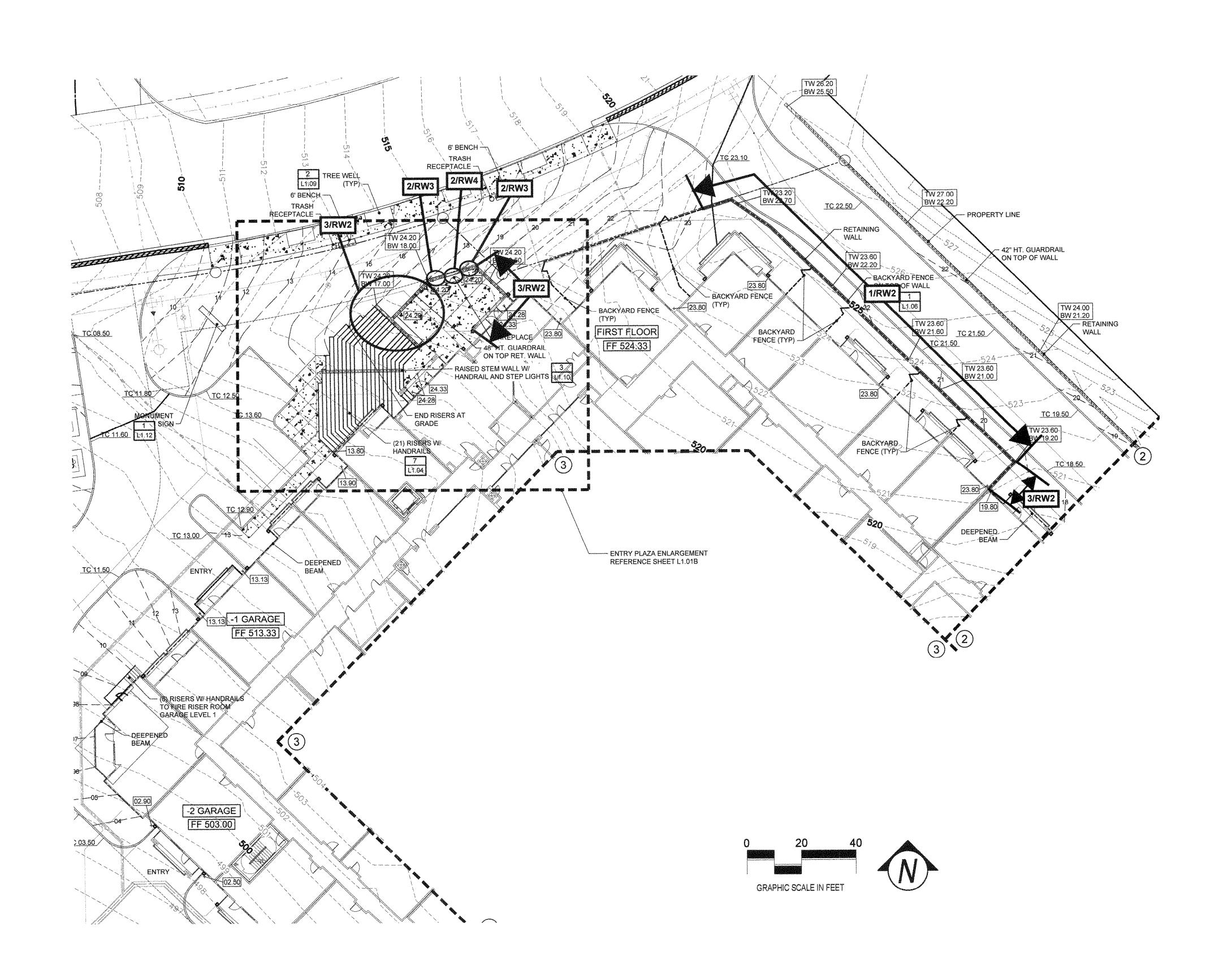
JOB NUMBER: DBA-1804

TREE SURVEY/ MITIGATION

TS-1







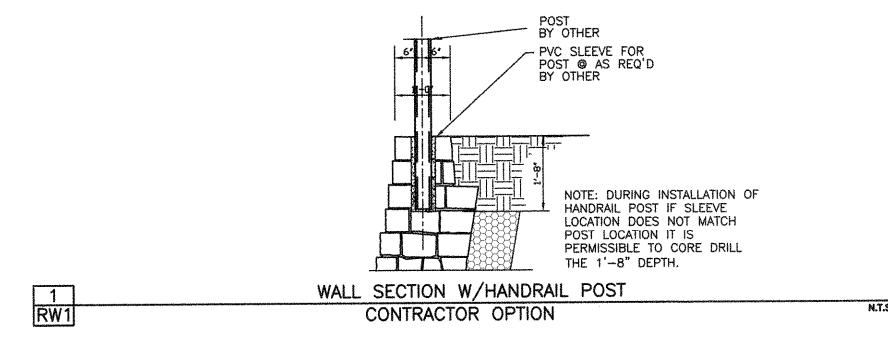
RELEASED FOR CONSTRUCTION
ALL RESPONSIBILITY FOR ALL THE CIT.
REMAINS WITH THE DILLEY FLAGVING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES THE RESPONSIBILITY FOR ADEQUACY OR ADCUPACY OF DESIGN.

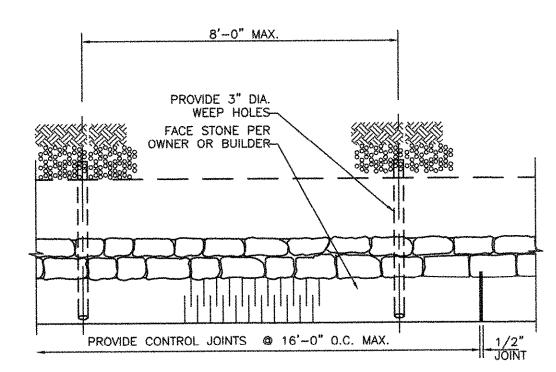
DATE

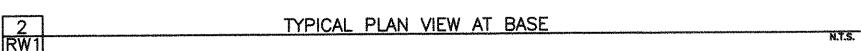
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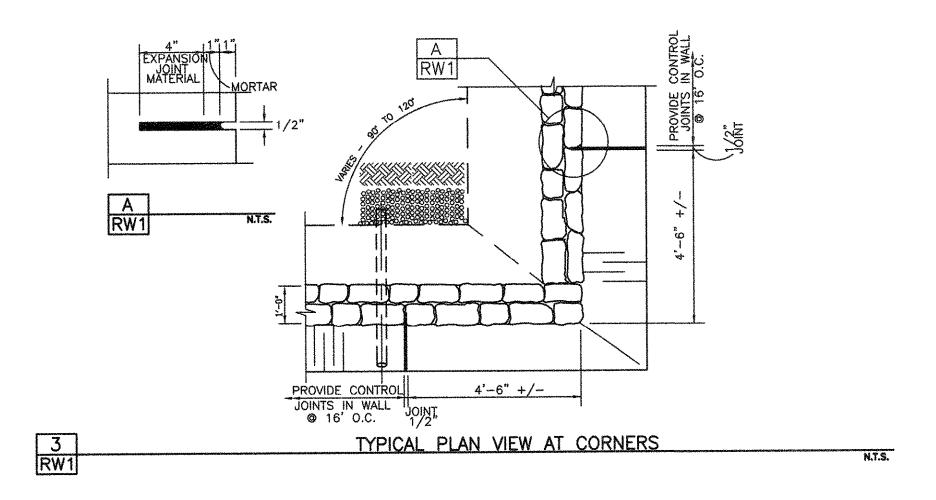
AARON M. BERKES

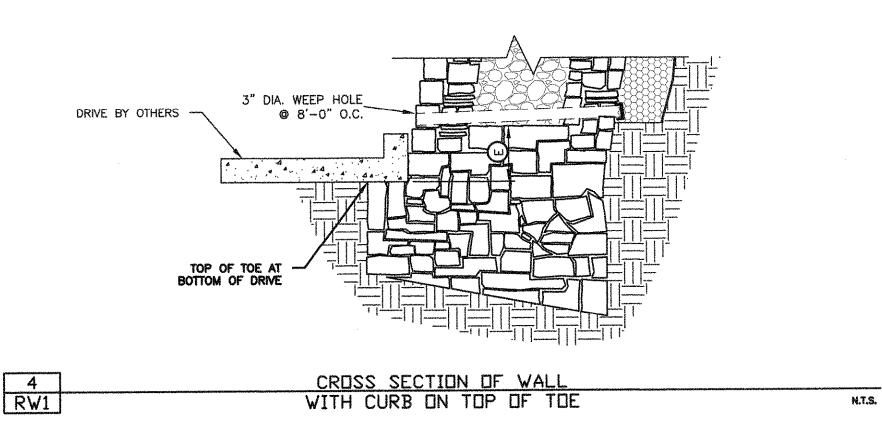
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GENERAL NOTES

1. Design

1.1. Design Codes

International Building Code, 2015 Edition

1.2. Geotechnical Report

Firm: RONE ENGINEERING, Inc Report No. 16-21084 Dated: May 12, 2016 Allowable Bearing Capacity _____1500 psf

1.3. Design Parameters

Soil Parameters:

Soil Type* Retained Backfill (On site clay) Foundation Soils (1500 psf)	Friction Angle	Cohesion (psf)	Unit Weight (pcf)
	26 deg	0 psf	120 pcf
	26 deg	0 psf	120 pcf
*See materials below for a description of each Soil Type.			

Factors of Safety:

External Stability a. Minimum Factor of Safety Against Base Sliding (Static Condition) b. Minimum Factor of Safety Against Overturning 2.0 c. Minimum Factor of Safety Against Global Stability 1.5

d. Minimum Factor of Safety for Bearing Capacity

Design Loading:

Lateral earth pressures are calculated using Coulombs Lateral Earth Pressure Theory. Designs have been performed to accept loading per the proposed loading conditions based on the Civil Grading Plans. A live loading of 250 psf has been used for all walls supporting areas subject to firelane loading.

3.0

Retaining walls should not have solid fence (such as wood fence) placed on top of wall other than that shown on these plans. Retaining walls shall not have additional surcharge placed above wall other than that shown on these plans. Retaining walls shall not have slope at base or top of wall that exceed that which is shown on these plans. The retaining walls noted above require special design.

2. Materials

2.1. Soil Types

- a. Retained Backfill a.a. On site clayey soils
- a.b. Properly compacted on-site fill soils, verification by others.

b. Foundation Soils (Allowable Bearing = 1500 psf min)

- b.a. Bearing on Stiff Natural Undisturbed Clayey or Sandy Soils or Compacted and Tested Fill Soils
- Friction Angle between Base of Wall and Soil 17 deg Bearing in fill soils. Fill soils supporting the retaining walls shall be placed in accordance with the recommendations for the Construction Requirements for temperatures between 40°F and 32°F: fill placement per the geotechincal report.
- c.a. Free draining granular backfill, clean, non-plastic, relatively well-graded.

2.2. Dimension Stone

- a. Average Density of masonry wall varies from 135pcf to 145pcf. b. Stone size varies from 4" to 18".
- c. Face stone shall be coordinated between contractor and owner/developer.
- d. Recycled concrete 4" to 18" may be used in place of dimension stone, contractors option.

