06/05/2018

GIDEON GROVE-NORTH

CONSTRUCTION PLANS

FOR

LOTS 1-26, BLOCK 1 LOTS 1-20, BLOCK 2

LOTS 1-11, BLOCK 3 LOTS 1-14, BLOCK 4

3 COMMON AREAS

CITY OF ROCKWALL ROCKWALL COUNTY, TEXAS

INDEX OF SHEETS

SHEET NO.	DESCRIPTION
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10,11	GRADING PLAN
12	GRADING DETAILS
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14	PROPOSED DRAINAGE AREA MAP
15,16	DRAINAGE CALCULATIONS
17-20	STORM SEWER PLAN & PROFILES
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29	STREET LIGHT AND SIGNAGE PLAN
DTLS	CONSTRUCTION DETAILS

OWNER/DEVELOPER:

ENGINEER'S NOTES TO CONTRACTOR

1. BY COMMENCING CONSTRUCTION

AND UNDERSTAND THE PLANS AND SPECIFICATIONS. ANY QUESTIONS AND

TO COMMENCING CONSTRUCTION.

START OF CONSTRUCTION.

& ONSITE CONDITIONS.

AREAS ARE GRADED TO DRAIN.

CONTRACTOR AFFIRMS THEY HAVE REVIEWED

DISCREPANCIES MUST BE ADDRESSED PRIOR

2. ALL CONSTRUCTION SPECIFICATIONS WITHIN CITY ROW AND EASEMENTS SHOULD COMPLY WITH CITY OF ROCKWALL STANDARDS. PRIOR APPROVAL TO USE ANY NON-STANDARD

3. CONTRACTOR MUST VERIFY THE LOCATION OF ALL EXISTING UTILITIES PRIOR TO THE

4. CONTRACTOR MUST ENSURE THAT ALL

5. CONTRACTOR MUST NOTIFY ENGINEER OF ANY DISCREPANCIES BETWEEN THESE PLANS

GIDEON GROVE, LTD 8214 WESTCHESTER DRIVE SUITE 710 DALLAS, TX 75225 PHONE: 214-522-4945 CONTACT: JOHN ARNOLD

ENGINEER:

MACATEE ENGINEERING

DAYTON MACATEE ENGINEERING, LLC

(Tex. Reg. No. F-456) 3519 MILES STREET DALLAS, TEXAS 75209

TEL 214-373-1180 * FAX 214-373-6580 E-MAIL: daytonm@macatee-engineering.com NORTHRIGE AR SOUTHRIDGE CH PARK CENTRAL RAL VINDY

SITE

VINDY

QUAIL

RUN

LOCATION MAP

MAPSCO: 20D-A
NOT TO SCALE

BEFORE ANY OTHER CONSTRUCTION CAN BEGIN, THE DETENTION POND SHALL BE CONSTRUCTED, INCLUDING THE CONCRETE FLUMES AND OUTFALL STRUCTURE AND THE SIDES AND BOTTOM SHALL BE STABILIZED WITH EITHER SOD OR ANCHORED SEEDED CURLEX

WITHIN THIS PLAN SET THE TERM "CITY OF ROCKWALL STANDARDS" IS MEANT TO INCLUDE THE CITY OF ROCKWALL STANDARDS OF DESIGN AND CONSTRUCTION MANUAL, DATED OCTOBER 2016 AND THE PUBLIC WORKS CONSTRUCTION STANDARDS AND SPECIFICATIONS, NORTH CENTRAL TEXAS, 4TH EDITION, DATED OCTOBER 2004 — PUBLISHED BY NORTH CENTRAL TEXAS COUNCIL OF GOVERMENTS. CONTRACTOR SHALL REFER TO BOTH MANUALS PRIOR TO BEGINNING CONSTRUCTION.

CITY OF ROCKWALL MONUMENT "COR-2"
ALUMINUM DISK STAMPED "CITY OF ROCKWALL SURVEY MONUMENT" +/-866'
EAST OF INTERSECTION OF WILLIAMS STREET AND CARUTH LANE, AND 50' SOUTH
OF CL OF WILLIAMS STREET. N: 7029731.124, E: 2598589.314, ELEV=529.10

CITY OF ROCKWALL MONUMENT "COR-4"
ALUMINUM DISK STAMPED "CITY OF ROCKWALL SURVEY MONUMENT" ON NORTH SIDE
OF DALTON ROAD +/-210' WEST OF INTERSECTION OF SH 205 AND DALTON ROAD,
AND 10' NORTH OF CURB LINE. N: 7040336.992, E: 2592422.633, ELEV=541.67

DAYTON C. MACATEE

65028

65028

06 05 18

DAYTON MACATEE
ENGINEERING, LLC
TX. REG. NO. F-456

ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OR ACCURACY OF DESIGN

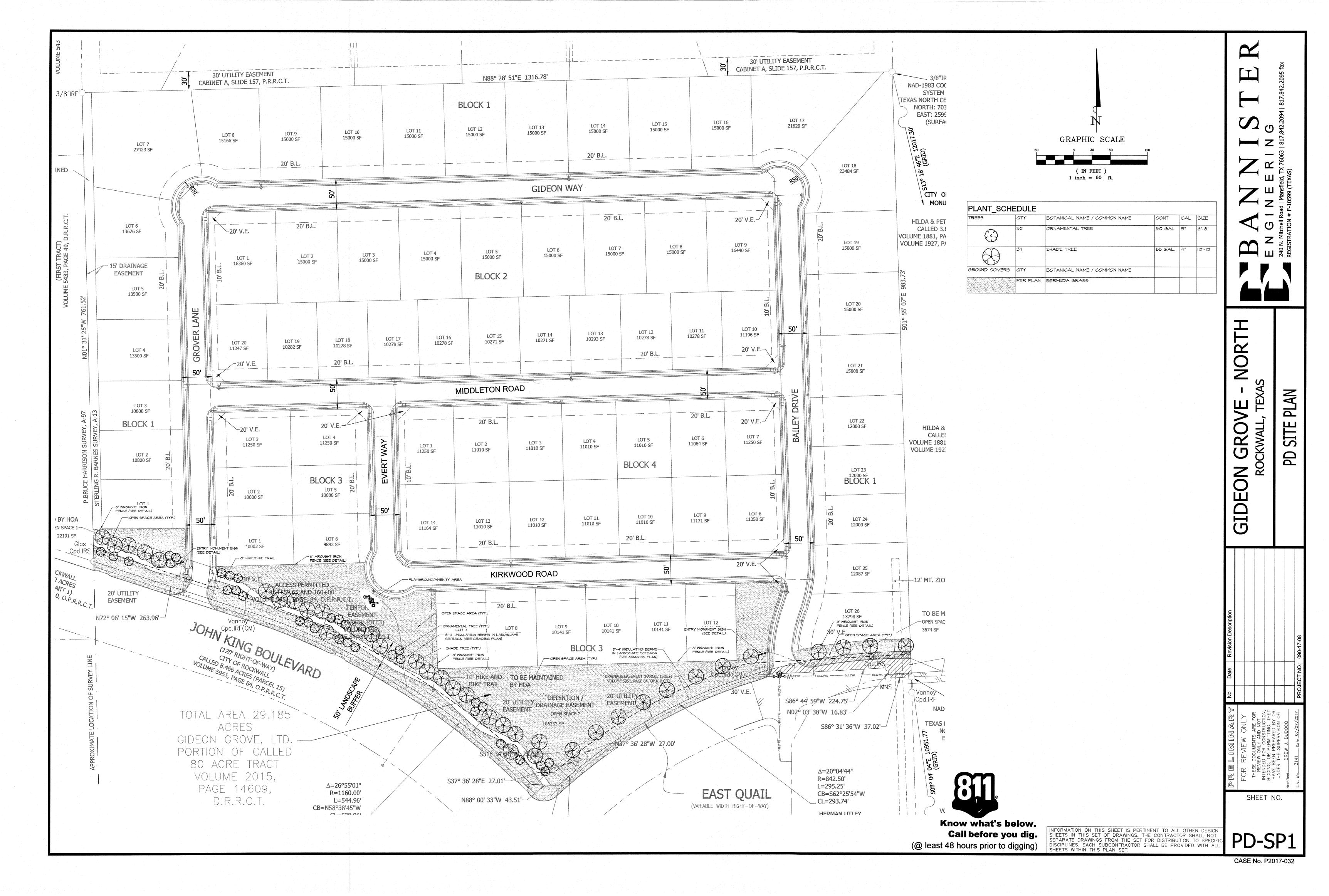
RECORD DRAWING

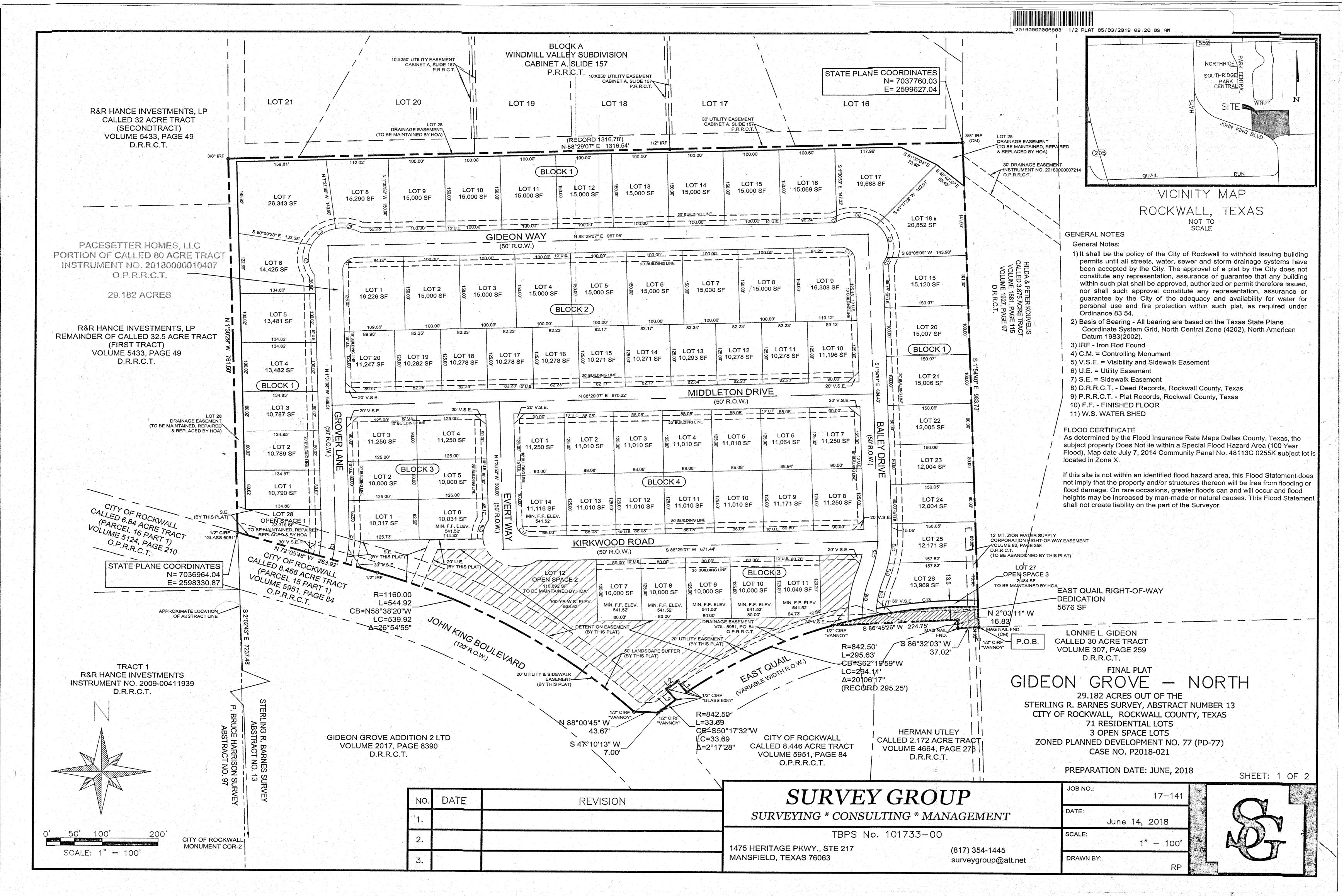
TO THE BEST OF OUR KNOWLEDGE, MACATEE ENGINEERING LLC, HEREBY STATES THAT THIS PLAN IS AN ACCURATE RECORD DRAWING. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE

Dayton C. Macatee, P.E. Date
12655 N. Central Expwy, Suite 420, Dallas, Texas 75243

REV. BY DATE DESCRIPTION

CHICLLINGIAL .





OWNERS CERTIFICATE

STATE OF TEXAS

WHEREAS, Pacesetter Homes, LLC is the owner of a 29,182 acre tract of land located in the Sterling R. Barnes Survey, Abstract No. 13, City of Rockwall Rockwall County, Texas, being a portion of that certain 80 acres tract of land conveyed to Pacesetter Homes, LLC, by deed as recorded in Instrument No. 2018 000010407, Official Public Records, Rockwall County, Texas and being more fully described by metes and bounds as follows:

BEGINNING at a mag. nail found at the southeast corner of the herein described tract, for the southwest corner of a called 3.875 acre tract of land as described in a deed to Hilda and Peter Kouvelis as recorded in Volume 1881, Page 115 and Volume 1927, Page 97, Deed Records, Rockwall County, Texas, being the northwest corner of a called 30 acre tract of land as described in a deed to Lonnie L. Gideon as recorded in Volume 307, Page 259 of the said deed records, and the northeast corner of a called 2.172 acre tract of land as described in a deed to Herman Utley as recorded in Volume 4664, Page 273 of the said deed records;

THENCE South 86 deg. 32 min. 26 sec. West, along the common line of this tract and the said Utley tract, a distance of 37.02 feet to a mag. nail found in the north right-of-way line of East Quail (Variable Width R.O.W.);

THENCE along the said East Quail right-of-way the following courses and distances:

North 02 deg. 03 min. 11 sec. West a distance of 16.83 feet to a mag. nail found for corner;

South 86 deg. 45 min. 26 sec. West a distance of 224.75 feet to a capped 1/2 inch iron rod stamped "VANNOY" found in the beginning of a curve to the left having a radius of 842.50 feet and a chord bearing South 62 deg. 19 min. 59 sec. West and a chord length of 294.11 feet;

Along said curve to the left a distance of 295.63 (Record 295.25 feet), to a capped 1/2 inch iron rod stamped "GLASS 6081";

North 37 deg. 36 min. 01 sec. West a distance of 27.00 feet to a capped 1/2 inch iron rod stamped "GLASS 60810";

South 51 deg. 34 min. 33 sec. West a distance of 25.00 feet to a capped 1/2 inch iron rod stamped "GLASS 6081";

South 37 deg. 43 min. 04 sec. East a distance of 27.01 feet to a capped 1/2 inch iron rod stamped "GLASS 6081" found in the beginning of a curve to the left having a radius of 842.50 feet and a chord bearing South 50 deg. 17 min. 32 sec. West and a chord length of 33.69 feet;

Along said curve to the left a distance of 33.69 feet to a capped 1/2 inch iron rod stamped "GLASS 6081";

South 47 deg. 10 min. 13 sec. West a distance of 7.00 feet to a capped 1/2 inch iron rod stamped "VANNOY";

THENCE North 88 deg. 00 min. 45 sec. West a distance of 43.67 feet to a capped 1/2 inch iron rod found in the northeasterly right-of-way line of John King Boulevard (120 foot R.O.W.), being in the beginning of a curve to the left having a radius of 1160.00 feet and a chord bearing North 58 deg. 38 min. 20 sec. West and a chord length of 539.92 feet;

THENCE along said curve to the left and the John King Boulevard right-of-way, a distance of 544.92 feet to a 1/2 inch iron rod found;

THENCE North 72 deg. 05 min. 48 sec. West along the common line of this tract and the said John King right-of-way, a distance of 263.92 feet to a capped 1/2 inch iron rod stamped "GLASS 6081" found at the southeast corner of the remainder of a called 32.5 acre tract of land as described in a deed to R&R Hance Investments, LP (First Tract) as recorded in Volume 5433, Page 49 of the said deed records;

THENCE North 01 deg. 30 min. 29 sec. West, along the common line of this tract and the said R&R Hance tract, pass the common east line of a called 32.5 acre tract of land as described in a deed to R&R Hance Investments, LP (Second Tract) as recorded in Volume 5433, Page 49 of the said deed records, a distance of 761.50 feet to a 3/8 inch iron rod fond at the southwest corner of Lot 21, Block A, Windmill Valley Subdivision as recorded in Cabinet A, Slide 157, Plat Records, Rockwall County, Texas;

THENCE North 88 deg. 29 min. 07 sec. East along the south line of Lots 21-16, Block A of said subdivision, a distance of 1316.54 feet (Record 1316.78 feet), to a 3/8 inch iron rod found in the west line of the said Kouvelis tract:

THENCE South 01 deg. 54 min. 40 sec. East along the common line of this tract and the said Kouvelis tract, a distance of 983.73 feet to the POINT OF BEGINNING and containing 29.182 acres of computed land more or less.

NOW, THEREFORE, KNOW ALL MEN BY THESE PRESENTS:

STATE OF TEXAS
COUNTY OF ROCKWALL

I (we) the undersigned owner(s) of the land shown on this plat, and designated herein as the GIDEON GROVE-NORTH, a subdivision to the City of Rockwall, Texas, and whose name is subscribed hereto, hereby dedicate to the use of the public forever all streets, alleys, parks, water courses, drains, easements and public places thereon shown on the purpose and consideration therein expressed. I (we) further certify that all other parties who have a mortgage or lien interest in the GIDEON GROVE-NORTH a subdivision have been notified and signed this plat. I (we) understand and do hereby reserve the easement strips shown on this plat for the purposes stated and for the mutual use and accommodation of all utilities desiring to use or using same. I (we) also understand the following;

- 1. No buildings shall be constructed or placed upon, over, or across the utility easements as described herein.
- 2. Any public utility shall have the right to remove and keep removed all or part of any buildings, fences, trees, shrubs, or other growths or improvements which in any way endanger or interfere with construction, maintenance or efficiency of their respective system on any of these easement strips; and any public utility shall at all times have the right of ingress or egress to, from and upon the said easement strips for purpose of construction, reconstruction, inspecting, patrolling, maintaining, and either adding to or removing all or part of their respective system without the necessity of, at any time, procuring the permission of anyone.
- 3. The City of Rockwall will not be responsible for any claims of any nature resulting from or occasioned by the establishment of grade of streets in the subdivision.
- 4. The developer and subdivision engineer shall bear total responsibility for storm drain improvements.
- 5. The developer shall be responsible for the necessary facilities to provide drainage patterns and drainage controls such that properties within the drainage area are not adversely affected by storm drainage from the development.
- 6. No house dwelling unit, or other structure shall be constructed on any lot in this addition by the owner or any other person until the developer and/or owner has complied with all requirements of the Subdivision Regulations of the City of Rockwall regarding improvements with respect to the entire block on the street or streets on which property abuts, including the actual installation of streets with the required base and paving, curb and gutter, water and sewer, drainage structures, storm structures, storm sewers, and alleys, all according to the specifications of the City of Rockwall; or

Until an escrow deposit, sufficient to pay for the cost of such improvements, as determined by the city's engineer and/or city administrator, computed on a private commercial rate basis, has been made with the city secretary, accompanied by any agreement signed by the developer and/or owner, authorizing the city to make such improvements at prevailing private commercial rates, or have the same made by a contractor and pay for the same out of the escrow deposit, should the developer and/or owner fail or refuse to install the required improvements within the time stated in such written agreement, but in no case shall the City be obligated to make such improvements itself. Such deposit may be used by the owner and/or developer as progress payments as the work progresses in making such improvements by making certified requisitions to the city secretary, supported by evidence of work done; or

Until the developer and/or owner files a corporate surety bond with the city secretary in a sum equal to the cost of such improvements for the designated area, guaranteeing the installation thereof within the time stated in the bond, which time shall be fixed by the city council of the City of Rockwall.

7. Property owner shall be responsible for maintaining, repairing, and replacing all systems in the detention and drainage easements.

8. No fencing shall be placed within or across any drainage easement.

I (we) further acknowledge that the dedications and/or exaction's made herein are proportional to the impact of the Subdivision upon the public services required in order that the development will comport with the present and future growth needs of the City; I (we), my (our) successors and assigns hereby waive any claim, damage, or cause of action that I (we) may have as a result of the dedication of exactions made herein.

Tom Lynch, President

STATE OF TEXAS COUNTY OF

Before me, the undersigned authority, on this day personally appeared *Tom Lynch* known to me to be the person whose name is subscribed to the foregoing instrument, and acknowledged to me that he executed the same for the purpose and consideration therein stated.

Given upon my hand and seal of office this 10th day of August, 2018.

Notary Public in and for the State of To

. ". "							
Comments	Carrier of	THE SECTION	Maria de la composición dela composición de la composición de la composición dela composición de la composición dela composición dela composición de la composición de la composición dela				
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NO.	DATE	REVISION
1.		
2.		
3.		

SURVEYORS CERTIFICATE

I, HEREBY CERTIFY, THAT I PREPARED THIS SURVEY PLAT SHOWN HEREON FROM AN ACTUAL SURVEY ON THE GROUND BY ME AND THAT ALL CORNERS ARE MARKED, AND THAT ALL DIMENSIONS SHOWN THEREON ARE TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF, AND FURTHERMORE THERE ARE NO ENCROACHMENTS, PROTRUSIONS, IMPROVEMENTS, EASEMENTS, RIGHT-OF- WAY, OR 100 YEAR FLOOD PLAIN AFFECTING THE PROPERTY EXCEPT AS SHOWN THEREON.

WITNESS UNDER MY HAND THIS THE

3rd DAY OF Angest, 201

WILLIAM P. PRICE STATE OF TEXAS R.P.L.S. NO. 3047



		Cı	urve	Table	
Curve #	Length	Radius	Delta	Chord Direction	Chord Length
C1	53.30	245.18	12.46	N5° 43' 19"E	53.19
C2	16.01	319.96	2.87	N1° 05' 22"E	16.01
C3	65.96	61.00	61.96	N5° 09' 11"W	62.80
C4	55.87	61.00	52.48	N52° 04' 02"E	53.94
C5	47.69	61.00	44.79	N79° 17′ 47″W	46.48
C6	39.27	25.00	90.00	N43° 29′ 00″E	35.36
C7	5.03	61.00	4.72	S56° 23′ 40″W	5.03
C8	77.22	61.00	72.53	N84° 58′ 39″W	72.17
C9	73.68	61.00	69.20	N14° 06′ 34″W	69.28
C10	12.82	61.00	12.05	N26° 30′ 57″E	12.80
C11	39.10	25.00	89.60	S46° 42′ 52″E	35.23
C12	65,57	275.00	13.66	N4° 54' 59"E	65.41
C13	35.49	275.00	7.40	N15° 26′ 40″E	35.47
C14	59.34	225.00	15.11	N11° 35′ 14″E	59.17
C15	91.87	250.00	21.06	N8° 36' 51"E	91.36
C16	87.50	250.00	20.05	S9' 06' 56"W	87.05
C17	39.27	25.00	90.00	N46° 31' 00"W	35.35
C18	39.51	61.00	37.11	S14° 27′ 11″W	38.82

PARCEL LINE TABLE														
LINE#	LENGTH	DIRECTION												
L1	27.00'	N37°36'01"W												
L2	25.00'	S51°34'33"W												
L3	27.01'	S37°43'04"E												

RECOMMENDED FOR FINAL APPROVAL:

Planning & Zoning Commission, Chairman

71/6/1

APPROVED:

I hereby certify that the above and foregoing plat of an addition to the City of Rockwall, Texas, was approved by the City Council of the City of Rockwall on the ______ day of _______, 2018.

This approval shall be invalid unless the approved plat for such addition is recorded in the office of the County Clerk of Rockwall, County, Texas, within one hundred eighty (180) days from said date of final approval.

