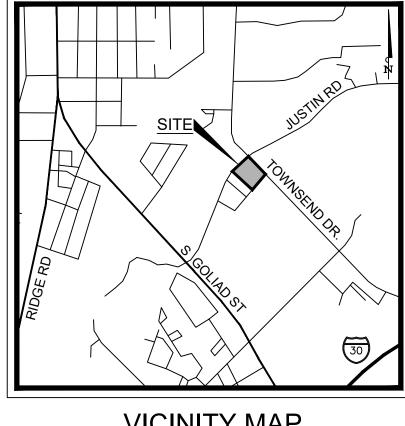
CIVIL PLANS

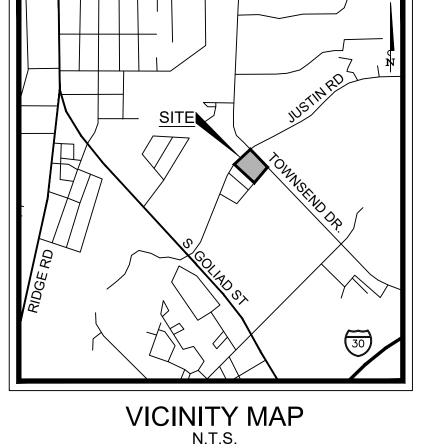
RCAD ROCKWALL

LOFLAND INDUSTRIAL PARK ADDITION LOT 1A-R1, BLOCK A

> 814 JUSTIN ROAD ROCKWALL, TX 75087

> > MAP ID: 2-4





ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OR ACCURACY OF DESIGN.

DESCRIPTION

1ST SUBMITTAL

2ND SUBMITTAL

3RD SUBMITTAL

CONSTRUCTION ASBUILTS

ENGINEER



1903 CENTRAL DR. SUITE #406 BEDFORD, TX 76021 PH. 817.281.0572 FAX 817.281.0574 CONTACT: DREW DONOSKY, PE EMAIL: DREW@CLAYMOOREENG.COM

OWNER

CENTRAL APPRAISAL DISTRICT OF ROCKWALL 841 JUSTIN ROAD ROCKWALL, TX 75087 PH: (972) 771 2034 **CONTACT: KEVIN PASSONS**

PLAN SUBMITTAL LOG

SUBMITTAL DATE

3/10/2021

5/21/2021

7/29/2021

9/22/2025

SEPTEMBER 2025

PROBABLE START OF CONSTRUCTION: 07/2021

This drawing has been revised to show those changes during the construction process reported by the contractor to ClayMoore Engineering, Inc. and considered to be significant. This drawing is not guaranteed to be "As Built" but is based on the information made

STOP! CALL BEFORE YOU DIG

SHEET LIST TABLE

REVISION DATE

9/22/2025

9/22/2025

9/22/2025

9/22/2025

9/22/2025

9/22/2025

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9/22/2025

9/22/2025

5/10/2021

5/10/2021

5/10/2021

SHEET TITLE

PROPOSED REPLAT

GENERAL NOTES

APPROVED SITE PLAN

CITY GENERAL NOTES

CITY GENERAL NOTES

EROSION CONTROL PLAN

EROSION CONTROL DETAILS

EXISTING DRAINAGE AREA MAP

PROPOSED DRAINAGE AREA MAP

STORM DRAIN HGL CALCULATIONS

DETENTION POND A CALCULATIONS

DETENTION POND B CALCULATIONS

DETENTION POND C CALCULATIONS

PRIVATE CONSTRUCTION DETAILS

CITY CONSTRUCTION DETAILS

IMPERVIOUS AREA DELTA CALCULATIONS

CONTECH DETENTION SYSTEM (1 OF 12) CONTECH DETENTION SYSTEM (2 OF 12) CONTECH DETENTION SYSTEM (3 OF 12) CONTECH DETENTION SYSTEM (4 OF 12) CONTECH DETENTION SYSTEM (5 OF 12) CONTECH DETENTION SYSTEM (6 OF 12) CONTECH DETENTION SYSTEM (7 OF 12)

CONTECH DETENTION SYSTEM (8 OF 12) CONTECH DETENTION SYSTEM (9 OF 12) CONTECH DETENTION SYSTEM (10 OF 12) CONTECH DETENTION SYSTEM (11 OF 12) CONTECH DETENTION SYSTEM (12 OF 12)