2.3. Rebar/Welded Wire Fabric (If Required)

- a. All steel reinforcement shall be new billet steel conforming to ASTM A-615, Grade 60 with fy=60ksi.
- b. All reinforcement shall not have deleterious material on it. c. All welded wire fabric shall have minimum fy=65ksi and be hot dip galvanized.

- a. Weep pipes shall be PVC or corrugated HDPE pipe.
- b. Drainage zone shall be separated from retained backfill by mirafi 140N filter fabric or approved equal.

2.5 Portland Cement Mortar for Retaining Wall Construction.

The portland cement mortar used for construction of the masonry stone retaining walls shall be provided with the following proportions per cubic yard of concrete. The portland cement mortar supplier shall provide "batch tickets" clearly indicating that the appropriate amount of materials are provided in each truck load. The batch tickets shall clearly indicate the amount batched, the date, the project name and shall be provided to Falkofske Engineering, Inc. for review, documentation, and file.

Contents	Amount per	cubic yard	Specific Gravity	Volume ft^3
Type 1 Portland cement:	451	lbs	3.15	2.29
Type F Fly Ash		ibs	2.93	0.62
Fine Aggregate (sand):	2746	lbs	2.59	16.99
Potable Water	367	lbs	44 Gallons	5.88
Sika Air (or equivalent)	(AS R	EQ'D) oz	4.5%	1.22 27.0 Total

Note: the portland cement mortar supplier material weights may vary slightly based on the specific gravity of the materials used.

Concrete retarders may be used at the discretion of the masonry wall contractor. A greater amount of retarder is typically used during hot periods and a less amount of retarder is typically used during cool weather.

Please note that the above proportions will provide a portland cement mortar with a compressive strength of about f'c = 2500 psi. Falkofske Engineering, Inc. does not require any concrete testing provided the above proportions are verified by way of the "batch

3. Construction

3.1 Preparation Work

- a. Prior to grading or excavation of the site, confirm the location of the retaining walls and all underground features, including utility location within the area of construction. Ensure
- surrounding structures are protected from effects of wall excavation, and construction.
- b. Coordinate installation of underground utilities and other improvements with wall installation.

3.2 Excavation

- a. If a mortared footing is over-excavated, then the dimension stone shall be placed mortared. If a dry stone footing is over excavated, then the dimension stone does not need to be mortared.
- b. Fill over-excavated area in front of the wall footing with compacted on site soils before the wall construction exceeds 4 feet in height.
- c. In areas where the walls are installed in a cut, the required excavation shall extend horizontally to the extent of the width of the retaining wall. The wall may be built to the cut. If the wall is over cut, then soil shall either be compacted or the drainage zone may be widened.

3.3 Wall Construction

- a. The wall shall be constructed to the dimensions as shown on these plans. Front leads, back leads, and string lines shall be set for each wall. Care shall be taken to install the mortar zones the correct thickness, and to place drainage behind the wall as required.
- b. Control joints shall be installed at a maximum of 16'-0" o.c. per these plans. c. Weep pipes shall be placed at 8'-0" o.c. max.
- d. Face rock type shall be coordinated between the architect, owner, and retaining wall contractor.

3.4 Retained Backfill Placement

- a. Retained backfill shall be placed per the recommendations of the geotechnical engineer, but should not be less than 93% Standard Proctor Maximum Dry Density (ASTM D698).
- b. Fill should be placed in maximum 8" thick compacted lifts. c. Large compaction equipment (equipment heavier than 7,500 lb) shall remain a minimum of 1.5x the height of the wall away from the back of the wall for a period of 2 weeks from the time of
- d. After a period of 2 weeks from the time of construction large compaction equipment may be used
- behind the wall but shall stay a minimum of 5'-0" away from the back of the wall. e. Soil placed with in 5'-0" of the back of the wall shall be placed using handheld compaction
- f. If the wall is in a cut situation the wall may be built up to the cut. If the wall is overcut the drainage zone may be widened to the cut or compacted fill may be placed between the drainage zone and the cut.

3.6 Retaining Wall Performance, Maintenance, and Other Comments

- a. Control joints are provided in the retaining wall to allow for minor movements due to settlement and shrink swell of the soils. Some cracking may occur in the face of the retaining wall. This
- cracking, if minor (less than 3/8"), may be cosmetically repaired as desired. b. The retaining walls are designed to allow surface water to flow over the tops of the retaining walls. Care should be taken during and after construction to not allow water to pond behind the retaining walls, as this can have a negative impact on the stability of the retaining walls.
- c. If downspouts are located near the back of the retaining wall they should either be plumbed through the retaining wall to drain below the wall or collected and tied into the storm sewer
- system. Perforated subsurface pipes shall not be used behind the retaining walls. d. Positive drainage over the top of the walls shall be maintained throughout the life of the structure. If swales are placed behind the wall they shall remain clean and free draining. If water is found to
- be ponding in the swale it shall be fixed to allow water to freely drain as soon as possible. e. Any broken sprinklers behind the retaining wall shall be turned off and repaired as soon as

3.7 Cold Weather Construction of Retaining Walls

 a. Water and aggregates used in mortar shall not be heated above 140°F. b. Mortar sand or mixing water shall be heated to produce mortar temperatures between 40°F and 120°F at the time of mixing.

Construction Requirements for temperatures between 32°F and 25°F:

- a. The guidelines above for construction requirements for temperatures between 40°F and 32°F and the following shall be met.
- b. The mortar temperature shall be maintained above freezing until used in masonry stone retaining
- c. Visible ice and snow shall be removed from the top surface of existing foundations and masonry to receive new construction. These surfaces shall be heated to above freezing, using methods that do not result in damage.
- d. Newly constructed masonry shall be completely covered with weather-resistive membrane for 48 hours after being completed.

Construction Requirements for temperatures between 25°F and 20°F:

- a. The guidelines above for construction requirements for temperatures between 40°F and 32°F, the construction guidelines for temperatures between 32°F and 25°F, and the following shall be met. b. Masonry (raw stone) surfaces under construction shall be heated to 40°F.
- c. Wind breaks or enclosures shall be provided when the wind velocity exceeds 15 miles per hour. d. Newly constructed masonry shall be completely covered with weather-resistive insulating blankets, or equal protection, for 48 hours after being completed.

The above procedures comes from sections 2104.3.2.1, 2104.3.2.2, 2104.3.2.3, 2104.3.3.3, and 2104.3.3.4 of the International Building Code, and is in compliance with Masonry Standards Joint Committee recommendations for cold weather construction of masonry structures.

4. Construction Observations

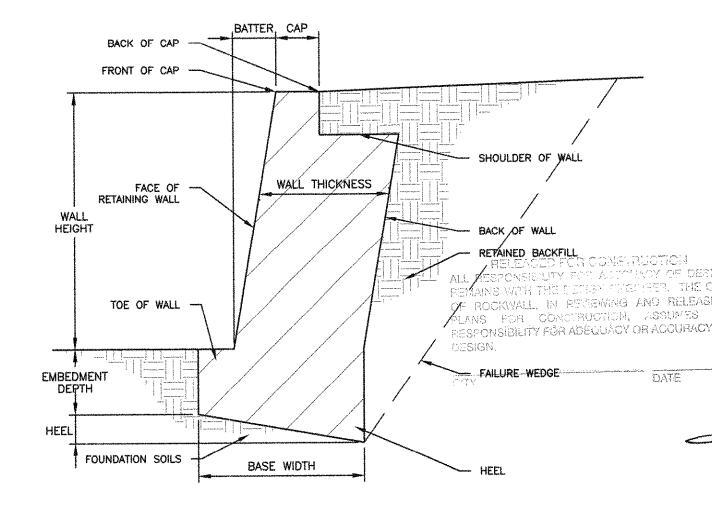
4.1 Construction Observations by Falkofske Engineering, Inc.

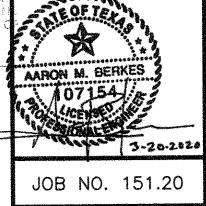
- a. Falkofske Engineering, Inc. will perform construction observation, but only as a means of verification of the contractors quality control performance.
- b. Falkofske Engineering, Inc. will act as the Special Inspector for this project. Contractor shall contact Falkofske Engineering to set up inspections, at least 1 day before construction starts.
- c. All required materials testing shall be performed by an approved materials testing
- d. Falkofske Engineering, inc. is not responsible for means, methods, and material furnished by the retaining wall contractor.

LEGEND

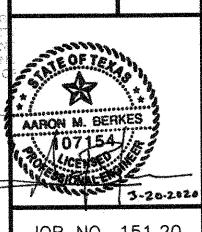
4.2 Construction Observations by Others

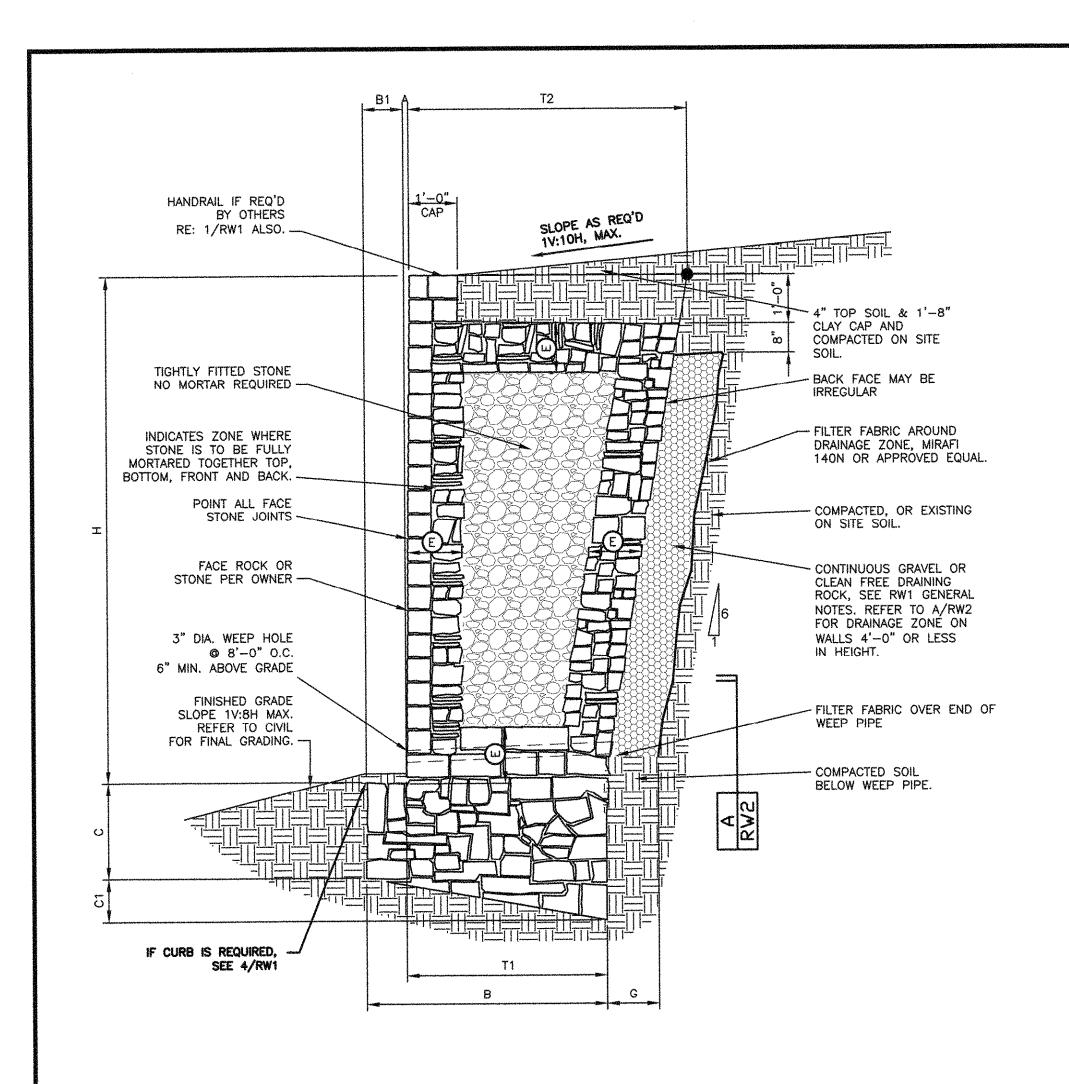
a. Construction observations as required by the city shall be coordinated by the contractor.