WITNESS OUR HANDS, this _5th day of Alphen

Mayor, City of Rockwall

City Secretary

Omywio ains,

City Engineer

Filed and Recorded
Official Public Records
Shelli Miller, County Clerk
Rockwall County, Texas
05/03/2019 09:20:09 AM
\$100.00

20190000006883

GIDEON GROVE - NORTH

29.182 ACRES OUT OF THE
STERLING R. BARNES SURVEY, ABSTRACT NUMBER 13
CITY OF ROCKWALL, ROCKWALL COUNTY, TEXAS
71 RESIDENTIAL LOTS
3 OPEN SPACE LOTS

ZONED PLANNED DEVELOPMENT NO. 77 (PD-77)

CASE NO. P2018-021

PREPARATION DATE: JUNE, 2018

17-14

SHEET: 2 OF 2

SURVEY GROUP

SURVEYING * CONSULTING * MANAGEMENT

TBPS No. 101733-00

1475 HERITAGE PKWY., STE 217 MANSFIELD, TEXAS 76063

(817) 354-1445 surveygroup@att.net DATE:

June 14, 2018

SCALE:

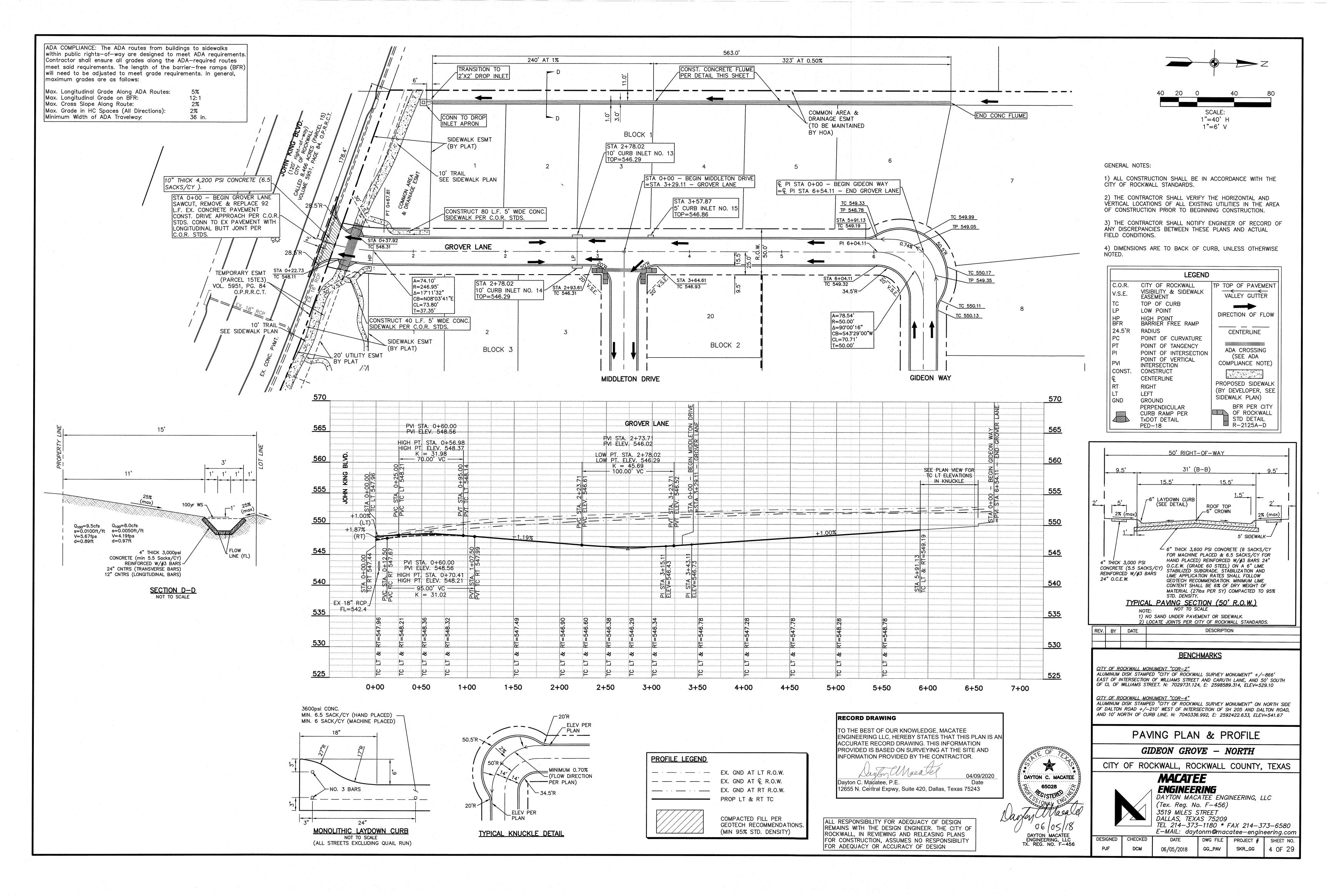
1" - 100'

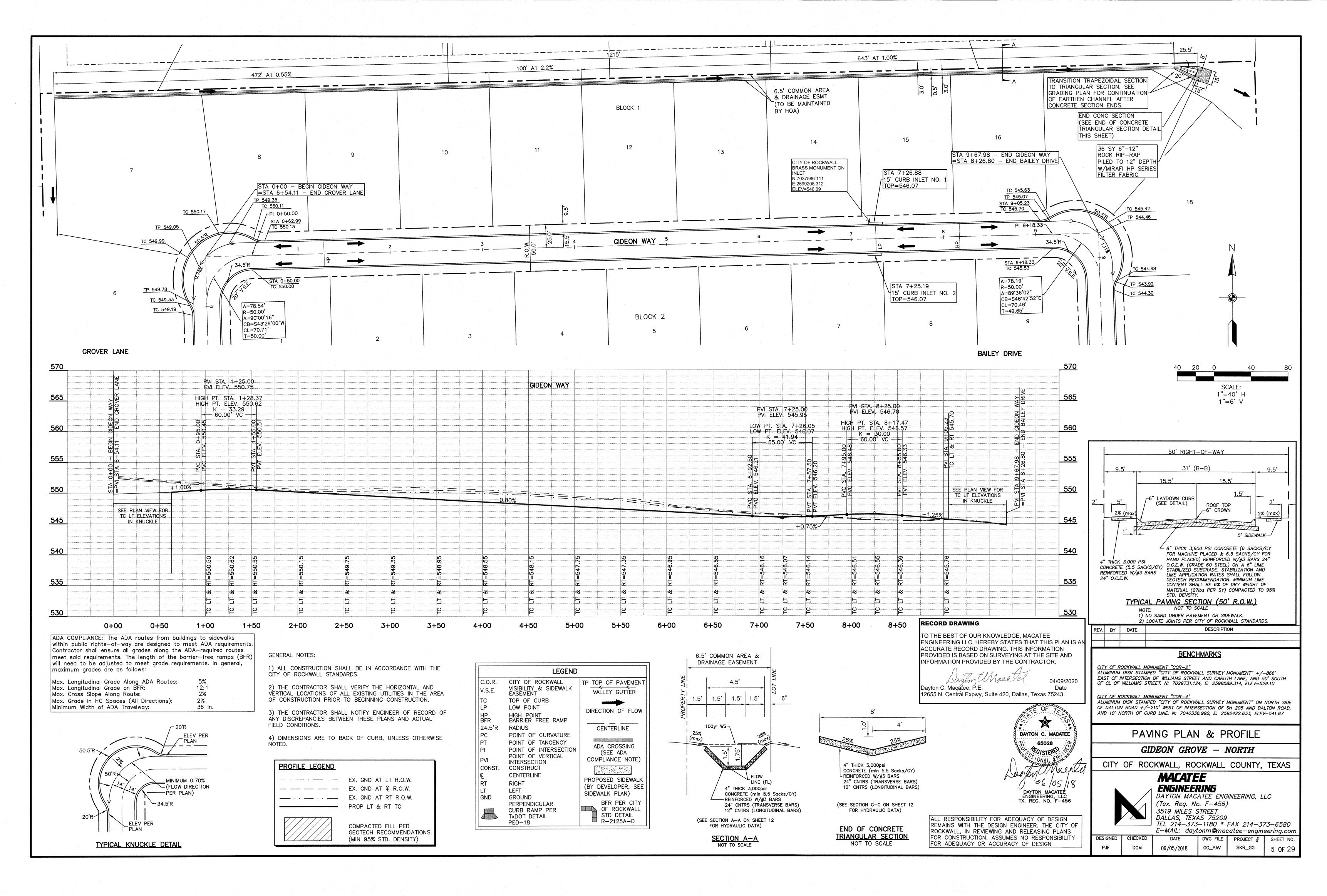
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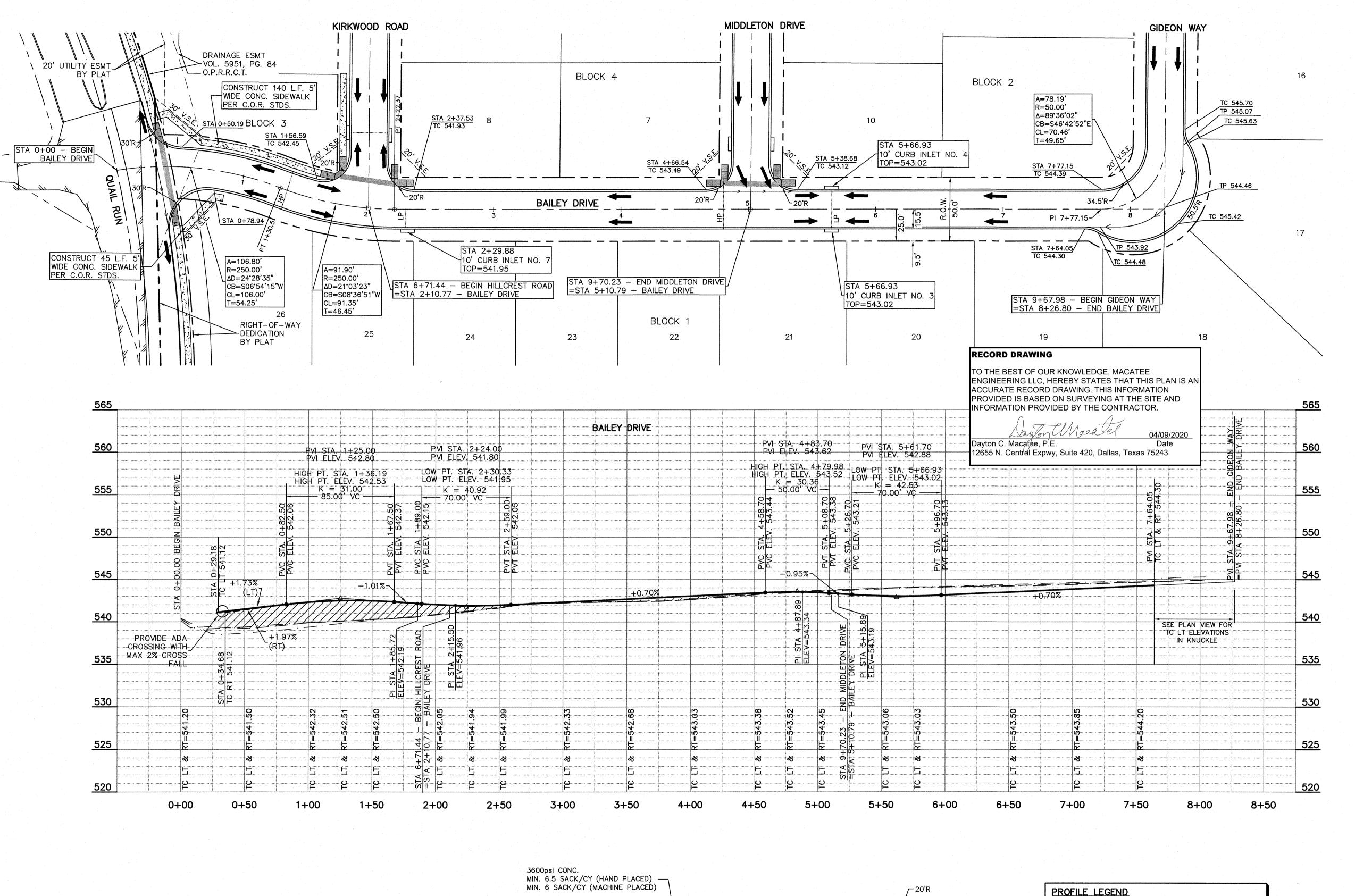
RP

JOB NO.:



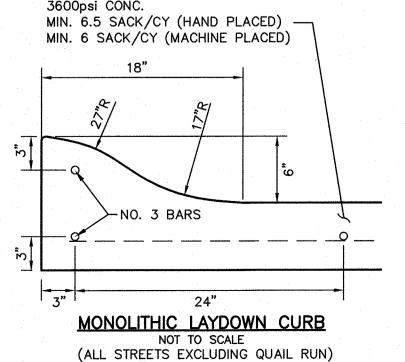


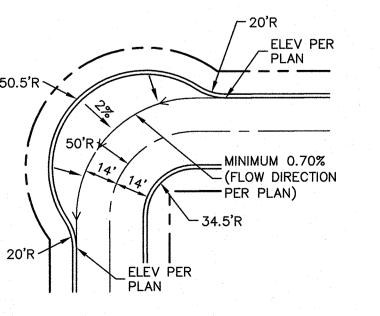




ADA COMPLIANCE: The ADA routes from buildings to sidewalks within public rights—of—way are designed to meet ADA requirements. Contractor shall ensure all grades along the ADA—required routes meet said requirements. The length of the barrier—free ramps (BFR) will need to be adjusted to meet grade requirements. In general, maximum grades are as follows:

Max. Longitudinal Grade Along ADA Routes: 12:1 Max. Longitudinal Grade on BFR: Max. Cross Slope Along Route: 2% Max. Grade in HC Spaces (All Directions): 2% Minimum Width of ADA Travelway: 36 in.



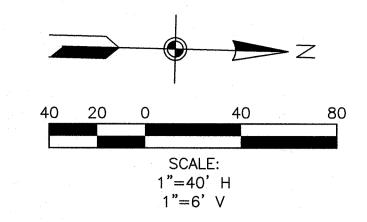


TYPICAL KNUCKLE DETAIL

EX. GND AT Q R.O.W. EX. GND AT RT R.O.W. PROP LT & RT TC COMPACTED FILL PER DAYTON C. MACATEE GEOTECH RECOMMENDATIONS. (MIN 95% STD. DENSITY)

EX. GND AT LT R.O.W.

ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OR ACCURACY OF DESIGN



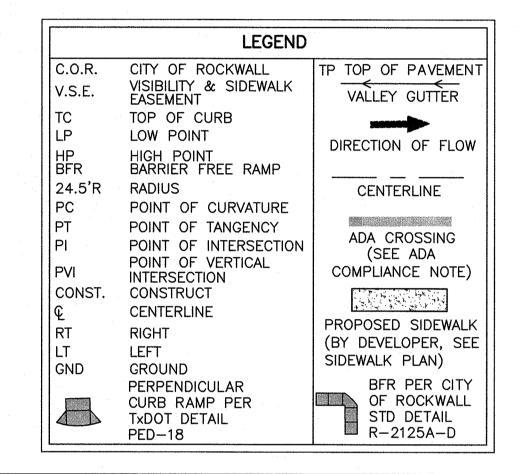
GENERAL NOTES:

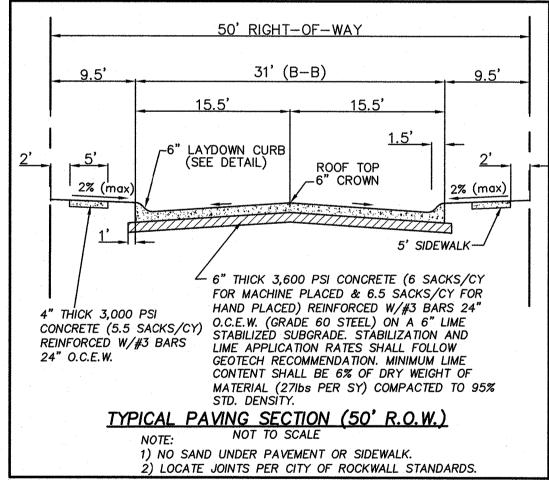
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3) THE CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD OF ANY DISCREPANCIES BETWEEN THESE PLANS AND ACTUAL FIELD CONDITIONS.

4) DIMENSIONS ARE TO BACK OF CURB, UNLESS OTHERWISE





BENCHMARKS

CITY OF ROCKWALL MONUMENT "COR-2"
ALUMINUM DISK STAMPED "CITY OF ROCKWALL SURVEY MONUMENT" +/-866' EAST OF INTERSECTION OF WILLIAMS STREET AND CARUTH LANE, AND 50' SOUTH OF CL OF WILLIAMS STREET. N: 7029731.124, E: 2598589.314, ELEV=529.10

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PAVING PLAN & PROFILE

GIDEON GROVE - NORTH

CITY OF ROCKWALL, ROCKWALL COUNTY, TEXAS

65028

A GISTERED

06/05/18

DAYTON MACATEE ENGINEERING, LLC TX. REG. NO. F-456

REV. BY DATE

MACATEE **ENGINEERING**

DAYTON MACATEE ENGINEERING, LLC (Tex. Reg. No. F-456) 3519 MILES STREET

DESCRIPTION

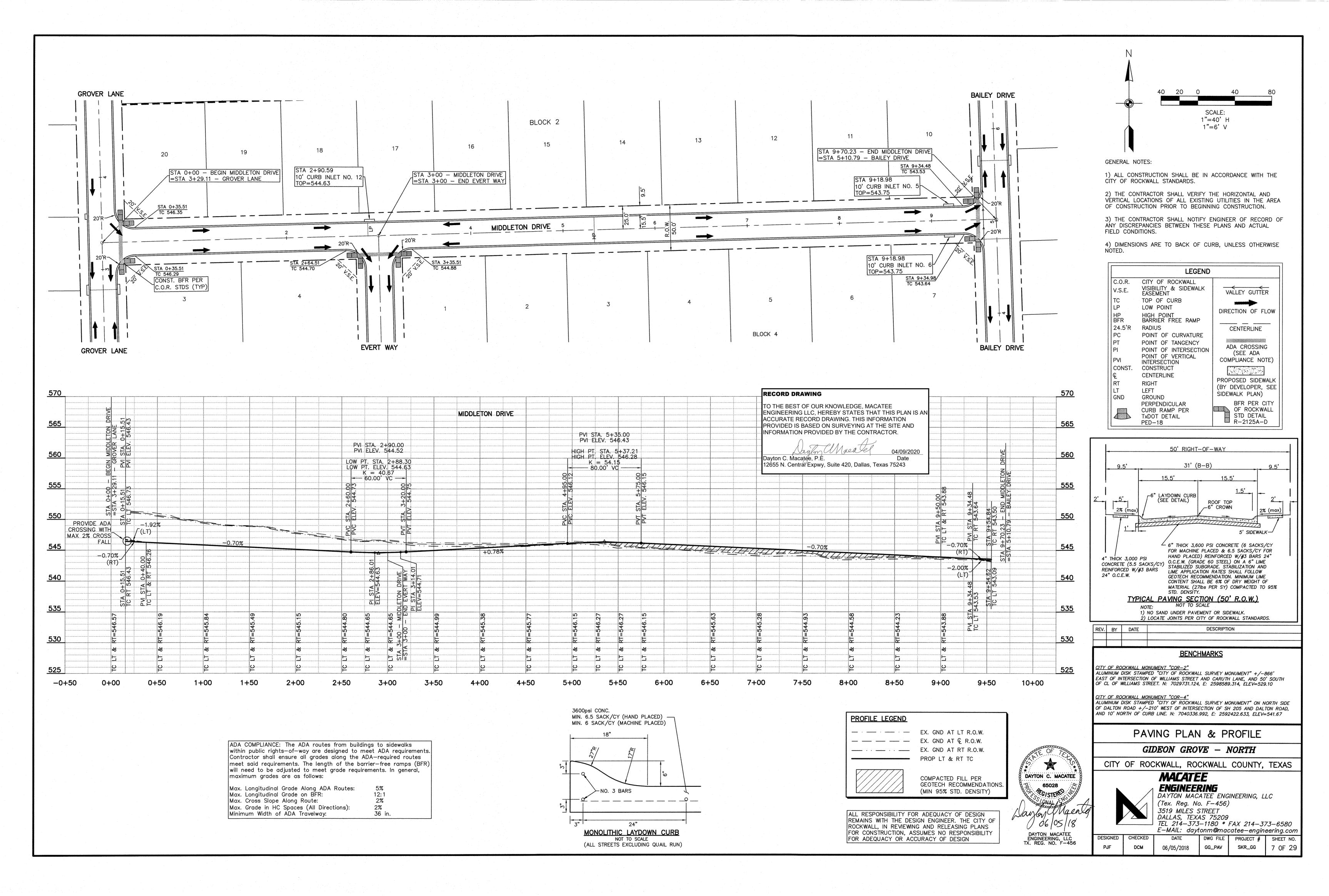
DALLAS, TEXAS 75209 TEL 214-373-1180 * FAX 214-373-6580 E-MAIL: daytonm@macatee-engineering.com

6 OF 29

DESIGNED DCM

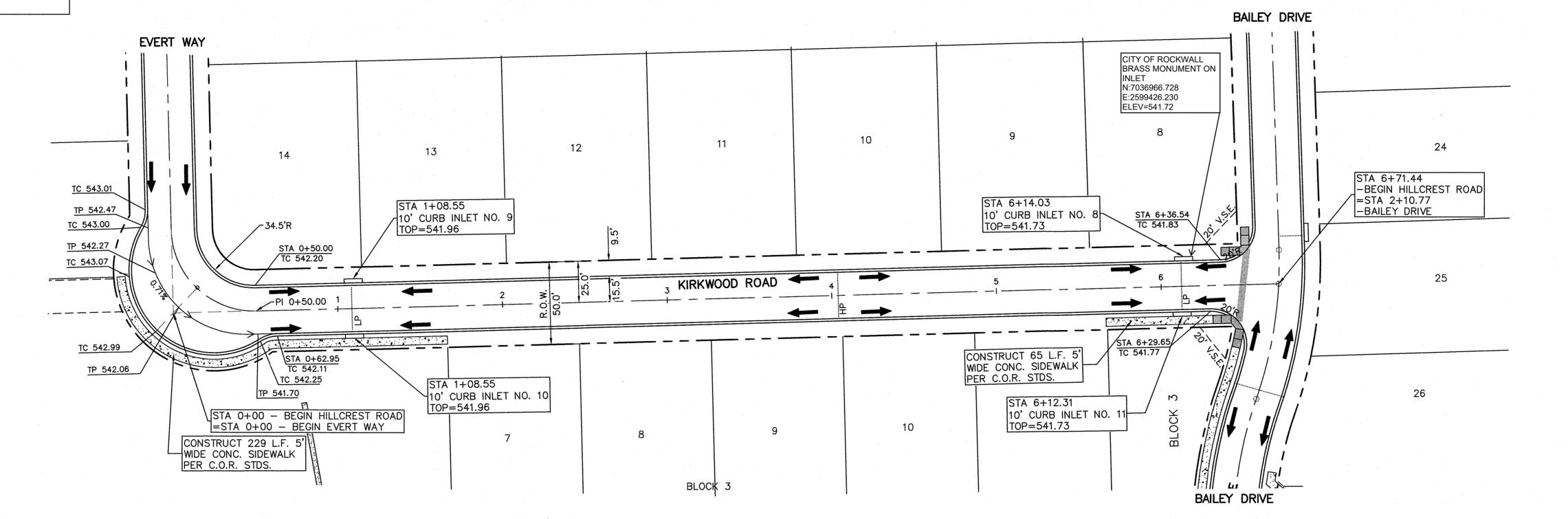
PROJECT # SHEET NO. GG_PAV SKR_GG

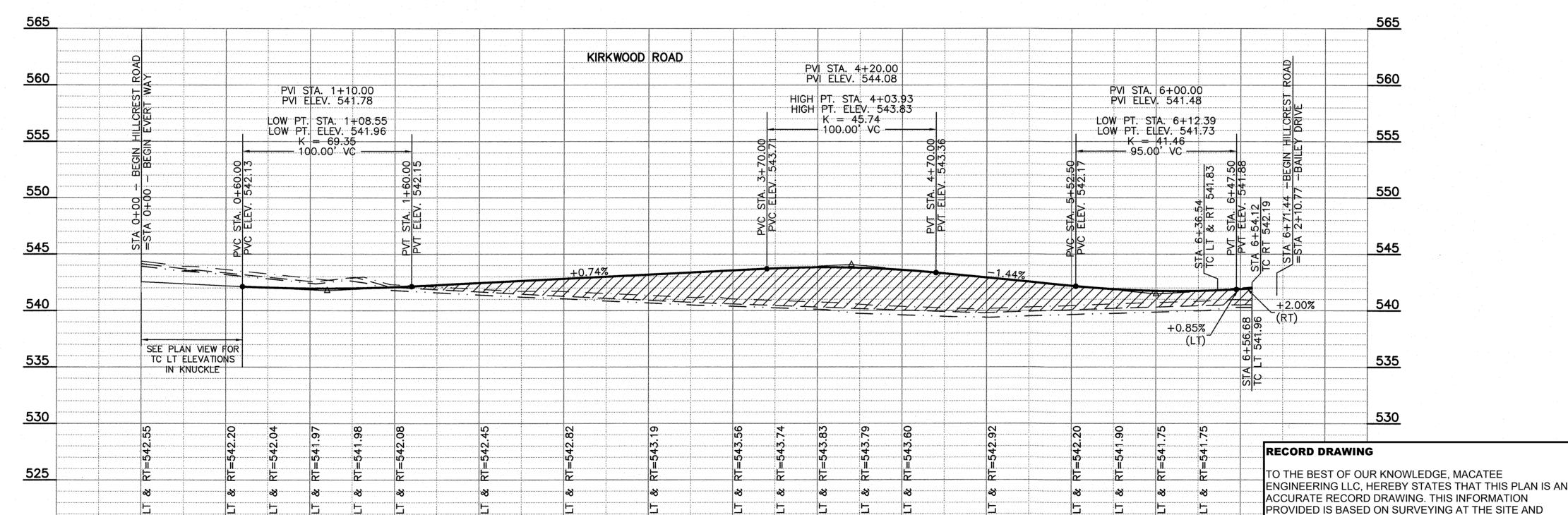
CHECKED 06/05/2018

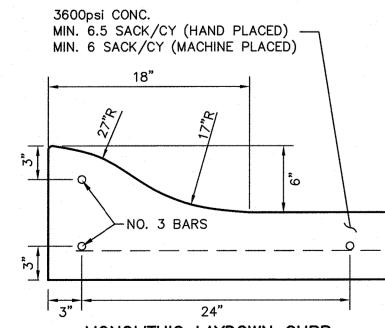


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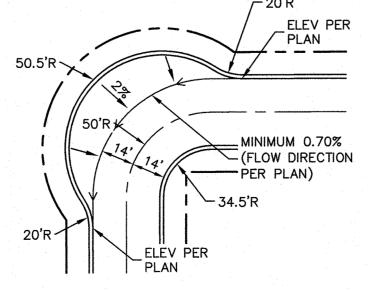
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MONOLITHIC LAYDOWN CURB NOT TO SCALE (ALL STREETS EXCLUDING QUAIL RUN)

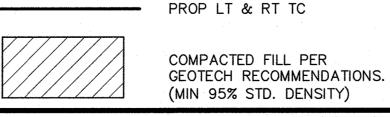


TYPICAL KNUCKLE DETAIL

2+00

2+50

PROFILE LEGEND EX. GND AT LT R.O.W. EX. GND AT Q R.O.W. EX. GND AT RT R.O.W.

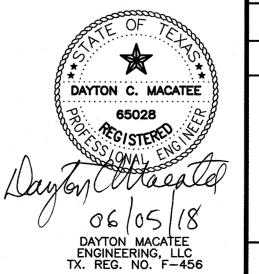


5+00

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6 + 50

6+00



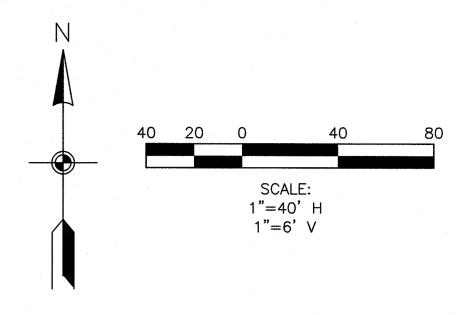
04/09/2020

Date

INFORMATION PROVIDED BY THE CONTRACTOR.