PLANTING DETAILS & SPECIFICATIONS

TREE DISPOSITION SPECS AND DETAILS

TREE DISPOSITION PLAN

DIMENSION CONTROL AND PAVING PLAN

DEMOLITION PLAN

GRADING PLAN

STORM DRAIN PLAN

STORM DRAIN PROFILES

SHEET NUMBER

C-0

SP-1

C-1

C-2

C-3

C-4

C-5

C-6

C-7

C-8

C-9

C-10

C-11

C-12

C-13

C-14

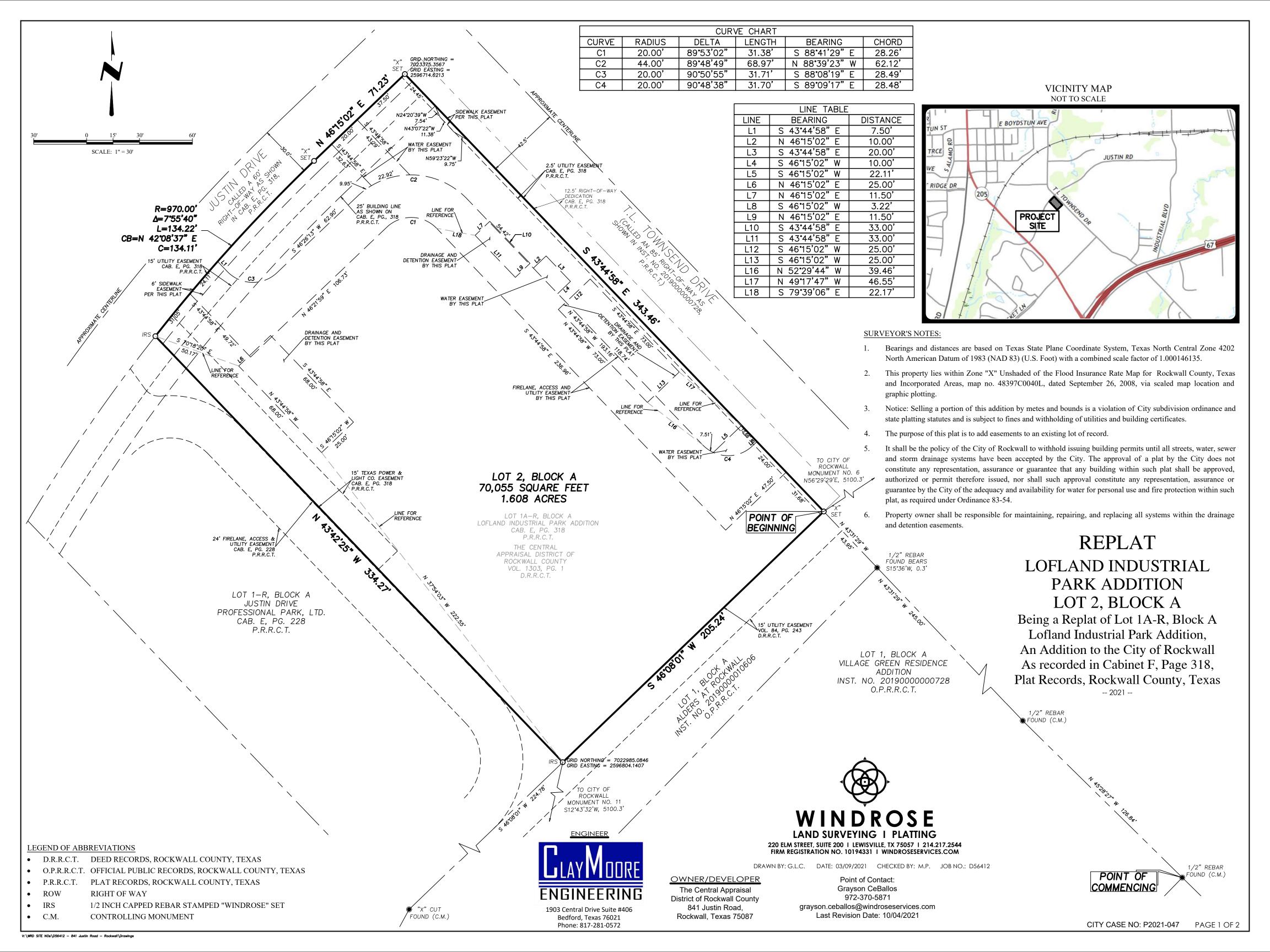
C-15

C-16

C-16.01

TD-1

DIG TESS 1-800-DIG-TESS (@ least 72 hours prior to digging)



STATE OF TEXAS

COUNTY OF ROCKWALL §

WHEREAS The Central Appraisal District of Rockwall County are the owners of a 1.608 acre tract of land situated in the B.J. Lewis Survey, Abstract Number 225, being all of Lot 1A-R, Block A of Lofland Industrial Park Addition, an addition to the City of Rockwall, as recorded in Cabinet E, Page 318, Plat Records, Rockwall County, Texas, being a tract of land described to The Central Appraisal District of Rockwall County, as recorded in Volume 1303, Page 01, Deed Records, Rockwall County, Texas and being more particularly described by metes and bounds as follows: (Bearings and Distances are based on the State Plane Coordinate System, Texas North Central Zone (4202), North American Datum of 1983 (NAD83) (US Foot) with a combined scale factor of 1.00015063);

COMMENCING from a 1/2 inch rebar found for the easternmost corner of Lot 1, Block A of Village Green Residence Addition, an addition to the City of Rockwall, as recorded in Instrument Number 20190000000728, Plat Records, Rockwall County, Texas, also being a point on the south right-of-way line of T.L. Townsend Drive (Called an 85 foot right-of-way as shown in Instrument Number 2019000000728, Plat Records, Rockwall County, Texas);

THENCE North 45 degrees 28 minutes 27 seconds West, with the south right-of-way line of said T.L. Townsend Drive, with a north line of said Lot 1, a distance of 126.84 feet to a 1/2 inch rebar found for an exterior "ell" corner of said Lot 1;

THENCE North 43 degrees 31 minutes 29 seconds West, with the south right-of-way line of said T.L. Townsend Drive, same being a north line of said Lot 1, a distance of 245.00 feet to a point for the northernmost corner of said Lot 1, same being the easternmost northeast corner of Lot 1, Block A of Alders At Rockwall as recorded in Instrument Number 20190000010606, Plat Records, Rockwall County, Texas from which a 1/2 inch rebar found bears South 15 degrees 36 minutes West, a distance of 0.3 feet;

THENCE North 43 degrees 31 minutes 29 seconds West, with the south right-of-way line of said T.L. Townsend Drive, with the northeasternmost line of said second referenced Lot 1, a distance of 43.95 feet to an "X" Cut set in concrete for the easternmost corner of said Lot 1A-R, same being the easternmost northwest corner of said second referenced Lot 1 and said point being THE POINT OF BEGINNING;

THENCE South 46 degrees 08 minutes 01 seconds West, departing the south right-of-way line of said T.L. Townsend Drive, with a northwest line of said second referenced Lot 1, same being the southeastern line of said Lot 1A-R, a distance of 205.24 feet to a 1/2 inch rebar capped "WINDROSE" set for the easternmost corner of Lot 1-R, Block A of Justin Drive Professional Park, LTD, an addition to the City of Rockwall, as recorded in Cabinet E, Page 228, Plat Records Rockwall County, Texas;

THENCE North 43 degrees 42 minutes 25 seconds West, departing a north line of said second referenced Lot 1, with an east line of said Lot 1-R, a distance of 334.27 feet to a 1/2 inch rebar capped "WINDROSE" set for the northern most corner of said Lot 1-R and lying on the south right-of-way line of Justin Drive (Called a 60 foot right-of-way as shown in Cabinet E, Page 318, Plat Records, Rockwall County, Texas) and being the beginning of a curve to the right with a radius of 970.00 feet, a central angle of 07 degrees 55 minutes 40 seconds and a chord bearing and distance of North 42 degrees 08 minutes 37 seconds East, a distance of 134.11 feet;

THENCE with said curve to the right, with the east right-of-way line of said Justin Drive, an arc length of 134.22 feet to an "X" Cut set in concrete for corner;

THENCE North 46 degrees 15 minutes 02 seconds East, with the southeast right-of-way line of said Justin Drive, a distance of 71.23 feet to an "X" Cut set in concrete at the intersection of the southeast right-of-way line of said Justin Drive, same being the southwest right-of-way line of said T.L. Townsend Drive;

THENCE South 43 degrees 44 minutes 58 seconds East, with the southwest right-of-way line of said T.L. Townsend Drive, a distance of 343.46 feet to THE POINT OF BEGINNING and containing 70,055 square feet or 1.608 acres of land, more or less.