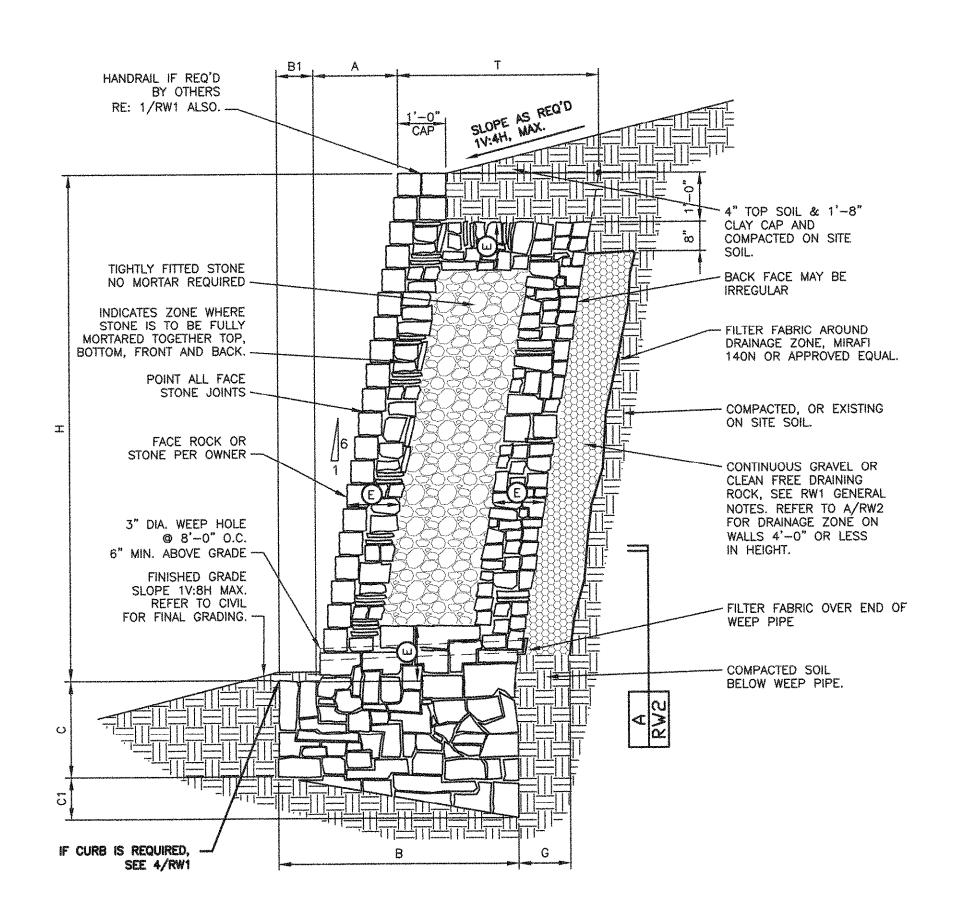


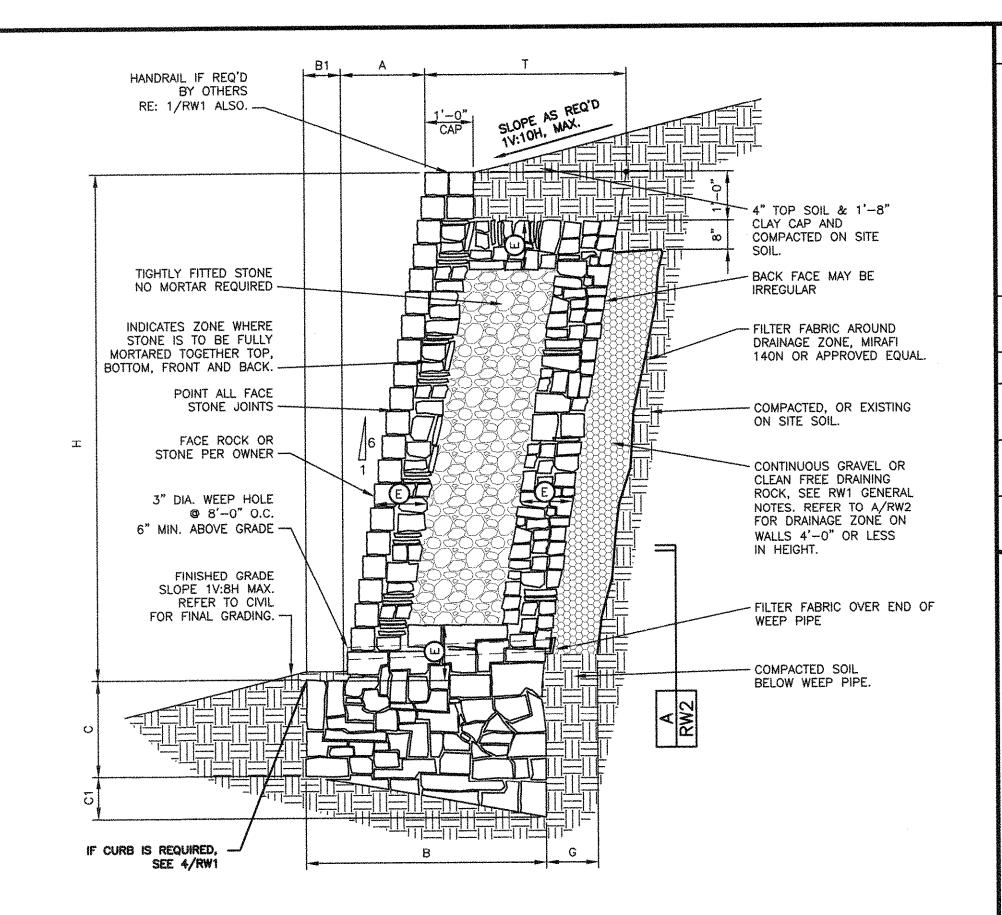


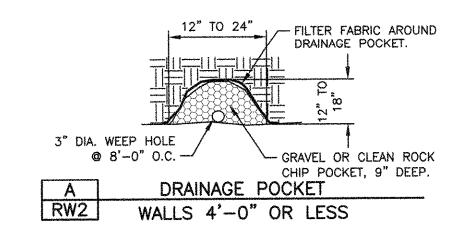
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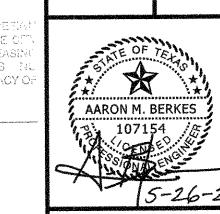


WALL HEIGHT H	Base Width B	70£ 81	BASE DEPTH (TOE) C	BASE DEPTH (HEEL) C1	BATTER A	MORAŽED E	THICKNESS OF WALL T1	THICKNESS OF WALL T2	Orainage zone Thickness G	BEARING CAPACIT
1'-0"	1'0"	0'-0"	0'-6"	0'-2"	1/4"	FULLY MORTARED	1'0"	1'-2"	SEE A/RW2	
2'-0"	1'-2"	0'-2"	0'-9"	0'-3"	1/2"	FULLY MORTARED	1'0"	1'-4"	SEE A/RW2	
3'-0"	1'-6"	0'-3"	0'-9"	0'-4"	3/4"	FULLY MORTARED	1'3"	1'-9"	SEE A/RW2	
4'-0"	2'-1"	0'-5"	1'-0"	0'-5"	1"	FULLY MORTARED	1'-8"	2'-4"	SEE A/RW2	1500 pt
5'-0"	2'-9"	0'-7"	1'-3"	0'-6"	0'-1 1/4"	0'8"	2'-2"	3'-0"	1'0"	
6'-0"	3'-5"	0'-10"	1'-6"	0'-8"	0'-1 1/2"	0'-10"	2'7"	3'-7"	1'-0"	
7'-0"	4'-0"	1'-0"	1'-9"	0'-9"	0'-1 3/4"	0'-10"	3'-0"	4'-2"	1'-0"	
8'-0"	4'-10"	1'-4"	2'-3"	0'-11"	0'-2"	1'-0"	3'-6"	4'-10"	1'-0"	
**************************************	Annual Control of the	WAL	L DESIGN C	RITERIA						
BEARING Qa	SLOPE TOP	SLOPE BOT	ACTIVE PRESSURE Фа	PASSIVE PRESSURE Pp	FRICTION ANGLE BASE Č	SLOPE OF BACK OF WALL	SURCHARGE 9			
1500PSF	5.71 deg	7.13 deg	26 deg	26 deg	17 deg	99.46 deg	0 psf			

	1600 pef -	BEARING CAF	MASONR ACITY (BARD)	y Wall Sch Iatural Undis		see genera	L NOTES SEL	ET RWI)	
WALL HEIGHT H	BASE WIDTH B	70€ 81	DEPTH (TOE)	BASE DEPTH (HEEL) C1	BATTER A	MOZONE E	THICKNESS OF WALL T	DRAINAGE ZONE THICKNESS G	BEARING CAPACITY
1'-0"	1'-0"	0'-0"	0'-6"	0'-2"	0'-2"	FULLY MORTARED	1'-0"	1'-0"	
2'-0"	1'-4"	0'-2"	0'-9"	0'-3"	0'-4"	WORTARED	1'-2"	1'0"	
3'-0"	1'9"	0'-3"	0,-9,	0'-4"	0'-6"	MORTARED	1'-6"	1'-0"	
4'-0"	2'-7"	0'-4"	1'-0"	0'-6"	0'-8"	FULLY MORTARED	2'-3"	1'-0"	1500 psf
5'-0"	3'-3"	0'-5"	1'-3"	0'-7"	0'-10"	0'-8"	2'-10"	1'0"	
6'-0"	4'-0"	0'-7"	1'-6"	0'-9"	1'-0"	0'-10"	3'-5"	1'0"	
7'-0"	4'-10"	0'-9"	1'-9"	0'-10"	1'-2"	0'-10"	4'-1"	1'0"	
8'-0"	5'-8"	0'-10*	2'-3"	1'-0"	1'-4"	1'-0"	4'-10"	1'0"	
9,-0,	6'-10"	0'-11"	2'-6"	1'-3"	1'-6"	1'-0"	5'-11"	1'-0"	
10'-0"	7'-7"	1'-0"	3'-0"	1'-5"	1'-8"	1'-2"	6'-7"	1'0"	1700 ps
11'-0"	8'-5"	1'1"	3'-6"	1'-6"	1'-10"	1'-2"	7'-4"	1'-0"	1900 ps
, a commune, a coperant construction confe	<u> Anna ann an an ann an an an an an an an a</u>	WAL	L DESIGN C	RITERIA			***************************************		
BEARING Go	SLOPE TOP	SLOPE BOT	ACTIVE PRESSURE Фа	PASSIVE PRESSURE Pp	FRICTION ANGLE BASE 5	SLOPE OF BACK OF WALL	SURCHARGE		
1500PSF	14 deg	7.13 deg	26 deg	26 deg	17 deg	99.46 deg	0 psf		