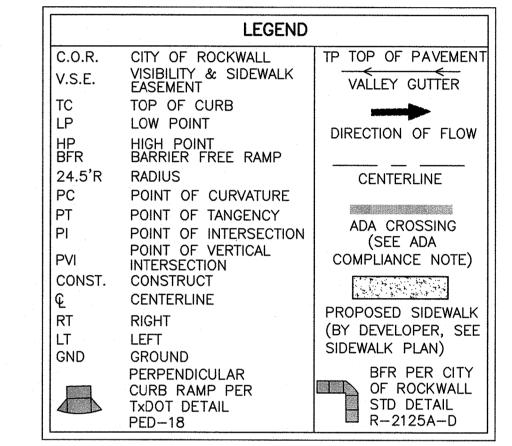
12655 N. Central Expwy, Suite 420, Dallas, Texas 75243

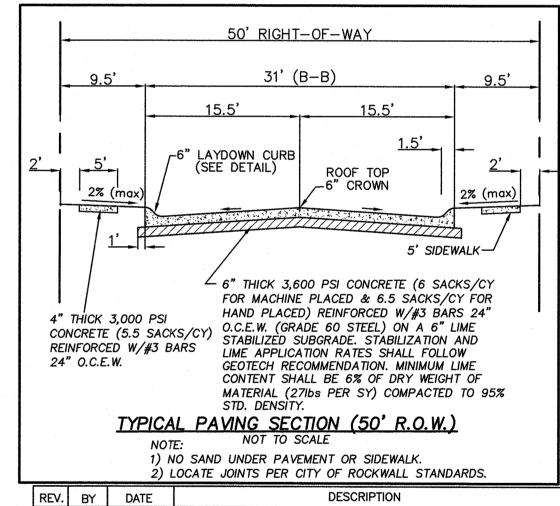
Dayton C. Ma¢atee, P.E.



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BENCHMARKS

CITY OF ROCKWALL MONUMENT "COR-2" ALUMINUM DISK STAMPED "CITY OF ROCKWALL SURVEY MONUMENT" +/-866' EAST OF INTERSECTION OF WILLIAMS STREET AND CARUTH LANE, AND 50' SOUTH OF CL OF WILLIAMS STREET. N: 7029731.124, E: 2598589.314, ELEV=529.10

CITY OF ROCKWALL MONUMENT "COR-4" ALUMINUM DISK STAMPED "CITY OF ROCKWALL SURVEY MONUMENT" ON NORTH SIDE OF DALTON ROAD +/-210' WEST OF INTERSECTION OF SH 205 AND DALTON ROAD, AND 10' NORTH OF CURB LINE. N: 7040336.992, E: 2592422.633, ELEV=541.67

PAVING PLAN & PROFILE

GIDEON GROVE - NORTH

CITY OF ROCKWALL, ROCKWALL COUNTY, TEXAS

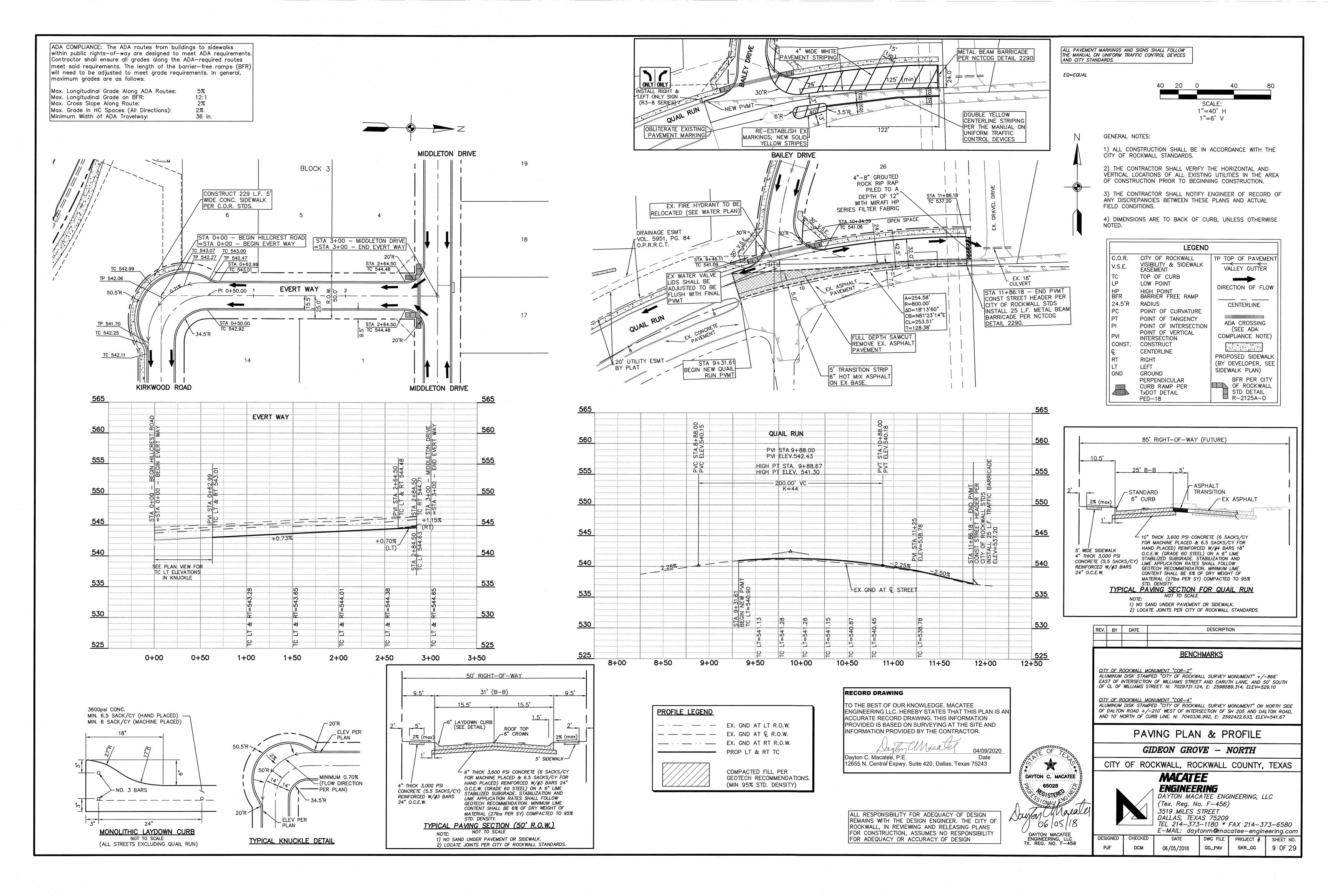


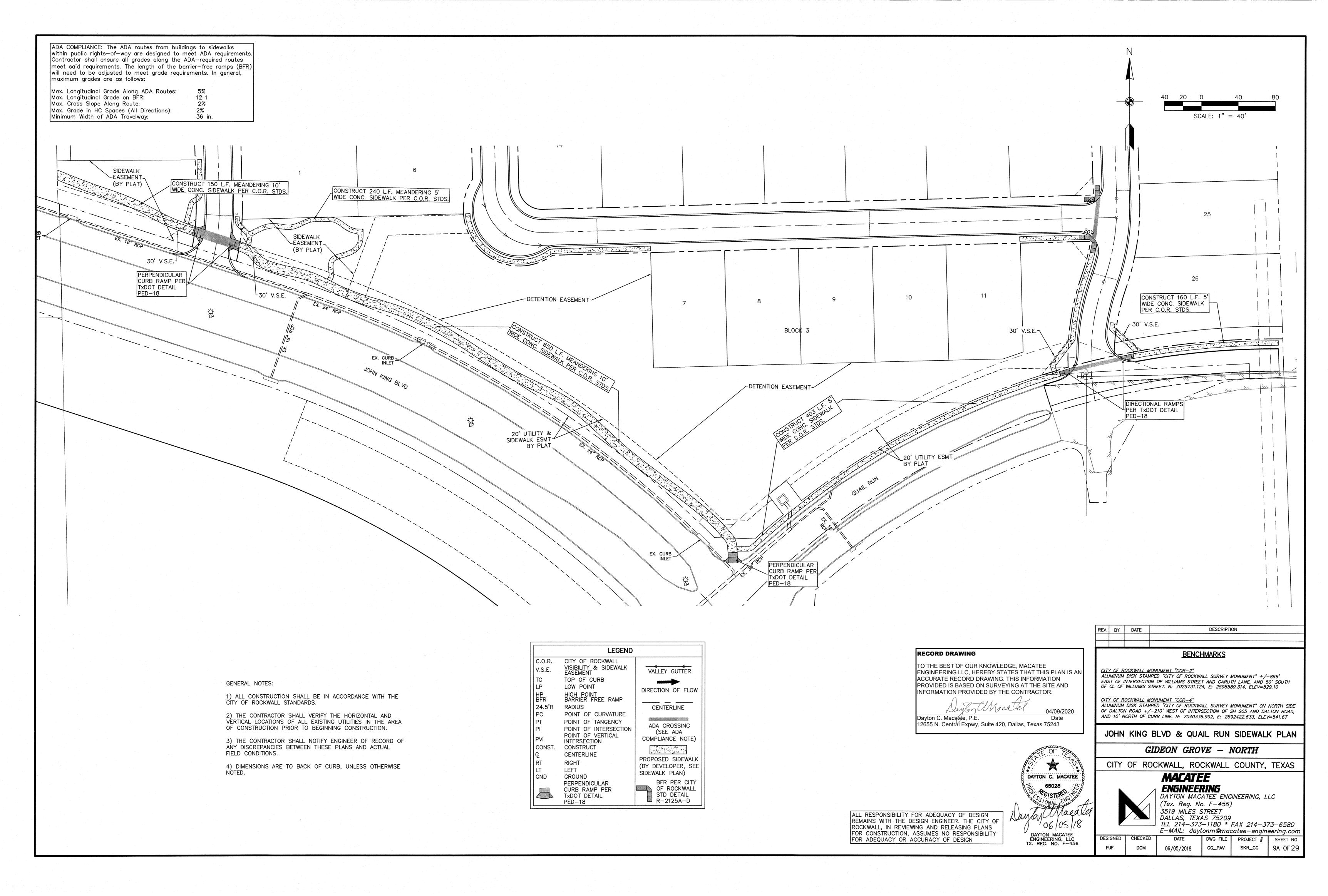
MACATEE **ENGINEERING**

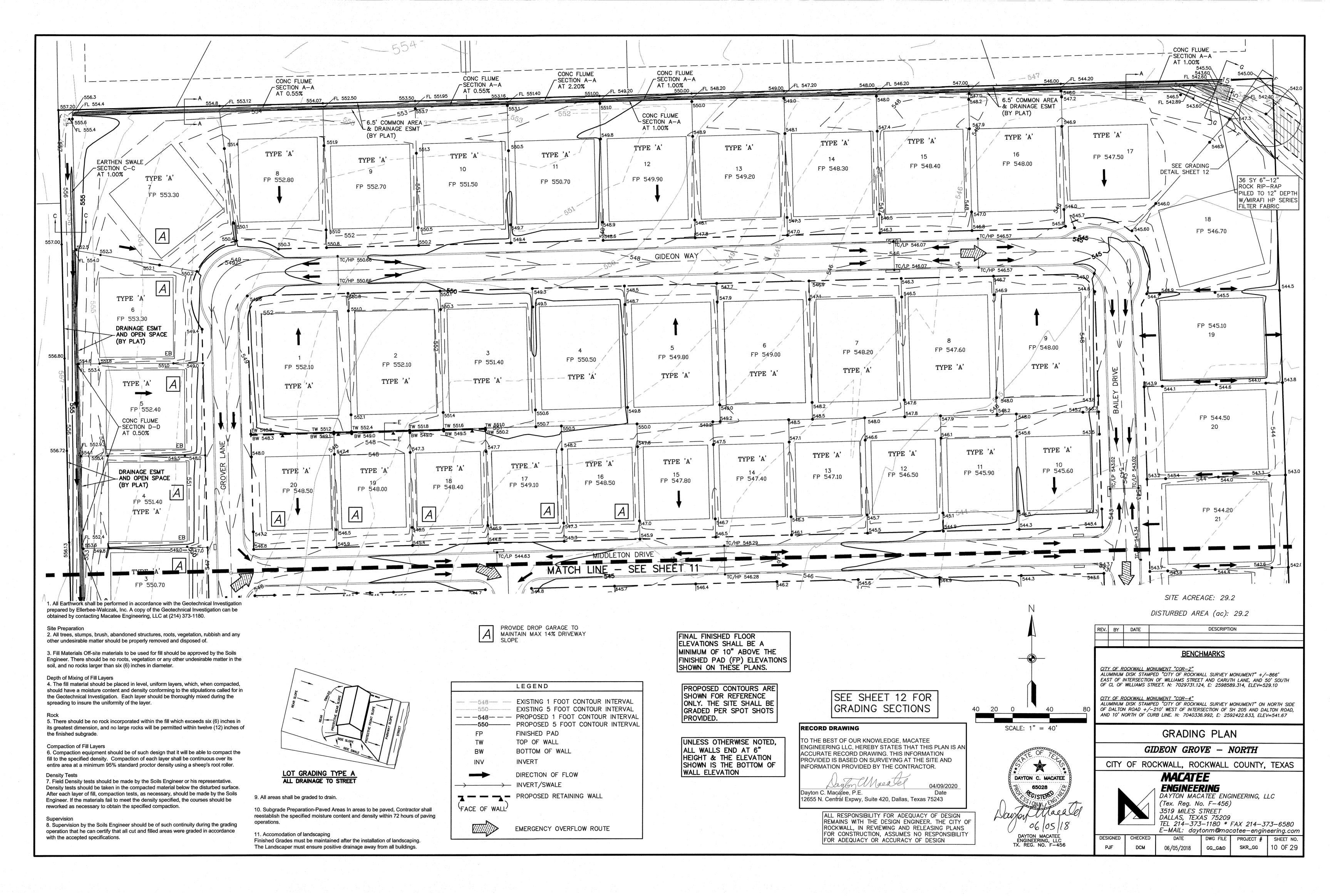
DAYTON MACATEE ENGINEERING, LLC (Tex. Reg. No. F-456) 3519 MILES STREET DALLAS, TEXAS 75209 TFI 214-373-1180 * FAX 214-373-6580

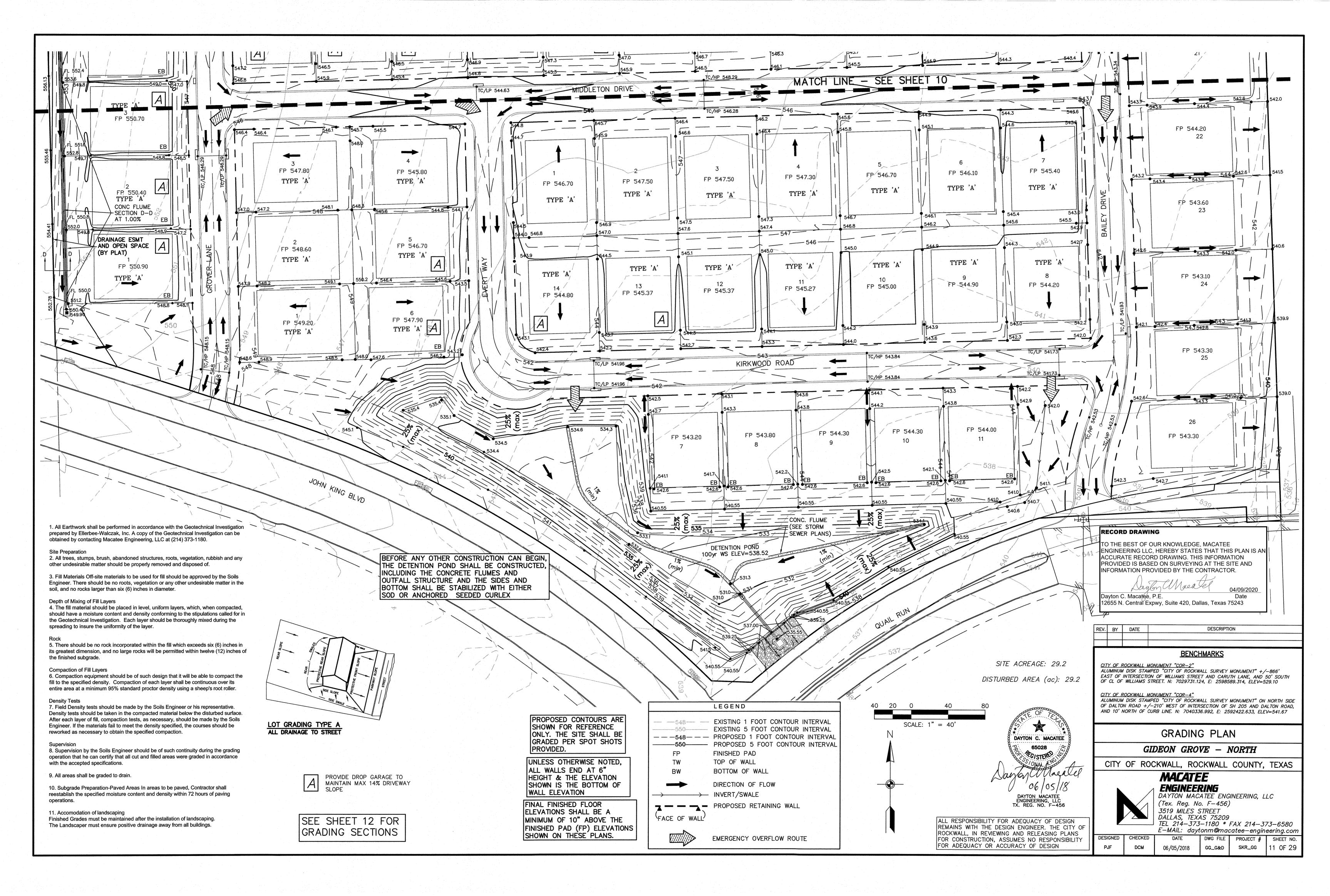
		E-MAIL: day			
SIGNED	CHECKED	DATE	DWG FILE	PROJECT #	SHEET NO.
PJF	DCM	06/05/2018	GG_PAV	SKR_GG	8 OF 29

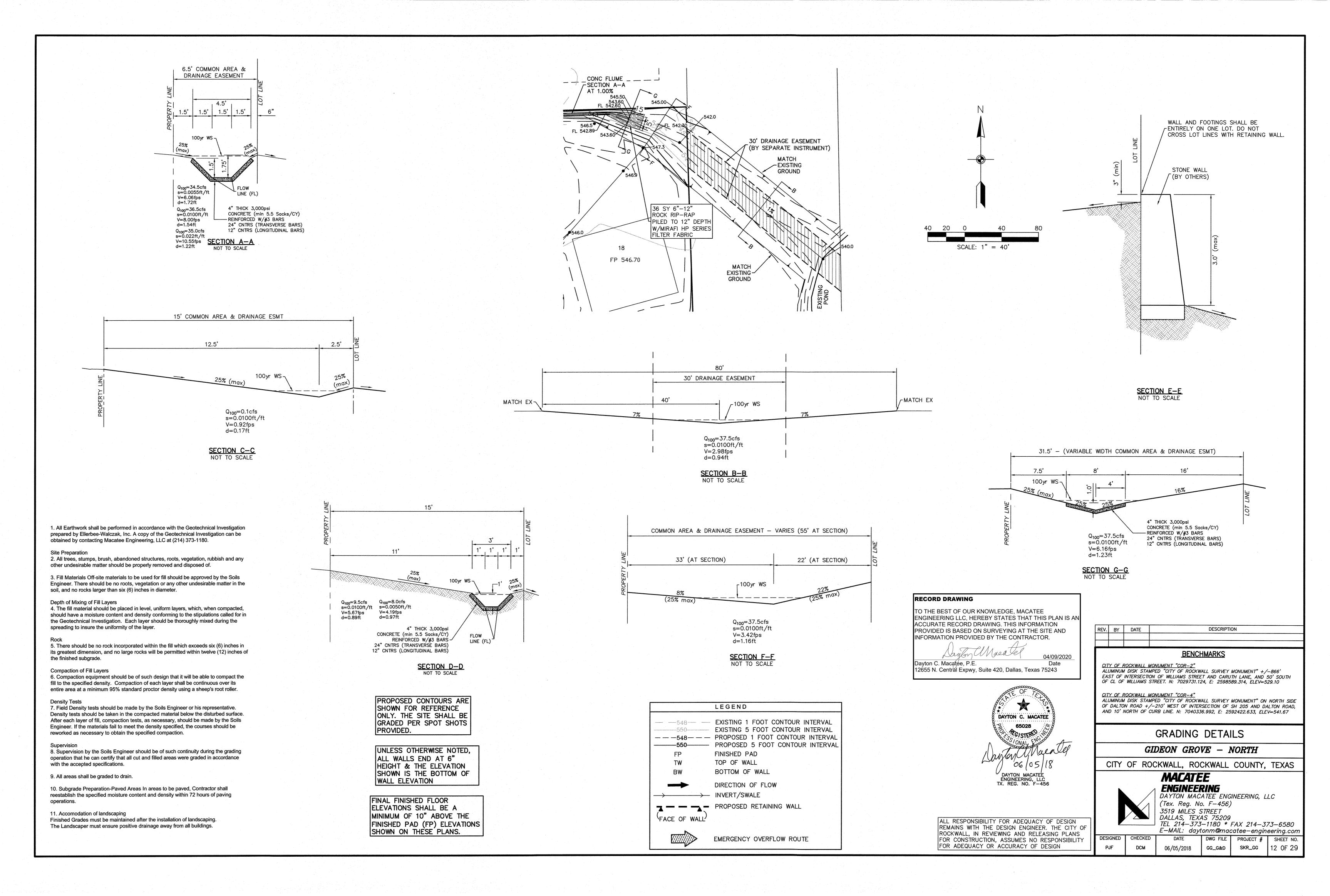
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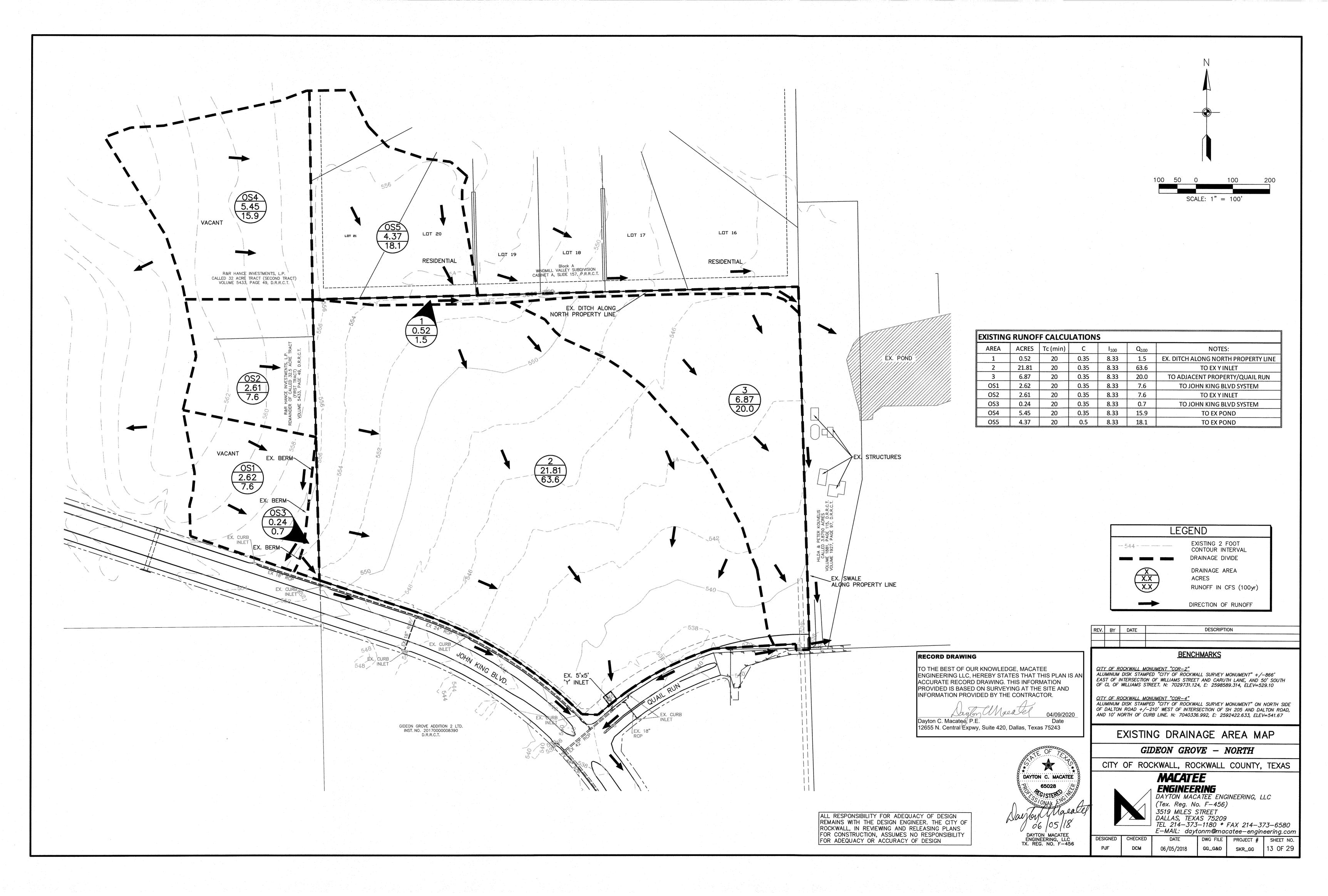