NOW, THEREFORE, KNOW ALL MEN BY THESE PRESENTS

I (we) the undersigned owner(s) of the land shown on this plat, and designated herein as the Lofland Industrial Park Addition subdivision to the City of Rockwall, Texas, and whose name is subscribed hereto, hereby dedicate to the use of the public forever all streets, alleys, parks, water courses, drains, easements and public places thereon shown on the purpose and consideration therein expressed. I (we) further certify that all other parties who have a mortgage or lien interest in the Lofland Industrial Park Addition subdivision have been notified and signed this plat. I (we) understand and do hereby reserve the easement strips shown on this plat for the purposes stated and for the mutual use and accommodation of all utilities desiring to use or using same. I (we) also understand the following:

- 1. No buildings shall be constructed or placed upon, over, or across the utility easements as described herein.
- 2. Any public utility shall have the right to remove and keep removed all or part of any buildings, fences, trees, shrubs or other growths or improvements which in any way endanger or interfere with construction, maintenance or efficiency of their respective system on any of these easements strips; and any public utility shall at all times have the right of ingress or egress to, from and upon the said easement strips for purpose of construction, reconstruction, inspecting, patrolling, maintaining, and either adding to or removing all or part of their respective system without the necessity of, at any time, procuring the permission of anyone.
- 3. The City of Rockwall will not be responsible for any claims of any nature resulting from or occasioned by the establishment of grade of streets in the subdivision.
- The developer and subdivision engineer shall bear total responsibility for storm drain improvements.
- 5. The developer shall be responsible for the necessary facilities to provide drainage patterns and drainage controls such that properties within the drainage area are not adversely affected by storm drainage from the development.
- No house dwelling unit, or other structure shall be constructed on any lot in this addition by the owner or any other person until the developer and/or owner has compiled with all requirements of the Subdivision Regulations of the City of Rockwall regarding improvements with respect to the entire block on the street or streets on which property abuts, including the actual installation of streets with the required base and paving, curb and gutter, water and sewer, drainage structures, storm sewers, and alleys, all according to the specifications of the City of Rockwall; or

Until an escrow deposit, sufficient to pay for the cost of such improvements as determined by the City's engineer and/or city administrator, computed on a private commercial rate basis, has been made with the city secretary, accompanied by an agreement signed by the developer and/or owner, authorizing the city to make such improvements at prevailing private commercial rates, or have the same made by a contractor and pay for the same out of the escrow deposit, should the developer and/or owner fail or refuse to install the required improvements within the time stated in such written agreement, but in no case shall the City be obligated to make such improvements itself. Such deposit may be used by the owner and/or developer as progress payments as the work progresses in making such improvements by making certified requisitions to the city secretary, supported by evidence of work done; or

Until the developer and/or owner files a corporate surety bond with the City Secretary in a sum equal to the cost of such improvements for the designated area, guaranteeing the installation thereof within the time stated in the bond, which time shall be fixed by the City Council of the City of Rockwall.

I (we) further acknowledge that the dedications and/or exaction's made herein are proportional to the impact of the Suvdivision upon the public services required in order that the development will comport with the present and future growth needs of the City; I (we), my (our) successors, and assigns hereby waive any claim, damage, or cause of action that I (we) may have as a result of the dedication of exactions made herein.

The Central Appraisal District of Rockwall County

Authorized Representative -	ature	
Printed Name		
Title		
Date		
STATE OF TEXAS	§	
COUNTY OF	§	
	authority, a Notary Public in and for the State of Texas asse name is subscribed to the foregoing instrument and erein expressed.	

day of

841 Justin Road,

Rockwall, Texas 75087

Notary Public in and for the State of Texas

GIVEN UNDER MY HAND AND SEAL OF OFFICE this

LAND SURVEYING I PLATTING

220 ELM STREET, SUITE 200 | LEWISVILLE, TX 75057 | 214.217.2544 FIRM REGISTRATION NO. 10194331 | WINDROSESERVICES.COM

DRAWN BY: G.L.C. DATE: 03/09/2021 CHECKED BY: M.P. JOB NO.: D56412

Point of Contact: Grayson CeBallos 972-370-5871 grayson.ceballos@windroseservices.com Last Revision Date: 10/04/2021

STATE OF TEXAS COUNTY OF DENTON

This is to certify that I, Mark N. Peeples, a Registered Professional Land Surveyor of the State of Texas, have platted the subdivision from an actual survey on the ground, and that this plat correctly represents that survey made by me or under my direction and supervision.

Mark N. Peeples, R.P.L.S. No. 6443

STATE OF TEXAS **COUNTY OF DENTON**

BEFORE ME, the undersigned authority, a Notary Public in and for the State of Texas, on this day personally appeared Mark N. Peeples, known to me to be the person whose name is subscribed to the foregoing instrument and acknowledged to me that he executed the same for the purposes and consideration therein

GIVEN	UNDER	MY	HAND	AND	SEAL	OF	OFFICE	this	day	of
			20						_	

Notary Public in and for the State of Texas

APPROVAL:

I hereby certify that the above and foregoing plat of an addition to the City of Rockwall, Texas was approved by the City Council of the City of Rockwall on the day of

This approval shall be invalid unless the approved plat for such addition is recorded in the office of the County Clerk of Rockwall, County, Texas. Within one hundred eighty (180) days from said date of final approval.

Mayor,	City of Rockwall	

City Secretary

Planning and Zoning Chairman

City Engineer

REPLAT LOFLAND INDUSTRIAL PARK ADDITION LOT 2, BLOCK A

Being a Replat of Lot 1A-R, Block A Lofland Industrial Park Addition, An Addition to the City of Rockwall As recorded in Cabinet F, Page 318, Plat Records, Rockwall County, Texas