	1500 paf -	BEARING CAP		Y WALL SCH NATURAL UNDIS		SEE GENERA	L Notes se	EET RW1)	
HEIGHT HEIGHT	BASE WIDTH B	TOE B1	BASE DEPTH (TOE) C	BASE DEPTH (HEEL) C1	BATTER A	MORTANED ZON	THICKNESS OF WALL T	DRAINAGE ZONE THICKNESS G	BEARING CAPACITY
1'-0"	1'-0"	0'-0"	0'-6"	0'-2"	0'-2"	FULLY MORTARED	1'-0"	SEE A/RW2	
2'-0"	1'-2"	0'-2"	0'-9"	0'-3"	0'-4"	FULLY MORTARED	1'-0"	SEE A/RW2	
3'-0"	1'-7"	0'-3"	0'-9"	0'-4"	0'-6"	FULLY MORTARED	1'-4"	SEE A/RW2	
4'-0"	2'-3"	0'-4"	1'-0"	0'-5"	0'-8"	MORTARED	1'-11"	SEE A/RW2	
5'-0"	2'-9"	0'-5"	1'-3"	0'-6"	0'-10"	0'-8"	2'-4"	1'-0"	1500 psf
6'-0"	3'-5"	0'-7"	1'-6"	0'-8"	1'-0"	0'-10"	2'-10"	1'-0"	
7'-0"	4'-1"	0'-9"	1'-9"	0,-8,	1'-2"	0'-10°	3'-4"	1'-0"	
8'-0"	4'-11"	0'-11"	2'-3"	0'-11"	1'-4"	1'0"	4'-0"	1'-0"	
9'-0"	5'-11"	1'-1"	2'-6"	1'-1"	1'-6"	1'-0"	4'-10"	1'-0"	
10'-0"	6'-6"	1'-3"	3'-0"	1'-2"	1'-8"	1'-2"	5'-5"	1'-0"	1600 ps
11'-0"	7'-4"	1'-4"	3'-6"	1'-4"	1'-10"	1'-2"	6'-0"	1'-3"	1700 ps
,		WAL	L DESIGN C	RITERIA					
BEARING Qo	SLOPE TOP	SLOPE BOT	ACTIVE PRESSURE ©8	PASSIVE PRESSURE Φp	FRICTION ANGLE BASE 5	SLOPE OF BACK OF WALL	SURCHARGE q		
1500PSF	5.71 deg	7.13 deg	26 deg	26 deg	17 deg	99.46 deg	0 psf	1	

RELEAGED FOR CUNS INJURIES
ALL BESPONSESURY FUR AU 11107 OF D
REMAINS WITH THE CLUBBY LAR LITE. THE
OF ROCKWALL, IN REMEWING AND RELE
PLANS FOR CONSTRUCTION, ASSUMES
RESPONSIBILITY FOR ADEQUACY OR ACCURA
DESIGN.



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TYPICAL WALL SECTION - 1V:10H MAX SLOPE ABOVE WALL

1V:8H MAX SLOPE BELOW WALL BEARING IN CLAYEY OR SANDY SOILS

1 RW2

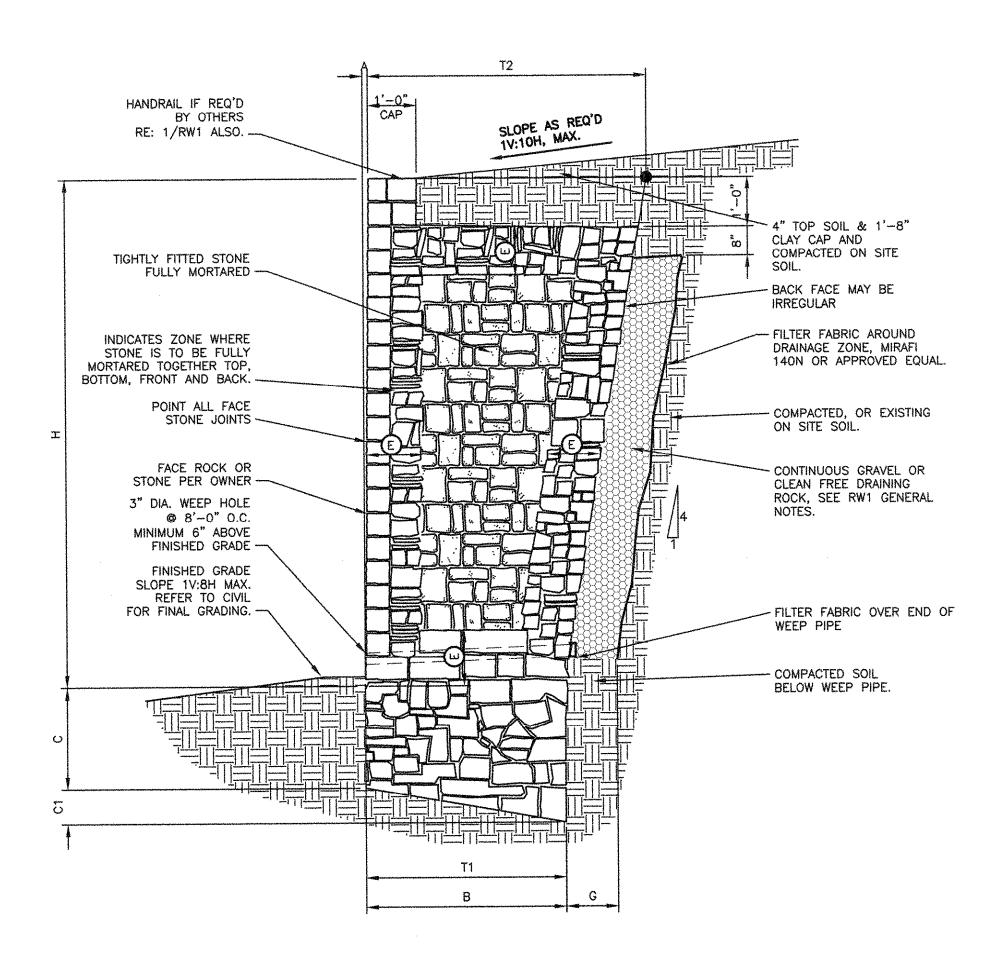
RW2

MAX. SLOPE ABOVE WALL 1V:4H MAX. SLOPE BELOW WALL 1V:8H

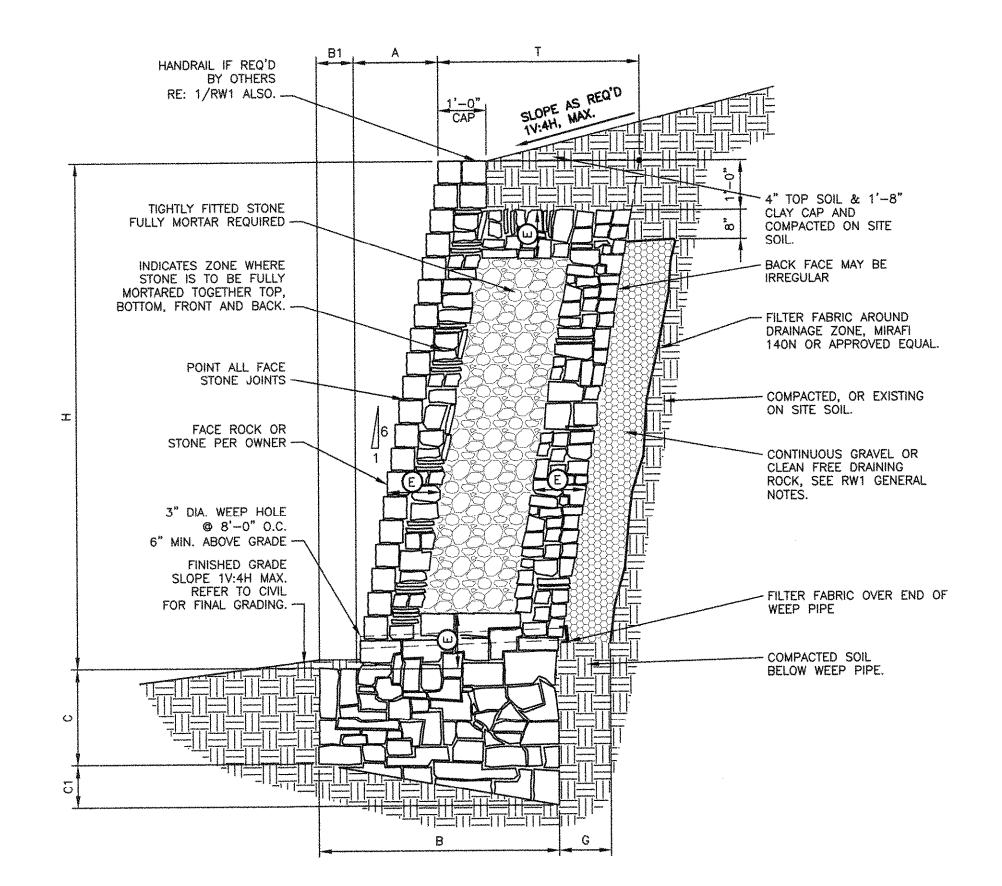
TYPICAL VERTICAL WALL SECTION - 1V:10H MAX SLOPE ABOVE WALL BEARING IN CLAYEY OR SANDY SOILS