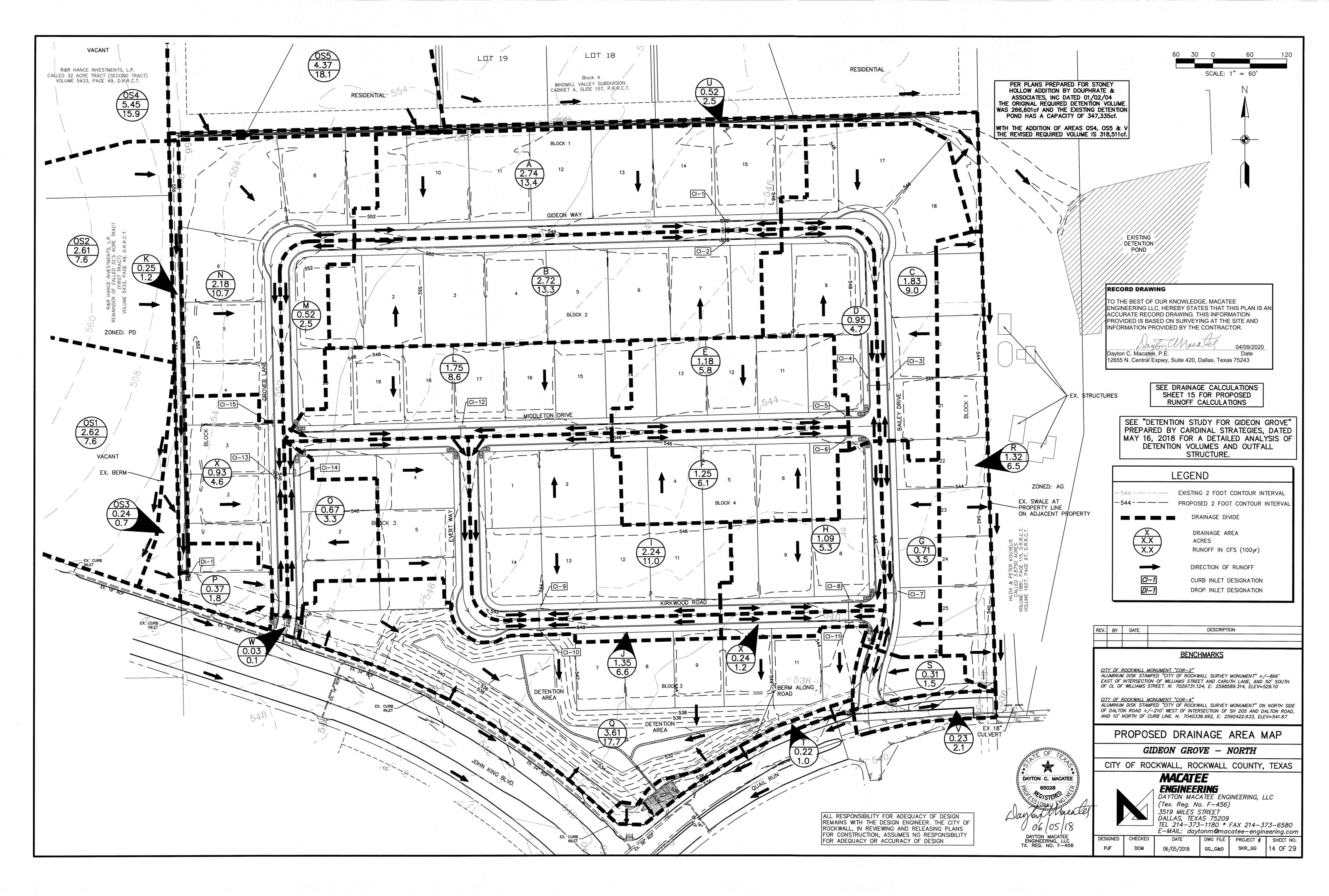












PIPE HYDRAULICS CALCULATIONS

											PII	² E F	HYD	RAUL	ICS	CA	LCU	LATI	ONS	5										
STOR	M LIN	EA																									,			
tations U/S	D/S	Length	Pipe Size	Туре	Pipe Area	Wetted Perimeter	Hydraulic Radius	Manning's	Basin Area	Runoff Coeff	Incre- mental	Accum- ulated	U/S Tc	Design Storm Freq	Intensity	Runoff	Velocity	/ Time in Conduit		Friction Headloss	HGL U/S	D/S	Headloss V ₁ ² /2g	Calculations V ₂ ² /2g	Jct Type	Coeff K _i	Headloss H _L	Design HGL	Top of Curb Elev.	HGL Depti
		(ft)	(in)		(ft ²)	Pw			(Ac)	С	C*A	C*A	(min)	(yr)		Q (cfs)	V (fps)		S _f	(ft)								1, 1		(ft)
1203	1181	22.00	21.00	RCP	2.41	5.50	0.44	0.013	2.72	0.50	1.36	1.36	10.00	100	9.80	13.3	5.54	0.07	0.0070	0.15	543.38	543.22	0.40	0.48	Inlet CI-2	1.25	0.60	543.97	546.07	2.10
179	1179 1176	3.00	33.00	RCP RCP	2.41 5.94	5.50 8.64	0.44	0.013	0.00	0.50	0.00	1.36	10.07	100	9.79	13.3	2.24	0.01	0.0070	0.01	543.05		0.48	0.48	45 Deg Bend Expansion	0.37	0.18	543.22 543.03	546.09 546.09	3.06
176	953	223.00	33.00	RCP	5.94	8.64	0.69	0.013	2.74	0.50	1.37	2.73	10.09	100	9.78	26.7	4.50	0.83	0.0025		542.47		0.08	0.31	45 Deg Wye	0.50	0.28	542.75	546.10	3.35
953	683	270.00	33.00	RCP	5.94	8.64	0.69	0.013	0.00	0.50	0.00	2.73	10.92	100	9.65	26.4	4.44	1.01	0.0025	0.67	541.77	541.11	0.31	0.31	MH	0.55	0.13	541.91	544.90	2.99
583	680	3.00	39.00	RCP	8.30	10.21	0.81	0.013	0.00	0.50	0.00	2.73	11.94	100	9.49	25.9	3.12	0.02	0.0010	0.00	541.02		0.31	0.15	Expansion		0.09	541.11	543.15	2.04
680 674	674	6.00 46.00	39.00 39.00	RCP RCP	8.30	10.21	0.81	0.013	1.83	0.50	0.48	3.21 4.12	11.95	100	9.49	30.4	3.67 4.71	0.03	0.0013	0.01	540.88	540.88	0.15	0.21	45 Deg Wye 45 Deg Wye	0.50	0.13	541.02	543.10 543.02	2.08
528	343	285.00	42.00	RCP	9.62	11.00	0.88	0.013	2.43	0.50	1.22	5.34	12.14	100	9.46	50.5	5.24	0.91	0.0025	0.71	540.19		0.34	0.43	MH	0.25	0.34	540.53		2.94
43	300	43.00	48.00	RCP	12.57	12.57	1.00	0.013	0.71	0.50	0.36	5.69	13.05	100	9.31	53.0	4.22	0.17	0.0014	0.06	539.42	539.36	0.43	0.28	45 Deg Wye	0.50	0.06	539.48	542.00	2.52
800	177	123.00	48.00	RCP	12.57	12.57	1.00	0.013	1.09	0.50	0.55	6.24	13.22	100	9.29	57.9	4.61	0.44	0.0016	0.20	539.17	538.97	0.28	0.33	45 Deg Wye	0.50	0.19	539.36	542.08	2.72
177	0	177.00	48.00	RCP	12.57	12.57	1.00	0.013	0.00	0.50	0.00	6.24	13.66	100	9.21	57.4	4.57	0.65	0.0016	0.28	538.80	538.52	0.33	0.32	MH	0.48	0.17	538.97	541.40	2.43
OR	M LAT	A-1	·															,									:			
tions		Length	Pipe Size	Туре	Pipe	Wetted		Manning's	Basin	Runoff	Incre-	Accum-	U/S	Design	Intensity	Runoff	Velocity	/ Time in	Friction	Friction		1		Calculations	Jct	Coeff	Headloss	Design	Top of	HGL Dept
J/S	D/S				Area	Perimeter Pw	Radius	n .	Area	Coeff C	mental C*A	ulated C*A	Тс	Storm Freq		· · · Q	V	Conduit	Slope S _f	Headloss	U/S	D/S	V ₁ ² /2g	V ₂ ² /2g	Туре	K _j	H _L	HGL	Curb Elev.	Below T/
69	33	(ft) 36.00	(in) 21.00	RCP	(ft ²) 2.41	5.50	0.44	0.013	(Ac) 1.09	0.50	0.55	0.55	(min) 10.00	(yr) 100	9.80	(cfs) 5.3	(fps) 2.22	0.27	0.0011	(ft) 0.04	539.54	539.50		0.08	Inlet CI-8	1.25	0.10	539.64	541.73	(ft) 2.09
33	0	33.00	21.00	RCP	2.41	5.50	0.44	0.013	0.24	0.50	0.12	0.67	10.27	100	9.76	6.5	2.70	0.20	0.0017	0.06	539.42	539.36	0.08	0.11	60 Deg Wye	0.35	0.09	539.50	541.85	2.35
OR	M LAT	A-1-1																												
ions		Length	Pipe Size	Туре	Pipe	Wetted	Hydraulic	Manning's	Basin	Runoff	Incre-	Accum-	U/S	Design	Intensity	Runoff	Velocity	Time in	Friction	Friction	HGL		Headloss	Calculations	Jct	Coeff	Headloss	Design	Top of	HGL Dept
J/S	D/S				Area	Perimeter Pw	Radius	n	Area	Coeff C	mental C*A	ulated C*A	Тс	Storm Freq		Q	V	Conduit	Slope S _f	Headloss	U/S	D/S	V ₁ ² /2g	V ₂ ² /2g	Туре	K _j	HL	HGL	Curb Elev.	Below T
26	0	(ft) 26.00	(in) 21.00	RCP	(ft ²) 2.41	5.50	0.44	0.013	(Ac) 0.24	0.50	0.12	0.12	(min) 10.00	(yr) 100	9.80	(cfs) 1.2	(fps) 0.49	0.89	0.0001	(ft) 0.00	539.50	539.50		0.00	Inlet CI-11	1.25	0.00	539.51	541.73	(ft) 2.22
			<u> </u>														1													
OR	M LAT	A-2				· · · · · · · · · · · · · · · · · · ·																								
ions I/S	D/S	Length	Pipe Size	Туре	Pipe Area	Wetted Perimeter	Hydraulic Radius	Manning's n	Basin Area	Runoff Coeff	Incre- mental	Accum- ulated	U/S Tc	Design Storm Freq	Intensity	Runoff	Velocity	Time in Conduit	Friction Slope	Friction Headloss	HGL U/S	D/S	Headloss V ₁ ² /2g	Calculations V ₂ ² /2g	Jct Type	Coeff K _j	Headloss H _L	Design HGL	Top of Curb Elev.	HGL Dept Below T/
		(ft)	(in)		(ft ²)	Pw			(Ac)	C	C*A	C*A	(min)	(yr)		Q (cfs)	(fps)		S _f	(ft)										(ft)
2	0	22.00	21.00	RCP	2.41	5.50	0.44	0.013	0.71	0.50	0.36	0.36	10.00	100	9.80	3.5	1.45	0.25	0.0005	0.01	539.49	539.48		0.03	Inlet CI-7	1.25	0.04	539.53	541.91	2.38
OR	M LAT	A-3							 	·																				
ions I/S	D/S	Length	Pipe Size	Туре	Pipe Area	Wetted Perimeter	Hydraulic Radius	Manning's	Basin Area	Runoff Coeff	Incre- mental	Accum- ulated	U/S Tc	Design Storm Freq	1	Runoff	Velocity	Time in	Friction Slope	Friction Headloss	HGL U/S	D/S	Headloss V ₁ ² /2g	Calculations V ₂ ² /2g	Jct Type	Coeff K _i	Headloss	Design HGL	Top of Curb Elev.	HGL Dept
		(ft)	(in)		(ft ²)	Pw			(Ac)	С	C*A	C*A	(min)	(yr)	l	Q (cfs)	V (fps)		S _f	(ft)				٠.						(ft)
59	37	22.00	21.00	RCP	2.41	5.50	0.44	0.013	1.18	0.50	0.59	0.59	10.00	100	9.80	5.8	2.40	0.15	0.0013		541.19			0.09	Inlet CI-5	1.25	0.11		543.75	2.45
37	0	3.00	21.00	RCP RCP	2.41	5.50	0.44	0.013	1.25	0.50	0.00	1.22	10.15	100	9.78	5.8 11.9	2.40 4.94	0.02	0.0013	0.00	541.06 540.72	541.05 540.53	0.09	0.09	45 Deg Bend 45 Deg Wye	0.37	0.10	541.16 541.05	543.50 543.50	2.34 2.45
ORI	M LAT	A-3-1																												
ions		Length	Pipe Size	Туре	Pipe	Wetted	Hydraulic	Manning's	Basin	Runoff	Incre-	Accum-	U/S	Design	Intensity	Runoff	Velocity	Time in	Friction	Friction	HGL		Headloss	Calculations	Jct	Coeff	Headloss	Design	Top of	HGL Dept
J/S	D/S				Area	Perimeter Pw	Radius	n	Area	Coeff C	mental C*A	ulated C*A	Tc	Storm Freq	1	Q	V	Conduit	Slope S _f	Headloss	U/S	D/S	V ₁ ² /2g	V ₂ ² /2g	Туре	K _j	HL	HGL	Curb Elev.	Below T/
22	0	(ft) 22.00	(in) 21.00	RCP	(ft ²)	5.50	0.44	0.013	(Ac) 1.25	0.50	0.63	0.63	(min) 10.00	(yr) 100	9.80	(cfs) 6.1	(fps) 2.55	0.14	0.0015	(ft) 0.03	541.08	541.05		0.10	Inlet CI-6	1.25	0.13	541.21	543.62	(ft) 2.41
'OD!	MIAT																			1										
	M LAT			·			T							r <u>-</u>																
ons /S	D/S	Length	Pipe Size	Туре	Pipe Area	Wetted Perimeter	Hydraulic Radius	Manning's n	Basin Area	Runoff Coeff	Incre- mental	Accum- ulated	U/S Tc	Design Storm Freq	Intensity		Velocity	Time in Conduit	Friction Slope	Friction Headloss	HGL U/S	D/S	Headloss V ₁ ² /2g	Calculations V ₂ ² /2g	Jct Type	Coeff K _j	Headloss H _L	Design HGL	Top of Curb Elev.	HGL Dept Below T/
10		(ft)	(in)	200	(ft²)	PW	0.44	0.013	(Ac)	C	C*A	C*A	(min)	(yr)	0.00	Q (cfs)	(fps)	0.16	S _f	(ft)	F40.00	F40.00		0.00	Indian CLA	1 25	0.07	E40.00	543.02	(ft) 2.03
18	0	18.00	21.00	RCP	2.41	5.50	0.44	0.013	0.95	0.50	0.48	0.48	10.00	100	9.80	4.7	1.94	0.16	0.0009	0.02	540.92	540.90		0.06	Inlet CI-4	1.25	0.07	540.99	543.02	2.03
ORI	M LAT	A-5				·																								
ions I/S	D/S	Length	Pipe Size	Туре	Pipe Area	Wetted Perimeter	Hydraulic Radius	Manning's	Basin Area	Runoff Coeff	Incre- mental	Accum- ulated	U/S Tc	Design Storm Freq	Intensity	Runoff	Velocity	Time in	Friction Slope	Friction Headloss	HGL U/S	D/S	Headloss V ₁ ² /2g	Calculations V ₂ ² /2g	Jct Type	Coeff K _i	Headloss H _i	Design HGL	Top of Curb Elev.	HGL Dept
, -	J, J	(ft)	(in)		(ft ²)	Pw		••	(Ac)	C	C*A	C*A	(min)	(yr)		Q (cfs)	V (fps)	Jonault	S _f	(ft)	5,5	<i>-,</i> -	-1/45	-	7,00		• • •			(ft)
22	0	22.00	24.00	RCP	3.14	6.28	0.50	0.013	1.81	0.50	0.91	0.91	10.00	100	9.80	8.9	2.82	0.13	0.0015	0.03	540.83	540.80		0.12	Inlet CI-3	1.25	0.15	540.99	543.02	2.03
ORI	M LAT	A-6							· · · · · · · · · · · · · · · · · · ·																			<u> </u>		, , , , , , , , , , , , , , , , , , ,
ions		Length	Pipe Size	Туре	Pipe	Wetted	Hydraulic	Manning's	Basin	Runoff	Incre-	Accum-	U/S	Design	Intensity	Runoff	Velocity	Time in	Friction	Friction	HGL		Headloss	Calculations	Jct	Coeff	Headloss	Design	Top of	HGL Dept
l/S	D/S		.,	1,700	Area	Perimeter Pw	Radius	n	Area	Coeff C	mental C*A	ulated C*A	• •	Storm Freq		Q	V	Conduit		Headloss		D/S	V ₁ ² /2g	V ₂ ² /2g	Туре	K _j	H _L		Curb Elev.	Below T/C
22	0	(ft) 22.00	(in) 21.00	RCP	(ft ²) 2.41	5.50	0.44	0.013	(Ac) 2.62	0.50	1.31	1.31	(min) 10.00	(yr) 100	9.80	(cfs) 12.8	(fps) 5.34	0.07	0.0065	(ft) 0.14	542.89	542.75		0.44	Inlet CI-1	1.25	0.55	543.45	546.06	(ft) 2.61
										· · · · ·					-	-														

PROPOS	ED RUNG	OFF CALC	ULATIO	NS		
AREA	ACRES	Tc (min)	C :	1100	Q100	NOTES:
Α	2.74	10	0.5	9.8	13.4	CI-1 (TO DETENTION POND)
В	2.72	10	0.5	9.8	13.3	CI-2 (TO DETENTION POND)
С	1.83	10	0.5	9.8	9.0	CI-3 (TO DETENTION POND)
D	0.95	10	0.5	9.8	4.7	CI-4 (TO DETENTION POND)
E	1.18	10	0.5	9.8	5.8	CI-5 (TO DETENTION POND)
F	1.25	10	0.5	9.8	6.1	CI-6 (TO DETENTION POND)
G	0.71	10	0.5	9.8	3.5	CI-8 (TO DETENTION POND)
Н	1.09	10	0.5	9.8	5.3	CI-7 (TO DETENTION POND)
ı	2.24	10	0.5	9.8	11.0	CI-9 (TO DETENTION POND)
J	1.35	10	0.5	9.8	6.6	CI-10 (TO DETENTION POND)
K	0.25	10	0.5	9.8	1.2	DI-1 (TO DETENTION POND)
L	1.75	10	0.5	9.8	8.6	CI-12 (TO DETENTION POND)
M	0.52	10	0.5	9.8	2.5	CI-12 (TO DETENTION POND)
N	2.18	10	0.5	9.8	10.7	CI-13 (TO DETENTION POND)
0	0.67	10	0.5	9.8	3.3	CI-14 (TO DETENTION POND)
Р	0.37	10	0.5	9.8	1.8	CI-13 (TO DETENTION POND)
Q	3.61	10	0.5	9.8	17.7	TO DETENTION POND
R	1.32	10	0.5	9.8	6.5	TO ADJACENT PROPERTY (BYPASS DETENTION)
S	0.31	10	0.5	9.8	1.5	TO QUAIL RUN (BYPASS DETENTION)
T	0.22	10	0.5	9.8	1.0	TO QUAIL RUN (BYPASS DETENTION)
U	0.52	10	0.5	9.8	2.5	TO EX POND (BYPASS DETENTION)
V	0.23	10	0.9	9.8	2.1	TO EX 18" CULVERT (BYPASS DETENTION)
W	0.03	10	0.5	9.8	0.1	TO JOHN KING BLVD (BYPASS DETENTION)
Х	0.93	10	0.5	9.8	4.6	CI-13 (TO DETENTION POND)
OS1	2.62	20	0.35	8.33	7.6	TO JOHN KING BLVD
OS2	2.61	20	0.35	8.33	7.6	TO DI-1 (TO DETENTION POND)
OS3	0.24	20	0.35	8.33	0.7	TO DI-1 (TO DETENTION POND)
OS4	5.45	20	0.35	8.33	15.9	TO EX POND
OS5	4.37	20	0.5	8.33	18.1	TO EX POND

nlet Calculations															· · · · · · · · · · · · · · · · · · ·	
Inlet Designation	Inlet Type	Runoff	Upstream Bypass	Total Gutter Flow	Long Slope	Cross Slope	Depression Depth	Depression Width	Flow Depth in Depressed	Ratio of Depressed flow to Total Flow	Equivalent Cross Section	Required Inlet Length	Actual Inlet Length	Inlet Capacity	Bypass Flow	To
		Q ₁₀₀	Q ₁₀₀	Q ₁₀₀	S	S _x	a	W	Section	(E _o)	(S _e)					
		(cfs)	(cfs)	(cfs)	(ft/ft)	(ft/ft)	(ft)	(ft)	(ft)		(ft/ft)	(ft)	(ft)	(cfs)	(cfs)	
		<u> </u>		- <u> </u>												
CI-1	Sag	13.4	0	13.4	0.0080	0.033	0.5	2	0.50	n/a	n/a	12.88	15	15.1	0	n/:
Cl-2	Sag	13.3	0	13.3	0.0080	0.033	0.5	2	0.50	n/a	n/a	12.76	15	15.1	0	n/a
Cl-3	Sag	9.0	0	9.0	0.0070	0.033	0.5	2	0.44	n/a	n/a	9.81	10	11.1	0	n/a
CI-4	Sag	4.7	0	4.7	0.0070	0.033	0.5	2	0.29	n/a	n/a	9.48	10	11.1	0	n/a
CI-5	On-Grade	5.8	0	5.8	0.0073	0.033	0.5	2	0.40	0.45	0.2355	7.74	10	10.6	0	n/a
CI-6	On-Grade	6.1	0	6.1	0.0073	0.033	0.5	2	0.40	0.45	0.2355	7.91	10	10.6	0	n/a
CI-7	Sag	3.5	0	3.5	0.0070	0.033	0.5	2	0.24	n/a	n/a	9.34	10	11.1	0	n/a
CI-8	Sag	5.3	0	5.3	0.0130	0.033	0.5	2	0.35	n/a	n/a	7.53	10	11.1	0	n/a
CI-9	Sag	11.0	0	11.0	0.0074	0.033	0.5	2	0.50	n/a	n/a	9.75	10	11.1	0	n/a
CI-10	Sag	6.6	0	6.6	0.0074	0.033	0.5	2	0.36	n/a	n/a	9.69	10	11.1	-0	n/a
CI-11	Sag	1.2	0	1.2	0.0130	0.033	0.5	2	0.15	n/a	n/a	5.38	10	11.1	0	n/a
CI-12	Sag	11.1	0	11.1	0.0078	0.033	0.5	2	0.50	n/a	n/a	10.00	10	11.1	0	n/a
CI-13	Sag	6.4	3.4	9.8	0.0119	0.033	0.5	2	0.47	n/a	n/a	5.04	10	11.1	0	n/a
CI-14	Sag	3.3	0	3.3	0.0119	0.033	0.5	2	0.23	n/a	n/a	9.41	10	11.1	0	n/a
CI-15	On-Grade	10.7	0	10.7	0.0100	0.033	0.5	2	0.46	0.45	0.249	10.64	5	7.2	3.4	CI-:
DI-1	Drop	10.5	0	10.5	n/a	n/a	n/a	n/a	0.50	n/a	n/a	9.62	20	21.8	0	n/a

GUTTER CAPA	CITY CHART
STREET SLOPE (%)	CAPACITY (cfs)
0.70	13.8
0.80	14.7
1.00	16.4
1.25	18.4
1.50	20.1
2.00	23.2

RECORD DRAWING

TO THE BEST OF OUR KNOWLEDGE, MACATEE ENGINEERING LLC, HEREBY STATES THAT THIS PLAN IS AN ACCURATE RECORD DRAWING. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.

Dayton C. Macatee, P.E. Date 12655 N. Central Expwy, Suite 420, Dallas, Texas 75243 04/09/2020

DAYTON C. MACATEE Dayloy Marales DAYTON MACATEE ENGINEERING, LLC TX. REG. NO. F-456

DRAINAGE CALCULATIONS

GIDEON GROVE - NORTH

CITY OF ROCKWALL, ROCKWALL COUNTY, TEXAS



MACATEE ENGINEERING

DAYTON MACATEE ENGINEERING, LLC

(Tex. Reg. No. F-456)

3519 MILES STREET

DALLAS, TEXAS 75209

TEL 214-373-1180 * FAX 214-373-6580

E-MAIL: daytonm@macatee-engineering.com

DESIGNED CHECKED DWG FILE PROJECT # SHEET NO.