CITY CASE NO: P2021-047

-- 2021 --

PAGE 2 OF 2

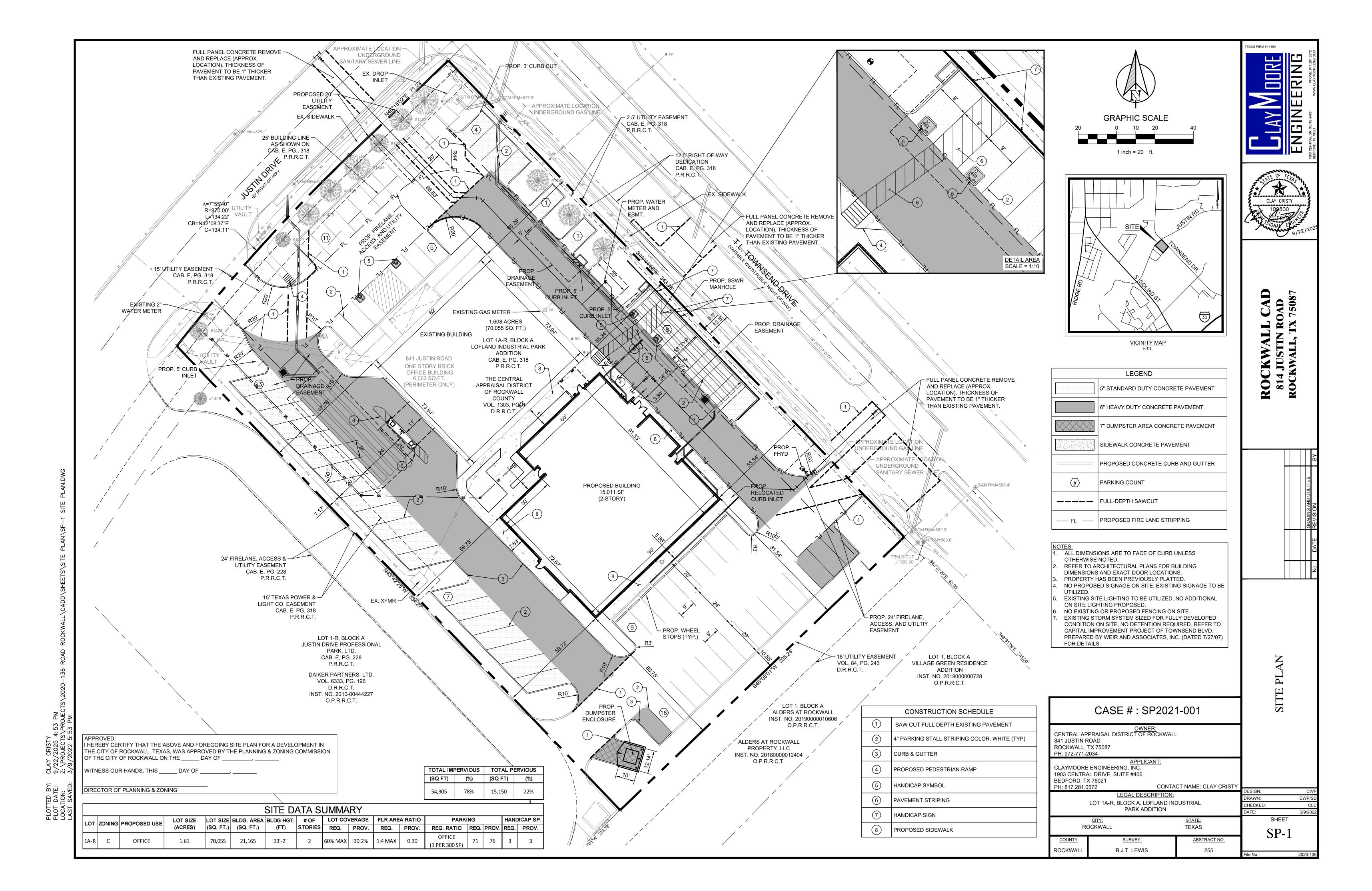
Bedford, Texas 76021

Phone: 817-281-0572

ENGINEER

OWNER/DEVELOPER The Central Appraisal District of Rockwall County 1903 Central Drive Suite #406