TYPICAL WALL SECTION
BEARING IN CLAYS

JOB NO. 151.20



		2500 1 OR CO	ef - BEARING	ASONRY WA. CAPACITY (ST. PESTED SOILS	FF NATURAL	UNDISTURBED S NOTES SHEET	OUS RW1)			
WALL HEIGHT H	BASE WIDTH B	BASE DEPTH (TOE) C	BASE DEPTH (HEEL) C1	BATTER A	MONTANED ZONE E	THICKNESS OF WALL T1	THICKNESS OF WALL T2	Drainage zone Thickness G	BEARING CAPACIT	
4'-0"	2'-0"	1'-0"	0'-5"	0'-1"	FULLY MORTARED	2'-0"	3'-0"	1'-0"	1500 ps	
5'-0"	2'-6"	1'-3"	0'-6"	0'-1 1/4"	FULLY MORTARED	2'-6"	3'-9"	1'-0"	1300 pt	
6'-0"	3'-0"	1'-6"	0'-7"	0'-1 1/2"	FULLY MORTARED	3'-0"	4'-6"	1'-0"	1850 ps	
7'-0"	3'-6"	1'-9"	0'-8"	0'-1 1/2"	FULLY MORTARED	3'-6"	5'-3"	1'-0"	2150 p	
anakan yang kambabbah kabilan kabilan dalah k		WAL	L DESIGN C	RITERIA						
BEARING Qo	SLOPE TOP	SLOPE BOT	ACTIVE PRESSURE Фа	PASSIVE PRESSURE ©p	FRICTION ANGLE BASE O	SLOPE OF BACK OF WALL	SURCHARGE q			
1500PSF	5.71 deg	7.13 deg	26 deg	26 deg	17 deg	104.04 deg	0 psf	1		



Angele Selvin de Sel		1500 pef OR COMP	- BEARING C	SONRY WALL APACITY (STIFF STED SOILS SE	NATURAL UNI	S Disturbed soi Otes sheet ri	LS FI)		
H HEIGHT WALL	BASE WIOTH B	TOE 81	BASE DEPTH (TOE) C	BASE DEPTH (HEEL) C1	BATTER A	FULLY MORTARED ZONE E	THICKNESS OF WALL T	Drainage zone Thickness G	BEARING CAPACITY
5'-0"	3'-6"	0'-5"	3'-0"	0'-8"	0'-10"	0'-9"	3'-1"	1'0"	
6'-0"	4'-3"	0'-6"	3'-6"	0'-9"	1'0"	1'-0"	3'-9"	1'-0"	1500 ps
7'-0"	5'-2"	0'-7"	4'-0"	0'-11"	1'-2"	1'-0"	4'-7"	1'-0"	•
8'-0"	6'-1"	0'-8"	4'-6"	1'1"	1'-4"	1'3"	5'-5"	1'-0"	
9'-0"	7'-3"	0'-9"	5'-0"	1'-3"	1'-6"	1'-3"	6'-6"	1'-0"	
10'-0"	8'-1"	0'-10"	6'0"	1'-6"	1'-8"	1'-6"	7'-3"	1'0"	2000 ps
11'0"	9'-0"	1'-0"	7'-0"	1'-7"	1'-10"	1'-6"	8'-0"	1'-0"	
	3	WAL	L DESIGN C	RITERIA					
BEAKING Qo	SLOPE TOP	SLOPE BOT β1	ACTIVE PRESSURE 0a	PASSIVE PRESSURE	FRICTION ANGLE BASE Ö	SLOPE OF BACK OF WALL	SURCHARGE q		
1500psf	14 deg	14 deg	26 deg	26 deg	17 deg	99.46 deg	0 psf	1	

RELEASED FOR CONSTRUCTION
ALL RESPONSIBILITY FOR A DISTRICT OF DERIGN
REMAINS WITH THE CLOTHAN LACTURED. THE OTH
OF ROCKWALL, IN REMAINING AND RELEASING
PLANS FOR CONSTRUCTION, ASSUMES IN
RESPONSIBILITY FOR ADEQUACY OR ACCURACY OF
DESIGN.

CHTY

AARON M. BERKES

107154

5-26-20

HRN HRN HRN HRN

| Falkofske Engineering, Inc. | Structural Engineering Constracts Registration F-4038 | 722 North Fielder Road | Arlington, Texas 76012 | (817) 261-8300

JOB NO. 151.20

RW3

TYPICAL VERTICAL WALL SECTION - 1V:10H MAX SLOPE ABOVE WALL

RW3

BEARING IN CLAYEY OR SANDY SOILS

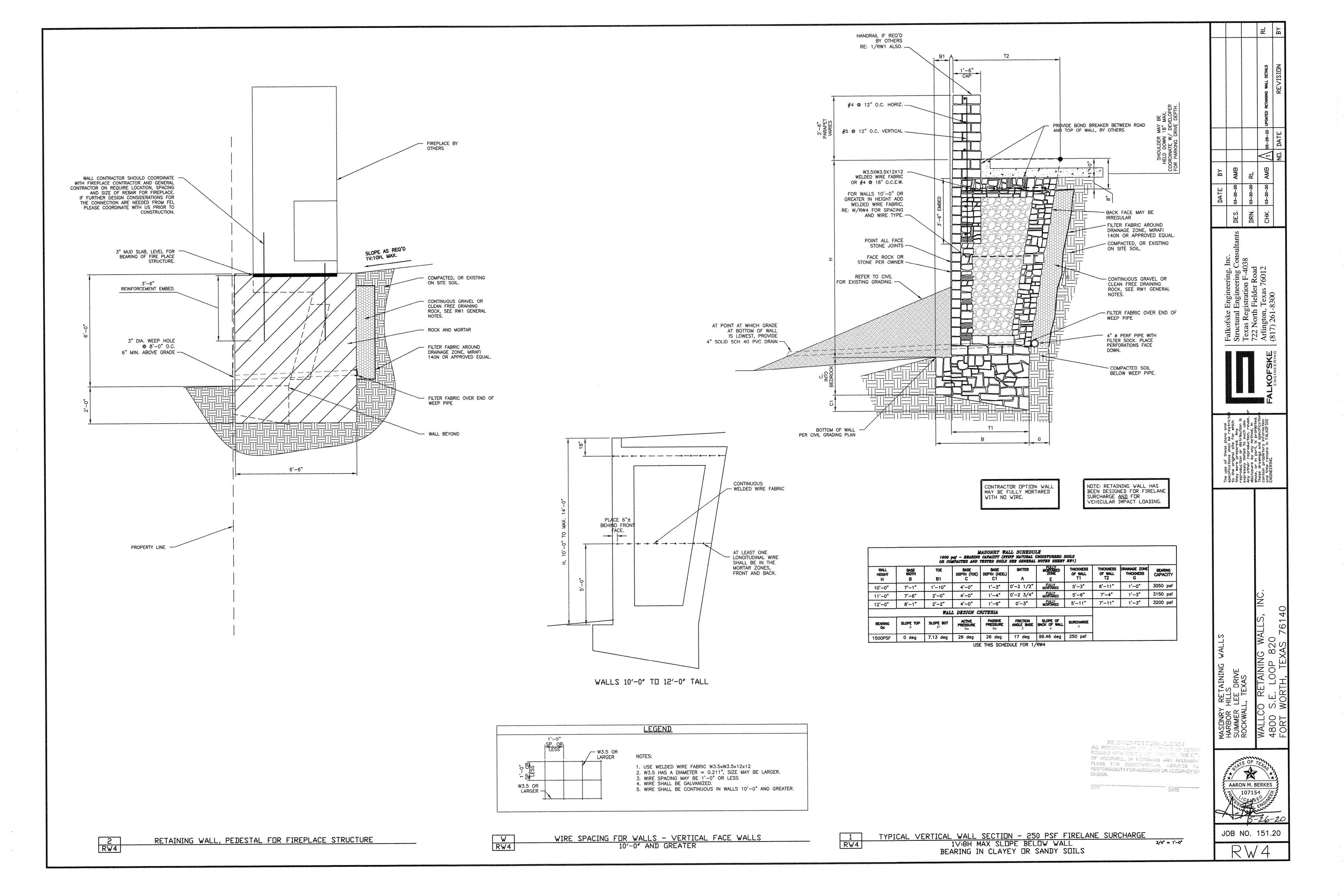
3/4" = 1'-0"