PJF DCM

06/05/2018

GG_G&D SKR_GG 15 0F 29

ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS
FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY
FOR ADEQUACY OR ACCURACY OF DESIGN

PIPE HYDRAULICS CALCULATIONS

STORM	L	IN	E	B

tions		Length	Pipe Size	Туре	Area	Wetted	Hydraulic	Manning's	Area	Runoff	Incre-	Accum-	U/S	Design	Intensity	Runoff	Velocity	Time in	Friction	Friction	HGL			Calculations	Jct	Coeff	Headloss	Design	Top of	HGL Dep
U/S	D/S	Lengui	r ipe size	1,500	7.1.50	Perimeter	Radius	n		Coeff	mental	ulated	Тс	Storm Freq				Conduit	Slope	Headloss	U/S	D/S	V ₁ ² /2g	V ₂ ² /2g	Туре	· K _j	H_L	HGL	Curb Elev.	Below
0/3	D/3					Pw				С	C*A	C*A			1	Q	٧		S _f											
		(£L)	/im\		(ft ²)				(Ac)				(min)	(yr)		(cfs)	(fps)			(ft)										(ft)
1444	056	(ft) 155.00	(in) 18.00	RCP	1.77	4.71	0.38	0.013	1.94	0.50	0.97	0.97	10.00	100	9.80	9.5	5.38	0.48	0.0081	1.26	547.19	545.93		0.45	Drop Inlet 1	1.25	0.56	547.75	549.90	2.15
1111	956	155.00	18.00	NCF	1.//	7.71	0.30	0.013	1.5	- 0.00																		F 4 F 00	540.00	2.0
956	949	7.00	18.00	RCP	1.77	4.71	0.38	0.013	0.00	0.50	0.00	0.97	10.48	100	9.72	9.4	5.34	0.02	0.0080	0.06	545.76	545.71	0.45	0.44	45 Deg Bend	0.37	0.16	545.93	548.00	2.07
-																			0.0000	4.40	E 4 E E 4	FAA 11	0.44	0.44	45 Deg Bend	0.37	0.16	545.71	547.90	2.19
949	770	179.00	18.00	RCP	1.77	4.71	0.38	0.013	0.00	0.50	0.00	0.97	10.50	100	9.72	9.4	5.34	0.56	0.0080	1.43	545.54	544.11	0.44	0.44	45 Deg Bend	0.57	0.10	3-13.71	J	
										6.50	0.00	0.07	11.00	100	9.63	9.3	1.90	0.03	0.0005	0.00	543.83	543.82	0.44	0.06	Expansion	-	0.28	544.11	546.30	2.19
770	767	3.00	30.00	RCP	4.91	7.85	0.63	0.013	0.00	0.50	0.00	0.97	11.06	100	9.63	9.5	1.50	0.03	0.0003	0.00	343.03	3.0.02								
		4.00	20.00	DCD.	4.01	7.85	0.63	0.013	2.00	0.50	1.00	1.97	11.09	100	9.63	19.0	3.86	0.02	0.0021	0.01	543.62	543.61	0.06	0.23	45 Deg Wye	0.50	0.20	543.82	546.30	2.48
767	763	4.00	30.00	RCP	4.91	7.65	0.03	0.013	2.00	0.50	1.00	1.37	11.03																	
763	732	31.00	30.00	RCP	4.91	7.85	0.63	0.013	0.67	0.50	0.34	2.31	11.10	100	9.62	22.2	4.52	0.11	0.0029	0.09	543.41	543.32	0.23	0.32	45 Deg Wye	0.50	0.20	543.61	546.30	2.69
703	732	31.00																				= 44 = 5	0.00	0.55	NAL.	0.55	0.30	543.32	546.57	3.25
732	432	300.00	30.00	RCP	4.91	7.85	0.63	0.013	1.48	0.50	0.74	3.05	11.22	100	9.60	29.2	5.96	0.84	0.0051	1.52	543.02	541.50	0.32	0.55	MH	0.55	0.30	343.32	340.37	3.23
													10.00	400	0.47	20.7	5.62	0.89	0.0035	1.06	541.15	540.09	0.55	0.49	MH	0.25	0.35	541.50	544.65	3.15
432	132	300.00	36.00	RCP	7.07	9.42	0.75	0.013	2.30	0.50	1.15	4.20	12.06	100	9.47	39.7	3.02	0.83	0.0035	1.00	341.13	3,0.03	0.00							
													10.05	400	0.22	39.1	5.54	0.22	0.0034	0.25	539.83	539.58	0.49	0.48	MH	0.55	0.26	540.09	542.55	2.46
132	59	73.00	36.00	RCP	7.07	9.42	0.75	0.013	0.00	0.50	0.00	4.20	12.95	100	9.33	39.1	3.34	0.22	0.0034	0.23	333.03	333.30								
			00.00	0.00	7.07	0.42	0.75	0.013	0.00	0.50	0.00	4.20	13.17	100	9.29	39.0	5.52	0.02	0.0034	0.03	539.40	539.37	0.48	0.47	45 Deg Bend	0.37	0.17	539.58	542.20	2.62
59	51	8.00	36.00	RCP	7.07	9.42	0.75	0.013	0.00	0.50	0.00	7.20	13.17																	
51	40	11.00	36.00	RCP	7.07	9.42	0.75	0.013	0.00	0.50	0.00	4.20	13.19	100	9.29	39.0	5.51	0.03	0.0034	0.04	539.20	539.16	0.47	0.47	45 Deg Bend	0.37	0.17	539.37	542.20	2.83
21	40	11.00	30.00		,																		0.47	0.70	AE Dog Miss	0.50	0.46	539.16	542.00	2.84
40	0	40.00	39.00	RCP	8.30	10.21	0.81	0.013	3.59	0.50	1.80	5.99	13.23	100	9.28	55.6	6.70	0.10	0.0045	0.18	538.70	538.52	0.47	0.70	45 Deg Wye	0.50	0.46	333.10	342.00	2.0
																														

STORM LAT B-1

										1 1								· · · · · · · · · · · · · · · · · · ·										0	T6	LICI Donth
Ctations		Longth	Pipe Size	Type	Area	Wetted	Hydraulic	Manning's	Area	Runoff	Incre-	Accum-	U/S	Design	Intensity	Runoff	Velocity	Time in	Friction	Friction	HGL		Headloss (Calculations	Jct	Coeff	Headloss	Design	Top of	HGL Depth
Stations U/S		Length	ripe size	Type	Aica	Perimeter	Radius	n		Coeff	mental	ulated	Тс	Storm Freq	1					Headloss	1 1	D/S	V ₁ ² /2g	V ₂ ² /2g	Type	, K _j	HL	HGL	Curb Elev.	Below T/C
0/3	0/3					Pw				С	C*A	C*A			e 1 1;	Q	V		S_f											
		(f+)	4, ,		/f+ ² \				(Ac)				(min)	(vr)		(cfs)	(fps)			(ft)										(ft)
		(10)	(in)	 	(11)	2.00	0.50	0.013	(1.0)	0.50	1.12	1.12	10.00	100	9.80	11.0	3.49	0.18	0.0023	0.09	539.68	539.60		0.19	Inlet CI-9	1.25	0.24	539.92	541.96	2.04
45	8	37.00	24.00	RCP	3.14	6.28	0.50	0.013	2.24	0.50	1.12	1.12	10.00	100	J.00	11.0														
						100							40.40	100	0.77	17.5	F 50	0.02	0.0060	0.05	539.21	539.16	0.19	0.48	45 Deg Wye	0.50	0.39	539.60	541.96	2.36
8	0	8.00	24.00	RCP	3.14	6.28	0.50	0.013	1.35	0.50	0.68	1.80	10.18	100	9.77	17.5	5.58	0.02	0.0000	0.03	333.21	333.10		<u> </u>	3					
																		<u> </u>								L		<u> </u>	L	

STORM LAT B-1-1

Cl. N		Longth	Dino Sizo	Type	Area	Wetted	Hydraulic	Manning's	Area	Runoff	Incre-	Accum-	U/S	Design	Intensity	Runoff	Velocity	Time in	Friction	Friction	HGL.		Headloss	Calculations	Jct	Coeff	Headloss	Design	Top of	HGL Dept
Stations U/S	D/S	Length	Pipe Size	Туре	Area	Perimeter	Radius	n	Aica	Coeff	mental	ulated		Storm Freq			1			Headloss		D/S	V ₁ ² /2g	V ₂ ² /2g	Type	K _j	HL	HGL	Curb Elev.	Below T
3,3	,,,					Pw				С	C*A	C*A			1	Q	V		S _f											/£+\
		(ft)	(in)		(ft ²)				(Ac)				(min)	(yr)		(cfs)	(fps)			(ft)									F 40 00	(11)
20	21	7.00	21.00	RCP	2.41	5.50	0.44	0.013	1.37	0.50	0.69	0.69	10.00	100	9.80	6.7	2.79	0.04	0.0018	0.01	539.69	539.68		0.12	Inlet CI-10	1.25	0.15	539.85	542.00	2.15
28	21	7.00	21.00	INCI	2,71	1 3.30	<u> </u>	1 0.0_0																						
21		21.00	21.00	RCP	2 41	5.50	0.44	0.013	0.00	0.50	0.00	0.69	10.04	100	9.79	6.7	2.79	0.13	0.0018	0.04	539.64	539.60	0.12	0.12	45 Deg Bend	0.37	0.04	539.68	542.00	2.32
21	<u> </u>	21.00	21.00	1.01	~T.L	3.30																						<u> </u>		

STORM LAT B-2

					***************************************															,							T - cc			Tf	UCI Donth
Statio	n.c	T	Length	Pipe Size	Type	Area	Wetted	Hydraulic	Manning's	Area	Runoff	Incre-	Accum-	U/S	Design	Intensity	Runoff	Velocity	Time in	Friction	Friction	HGL	1	Headloss	Calculations	Jct	Coeff	Headloss	Design	1 '	HGL Depth
Statio U/S		D/S	rengui	ripe size	Type	Alea	Perimeter	Radius	n			mental	ulated	Тс	Storm Freq				Conduit	Slope	Headloss	U/S	D/S	V ₁ ² /2g	$V_2^2/2g$	Type	Kj	HL	HGL	Curb Elev.	Below T/C
"	´ '						Pw				С	C*A	C*A			1	Q	V		S _f											
l			(ft)	/in\		(ft ²)				(Ac)				(min)	(yr)		(cfs)	(fps)			(ft)										(ft)
			(10)	31.00	RCP	2.41	5.50	0.44	0.013	2.30	0.50	1.15	1.15	10.00	100	9.80	11.3	4.69	0.06	0.0050	0.09	541.59	541.50		0.34	Inlet CI-12	1.25	0.43	542.01	544.63	2.62
1/		0	17.00	21.00	NCP	Z.41	3.30	0.44	0.013	2.50	0.50										,									<u></u> '	

STORM LAT B-3

			1	T _		187-44-31	Livelegatio	Mannings	Aron	Punoff	Incre-	Accum-	U/S	Design	Intensity	Runoff	Velocity	Time in	Friction	Friction	HGL		Headloss	Calculations	Jct	Coeff	Headloss	Design	Top of	HGL Deptl
Stations		Length	Pipe Size	Туре	Area	Wetted		Manning's	Area	Runoff				T				1		Headloss	1	D/S	V ₁ ² /2g	$V_{2}^{2}/2g$	Type	K;	н	HGL	Curb Elev.	Below T/
U/S	D/S			1		Perimeter	Radius	n		Coeff	mental	ulated	Tc	Storm Freq				Conduit	Slope	neauloss	0/3	0,3	V1/28	127-6	1	,			1	
, ,	,		,			Pw				С	C*A	C*A				Q	V		Sf											
		(ft)	(in)		(ft ²)				(Ac)				(min)	(yr)		(cfs)	(fps)			(ft)							<u> </u>			(ft)
		(,	(111)	 	(10)	F F0	0.44	0.013	0.67	0.50	0.24	0.34	10.00	100	9.80	3.3	1.36	0.27	0.0004	0.01	543.62	543.61		0.03	Inlet CI-14	1.25	0.04	543.66	546.29	2.63
22	0	22.00	21.00	RCP	2.41	5.50	0.44	0.013	0.67	0.30	0.34	0.34	10.00	1	3.00	+										1	i i			
																			<u> </u>	<u> </u>	1			<u> </u>				<u> </u>	· · · · · · · · · · · · · · · · · · ·	

STORM LAT B-4

					,			· · · · · · · · · · · · · · · · · · ·				- 66	.		11/6	Dasies	Intensity	Punoff	Volocity	Time in	Friction	Friction	HGI		Headloss	Calculations	Jct	Coeff	Headloss	Design	Top of	HGL Depth
Stations	T = /s	_ L	Length	Pipe Size	Туре	Area		Vetted rimeter	Hydraulic Radius	Manning's	Area	Runoff Coeff	Incre- mental	Accum- ulated	U/S Tc	Design Storm Freq	Intensity	Kulloli	Velocity	Conduit		Headloss		D/S	V ₁ ² /2g	V ₂ ² /2g	Туре	K _j	H_L	HGL	Curb Elev.	Below T/C
U/S	D/S						Pei	Pw	Naulus			C	C*A	C*A			1	Q	٧		S _f											
			/f+\	(in)		(ft ²)					(Ac)				(min)	(yr)		(cfs)	(fps)			(ft)										(ft)
22	0		22.00	21.00	RCP	2.41		5.50	0.44	0.013	2.00	0.50	1.00	1.00	10.00	100	9.80	9.8	4.07	0.09	0.0038	0.08	543.91	543.83		0.26	Inlet CI-13	1.25	0.32	544.24	546.29	2.05
44-	+																															

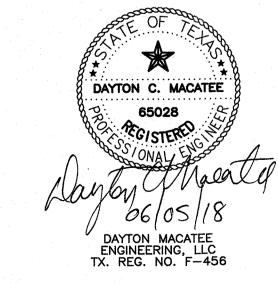
STORM LAT B-5

							,																				T				
							1	1		A	Dunoff	Inoro	Accum	U/S	Design	Intensity	Runoff	Velocity	Time in	Friction	Friction	HGL		Headloss	Calculations	Jct	Coeff	Headloss	Design	Top of	HGL Depth
Stati	ons		Length	Pipe Size	Туре	Area	Wetted	Hydraulic	Manning's	Area	Runoff	Incre-	Accum-				11011011		1		1		D/s	V ₁ ² /2g	V ₂ ² /2g	Type	κ.	н.	HGL	Curb Elev.	Below T/C
	/S	D/S					Perimeter	Radius	n		Coeff	mental	ulated	Тс	Storm Freq	1			Conduit	Slope	Headloss	U/S	D/S	V1/48	V2 / 48	Турс	"	• • •			
ľ	/3	0/3			1						_	C*A	C*A				O	v		Sf									1	·	
							PW					CA	C A				_	•			(6.)					· .			1		(ft)
			(f+)	(in)		(ft ²)				(Ac)				(min)	(yr)		(cfs)	(fps)			(ft)										(10)
			(11)	(111)		(10)				4.40	0.50	0.74	0.74	10.00	100	9.80	7.3	4.10	0.13	0.0047	0.15	543.47	543.32		0.26	Inlet CI-15	1.25	0.33	543.79	546.86	3.07
	1	0	31.00	18.00	RCP	1.77	4.71	0.38	0.013	1.48	0.50	0.74	0.74	10.00	100	9.60	7.5	7.20	0.20												
<u> </u>					T			1			[1	l	1		1	ļ	1		i .			l	·		<u> </u>	<u> </u>		<u> </u>		<u> </u>

RECORD DRAWING

TO THE BEST OF OUR KNOWLEDGE, MACATEE ENGINEERING LLC, HEREBY STATES THAT THIS PLAN IS AN ACCURATE RECORD DRAWING. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.

Dayton C. Macatee, P.E. Date 12655 N. Central Expwy, Suite 420, Dallas, Texas 75243



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DRAINAGE CALCULATIONS

GIDEON GROVE - NORTH

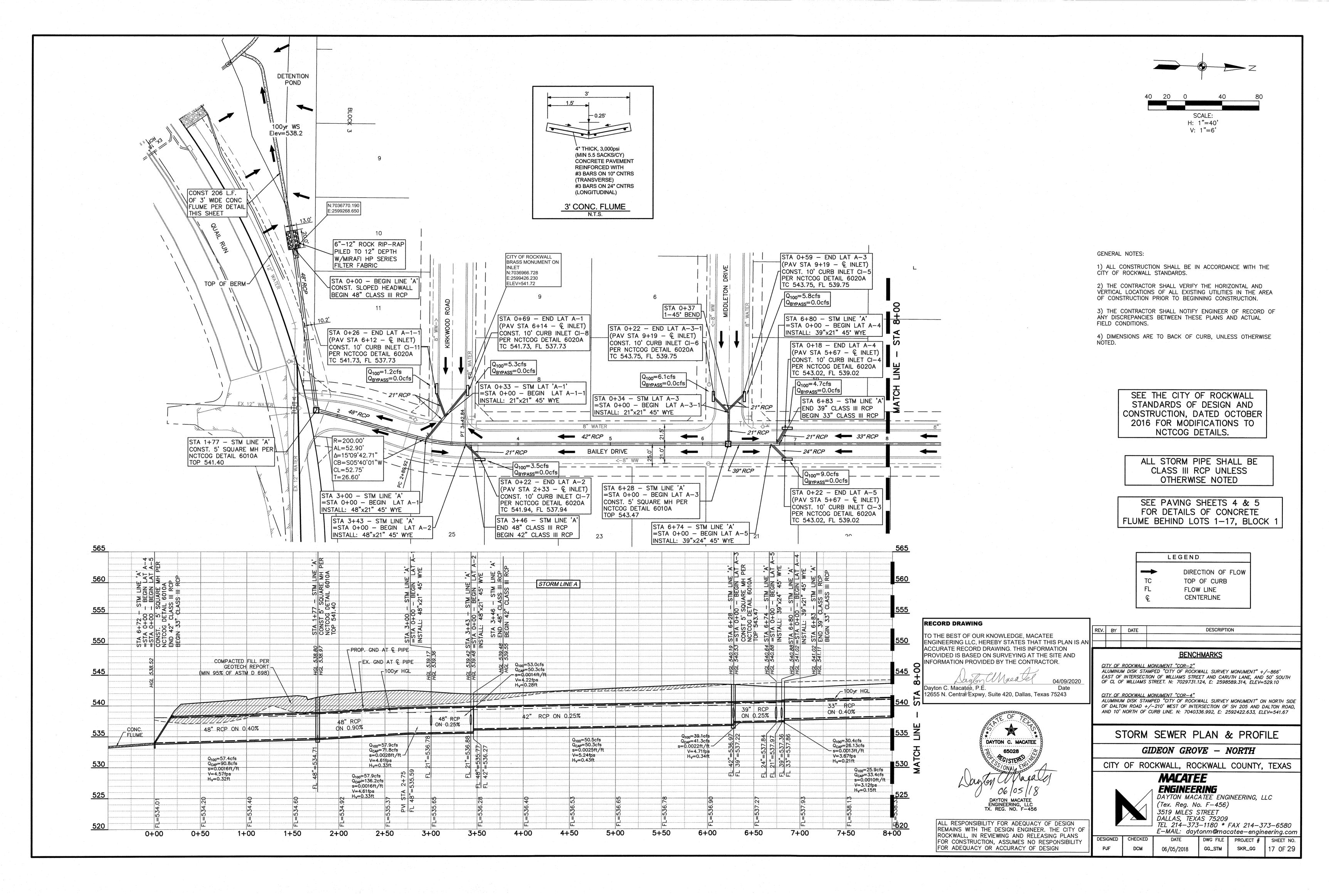
CITY OF ROCKWALL, ROCKWALL COUNTY, TEXAS

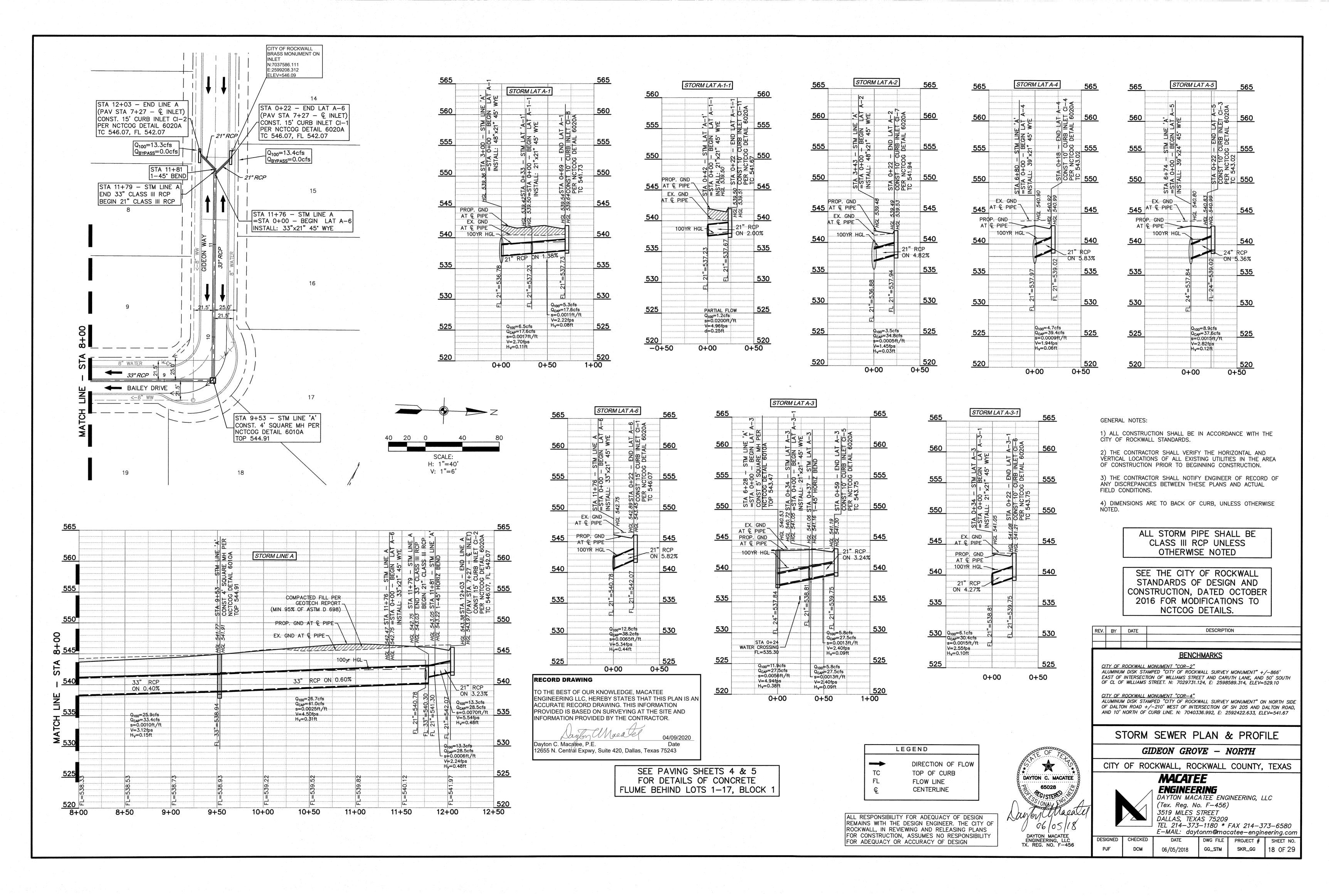


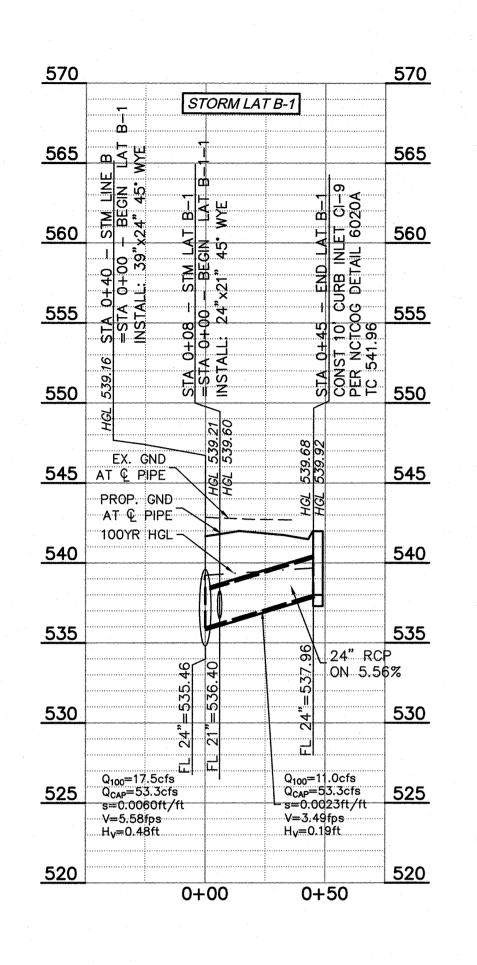
MACATEE
ENGINEERING
DAYTON MACATEE ENGINEERING, LLC
(Tex. Reg. No. F-456)

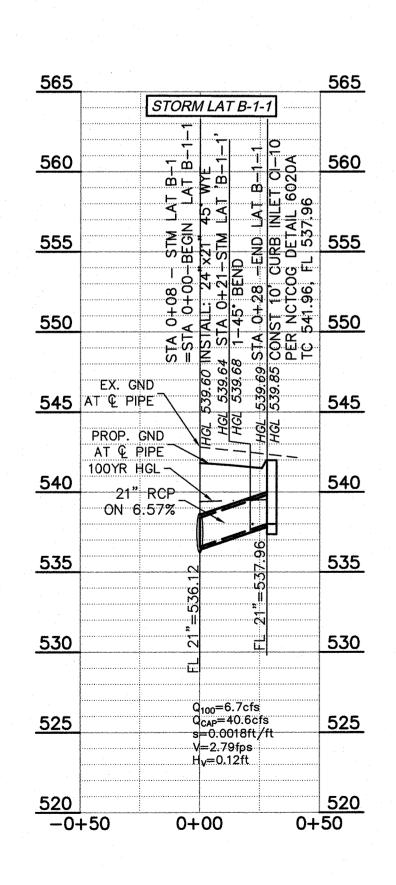
(Tex. Reg. No. F-456)
3519 MILES STREET
DALLAS, TEXAS 75209
TEL 214-373-1180 * FAX 214-373-6580
E-MAIL: daytonm@macatee-engineering.com

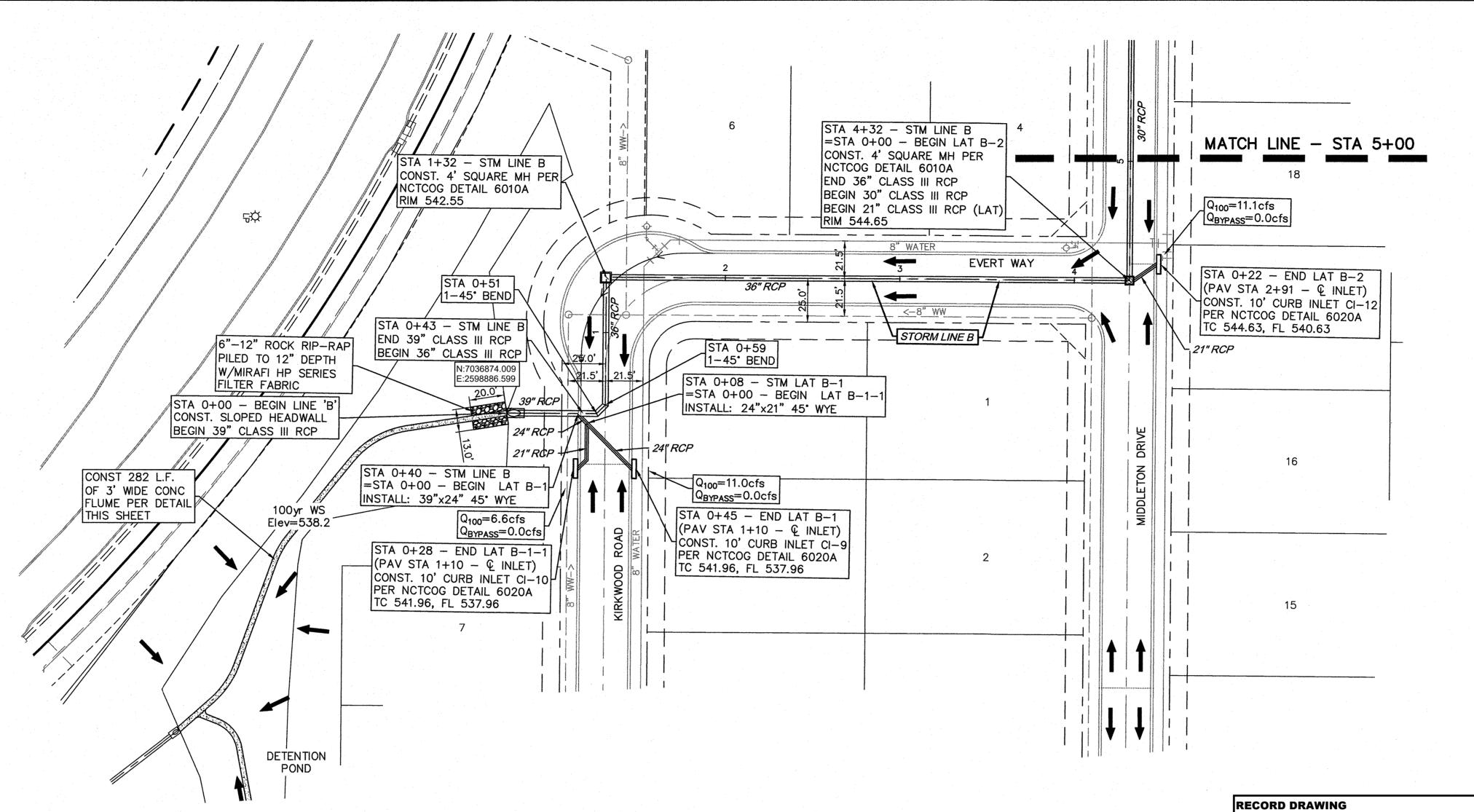
DESIGNED CHECKED DATE DWG FILE PROJECT # SHEET NO.
PJF DCM 06/05/2018 GG_G&D SKR_GG 16 OF 29