V:\WRD SITE NOs\D56412 - 841 Justin Road - Rockwall\Drawing



- 2. THE CONTRACTOR SHALL BE RESPONSIBLE TO FURNISH ALL MATERIALS AND LABOR TO CONSTRUCT THE FACILITY AS SHOWN AND DESCRIBED IN THE CONSTRUCTION DOCUMENTS IN ACCORDANCE WITH THE APPROPRIATE APPROVING AUTHORITIES, SPECIFICATIONS AND REQUIREMENTS. ALL WORK REQUIRED BY THESE PLANS SHALL BE CONDUCTED IN CONFORMANCE WITH CURRENT SAFETY CODES AND STANDARDS WITH JURISDICTION OVER THIS
- 3. THE CONTRACTOR SHALL CONTACT ALL FRANCHISE UTILITY COMPANIES TO HAVE THEM LOCATE EXISTING UTILITIES PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL COORDINATE THE EXACT LOCATION AND DEPTH OF ALL FRANCHISE UTILITY SERVICES AND ANY REQUIRED RELOCATION AND/OR EXTENSIONS. SERVICES SHOWN ON THE PLANS, IF ANY, ARE CONCEPTUAL.
- 4. THE CONTRACTOR SHALL PROTECT ALL PUBLIC AND PRIVATE UTILITIES IN THE CONSTRUCTION OF THIS PROJECT. ALL MANHOLES, CLEANOUTS, VALVE BOXES, POWER POLES, SIGNS, FIRE HYDRANTS, ETC., MUST BE ADJUSTED TO PROPER GRADE BY THE CONTRACTOR PRIOR TO AND AFTER PLACING OF PERMANENT PAVING. UTILITIES MUST BE MAINTAINED TO PROPER LINE AND GRADE DURING CONSTRUCTION OF THE PAVING FOR THIS PROJECT.
- 5. BRACING OF UTILITY POLES MAY BE REQUIRED BY UTILITY COMPANIES WHEN TRENCHING OR EXCAVATION IS IN CLOSE PROXIMITY TO THE POLES. THE COST OF BRACING POLES WILL BE BORNE BY THE CONTRACTOR. THERE IS NO SEPARATE PAY ITEM FOR THIS WORK. THE COST IS INCIDENTAL TO THE VARIOUS PAY ITEMS FOR INSTALLATION OF PIPE
- 6. THE LOCATIONS, ELEVATIONS, AND DIMENSIONS OF EXISTING UTILITIES SHOWN ON THE PLANS WERE OBTAINED FROM AVAILABLE RECORDS AND ARE CONSIDERED APPROXIMATE. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LOCATIONS, ELEVATIONS, AND DIMENSIONS OF ADJACENT AND/OR CONFLICTING UTILITIES SUFFICIENTLY IN ADVANCE OF CONSTRUCTION IN ORDER THAT ADJUSTMENTS CAN BE MADE TO PROVIDE ADEQUATE CLEARANCES. THE CONTRACTOR SHALL PRESERVE AND PROTECT PUBLIC UTILITIES AT ALL TIMES DURING CONSTRUCTION. ANY DAMAGE TO UTILITIES RESULTING FROM CONTRACTOR'S OPERATIONS SHALL BE RESTORED AT THE CONTRACTOR'S EXPENSE. THE ENGINEER SHALL BE NOTIFIED WHEN PROPOSED FACILITY GRADES CONFLICT WITH EXISTING UTILITY GRADES.
- 7. THE CONTRACTOR SHALL IMMEDIATELY REPAIR OR REPLACE ANY PHYSICAL DAMAGE TO PRIVATE PROPERTY, INCLUDING, BUT NOT LIMITED TO FENCES, WALLS, PAVEMENT, GRASS, TREES, AND LAWN SPRINKLER AND IRRIGATION SYSTEMS AT NO COST TO THE OWNER. THIS WORK SHALL BE SUBSIDIARY TO THE CONTRACT (UNLESS OTHERWISE NOTED) AND IS NOT A SEPARATE PAY
- 8. THE CONTRACTOR SHALL REMOVE SURPLUS MATERIAL FROM THE PROJECT AREA. THIS WORK SHALL BE SUBSIDIARY TO THE CONTRACT AND IS NOT A SEPARATE PAY ITEM.
- 9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS PRIOR TO CONSTRUCTION.
- 10. THE CONTRACTOR SHALL HAVE AVAILABLE AT THE JOB SITE AT ALL TIMES A COPY OF THE CONTRACT DOCUMENTS INCLUDING PLANS, SPECIFICATIONS, AND SPECIAL CONDITIONS, COPIES OF ANY REQUIRED CONSTRUCTION PERMITS, EROSION CONTROL PLANS, SWPPP AND INSPECTION REPORTS.
- 11. ANY DISCREPANCIES ON THE DRAWINGS SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE COMMENCING WORK. NO FIELD CHANGES OR DEVIATIONS FROM DESIGN SHALL BE MADE WITHOUT PRIOR APPROVAL OF THE OWNER AND NOTIFICATION TO THE ENGINEER. NO CONSIDERATION WILL BE GIVEN TO CHANGE ORDERS FOR WHICH THE OWNER AND ENGINEER WERE NOT CONTACTED PRIOR TO CONSTRUCTION OF THE AFFECTED ITEM.
- 12. ALL COPIES OF COMPACTION. CONCRETE AND OTHER REQUIRED TEST RESULTS SHALL BE SENT TO THE ARCHITECT, CIVIL ENGINEER, CONTRACTOR AND OWNER DIRECTLY FROM THE TESTING AGENCY.
- 13. ALL NECESSARY INSPECTIONS AND/OR CERTIFICATIONS REQUIRED BY CODES, JURISDICTIONAL AGENCIES AND/OR UTILITY SERVICE COMPANIES SHALL BE OBTAINED BY THE CONTRACTOR PRIOR TO BUILDING POSSESSION AND THE FINAL CONNECTION OF SERVICES.
- 14. CONTRACTOR SHALL VERIFY BENCHMARKS AND DATUM PRIOR TO COMMENCING CONSTRUCTION OR STAKING OF IMPROVEMENTS.
- 15. CONTRACTOR SHALL THOROUGHLY CHECK COORDINATION OF CIVIL, LANDSCAPE, MEP, ARCHITECTURAL, AND OTHER PLANS PRIOR TO COMMENCING CONSTRUCTION. OWNER AND ENGINEER SHALL BE NOTIFIED OF
- ANY DISCREPANCY PRIOR TO COMMENCING WITH CONSTRUCTION. 16. ALL HORIZONTAL DIMENSIONS GIVEN ARE TO FACE OF CURB AND TO PIPE
- CENTERLINES UNLESS OTHERWISE NOTED ON PLANS. 17. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING RELOCATION AND INSTALLATION OF FRANCHISE UTILITIES NECESSARY FOR ON AND OFF SITE CONSTRUCTION. PAYMENT FOR RELOCATION AND INSTALLATION WILL BE NEGOTIATED ONCE IDENTIFIED.
- 18. ALL CUT OR FILL SLOPES SHALL BE 4:1 OR FLATTER UNLESS OTHERWISE
- 19. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL OF DUST AND DIRT RISING AND SCATTERING IN THE AIR DURING CONSTRUCTION AND SHALL PROVIDE WATER SPRINKLING OR OTHER SUITABLE METHODS OF CONTROL. THE CONTRACTOR SHALL COMPLY WITH ALL GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.
- 20.UPON COMPLETION OF CONSTRUCTION, THE CONTRACTOR SHALL PROVIDE THE CIVIL ENGINEER A COPY OF RECORD DRAWINGS IDENTIFYING ALL DEVIATIONS OR VARIATIONS FROM THE ORIGINAL PLANS.
- 21.CONTRACTOR SHALL GIVE NOTICE TO ALL AFFECTED PARTIES AND ALL AUTHORIZED INSPECTORS, SUPERINTENDENTS, OR PERSONS IN CHARGE OF PRIVATE AND PUBLIC UTILITIES OR RAILROADS AFFECTED BY HIS OPERATIONS, AT LEAST 48 HOURS PRIOR TO COMMENCEMENT OF WORK.
- 22.ALL "RECORD" DIMENSIONS SHALL CONFORM TO THE DESIGN DIMENSIONS PLUS OR MINUS 0.02 FEET. ALL "RECORD" SLOPES SHALL CONFORM TO THE DESIGNED SLOPES PLUS OR MINUS 0.005 FOOT/FOOT.
- 23.CONTRACTOR SHALL CONTACT CITY BUILDING OFFICIAL TO LEARN OF ANY UNUSUAL CONSTRUCTION SEQUENCING REQUIREMENTS THAT THE CITY MAY REQUIRE. THE CONTRACTOR IS CAUTIONED THAT THIS AND PERHAPS OTHER SUCH REQUIREMENTS MAY EXIST AND IT IS THE CONTRACTOR'S RESPONSIBILITY TO INVESTIGATE AND COMPLY WITH THEM.

PAVING AND STRIPING NOTES

- 1. THE REINFORCED PORTLAND CEMENT CONCRETE SHOULD HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS FOR STANDARD DUTY CONCRETE AND 3,600 PSI FOR MEDIUM DUTY CONCRETE AND DUMPSTER AREAS, AND A MINIMUM REINFORCING OF #3 BARS @ 18" O.C.E.W. AND SHALL STRICTLY ADHERE TO DETAILS INCLUDED IN THIS SET. A BASE SUB-GRADE PER THE GEOTECHNICAL REPORT IS REQUIRED BENEATH ALL PAVING.
- 2. TESTING OF MATERIALS REQUIRED FOR THE CONSTRUCTION OF THE PAVING IMPROVEMENTS SHALL BE PERFORMED BY AN AGENCY, APPROVED BY THE OWNER, FOR TESTING MATERIALS. PROCUREMENT OF THE TESTING LABORATORY AND THE PAYMENT OF SUCH TESTING SERVICES SHALL BE MADE BY THE OWNER. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ENSURE, BY THE STANDARD TESTING PROCEDURES, THAT THE WORK CONSTRUCTED MEETS THE REQUIREMENTS OF THE CITY AND PROJECT SPECIFICATIONS.
- 3. ALL SIGNS, PAVEMENT MARKINGS, AND OTHER TRAFFIC CONTROL DEVICES SHALL CONFORM TO THE "TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL
- 4. THE CONTRACTOR SHALL REVIEW LOCATION OF ALL TRAFFIC CONTROL
- DEVICES WITH THE OWNER PRIOR TO INSTALLATION. 5. SEE M.E.P. PLANS FOR LOCATION OF PROPOSED SLEEVING AND CONDUITS.
- 6. ALL HANDICAP RAMPING, STRIPING, AND PAVEMENT MARKINGS SHALL CONFORM TO THE MOST RECENT VERSION OF THE AMERICANS WITH DISABILITIES ACT OF 1994 AND THE TEXAS ARCHITECTURAL BARRIERS ACT OF 1994, AND ALL ADDENDUMS OR UPDATES.
- 7. CONTRACTOR SHALL SUBMIT A PAVEMENT JOINTING PLAN TO THE ENGINEER AND OWNER PRIOR TO THE BEGINNING OF ANY CONCRETE PAVING WORK.
- 8. ANY EXISTING CONCRETE OR ASPHALT SHOWN TO BE REMOVED SHALL BE PROPERLY DISPOSED OF BY THE CONTRACTOR OFF SITE. THIS WORK SHALL BE SUBSIDIARY TO THE CONTRACT AND IS NOT A SEPARATE PAY ITEM.
- CONSTRUCTION JOINTS SHALL BE REQUIRED AT INTERRUPTIONS OF PAVING OPERATIONS SUCH AS THOSE OCCURRING AT THE END OF THE DAY OR DUE TO WEATHER OR EQUIPMENT BREAKDOWN. PLACE AT LONGITUDINAL CONSTRUCTION OR ISOLATION JOINT LOCATIONS.
- 10. CONTRACTOR TO INSTALL CONSTRUCTION JOINTS IN CONCRETE PAVEMENT AT ALL PC'S AND AS CONVENIENT TO PHASING OF POURS. CONCRETE PAVEMENT TO BE CONSTRUCTED WITH ISOLATION JOINTS AROUND THE PERIMETER OF ANY BLOCK OUT IN PAVEMENT AND SAWED DUMMY JOINTS EVERY 12' IN BOTH
- 11. ALL JOINTS ARE TO CONTINUE THROUGH THE CURB.
- 12. RADIAL JOINTS SHALL BE NO SHORTER THAN 24".
- 13. ALL CONSTRUCTION JOINTS SHALL BE SAWED, CLEANED OF DEBRIS, BLOWN DRY AND IMMEDIATELY SEALED WITH HOT POURED RUBBER JOINT SEALING COMPOUND.
- 14.NO SAND ALLOWED UNDERNEATH PAVING.