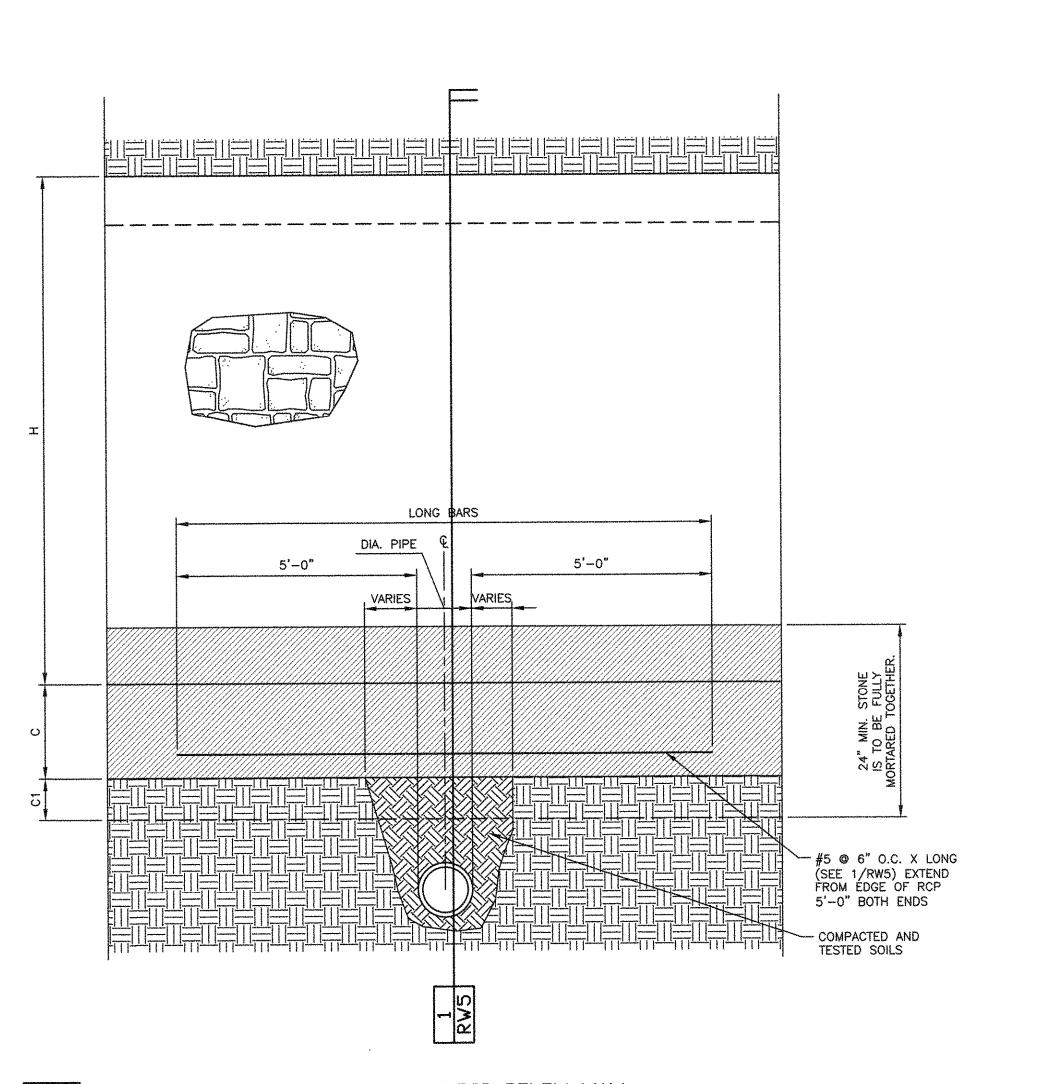
RW3

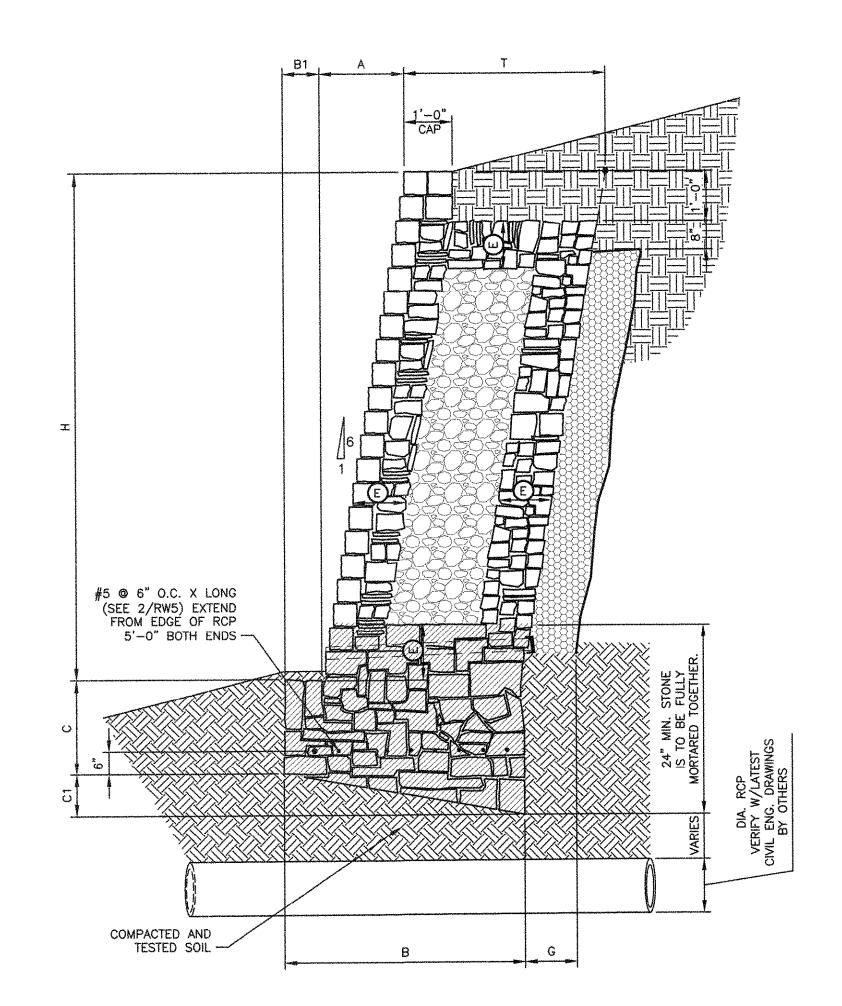
TYPICAL WALL SECTION - 1V:4H MAX SLOPE ABOVE WALL

1V:4H MAX SLOPE BELOW WALL

BEARING IN CLAYEY SOILS







RELEASED FOR CONSTRUCTION
ALL RESPONSIBILITY FOR AUTOMATA OF DERIGHT
REMAINS WITH THE CECEPY CARRESTED. THE OFF
OF ROCKYALL, IN RESEARING AND RELEASING
PLANS FOR CONSTRUCTION, ASSUMES IN
RESPONSIBILITY FOR ADEQUACY OF ACCURACY OF
DESIGN.

AARON M. BERKES

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O/cense

3-20-2020 JOB NO. 151.20

RW5

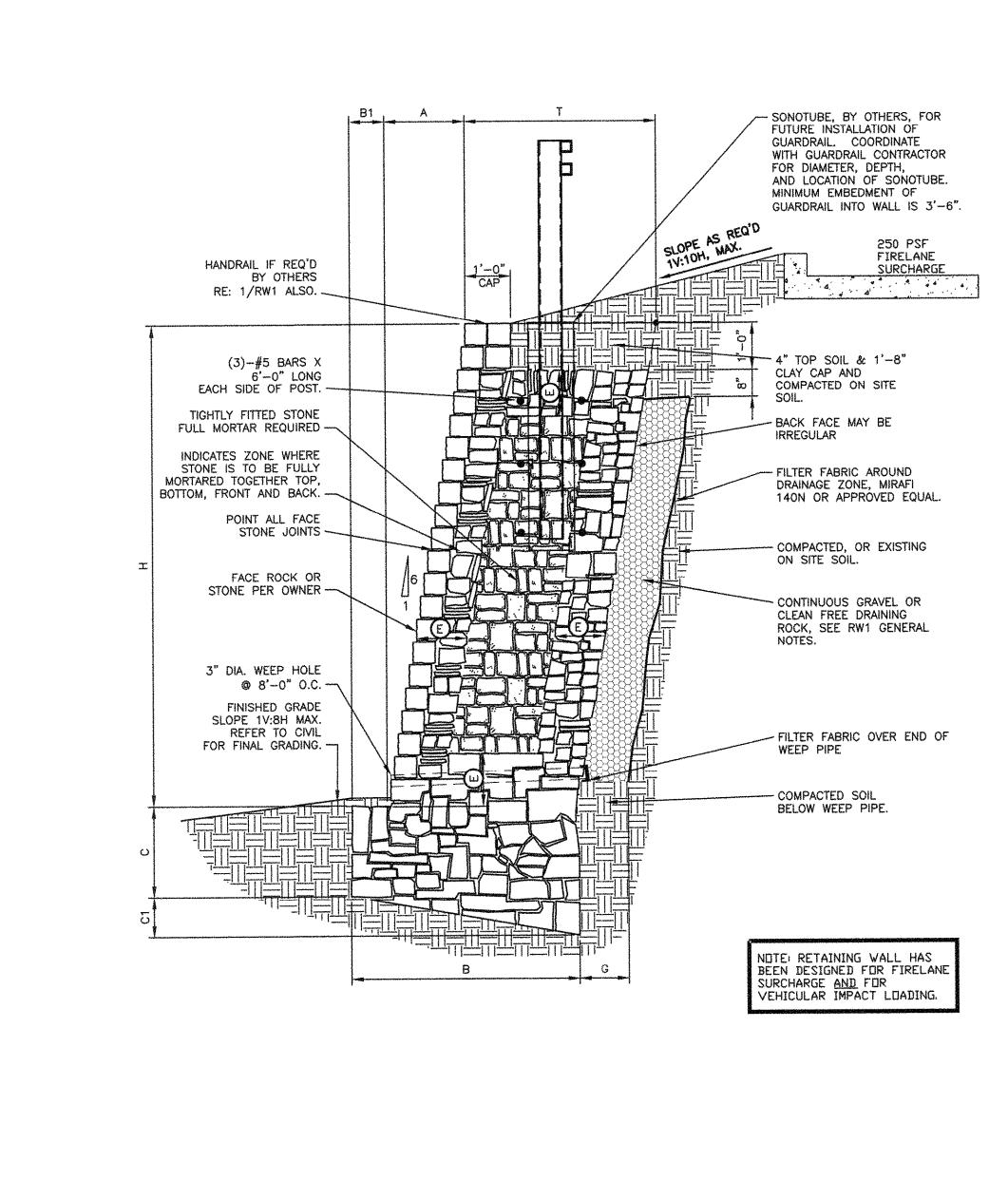
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1 RW5

WALL SECTION W/RCP BELOW WALL

2 RW5

WALL ELEVATION W/RCP BELOW WALL



		1500 OR CO	nef - BEARING	MASONRY W. CAPACITY (SI TESTED SOILS	IFF NATURAL	ULE UNDISTURBED L NOTES SHEET	SOUS RW6)		
WALL HEXGHT H	BASE WIDTH B	TOE B1	BASE DEPTH (TOE) C	BASE DEPTH (HEEL) C1	BATTER A	FULLY MORTARED ZONE E	THICKNESS OF WALL T	DRAINAGE ZONE THICKNESS G	BEARING CAPACITY
6'-0"	5'-3"	1'-6"	2'-0'	1'-0"	1'-0"	FULLY MORTARED	3'-9"	1'-0"	
7'-0"	5'-7"	1'-6"	2'-0"	1'-1"	1'-2"	MORTARED	4'-1"	1'-0"	1500 psf
8'-0"	6'-4"	1'-6"	2'-0"	1'-2"	1'4"	FULLY MORTARED	4'-11"	1'-0"	
9'-0"	7'-0"	1'-6"	2'-3"	1'-3"	1'-6"	MORTARED	5'-6"	1'-0"	
10'-0"	7'-6"	1'6"	2'-6"	1'-4"	1'-8"	FULLY MORTARED	6'-0"	1'-0"	1800 psf
11'-0"	8'-0"	1'-6"	2'-9"	1'-5"	1'-10"	MORTARED	6'6"	1'-3"	2000 psi
***************************************	Personne (mercens) produced and desired an	Y AL	L DESIGN C	RITERIA					
BEARING Qa	SLOPE TOP	SLOPE BOT	ACTIVE PRESSURE Фа	PASSIVE PRESSURE PP	FRICTION ANGLE BASE	SLOPE OF BACK OF WALL a	SURCHARGE		
1500PSF	0 deg	7.13 deg	26 deg	26 deg	17 deg	99.46 deg	250 psf		

AARON M. BERKES DATE

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TYPICAL WALL SECTION — 250PSF FIRELANE SURCHARGE

1V:10H MAX SLOPE ABOVE WALL, 1V:8H MAX SLOPE BELOW WALL

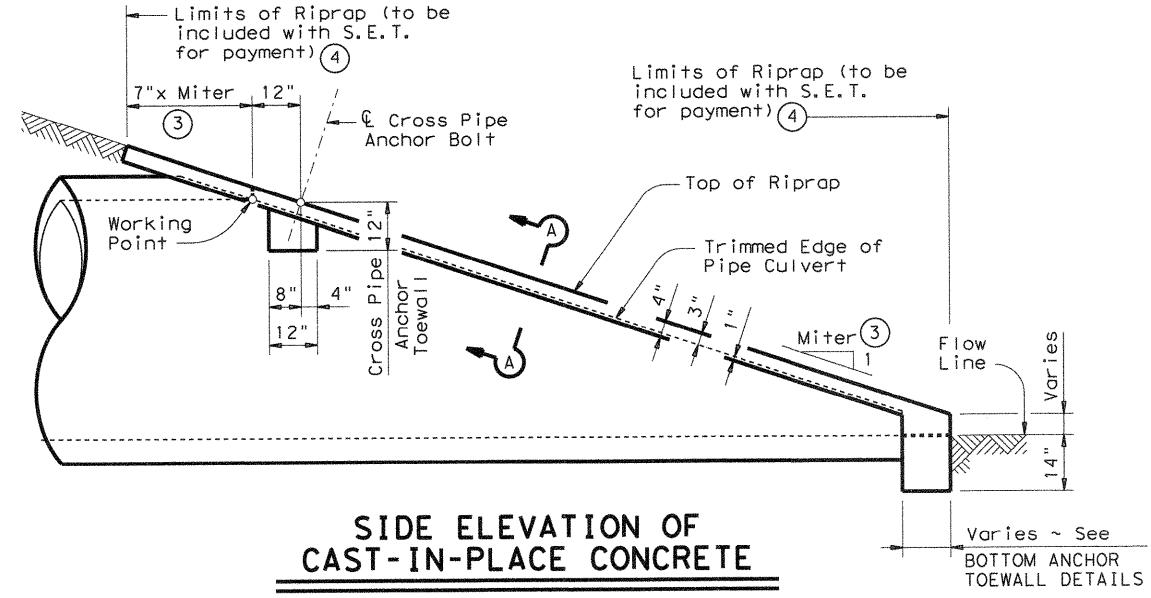
BEARING IN CLAYEY SOILS

RELEAGED FOR CONSTRUCTION
ALL RESPONSIBILITY FOR ALL DIVINITY OF DERIGN
REMAINS WITH THE DIDDER CONSTRUCT
OF ROCKWALL, IN REVIEWING AND RELEASING
PLANS FOR CONSTRUCTION, ASSUMES NO
RESPONSIBILITY FOR ADEQUACY OF ACCUPACY OF
DESIGN.