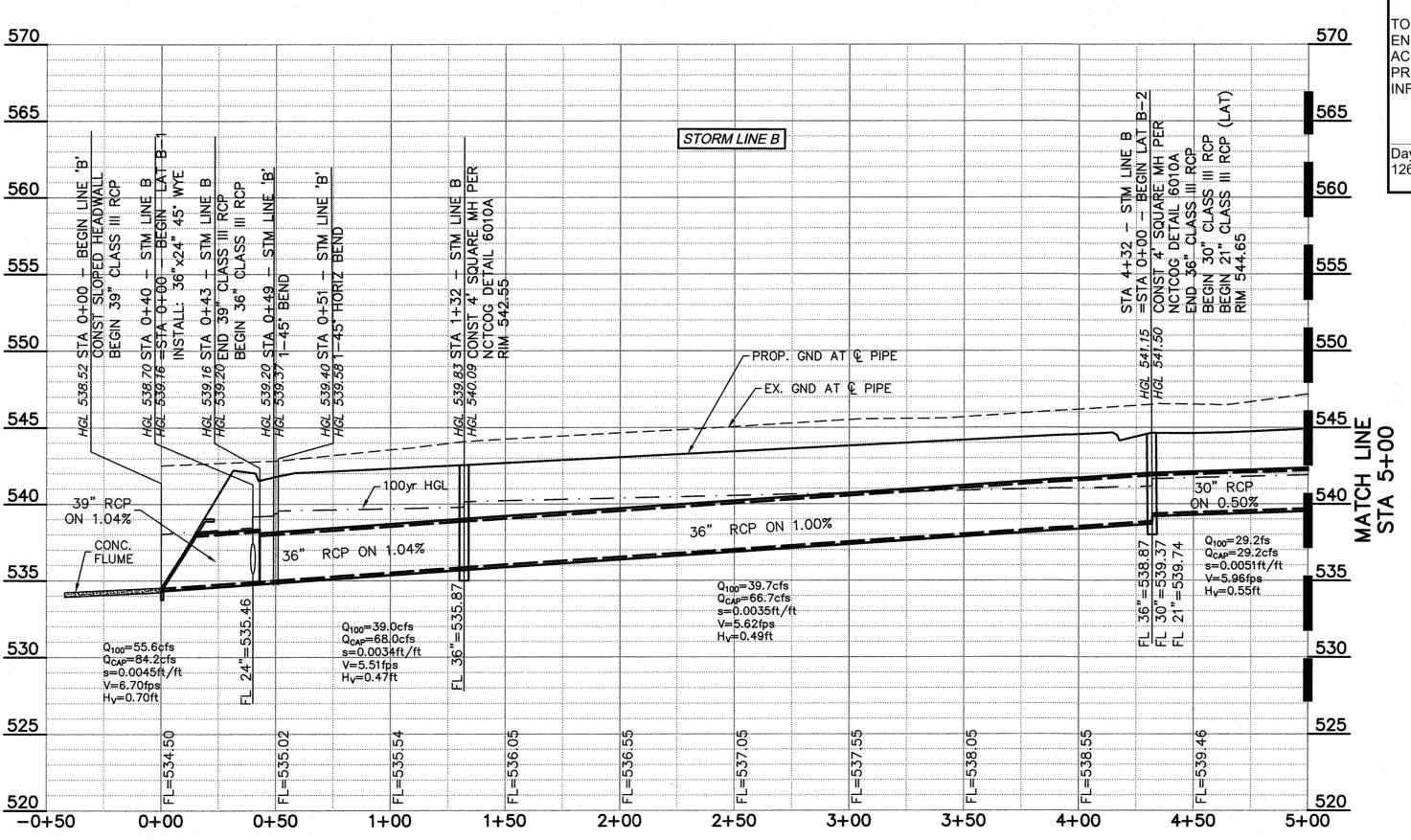






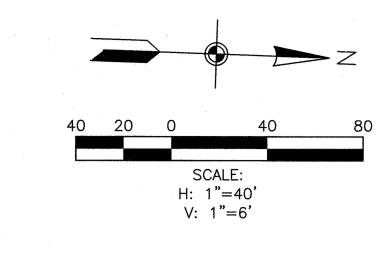


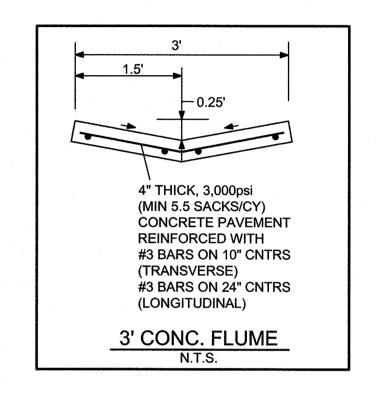




65028 *GISTERED ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN 06/05/18 REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS DAYTON MACATEE ENGINEERING, LLC TX. REG. NO. F-456 FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OR ACCURACY OF DESIGN

DAYTON C. MACATEE





GENERAL NOTES:

1) ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE CITY OF ROCKWALL STANDARDS.

2) THE CONTRACTOR SHALL VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL EXISTING UTILITIES IN THE AREA OF CONSTRUCTION PRIOR TO BEGINNING CONSTRUCTION.

3) THE CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD OF ANY DISCREPANCIES BETWEEN THESE PLANS AND ACTUAL FIELD CONDITIONS.

4) DIMENSIONS ARE TO BACK OF CURB, UNLESS OTHERWISE

TO THE BEST OF OUR KNOWLEDGE, MACATEE NGINEERING LLC, HEREBY STATES THAT THIS PLAN IS AN ACCURATE RECORD DRAWING. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.

Daylon C. Macates
Dayton C. Macates 04/09/2020

Date 12655 N. Central Expwy, Suite 420, Dallas, Texas 75243

> LEGEND DIRECTION OF FLOW \rightarrow TOP OF CURB FLOW LINE CENTERLINE

ALL STORM PIPE SHALL BE CLASS III RCP UNLESS OTHERWISE NOTED

SEE THE CITY OF ROCKWALL STANDARDS OF DESIGN AND CONSTRUCTION, DATED OCTOBER 2016 FOR MODIFICATIONS TO NCTCOG DETAILS.

SEE PAVING SHEETS 4 & 5 FOR DETAILS OF CONCRETE FLUME BEHIND LOTS 1-17, BLOCK 1

BENCHMARKS

DESCRIPTION

CITY OF ROCKWALL MONUMENT "COR-2" ALUMINUM DISK STAMPED "CITY OF ROCKWALL SURVEY MONUMENT" +/-866' EAST OF INTERSECTION OF WILLIAMS STREET AND CARUTH LANE, AND 50' SOUTH OF CL OF WILLIAMS STREET. N: 7029731.124, E: 2598589.314, ELEV=529.10

CITY OF ROCKWALL MONUMENT "COR-4" ALUMINUM DISK STAMPED "CITY OF ROCKWALL SURVEY MONUMENT" ON NORTH SIDE OF DALTON ROAD +/-210' WEST OF INTERSECTION OF SH 205 AND DALTON ROAD. AND 10' NORTH OF CURB LINE. N: 7040336.992, E: 2592422.633, ELEV=541.67

STORM SEWER PLAN & PROFILE

GIDEON GROVE - NORTH

CITY OF ROCKWALL, ROCKWALL COUNTY, TEXAS



PJF

REV. BY DATE

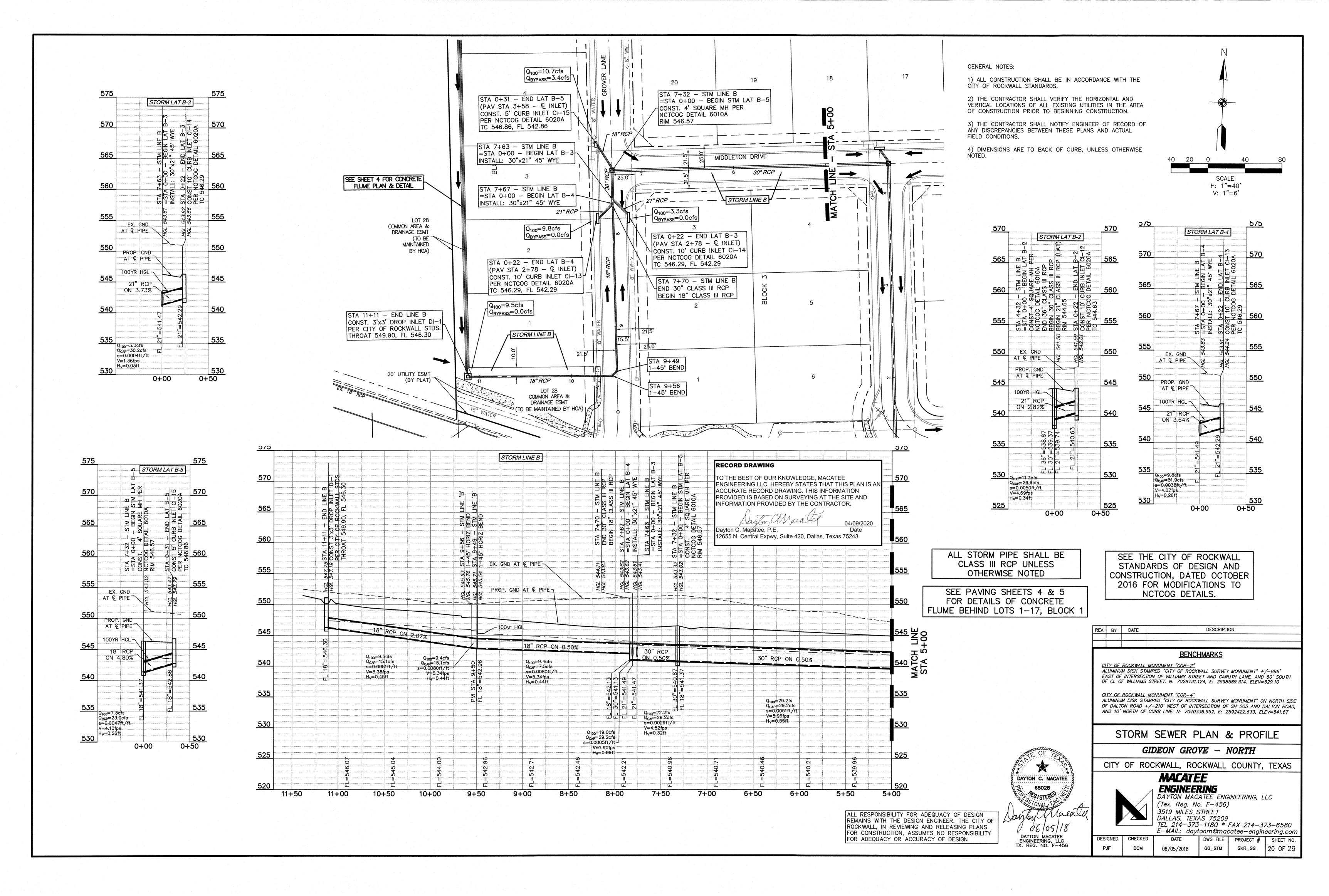
MACATEE

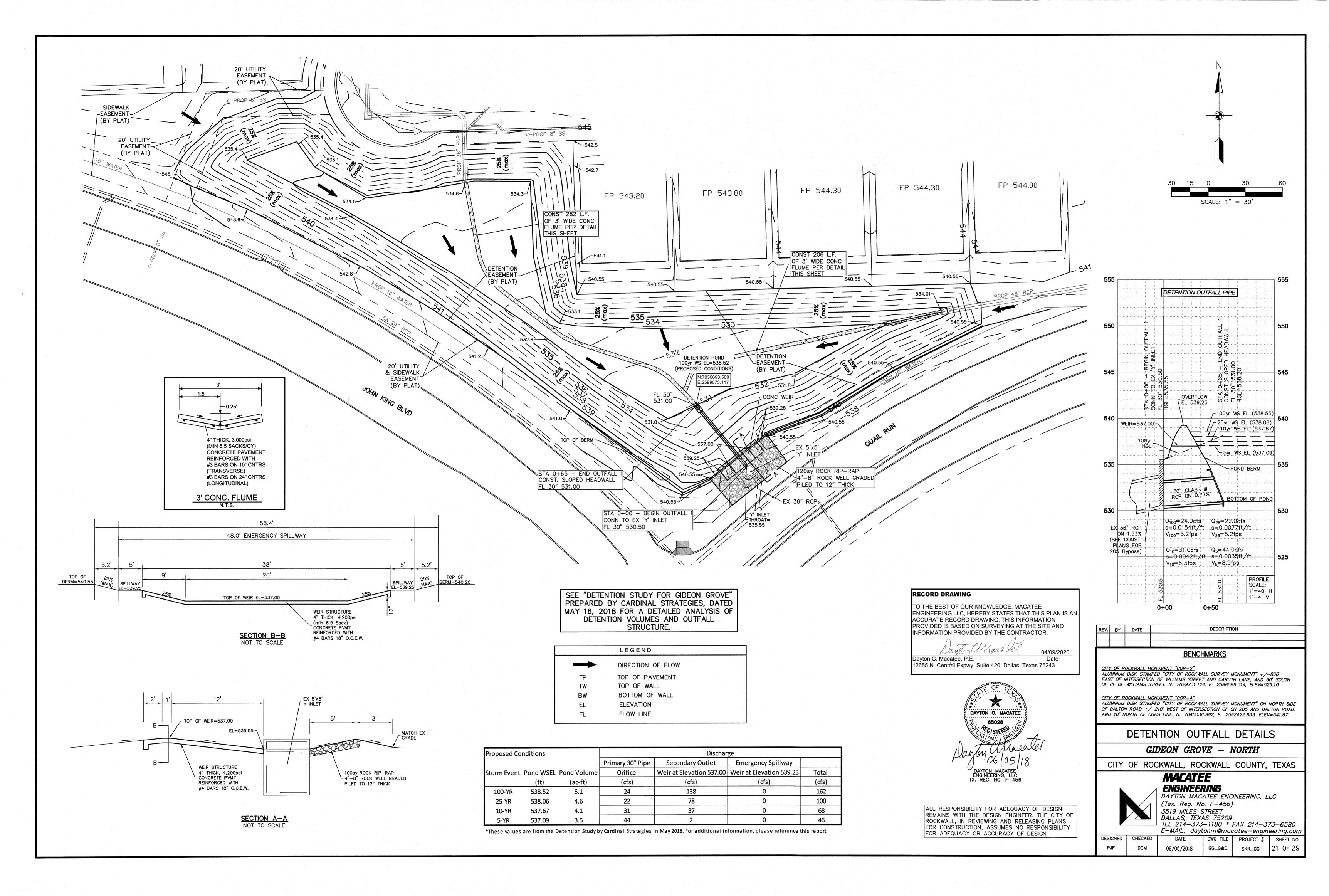
ENGINEERING DAYTON MACATEE ENGINEERING, LLC (Tex. Reg. No. F-456) 3519 MILES STREET DALLAS, TEXAS 75209

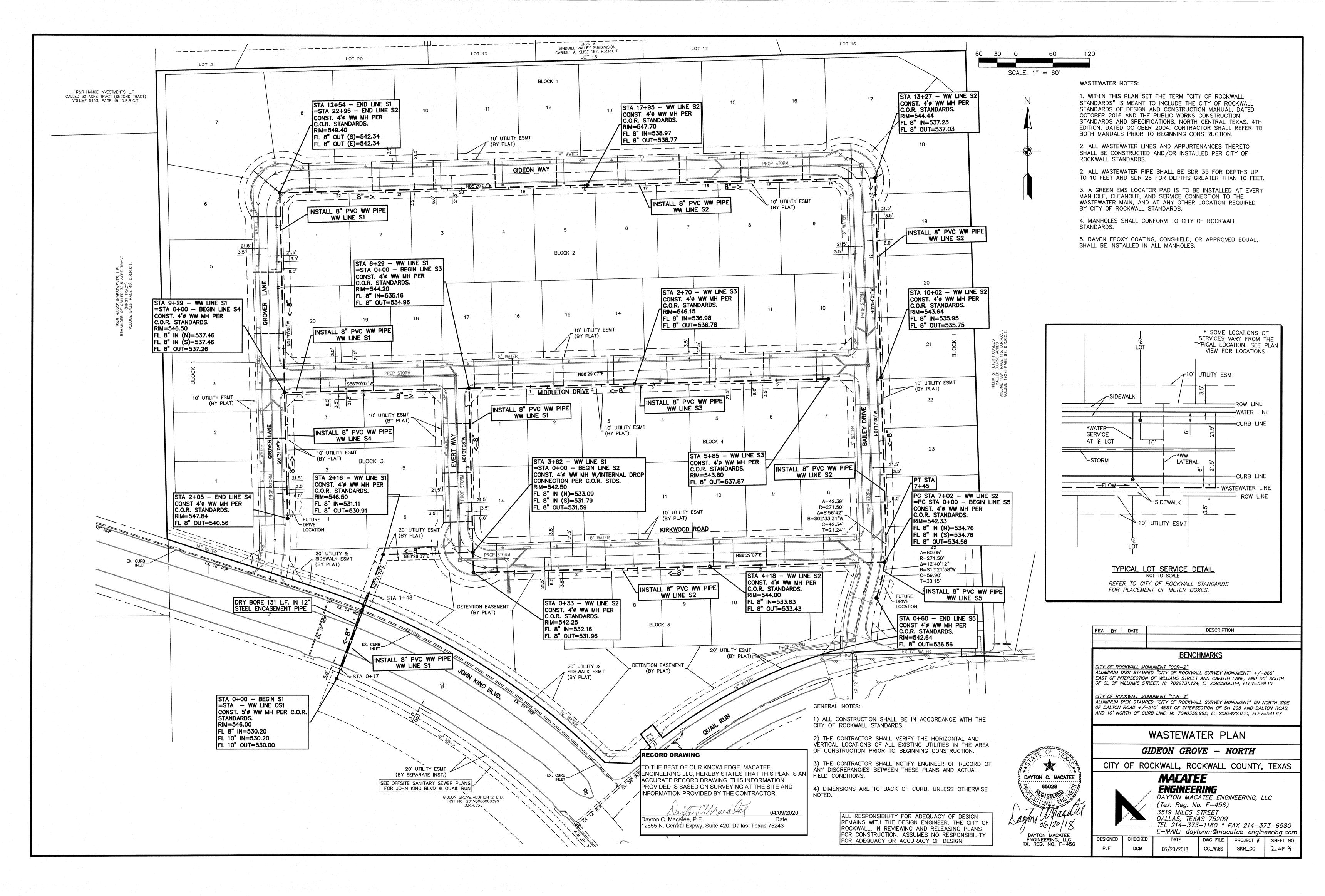
TEL 214-373-1180 * FAX 214-373-6580 E-MAIL: daytonm@macatee-engineering.com DESIGNED CHECKED

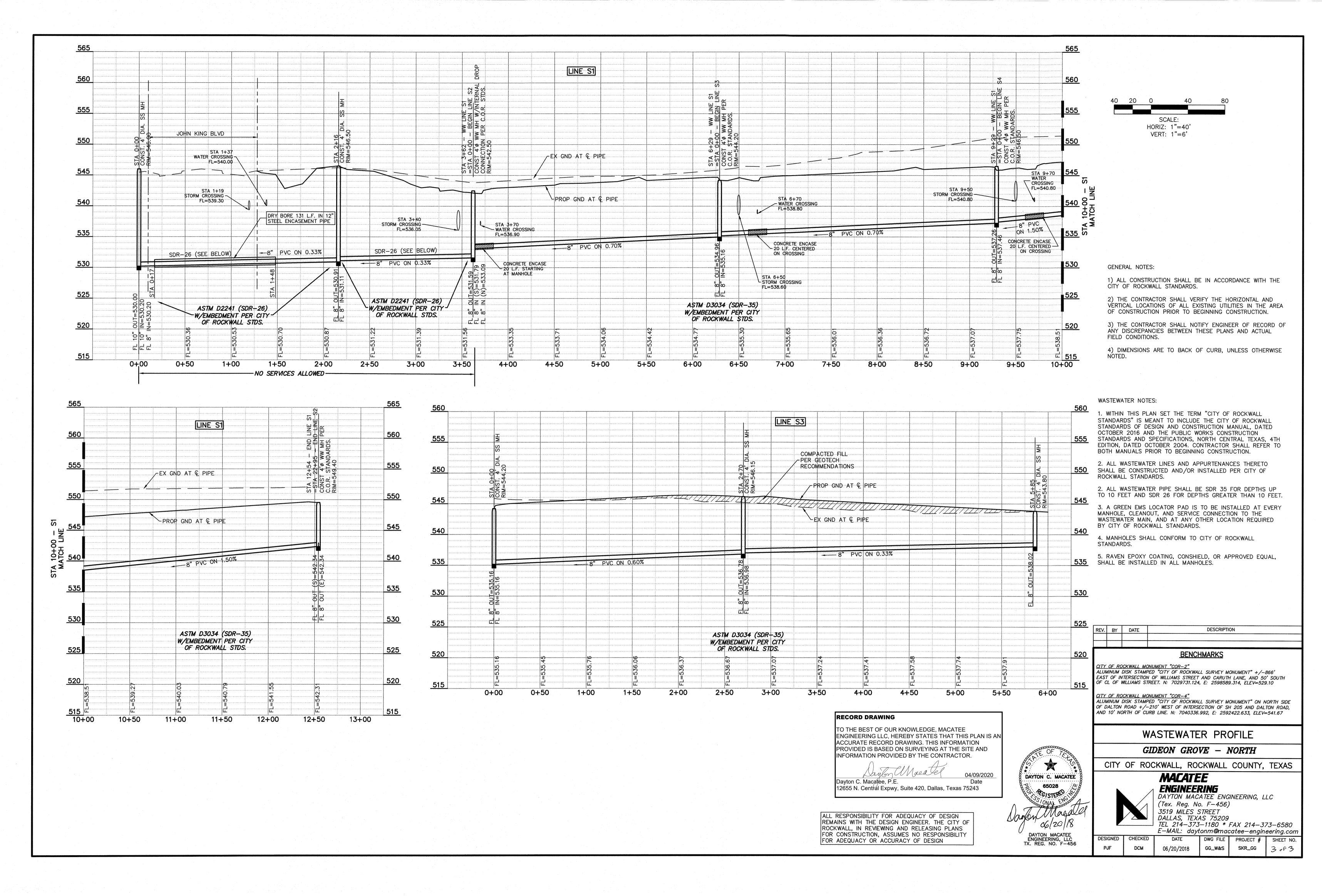
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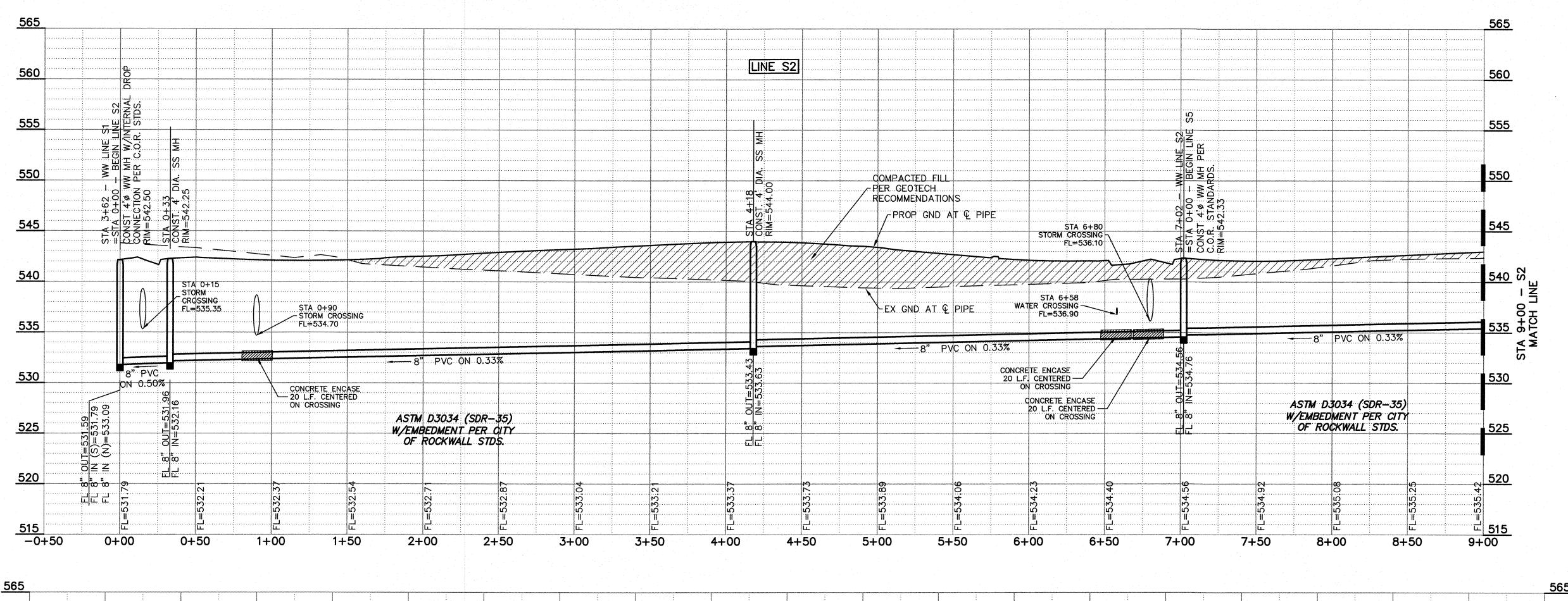
PROJECT # SHEET NO. 19 OF 29 GG_STM SKR_GG