STORM SEWER NOTES

- CONTRACTOR SHALL FIELD VERIFY THE VERTICAL AND HORIZONTAL LOCATIONS OF ALL EXISTING UTILITIES PRIOR TO START OF CONSTRUCTION. THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND CONSTRUCTION MANAGER IMMEDIATELY IF A CONFLICT IS DISCOVERED.
- 2. CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS SHOWN, INCLUDING THE HORIZONTAL AND VERTICAL LOCATION OF CURB INLETS, GRATE INLETS, AND ALL UTILITIES CROSSING THE STORM SEWER. FLOW LINES AND RIMS OF PROPOSED INLETS SHALL BE VERIFIED WITH THE PROPOSED GRADE PRIOR TO CONSTRUCTION.
- 3. THE END OF ALL STORM SEWER LATERALS THAT CONNECT TO WORK BY PLUMBER SHALL BE TIGHTLY PLUGGED OR CAPPED AND MARKED 5.0 FEET OUTSIDE THE BUILDING UNTIL FINAL CONNECTIONS ARE MADE BY PLUMBING
- 4. THE SITE UTILITY CONTRACTOR SHALL PROVIDE ALL MATERIALS AND APPURTENANCES NECESSARY FOR COMPLETE INSTALLATION OF THE STORM
- 5. CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL CONSTRUCTION PERMITS.
- 6. THE GENERAL CONTRACTOR AND ALL SUBCONTRACTORS SHALL VERIFY THE SUITABILITY OF ALL EXISTING AND PROPOSED SITE CONDITIONS INCLUDING GRADES AND DIMENSIONS BEFORE COMMENCEMENT OF CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY OF ANY DISCREPANCIES.
- 7. EXISTING MANHOLE TOPS AND ALL OTHER DRAINAGE FACILITIES SHALL BE ADJUSTED AS REQUIRED TO MATCH FINAL GRADES AS SHOWN ON GRADING PLAN. NO SEPARATE PAY ITEM.
- 8. ALL RCP SHALL BE CLASS 3 OR APPROVED EQUAL.

STORM SEWER DISCHARGE AUTHORIZATION

- 1. IF THE TOTAL DISTURBED AREA EXCEEDS ONE (1) ACRE A NOTICE OF INTENT (N.O.I.) SHALL BE SUBMITTED BY THE CONTRACTOR TO THE TCEQ NO LESS THAN 48 HOURS PRIOR TO COMMENCEMENT OF CONSTRUCTION ACTIVITIES.
- 2. ALL CONTRACTORS AND SUBCONTRACTORS PROVIDING SERVICES RELATED TO THE SWPPP SHALL SIGN A CONTRACTOR CERTIFICATION STATEMENT ACKNOWLEDGING THEIR RESPONSIBILITIES AS SPECIFIED IN THE SWPPP.
- 3. A COPY OF THE SWPPP, INCLUDING CONTRACTOR CERTIFICATIONS AND ANY REVISIONS, SHALL BE SUBMITTED TO THE CITY AND FILED WITH THE CONSTRUCTION PLANS, AND SHALL BE RETAINED ON-SITE DURING CONSTRUCTION.
- 4. A NOTICE OF TERMINATION (N.O.T.) SHALL BE SUBMITTED TO THE TCEQ BY THE CONTRACTOR WHEN THE SITE HAS 100% OF THE DISTURBED AREAS STABILIZED AND THE SITE NO LONGER HAS STORM WATER DISCHARGES ASSOCIATED WITH INDUSTRIAL ACTIVITIES (CONSTRUCTION), OR THE N.O.T. PERMITTEE OR CO-PERMITTEE NO LONGER HOLDS OPERATIONAL CONTROL OF THE CONSTRUCTION.