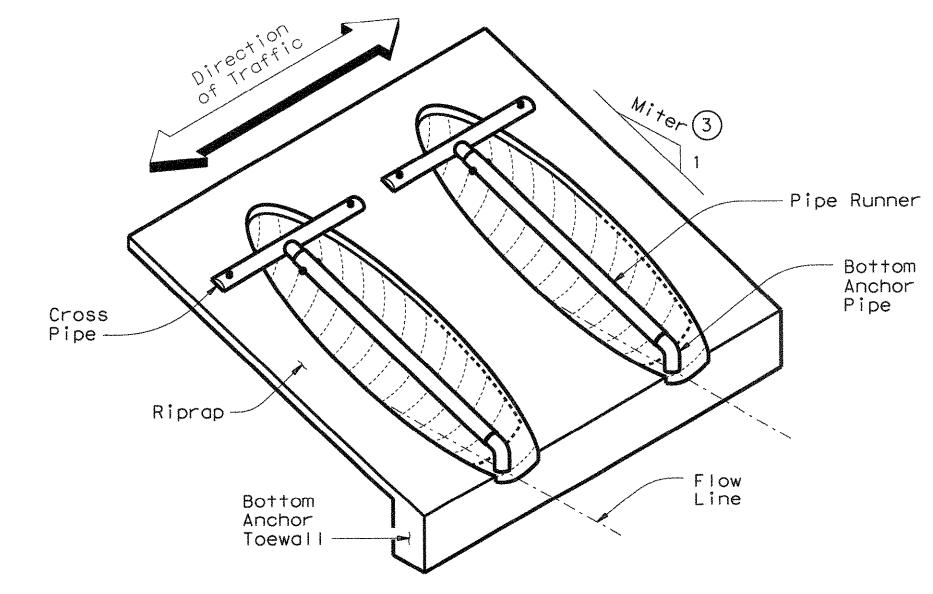
NOTE: All Pipe Runners, calculations, and dimensions are based on the pipe culverts mitered as shown in this detail. Alternate styles of mitered ends will require that appropriate adjustments be made to the values presented on this standard.

SIDE ELEVATION OF TYPICAL PIPE CULVERT MITER

(Showing Corrugated Metal Pipe Culvert. Details of Concrete Pipe Culvert are similar.)



(Showing Concrete Pipe Culvert.
Details of Corrugated Metal Pipe Culvert are similar.
Pipe Runners not shown for clarity)



ISOMETRIC VIEW OF TYPICAL INSTALLATION

(Showing installation with no skew.)

CROSS PIPE LENGTHS & PIPE RUNNER LENGTHS 12

				•	the second se									
		Cross		enger som generalisetet av kalende her blev state det sette state som state state som state som state state st	and deposit in the second s			Pipe Runn	er Length					
Nominal Culvert	Pipe Culvert	Pipe		3:1 Sic	de Slope	31177		4:1 Sic	de Slope			6:1 Sic	e Slope	
I.D.	Spa ~ G	Length	0° Skew	15° Skew	30° Skew	45° Skew	0° Skew	15° Skew	30° Skew	45° Skew	0° Skew	15° Skew	30° Skew	45° Skew
24"	1'- 7"	3'- 5"	N/A	N/A	N/A	5′-10"	N/A	N/A	N/A	8'-1"	N/A	N/A	N/A	12'- 9"
27"	1'-8"	3′ - 8″	N/A	N/A	5' - 5"	6'-11"	N/A	N/A	7' - 7"	9'- 7"	N/A	N/A	11'-11"	14'-11"
30"	1'-10"	3'-11"	N/A	N/A	6'-4"	8'-0"	N/A	N/A	8'-9"	11'- 0"	N/A	N/A	13' - 8"	17'- 0"
33"	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	4'- 2"	6'-2"	6′ - 5"	7′ - 3"	9'-1"	8' - 6"	8'-10"	10'- 0"	12'- 5"	13'- 3"	13'- 9"	15'- 5"	19' - 2"
36"	2'-1"	4' - 5"	l 6'-11"	7′ - 3"	8'-2"	10' - 2"	9'- 6"	9′-11"	11'- 2"	13'-10"	14'- 9"	15' - 3"	17'- 2"	21'- 3"
42"	2'- 4"	4'-11"	8'-6"	8'-10"	9'-11"	12' - 4"	11'- 7"	12'- 0"	13' - 6"	16' - 8"	17'- 9"	18' - 5"	20' - 8"	25' - 7"
48"	2'- 7"	5' - 5"	10'-1"	10'- 5"	11'- 9"	N/A	13'- 7"	14'- 2"	15′-10"	N/A	20' - 9"	21'- 6"	24' - 2"	N/A
54"	3'-0"	5′-11"	11'- 8"	12'-1"	N/A	N/A	15'- 8"	16' - 3"	N/A	N/A	23′-10"	24' - 8"	N/A	N/A
60"	3'- 3"	6' - 5"	13'- 3"	N/A	N/A	N/A	17'- 9"	N/A	N/A	N/A	26′-10"	N/A	N/A	N/A

TYF	PICAL P	IPE CULV	ERT MIT	ERS ③	CONDI	TIONS WHERE PII	PE RUNNERS 2	STA MAX	NDARD P PIPE RU	IPE SIZ NNER LE	ES & 1 NGTHS
Side Slope	0° Skew	15° Skew	30° Skew	45° Skew	Nominal Culvert I.D.	Single Pipe Culvert	Multiple Pipe Culverts	Pipe Size	Pipe O.D.	Pipe I.D.	Max Pipe Runner Length
3: 1	3:1	3,106:1	3, 464: 1	4.243:1	12" thru 21"	Skews thru 45°	Skews thru 45°	2" STD	2.375"	2.067"	N/A
4:1	4:1	4.141:1	4.619:1	5.657:1	24"	Skews thru 45°	Skews thru 30°	3" STD	3.500"	3.068"	10'- 0"
6: 1	6: 1	6.212:1	6.928:1	8,485:1	27"	Skews thru 30°	Skews thru 15°	4" STD	4.500"	4.026"	19' - 8"
	***************************************		And the state of t		30"	Skews thru 15°	Skews thru 15°	5" STD	5.563"	5.047"	34' - 2"
			**************************************		33"	Skews thru 15°	Always required				
			ernomen en processor de la companya		36"	Normal(No Skew)	Always required				
massizabet kasamat kenn ausa apa gampan opan logist sapit pinnahi da massi da bersoci kradekt.		***************************************			42" to 60"	Always required	Always required				

ESTIMATED CONCRETE RIPRAP QUANTITIES (CY) (5)

Nominal		3:1 Sic	le Slope			4:1 Sid	le Slope		6:1 Side Slope			
Culvert I.D.	0° Skew	15° Skew	30° Skew	45° Skew	0° Skew	15° Skew	30° Skew	45° Skew	0° Skew	15° Skew	30° Skew	45° Skew
12"	0.4	0.4	0.5	0.5	0.5	0,5	0.5	0.6	0.7	0.7	0.7	0.8
15"	0.5	0,5	0.5	0.6	0.6	0.6	0.6	0.7	0.7	0.7	0.8	0.9
18"	0.5	0.5	0.6	0.6	0.6	0.7	0.7	0.8	0.8	0.8	0.9	1.0
21"	0.6	0.6	0.6	0.7	0.7	0.7	0.8	0.9	0.9	0.9	1.0	1.2
24"	0.6	0.7	0.7	0.8	0.8	0.8	0.8	1.0	1.0	1.0	1.1	1.3
27"	0.7	0.7	0.8	0.9	0.8	0.9	0.9	1 1	1,1	1.1	1.2	1,4
30"	0.8	0.8	0.8	0.9	0.9	0.9	1.0	1.2	1.2	1.2	1.3	1.6
33"	0.8	0.8	0.9	1.0	1.0	1.0	1.1	1.3	1.3	1.4	1.5	1.7
36"	0.9	0.9	0.9	1.1	1.1	1 1	1.2	1.4	1.4	1.5	1.6	1.8
42"	1.0	1.0	1.1	1.3	1.2	1.3	1.3	1.6	1.6	1.7	1.8	2.1
48"	1 1	1.1	1.2	N/A	1.4	1.4	1,5	N/A	1.9	1.9	2.1	N/A
54"	1.3	1.3	N/A	N/A	1.6	1.6	N/A	N/A	2.1	2.1	N/A	N/A
60"	1.4	N/A	N/A	N/A	1.7	N/A	N/A	N/A	2.3	N/A	N/A	N/A

- Size of Pipe Runner shall be as shown in the tables. Cross Pipe shall be the same size as the Pipe Runner. Cross Pipe Stub Out and Bottom Anchor Pipe shall be the next smaller size pipe as shown in the STANDARD PIPE SIZES table.
- 2 This standard allows for the placement of only one pipe runner across each culvert pipe opening. In order to limit the clear opening to be traversed by an errant vehicle, the following conditions must be met:

For 60" culvert pipes, the skew must not exceed 0°. For 54" culvert pipes, the skew must not exceed 15°. For 48" culvert pipes, the skew must not exceed 30°. For all culvert pipe sizes 42" and less, the skew must not exceed 45°.

If the above conditions cannot be met, the designer should consider using a safety end treatment with flared wings. For further information, refer to the TxDOT "Roadway Design Manual".

- (3) Miter = Slope of Mitered Pipe Culvert End
- 4) Riprap placed beyond the limits shown will be paid as Concrete Riprap in accordance with Item 432, "Riprap".
- Quantities shown are for one end of one reinforced Concrete Pipe Culvert. For multiple Pipe Culverts or for Corrugated Metal Pipe Culverts, quantities will need to be adjusted. Riprap quantities are for Contractor's information only.

- 1. All Concrete for Structure Shall be Class F (4200 psi 7.0 sack Cement).
- 2. No Fly Ash is Allowed for Structures.