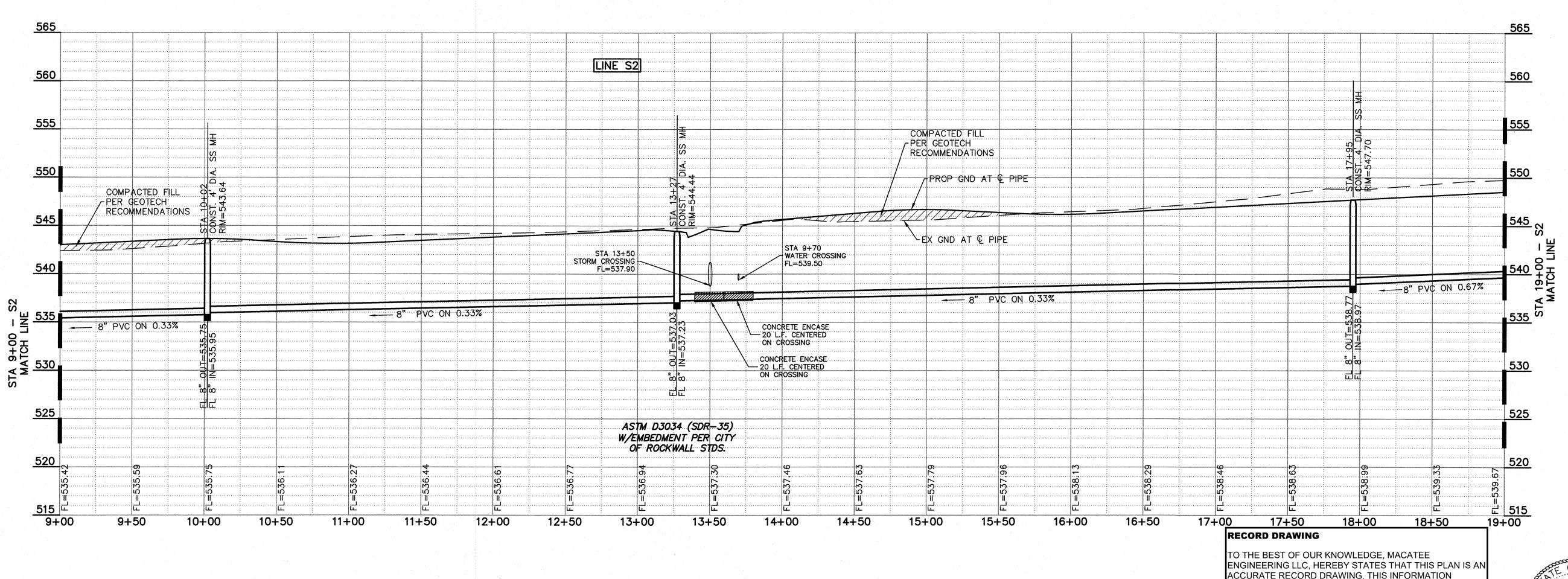


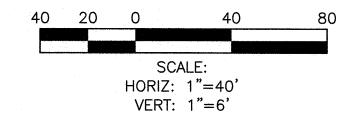










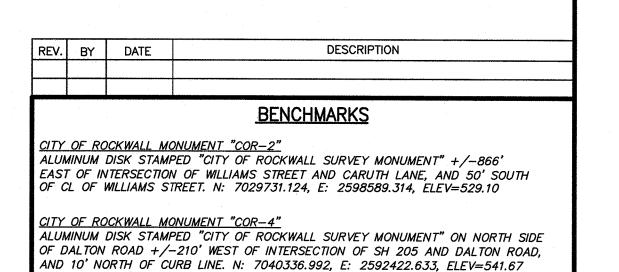


GENERAL NOTES:

- 1) ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE CITY OF ROCKWALL STANDARDS.
- 2) THE CONTRACTOR SHALL VERIFY THE HORIZONTAL AND VERTICAL LOCATIONS OF ALL EXISTING UTILITIES IN THE AREA OF CONSTRUCTION PRIOR TO BEGINNING CONSTRUCTION.
- 3) THE CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD OF ANY DISCREPANCIES BETWEEN THESE PLANS AND ACTUAL FIELD CONDITIONS.
- 4) DIMENSIONS ARE TO BACK OF CURB, UNLESS OTHERWISE

WASTEWATER NOTES:

- 1. WITHIN THIS PLAN SET THE TERM "CITY OF ROCKWALL STANDARDS" IS MEANT TO INCLUDE THE CITY OF ROCKWALL STANDARDS OF DESIGN AND CONSTRUCTION MANUAL, DATED OCTOBER 2016 AND THE PUBLIC WORKS CONSTRUCTION STANDARDS AND SPECIFICATIONS, NORTH CENTRAL TEXAS, 4TH EDITION, DATED OCTOBER 2004. CONTRACTOR SHALL REFER TO BOTH MANUALS PRIOR TO BEGINNING CONSTRUCTION.
- 2. ALL WASTEWATER LINES AND APPURTENANCES THERETO SHALL BE CONSTRUCTED AND/OR INSTALLED PER CITY OF ROCKWALL STANDARDS.
- 2. ALL WASTEWATER PIPE SHALL BE SDR 35 FOR DEPTHS UP TO 10 FEET AND SDR 26 FOR DEPTHS GREATER THAN 10 FEET.
- 3. A GREEN EMS LOCATOR PAD IS TO BE INSTALLED AT EVERY MANHOLE, CLEANOUT, AND SERVICE CONNECTION TO THE WASTEWATER MAIN, AND AT ANY OTHER LOCATION REQUIRED BY CITY OF ROCKWALL STANDARDS.
- 4. MANHOLES SHALL CONFORM TO CITY OF ROCKWALL STANDARDS.
- 5. RAVEN EPOXY COATING, CONSHIELD, OR APPROVED EQUAL. SHALL BE INSTALLED IN ALL MANHOLES.



WASTEWATER PROFILE

GIDEON GROVE - NORTH

CITY OF ROCKWALL, ROCKWALL COUNTY, TEXAS



DAYTON C. MACATEE

COSTERE!

06/05/18

DAYTON MACATEE ENGINEERING, LLC TX. REG. NO. F-456

PROVIDED IS BASED ON SURVEYING AT THE SITE AND

04/09/2020

INFORMATION PROVIDED BY THE CONTRACTOR.

Saylon Whallet

ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN

FOR ADEQUACY OR ACCURACY OF DESIGN

REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS

FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY

12655 N. Central Expwy, Suite 420, Dallas, Texas 75243

Dayton C. Maçatee, P.E.

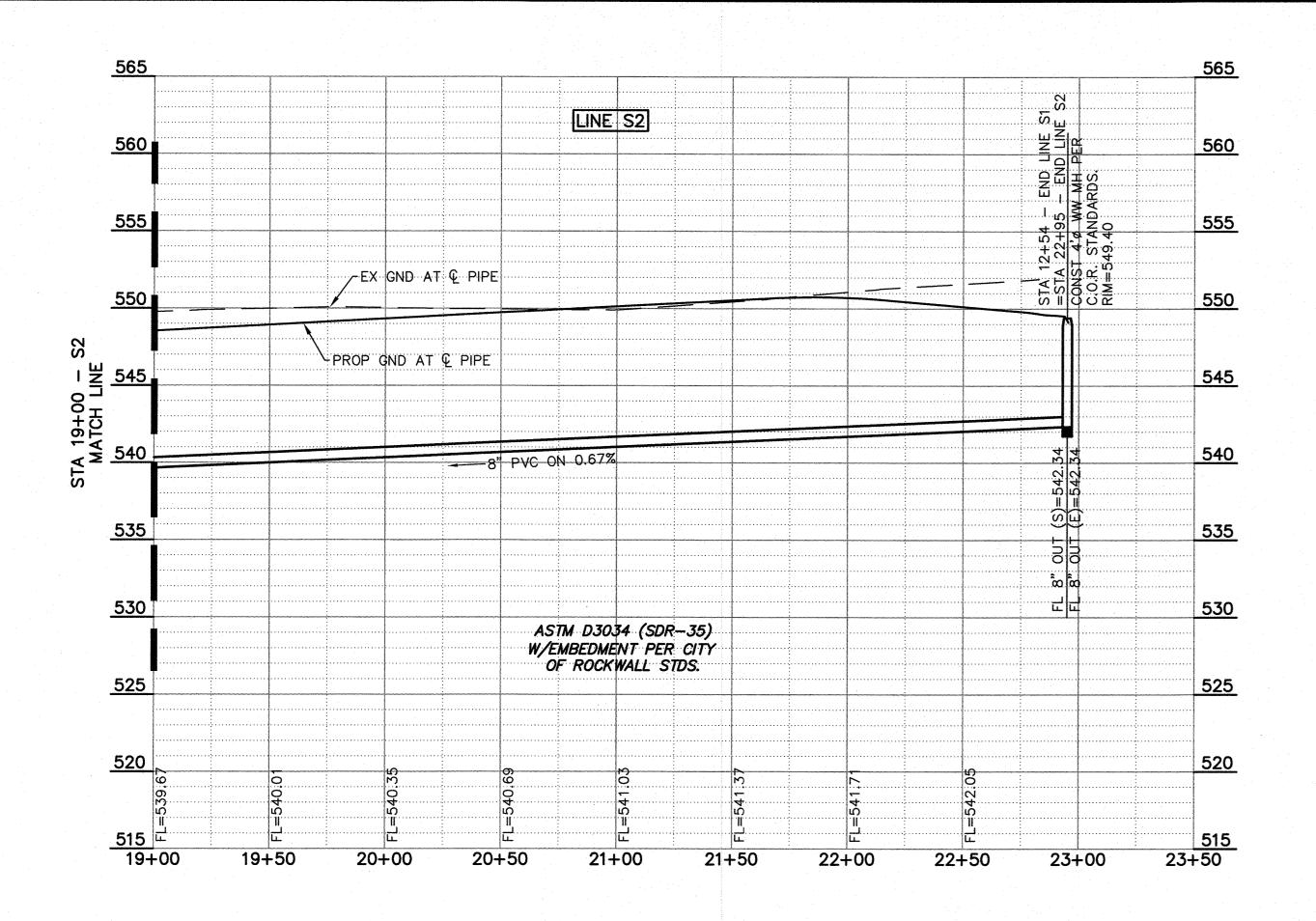
MACATEE

ENGINEERING DAYTON MACATEE ENGINEERING, LLC (Tex. Reg. No. F-456) 3519 MILES STREET

<u> </u>		TEL 214-37 E-MAIL: day	3-1180 *	FAX 214-3	
)	CHECKED	DATE	DWG FILE	PROJECT #	SHEET NO

DESIGNED PJF

06/05/2018 GG_W&S



LINE S5

ASTM D3034 (SDR-35)

W/EMBEDMENT PER CITY

OF ROCKWALL STDS.

0+50

0+00

EX GND

AT & PIPE

530

<u>515</u>

STA 0+60 CONST: 4'

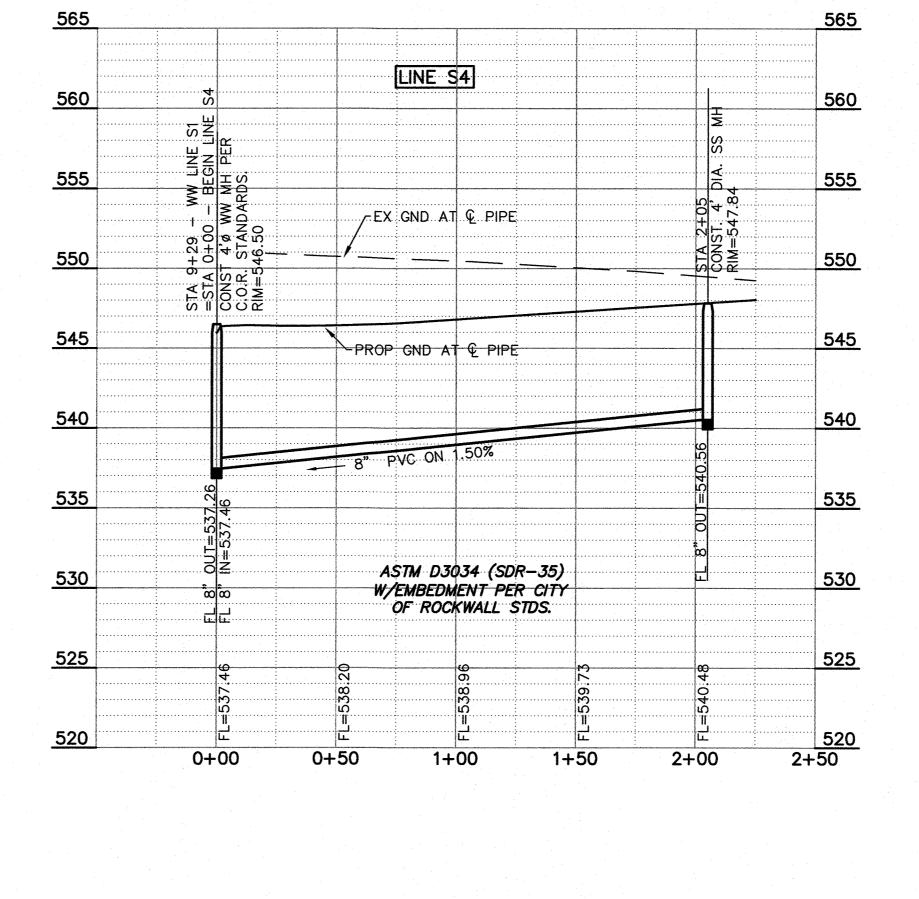
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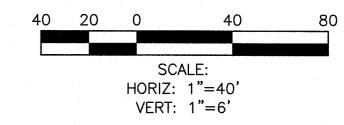
RECOMMENDATIONS

<u>530</u>

1+00

~PER GEOTECH





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RECORD DRAWING

TO THE BEST OF OUR KNOWLEDGE, MACATEE ENGINEERING LLC, HEREBY STATES THAT THIS PLAN IS AN ACCURATE RECORD DRAWING. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND

04/09/2020 Dayton C. Macatee, P.E.

REV. BY DATE DESCRIPTION

BENCHMARKS

<u>CITY OF ROCKWALL MONUMENT "COR-2"</u>
ALUMINUM DISK STAMPED "CITY OF ROCKWALL SURVEY MONUMENT" +/-866' EAST OF INTERSECTION OF WILLIAMS STREET AND CARUTH LANE, AND 50' SOUTH OF CL OF WILLIAMS STREET. N: 7029731.124, E: 2598589.314, ELEV=529.10

CITY OF ROCKWALL MONUMENT "COR-4" ALUMINUM DISK STAMPED "CITY OF ROCKWALL SURVEY MONUMENT" ON NORTH SIDE OF DALTON ROAD +/-210' WEST OF INTERSECTION OF SH 205 AND DALTON ROAD,

AND 10' NORTH OF CURB LINE. N: 7040336.992, E: 2592422.633, ELEV=541.67

WASTEWATER PROFILE

GIDEON GROVE - NORTH

CITY OF ROCKWALL, ROCKWALL COUNTY, TEXAS



MACATEE **ENGINEERING**

DAYTON MACATEE ENGINEERING, LLC (Tex. Reg. No. F-456) 3519 MILES STREET DALLAS, TEXAS 75209 TEL 214-373-1180 * FAX 214-373-6580

DESIGNED CHECKED PJF DCM

06/05/2018

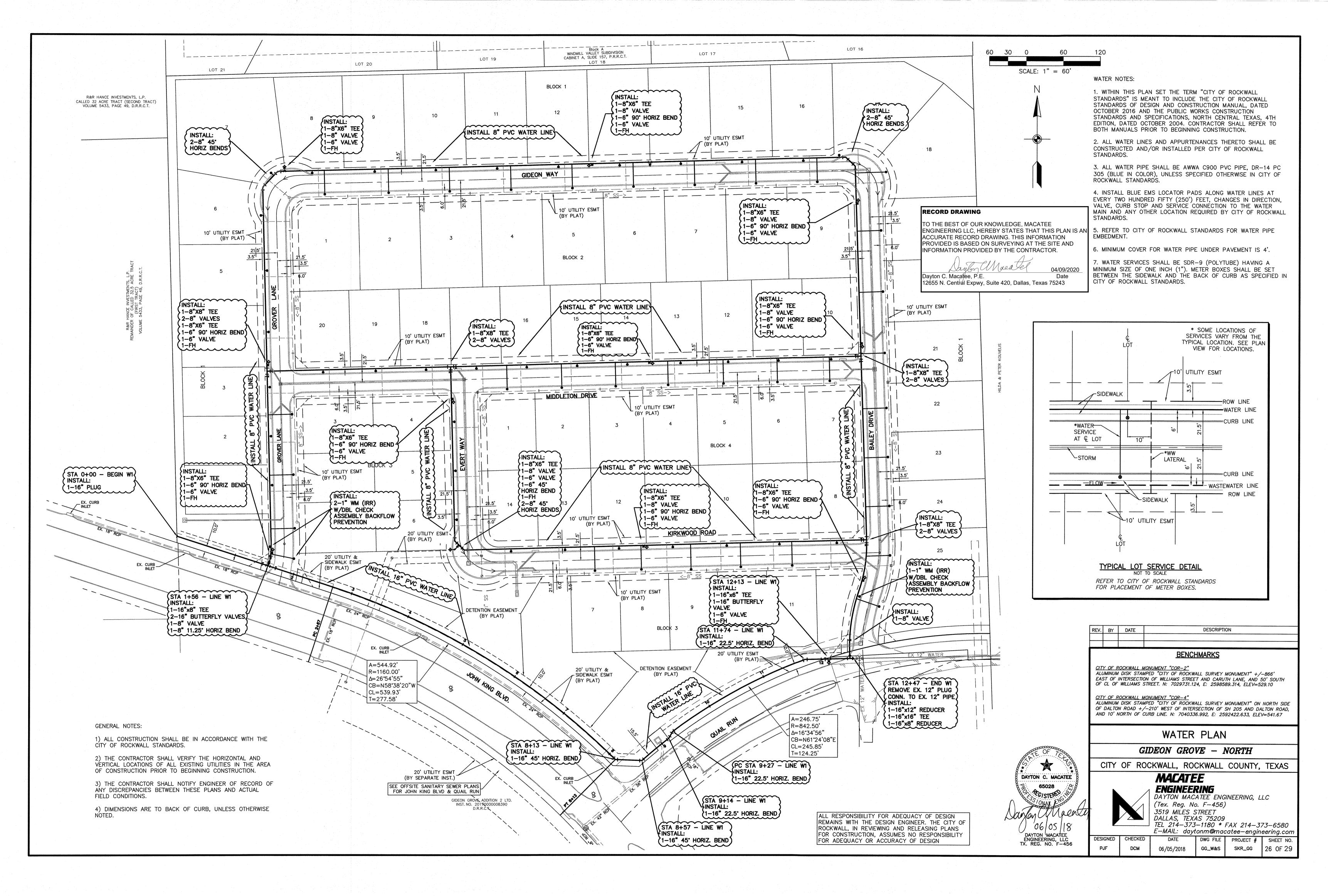
E-MAIL: daytonm@macatee-engineering.com PROJECT # SHEET NO. 25 OF 29 SKR_GG GG_W&S

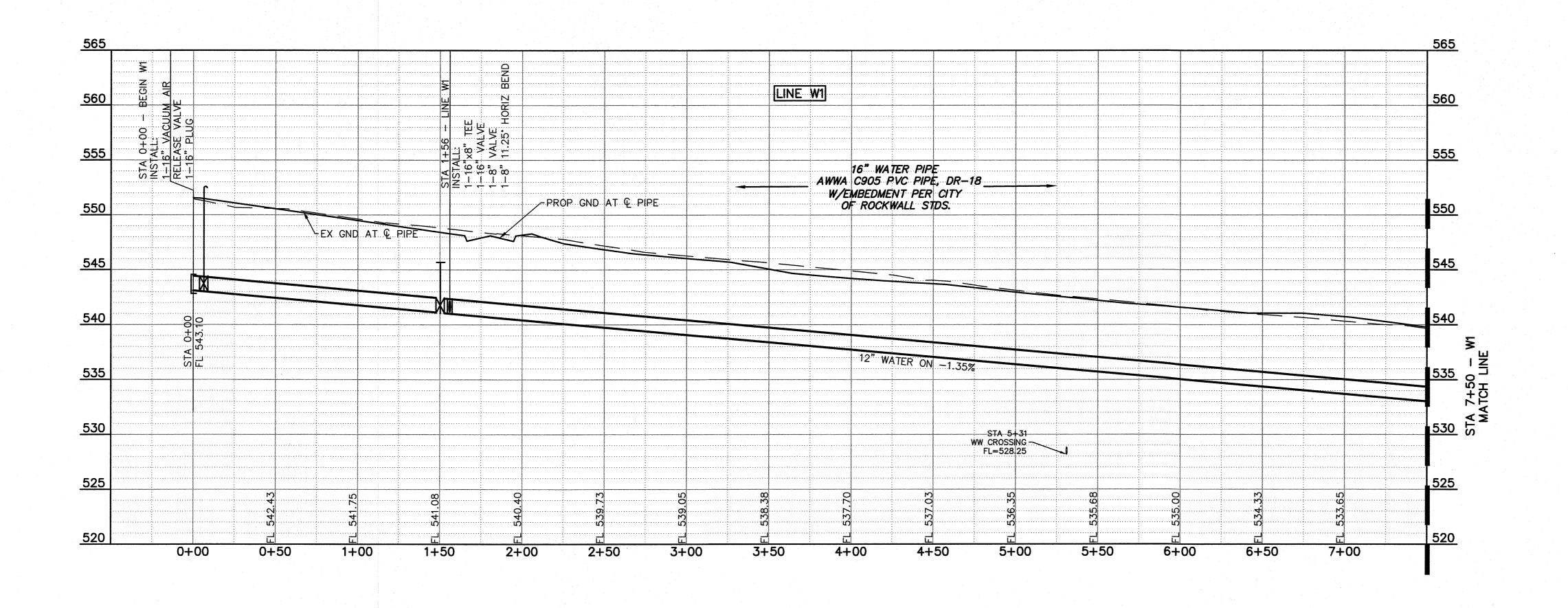
* GISTERED ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF 06/05/18 ROCKWALL, IN REVIEWING AND RELEASING PLANS DAYTON MACATEE ENGINEERING, LLC TX. REG. NO. F-456 FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OR ACCURACY OF DESIGN

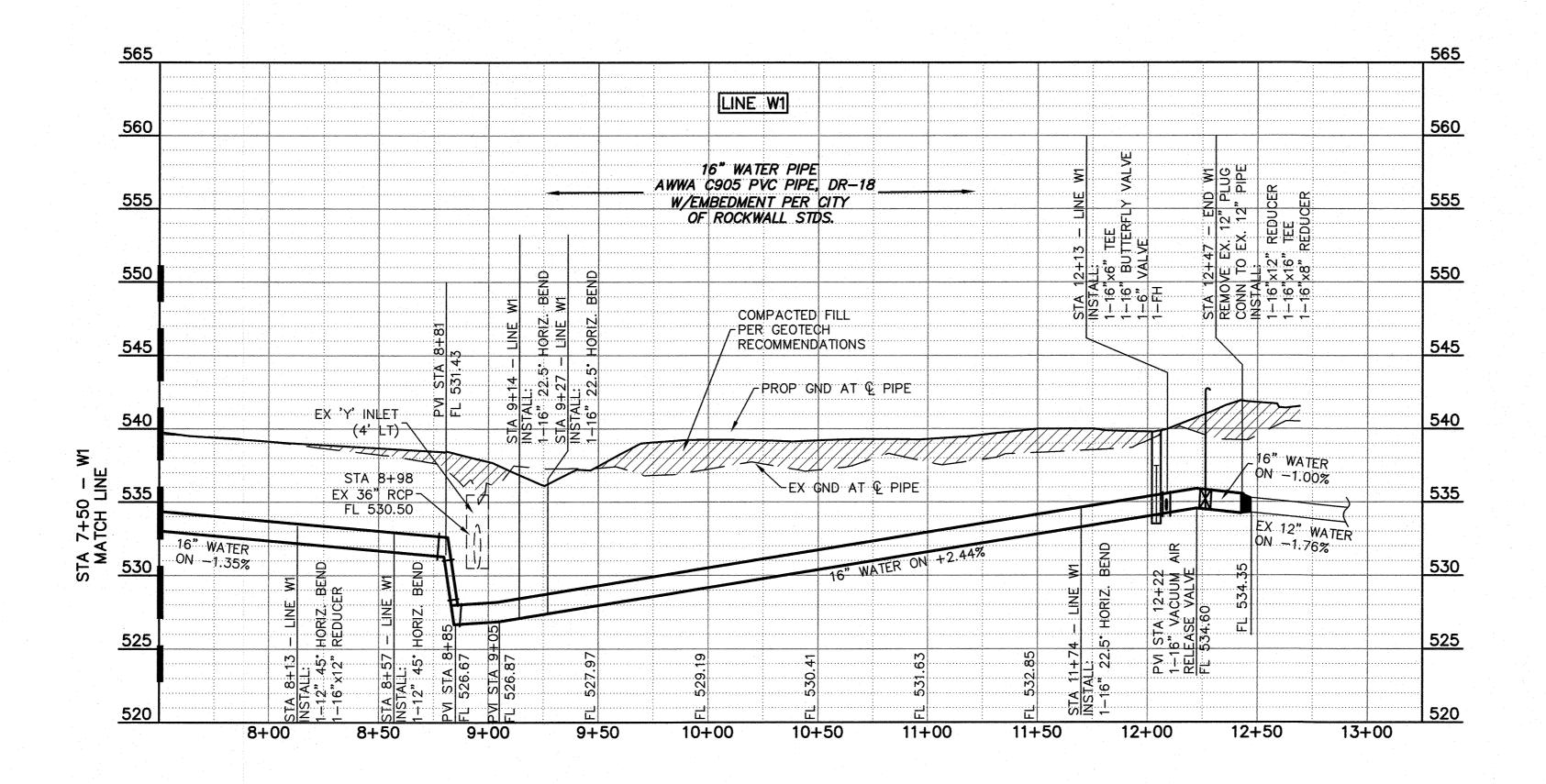
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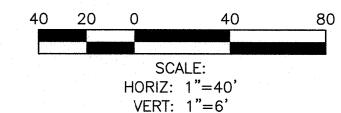
12655 N. Centra Expwy, Suite 420, Dallas, Texas 75243

DAYTON C. MACATEE 65028









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2. ALL WATER LINES AND APPURTENANCES THERETO SHALL BE CONSTRUCTED AND/OR INSTALLED PER CITY OF ROCKWALL STANDARDS.

3. ALL WATER PIPE SHALL BE AWWA C900 PVC PIPE, DR-14 PC 305 (BLUE IN COLOR), UNLESS SPECIFIED OTHERWISE IN CITY OF ROCKWALL STANDARDS.

4. INSTALL BLUE EMS LOCATOR PADS ALONG WATER LINES AT EVERY TWO HUNDRED FIFTY (250') FEET, CHANGES IN DIRECTION, VALVE, CURB STOP AND SERVICE CONNECTION TO THE WATER MAIN AND ANY OTHER LOCATION REQUIRED BY CITY OF ROCKWALL STANDARDS.

5. REFER TO CITY OF ROCKWALL STANDARDS FOR WATER PIPE EMBEDMENT.

6. MINIMUM COVER FOR WATER PIPE UNDER PAVEMENT IS 4'.

7. WATER SERVICES SHALL BE SDR-9 (POLYTUBE) HAVING A MINIMUM SIZE OF ONE INCH (1"). METER BOXES SHALL BE SET BETWEEN THE SIDEWALK AND THE BACK OF CURB AS SPECIFIED IN CITY OF ROCKWALL STANDARDS.