WATER NOTES

- 1. EXISTING UTILITY DATA IS PROVIDED FOR INFORMATION ONLY. ALTHOUGH THIS DATA IS SHOWN AS ACCURATELY AS POSSIBLE, THE CONTRACTOR IS CAUTIONED THAT THE DEVELOPER AND THE ENGINEER NEITHER ASSUMES NOR IMPLIES ANY RESPONSIBILITY FOR THE ACCURACY OF THIS DATA.
- 2. THE CONTRACTOR IS TO VERIFY LOCATION AND ELEVATION OF EXISTING UTILITIES PRIOR TO CONSTRUCTION.
- 3. HORIZONTAL AND VERTICAL BLOCKING FOR WATER LINES HAS BEEN OMITTED FOR CLARITY. HOWEVER, BLOCKING SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY STANDARDS AND SPECIFICATIONS.
- 4. TRENCHES WHICH LAY OUTSIDE EXISTING OR FUTURE PAVEMENTS SHALL BE BACK FILLED ABOVE THE TOP OF THE EMBEDMENT WITH TYPE 'C' BACKFILL MATERIALS. WHEN TYPE 'C' BACKFILL MATERIAL IS NOT SUITABLE AND AT THE DIRECTION OF THE ENGINEER TYPE 'B' MATERIAL SHALL BE USED. ALL BACKFILL MATERIAL SHALL BE COMPACTED TO A MINIMUM OF 95% PROCTOR DENSITY BY MEANS OF TAMPING ONLY. TRENCHES WHICH CROSS UNDER EXISTING OR FUTURE PAVEMENT SHALL BE BACK FILLED PER FIGURE 'A' WITH 95% PROCTOR STANDARD DENSITY OF -2, +4 OF OPTIMUM MOISTURE CONTENT.
- 5. TOP OF WATER LINES SHALL BE A MINIMUM OF 42" BELOW TOP OF CURB EXCEPT WHERE SHOWN OTHERWISE IN THESE PLANS.
- 6. FIRE HYDRANTS SHALL BE A MINIMUM 3' BEHIND THE FACE OF THE CURB UNLESS OTHERWISE DIRECTED BY THE CITY. FIRE HYDRANTS AND VALVES AS SHOWN ON THESE PLANS ARE SYMBOLIC ONLY.
- 7. CORPORATION STOPS SHALL BE TESTED FOR FULL FLOW WHEN THE SYSTEM IS PRESSURE TESTED.
- 8. ALL NEW WATER MAINS SHALL BE FULLY PURGED.
- 9. ALL 6", 8", 10" & 12" WATER MAINS SHALL BE PVC AWWA C900, DR-14. ALL WATER MAINS USING POLY-WRAPPED DUCTILE IRON PIPE SHALL BE CLASS 51.
- 10.FITTINGS SHALL BE DUCTILE IRON AND MECHANICAL JOINT TYPE, WITH "COR-BLUE" BOLTS AND SHALL BE CLASS 250.

SANITARY SEWER NOTES

- 1. EXISTING UTILITY DATA IS PROVIDED FOR INFORMATION ONLY. ALTHOUGH THIS DATA IS SHOWN AS ACCURATELY AS POSSIBLE, THE CONTRACTOR IS CAUTIONED THAT THE DEVELOPER AND THE ENGINEER NEITHER ASSUMES NOR IMPLIES ANY RESPONSIBILITY FOR THE ACCURACY OF THIS DATA.
- 2. THE CONTRACTOR IS TO VERIFY LOCATION AND ELEVATION OF EXISTING UTILITIES PRIOR TO CONSTRUCTION.
- 3. TRENCHES WHICH LIE OUTSIDE EXISTING PAVEMENTS SHALL BE BACKFILLED ABOVE THE TOP OF THE EMBEDMENT WITH TYPE "C" BACKFILL MATERIAL. WHEN TYPE "C" BACKFILL MATERIAL IS NOT SUITABLE AND AT THE DIRECTION OF THE ENGINEER, TYPE "B" MATERIAL SHALL BE USED. ALL BACKFILL MATERIAL SHALL BE COMPACTED TO A MINIMUM OF 90% OF THE MAXIMUM DRY DENSITY BY MEANS OF TAMPING ONLY. TRENCHES THAT CROSS UNDER EXISTING OR FUTURE PAVEMENT SHALL BE BACKFILLED AND COMPACTED TO A MINIMUM OF 95% OF THE MAXIMUM DRY DENSITY WITH MOISTURE CONTENT-2 AND +4% OF OPTIMUM MOISTURE CONTENT.
- 4. TYPICAL LOCATION OF SANITARY SEWER PIPE SHALL BE A MINIMUM OF 4'-0" BELOW TOP OF CURB EXCEPT WHERE SHOWN OTHERWISE IN THESE PLANS
- 5. ALL FLEXIBLE SANITARY SEWER MAINS SHALL BE TESTED WITH STANDARD 5% DEFLECTION MANDREL.
- 6. ALL SANITARY SEWER LINES SHALL BE CAPPED WITH AN APPROPRIATE CAP AT THE END OF EACH WORKDAY. 7. WHEN EXISTING GRADES ARE LOWER THAN PROPOSED MAINS, THE FILL AREA
- OVER THE PIPE SHALL BE FILLED AND COMPACTED TO A MINIMUM OF 95% OF THE MAXIMUM DRY DENSITY TO THE PROPOSED FINISHED GRADE PRIOR TO INSTALLING ANY MAIN.
- 8. ALL SEWER SERVICES SHALL BE CONSTRUCTED OF SDR-35 PIPE.

TRAFFIC CONTROL NOTES

- 1. CONTRACTOR SHALL PROVIDE TRAFFIC CONTROL PLANS TO THE OWNER, AT LEAST 48 HOURS PRIOR TO CONSTRUCTION ACTIVITY.
- 2. ALL TRAFFIC CONTROL MEASURES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (TMUTCD), LATEST VERSION.
- 3. THE CONTRACTOR SHALL COVER EXISTING SIGNS AND OBLITERATE EXISTING PAVEMENT MARKINGS THAT CONFLICT WITH THE INTENT OF THESE TRAFFIC CONTROL PLANS TO AVOID CONFUSION TO THE TRAVELING PUBLIC.
- 4. THE CONTRACTOR SHALL UNCOVER EXISTING SIGNS AND REPLACE PAVEMENT MARKINGS IN-KIND AS ORIGINALLY CONFIGURED AT THE END OF CONSTRUCTION OPERATIONS AND PRIOR TO FINAL ACCEPTANCE BY THE
- 5. ALL TEMPORARY SIGNS, BARRICADES, WARNING LIGHTS AND OTHER MISCELLANEOUS TRAFFIC CONTROL MEASURES SHALL BE REMOVED AND ORIGINAL TRAFFIC CONTROL MEASURES REPLACED AT THE END OF THE CONTRACTOR'S CONSTRUCTION OPERATIONS.