SHEET 1 OF 2

Texas Department of Transportation SAFETY END TREATMENT

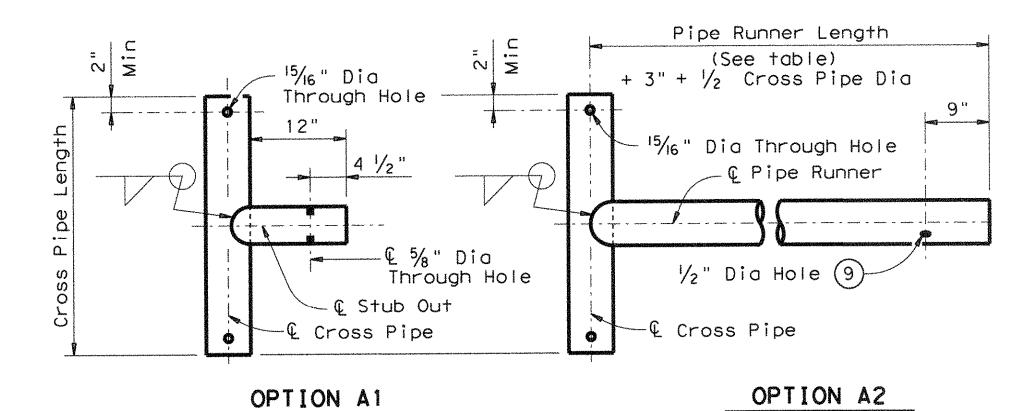
FOR 12" DIA TO 60" DIA PIPE CULVERTS TYPE II ~ CROSS DRAINAGE

SETP-CD

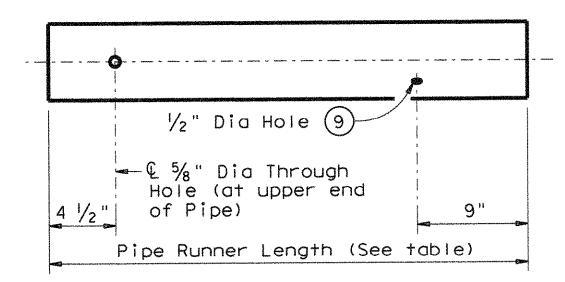
Bridge Division Standard

ILE: setpcdse.dgn	DN: GAI	E	CK:	CAT	DW:	JRP	CK: GAF
CTxDOT February 2010	CONT	SECT		J0B		f	HIGHWAY
REVISIONS							
11-10: Add note for synthetic fibers.	DIST			COUNT	/		SHEET NO.
Synthesia 1020. u.							

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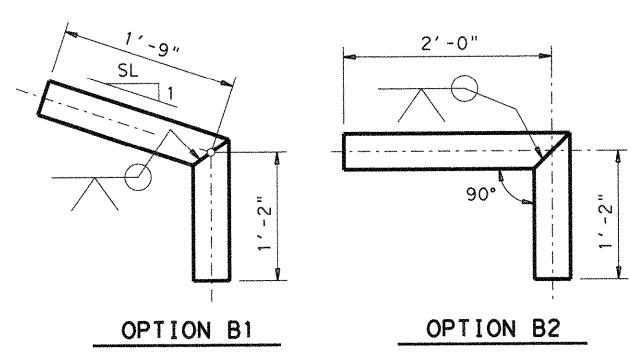


CROSS PIPE AND CONNECTIONS DETAILS

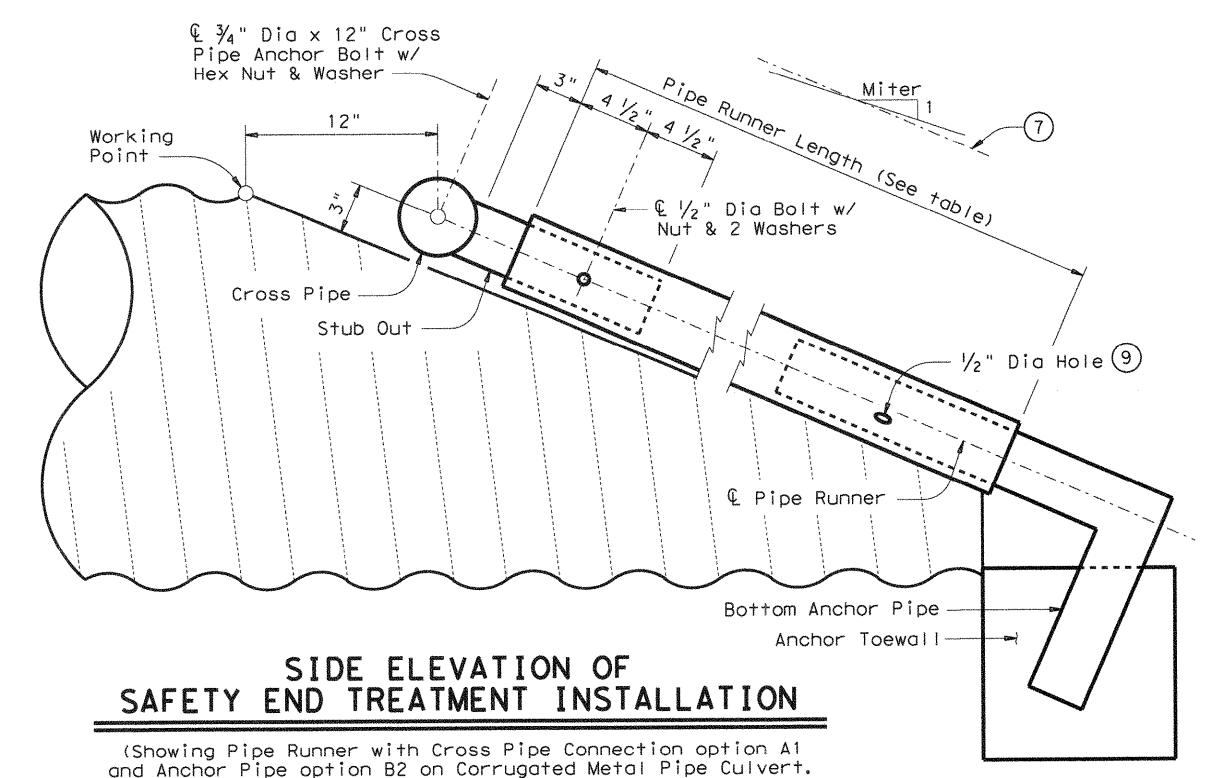


NOTE: The separate Pipe Runner shown is required when Cross Pipe Connection Option A1 is used.

PIPE RUNNER DETAILS



BOTTOM ANCHOR PIPE DETAILS 10

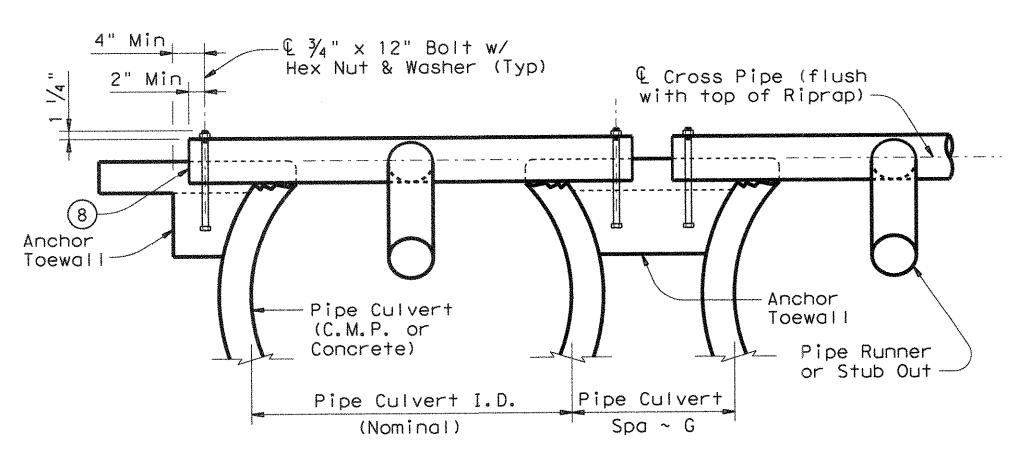


Concrete Pipe Culvert details are similar. Riprap not shown for clarity)

Runner Runner 9)-Bottom Anchor Bottom Anchor Pipe -Pipe -Bottom Anchor Bottom Anchor Toewall Toewall-3" Min Clear 12" OPTION B2 OPTION B1

BOTTOM ANCHOR TOEWALL DETAILS

(Culvert & Riprap not shown for clarity)



SHOWING CROSS PIPE & ANCHOR TOEWALL

SHOWING TYPICAL PIPE CULVERT & RIPRAP

Limits of Riprap (to be

included with S.E.T.

widest portion

-Pipe Culvert

Bridge Division

Standard

(C.M.P. or

Concrete)

-Riprap

of Pipe Culvert

for payment) (4)

1'-6"

(Typ)

SECTION A-A

— € Roadway

Limits of

Riprap

Side Slope (6)

PLAN OF SKEWED

INSTALLATION

(4) Riprap placed beyond the limits shown will be paid as Concrete Riprap in accordance with Item 432, "Riprap".

(6) Recommended values of side slope are 3:1, 4:1, & 6:1. All quantities, calculations, and dimensions shown herein are based on these recommended values. Slope of 3:1 or flatter is required for vehicle safety.

- 7 Note that actual slope of Pipe Runner may vary slightly from Side Slope of Riprap and trimmed Culvert Pipe edge.
- (8) Care shall be taken to ensure that Riprap concrete does not flow into the Cross Pipe so as to permit disassembly of the bolted connection to allow cleanout access.
- (9) After installation, the $\frac{1}{2}$ " hole shall be inspected to ensure that the lap of the Pipe Runner with the Bottom Anchor Pipe is adequate.
- (10) At fabricator's option, a heat bend to a smooth 5" radius or a manufactured elbow (of the same material as the Runner) may be substituted for the mitered and welded joint in the Bottom Anchor Pipe.

GENERAL NOTES:

as recommended by Research Report 280-1, "Safety Treatment of Roadside Cross-Drainage Structures", Texas Transportation Institute, March 1981. The Safety End Treatments shown herein are intended for use in those installations where out of control vehicles are likely to traverse the Riprap and all necessary inverts shall be Concrete Riprap conforming to the requirements of Item 432, "Riprap". Synthetic fibers listed on the "Fibers for Concrete" Material Producer

List (MPL) may be used in lieu of steel reinforcing in riprap concrete

Bolts and nuts shall conform to ASTM A307. All steel components, except concrete reinforcing, shall be galvanized after fabrication. Galvanizing damaged during transport or construction

SHEET 2 OF 2

Texas Department of Transportation

SAFETY END TREATMENT FOR 12" DIA TO 60" DIA PIPE CULVERTS TYPE II ~ CROSS DRAINAGE

SETP-CD

DN: GAF CK: CAT DW: JRP setpcdse.dgn CONT SECT HIGHWAY JOB ©TxDOT February 2010 REVISIONS 11-10: Add note for SHEET NO. COUNTY synthetic fibers.

Pipe Runners are designed for a traversing load of 1,800 pounds at yield openings approximately perpendicular to the Pipe Runners. unless noted otherwise. Payment for riprap and toewall is included in the Price Bid for each Safety End Treatment. Pipe Runners, Cross Pipes, and Anchor Pipes shall conform to the requirements of ASTM A53 (Type E or S, Grade B), ASTM A500 (Grade B), or API 5LX52. shall be repaired in accordance with the specifications.