RECORD DRAWING

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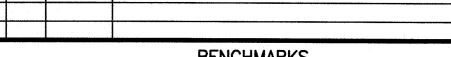
Saylon Macatel 04/09/2020 Dayton C. Macatee, P.É. Date 12655 N. Central Expwy, Suite 420, Dallas, Texas 75243

> DAYTON C. MACATEE 65028

AGISTERED.

DAYTON MACATEE ENGINEERING, LLC TX. REG. NO. F-456

106/05/18



BENCHMARKS

DESCRIPTION

<u>CITY OF ROCKWALL MONUMENT "COR-2"</u> ALUMINUM DISK STAMPED "CITY OF ROCKWALL SURVEY MONUMENT" +/-866' EAST OF INTERSECTION OF WILLIAMS STREET AND CARUTH LANE, AND 50' SOUTH OF CL OF WILLIAMS STREET. N: 7029731.124, E: 2598589.314, ELEV=529.10

CITY OF ROCKWALL MONUMENT "COR-4"
ALUMINUM DISK STAMPED "CITY OF ROCKWALL SURVEY MONUMENT" ON NORTH SIDE OF DALTON ROAD +/-210' WEST OF INTERSECTION OF SH 205 AND DALTON ROAD, AND 10' NORTH OF CURB LINE. N: 7040336.992, E: 2592422.633, ELEV=541.67

16" WATER PROFILE

GIDEON GROVE - NORTH

CITY OF ROCKWALL, ROCKWALL COUNTY, TEXAS



REV. BY DATE

MACATEE **ENGINEERING** DAYTON MACATEE ENGINEERING, LLC (Tex. Reg. No. F-456)

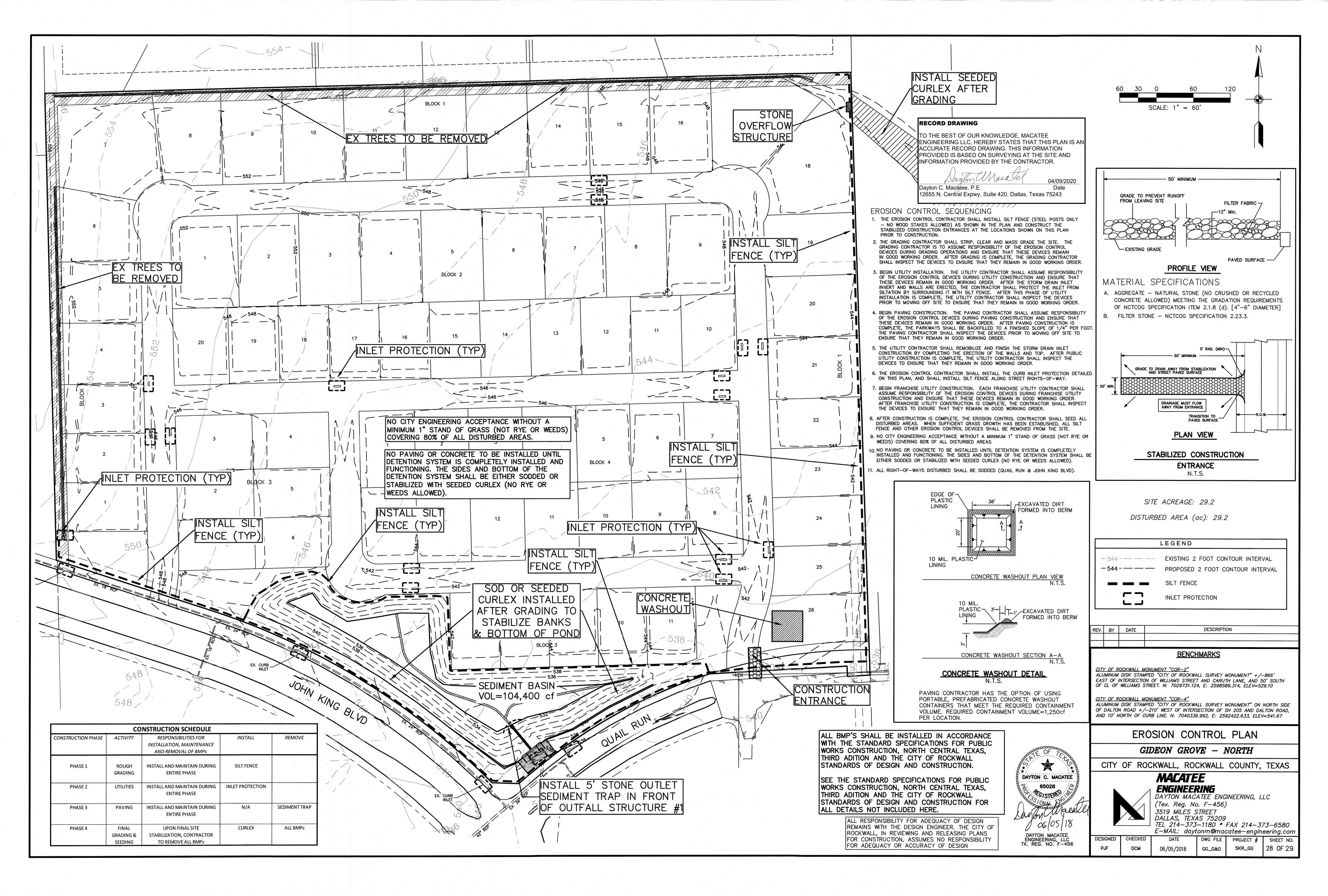
3519 MILES STREET DALLAS, TEXAS 75209 TEL 214-373-1180 * FAX 214-373-6580 E-MAIL: daytonm@macatee-engineering.com

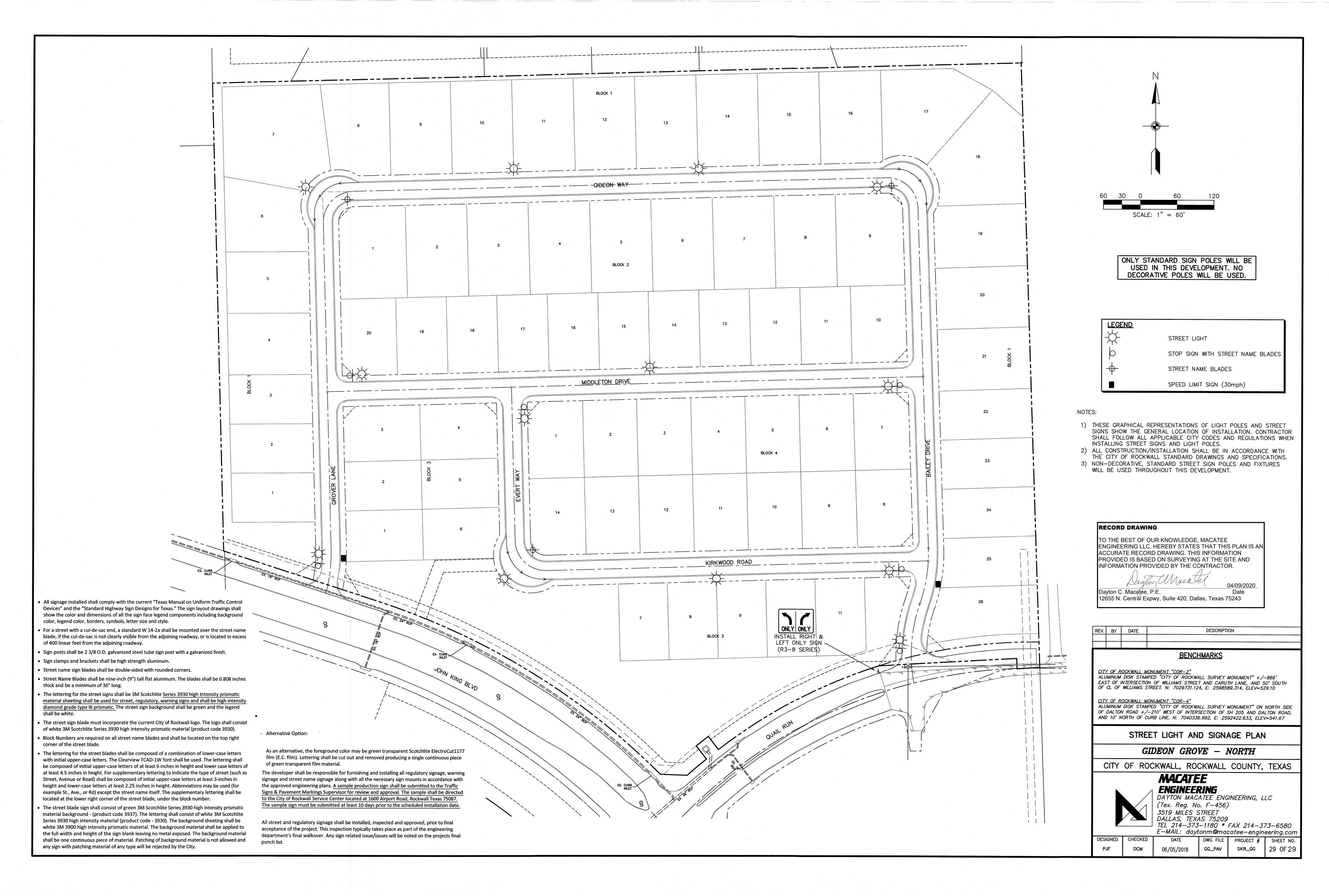
27 OF 29

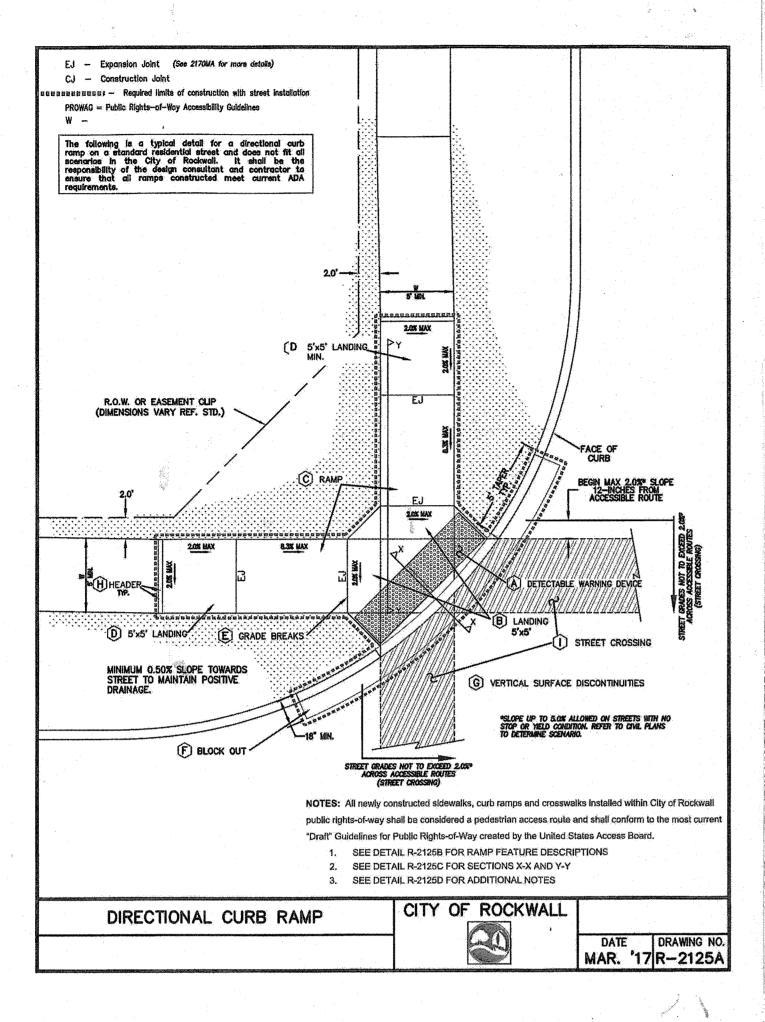
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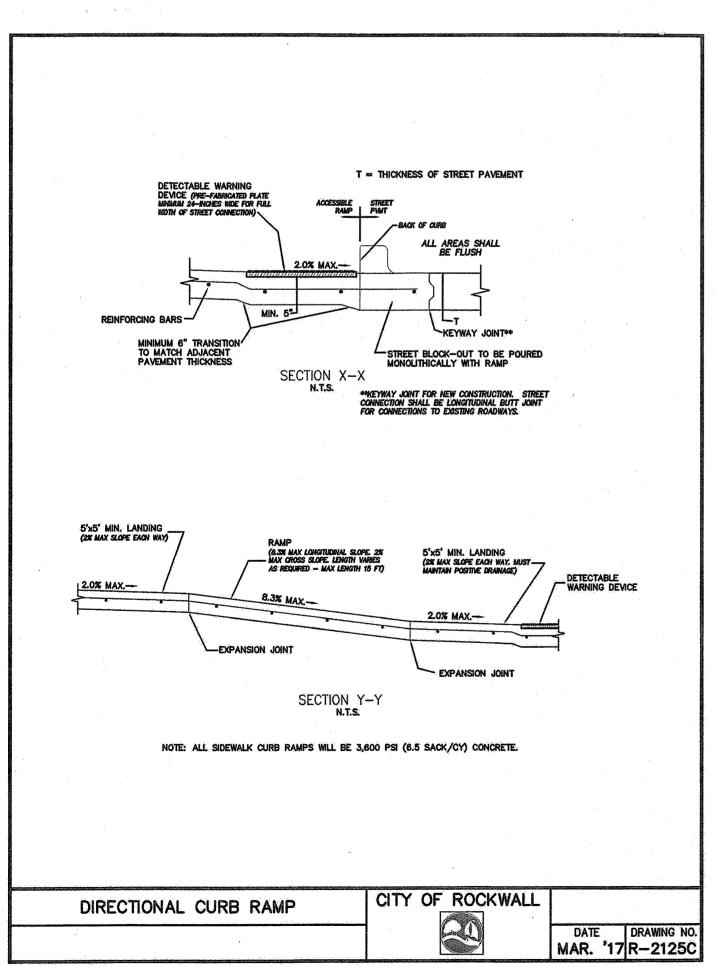
PROJECT # SHEET NO. SKR_GG 06/05/2018 GG_W&S

ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OR ACCURACY OF DESIGN









- Detectable Warning Devices (DWD) shall be pre-manufactured cast-in-place truncated dome plates installed to the manufacturer's specifications, and shall meet all ADA requirements. No Brick Pavers allowed. Color to be approved by the City. DWD shall be 24 inches in length for the full width of the street connection starting at the back of curb. A maximum 2-inch border shall be allowed on the sides of the DWD for proper installation. Also known as "Clear Space" per ADA PROWAG, the City requires a minimum landing space of 5—foot by 5—foot at the bottom of every ramp. This landing space shall have a cross slope in both directions that does not exceed 2.0% and shall be wholly outside the parallel vehicular travel The ramp component of the directional curb ramp shall have a continuous longitudinal slope more than 5% and less than 8.3%. The ramp shall also have a cross slope of no more than 2.0%. Length of ramp can vary, but shall not exceed 15 feet to achieve desired elevation change.
- Also known as "Turning Space" per ADA PROWAG, a minimum landing space of 5—foot by 5—foot shall be at the top of every ramp. This landing (turning) space shall have a cross slope in both directions that does not exceed 2.0%. Landing must match width of sidewalk and length shall be the same distance ("Squared" Landing).
- All curb ramps shall have grade breaks at the top and bottom that are perpendicular to the direction of the ramp run. Where the ends of the bottom grade break are less than or equal to 5 feet, the DWD shall be placed within the ramp at the bottom grade break. Where either end of the bottom grade break is greater than 5 feet, the DWD shall be placed behind the back of the
- Paving contractor shall leave block out with a keyway joint installed, minimum of 18 inches measured from back of curb. Block out shall be poured monolithically with Curb Ramp. Concrete shall tie to street paving with a keyway joint per NCTCOG detail 2050. No curb shall be constructed where a DWD is provided. The curb on either side shall have a typical 5 foot taper to transition from the standard 6-inch curb height to be flush with ramp.
- All work associated with accessible routes shall be installed flush with all features to minimize vertical surface discontinuities. Each segment along accessible route shall be flush with no more (zero tolerance) than a 14-inch grade separation (elevation difference), or 15-inch grade separation if beveled (bevel slope shall not be steeper than 50%).
- A sidewalk header shall be constructed at ends of all work performed.
- Street crossings shall adhere to same guidelines as other accessible routes within public right—of—way, and shall be for the full width of the in—line accessible route. Cross slope shall not exceed 2%*. New street construction shall incorporate all ADA design requirements. It shall be the responsibility of the Design Professional and Contractor to ensure all street crossings meet the requirements of PROWAG. Street alterations on existing streets to bring to compliance shall be at the City Engineer's discretion.
- All curbs constructed as part of an ADA Ramp shall match City curb standards. * See PROWAG special design considerations when street crossing has no stop or yield condition.

CITY OF ROCKWALL DIRECTIONAL CURB RAMP MAR. '17 R-2125E

PEDESTRIAN ACCESSIBILITY (WITHIN PUBLIC R.O.W.)

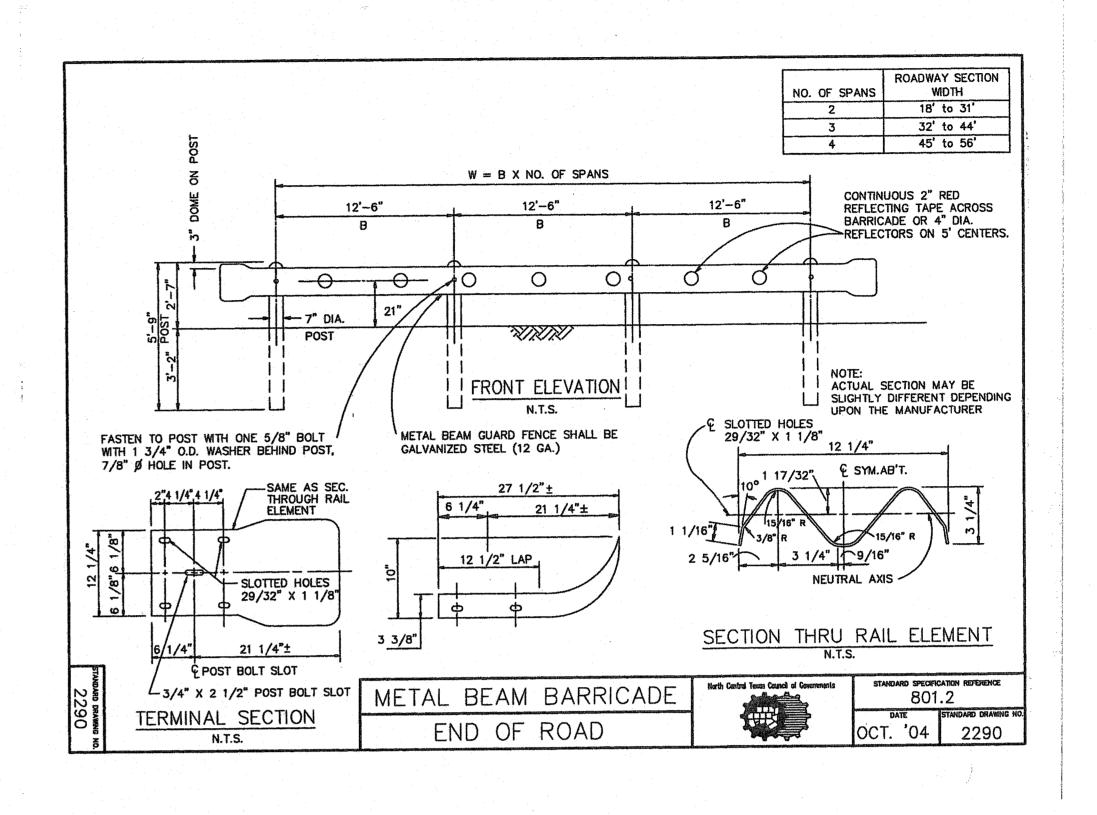
All newly constructed sidewalks, curb ramps and crosswalks installed within City of Rockwall public rights—of—way shall be considered a pedestrian access route and shall conform to the most current Guidelines for Public Rights-of-Way created by the United States Access Board.

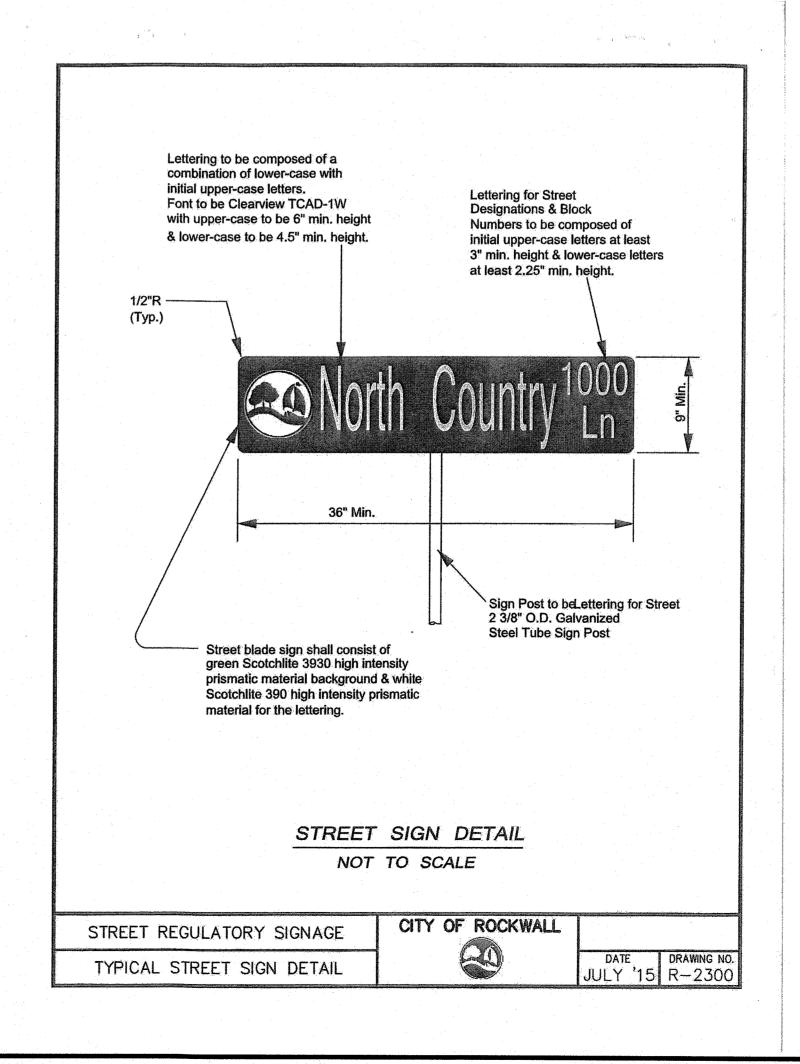
- All slopes shown are <u>MAXIMUM ALLOWABLE</u>. Lesser slopes that will still drain properly should be used. Adjust curb ramp length or grade of approach sidewalks as directed. 2. Landings shall be 5'x 5' minimum with a maximum 2% slope in the transverse and
- 3. Clear space at the bottom of curb ramps shall be a minimum of 5'x 5' wholly contained within the crosswalk and wholly outside the parallel vehicular travel path. 4. Maximum allowable cross slope on sidewalk and curb ramp surfaces is 2%.
- 5. Additional information on curb ramp location, design, light reflective value and texture may be found in the most current edition of the Texas Accessibility Standards (TAS) and 16 TAC 68.102. Federal guidelines shall supersede any conflicts.
- Crosswalk dimensions, crosswalk markings and stop bar locations shall be as shown elsewhere in the plans. At intersections where crosswalk markings are not required, curb ramps and accessible routes shall align with theoretical crosswalks unless otherwise directed.
- 7. Handrails are not required on curb ramps. 8. Provide a flush transition where the curb ramps connect to the street.
- 9. Accessible routes are considered "ramps" when longitudinal slopes are between 5% and 8.3% (maximum allowable). Sidewalks under 5% longitudinal slope are deemed accessible routes and must follow all applicable guidelines.
- DETECTABLE WARNING DEVICE 10. Curb ramps must contain a detectable warning surface that consists of raised truncated domes complying with Section 705 of the TAS. The surface must contrast visually with adjoining surfaces. Furnish and install an approved cast-in-place dark red detectable warning surface material adjacent to uncolored concrete, unless specified elsewhere in the
- 11. Detectable Warning Materials shall be truncated dome plates in the color approved by the City. Install products in accordance with manufacturer's specifications.
- 12. Detectable warning surfaces must be slip resistant and not allow water to accumulate. 13. Detectable warning surfaces shall be a minimum of 24" in depth in the direction of pedestrian travel, and extend the full width of the curb ramp or landing where the pedestrian
- 14. Detectable warning surfaces shall be located so that the edge nearest the curb line is at the back of curb. When placed on the ramp, align the rows of domes to be perpendicular to the grade break between the ramp run and the street. Where detectable warning surfaces are provided on a surface with a slope that is less than 5 percent, dome orientation is less critical. Detectable warning surfaces may be curved along the corner radius.

SIDEWALKS

- 15. Provide clear ground space at operable parts, including pedestrian push buttons. Operable parts shall be placed within one or more reach ranges specified in TAS 308.
- 16. Place traffic signal or illumination poles, ground boxes, controller boxes, signs, drainage facilities and other items so as not to obstruct the pedestrian access route or clear ground
- 17. Street grades and cross slopes shall be as shown elsewhere in the plans. 18. Changes in level greater than 1/4 inch are not permitted (1/2 inch with bevel).
- 19. The least possible grade should be used to maximize accessibility. The running slope of sidewalks and crosswalks within the public right of way may follow the grade of the parallel roadway. Where a continuous grade greater than 5% must be provided, handrails may be desirable to improve accessibility. Handrails may also be needed to protect pedestrians from potentially hazardous conditions. If provided, handrails shall comply with TAS 505.
- 20. Handrall extensions shall not protrude into the usable landing area or into intersecting

CITY OF ROCKWALL DIRECTIONAL CURB RAMP DATE DRAWING NO MAR. '17 R-2125D





RECORD DRAWING

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CONSTRUCTION DETAILS

GIDEON GROVE - NORTH

CITY OF ROCKWALL, ROCKWALL COUNTY, TEXAS

MACATEE **ENGINEERING**

DAYTON MACATEE ENGINEERING, LLC (Tex. Reg. No. F-456) 3519 MILES STREET DALLAS. TEXAS 75209 TEL 214-373-1180 * FAX 214-373-6580

E-MAIL: daytonm@macatee-engineering.com DESIGNED CHECKED

05/03/2018

DWG FILE PROJECT # SHEET NO. GG_PAV

SKR_GG