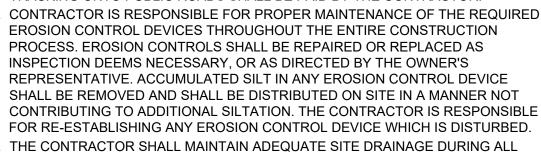
EROSION CONTROL NOTES

- 1. THE CONTRACTOR SHALL COMPLY WITH ALL FEDERAL, STATE, AND LOCAL EROSION, CONSERVATION, AND SILTATION ORDINANCES. THE CONTRACTOR SHALL USE SEDIMENT FILTERS OR OTHER MEASURES APPROVED BY THE ENGINEER AND CONSTRUCTION MANAGER TO PREVENT SILT AND CONSTRUCTION DEBRIS FROM CLOGGING STORM SEWER PIPES OR PROPOSED OR EXISTING INLETS, OR FROM BEING TRANSPORTED TO ADJACENT PROPERTIES AND STREET RIGHT-OF-WAYS. ALL EROSION CONTROL DEVICES SHALL BE INSTALLED PRIOR TO SITE DISTURBANCE AND SHALL REMAIN IN PLACE UNTIL FINAL GRADING AND PAVING IS COMPLETE AND PERMANENT SOIL STABILIZATION IS ACHIEVED.
- 2. CONSTRUCTION OPERATIONS SHALL BE MANAGED SO THAT AS MUCH OF THE SITE AS POSSIBLE IS LEFT COVERED WITH EXISTING TOPSOIL AND VEGETATION.
- 3. ALL SLOPES AND AREAS DISTURBED BY CONSTRUCTION SHALL BE GRADED SMOOTH. THE AREAS SHALL THEN BE SEEDED (OR SODDED), IRRIGATED, AND MAINTAINED UNTIL PERMANENT STAND OF GRASS IS ACHIEVED WITH A MINIMUM OF 70% COVERAGE. UNLESS OTHERWISE NOTED, PRIVATE LAWN AREAS AND PARKWAYS IN FRONT OF PRIVATE LAWN AREAS DISTURBED BY CONSTRUCTION SHALL BE REPLACED WITH BLOCK SOD SIMILAR TO THAT
- 4. CONTRACTOR SHALL CONSTRUCT A STABILIZED CONSTRUCTION ENTRANCE AT ALL PRIMARY POINTS OF ACCESS. CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT ALL CONSTRUCTION TRAFFIC UTILIZES THE STABILIZED ENTRANCE AT ALL TIMES FOR INGRESS/EGRESS TO THE SITE.
- 5. CONSTRUCTION ENTRANCE:
 - MINIMUM SIZE STONE: 4-6 INCHES DIAMETER
 - THICKNESS: NOT LESS THAN 12-INCHES LENGTH: 50-FEET MINIMUM
 - WIDTH: NOT LESS THAN 20-FEET MINIMUM OF ALL POINTS OF INGRESS
 - AND EGRESS. MAINTENANCE REQUIREMENTS: AS NECESSARY TO PREVENT TRACKING OR FLOWING MUD INTO PUBLIC RIGHT-OF-WAY OR PARKING

- WHICH SHALL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC ROADWAYS. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ON A PUBLIC ROADWAY SHALL BE REMOVED IMMEDIATELY. WHEN WASHING IS REQUIRED TO REMOVE SEDIMENT PRIOR TO ENTRANCE TO A PUBLIC ROADWAY, IT SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE WHICH DRAINS INTO AN APPROVED SEDIMENT BASIN. ALL FINES IMPOSED FOR TRACKING ONTO PUBLIC ROADS SHALL BE PAID BY THE CONTRACTOR.
- 7. CONTRACTOR IS RESPONSIBLE FOR PROPER MAINTENANCE OF THE REQUIRED EROSION CONTROL DEVICES THROUGHOUT THE ENTIRE CONSTRUCTION PROCESS. EROSION CONTROLS SHALL BE REPAIRED OR REPLACED AS INSPECTION DEEMS NECESSARY, OR AS DIRECTED BY THE OWNER'S REPRESENTATIVE. ACCUMULATED SILT IN ANY EROSION CONTROL DEVICE SHALL BE REMOVED AND SHALL BE DISTRIBUTED ON SITE IN A MANNER NOT
- 8. THE CONTRACTOR SHALL MAINTAIN ADEQUATE SITE DRAINAGE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL USE FILTER BARRIER (OR OTHER METHOD APPROVED BY THE ENGINEER AND CITY) AS REQUIRED TO AND CONSTRUCTION DEBRIS FLOWING ONTO ADJACENT PROPERTIES AS
- AT ALL TIMES DURING THE COURSE OF THE PROJECT. THE GRADING CONTRACTOR SHALL PROVIDE ALL NECESSARY ENGINEERING AND SURVEYING
- BEGINNING CONSTRUCTION.
- WATER POLLUTION PREVENTION PLAN WITH ALL CONDITIONS, ATTACHMENTS, EXHIBITS, AND PERMIT MODIFICATIONS IN GOOD CONDITION AT THE CONSTRUCTION SITE. THE COMPLETE PERMIT MUST BE AVAILABLE FOR

- 1. IF A GRADING PERMIT IS REQUIRED FROM THE CITY PRIOR TO STARTING CONSTRUCTION, CONTRACTOR IS RESPONSIBLE FOR OBTAINING PERMIT AND
- 2. CONTRACTOR SHALL FIELD VERIFY HORIZONTAL AND VERTICAL LOCATION OF ALL EXISTING UTILITIES PRIOR TO CONSTRUCTION. CONTRACTOR IS THE CONTRACTOR SHALL REPLACE THEM AT HIS OWN EXPENSE.
- 3. ALL SPOT ELEVATIONS SHOWN ARE TO TOP OF PAVING SURFACE OR FINISHED EARTH GRADE UNLESS NOTED OTHERWISE.
- 4. CONTRACTOR TO ENSURE POSITIVE DRAINAGE FROM THE EXISTING AND PROPOSED BUILDINGS AND NO PONDING IN PAVED AREAS. CONTRACTOR ADJUSTMENTS TO SPOT GRADES TO MAINTAIN POSITIVE DRAINAGE IS ALLOWED WITH THE PRIOR APPROVAL OF THE ENGINEER. CONTRACTOR SHALL CONTACT THE ENGINEER PRIOR TO PAVING IF ANY AREAS OF POOR
- 5. THE CONTRACTOR SHALL PROTECT ALL MANHOLE COVERS, VALVE COVERS, VAULT LIDS, FIRE HYDRANTS, POWER POLES, GUY WIRES, AND TELEPHONE BOXES WHICH ARE TO REMAIN IN PLACE AND UNDISTURBED DURING
- 6. ALL EXISTING CONCRETE PAVING, CHANNEL IMPROVEMENTS, SIDEWALK AND DISPOSED OF BY THE CONTRACTOR, OFFSITE UNLESS OTHERWISE
- 7. ALL CLEARING, GRADING, COMPACTION AND SUBGRADE PREPARATION SHALL
- 8. GRADING CONTRACTOR TO COORDINATE WITH THE FRANCHISE UTILITY
- COMPANIES FOR ANY REQUIRED UTILITY ADJUSTMENTS AND/OR RELOCATIONS. 9. THE CONTRACTOR SHALL CALCULATE HIS OWN EARTHWORK QUANTITIES AND USE TO DETERMINE HIS BID ACCORDINGLY.
- IN NO CASE SHALL HANDICAP RAMP SLOPES EXCEED 1 VERTICAL TO 12 HORIZONTAL. IN NO CASE SHALL SIDEWALK CROSS SLOPES EXCEED 2.0 PERCENT. IN NO CASE SHALL LONGITUDINAL SIDEWALK SLOPES EXCEED 5.0 WILL BE ACCEPTED FOR A.D.A. AND T.A.S. COMPLIANCE ISSUES.

6. SITE ENTRY AND EXIT LOCATIONS SHALL BE MAINTAINED IN A CONDITION



- PREVENT ADVERSE OFF SITE IMPACTS OR STORM WATER QUALITY FROM SILT REQUIRED BY THE CITY. 9. BEFORE ANY EARTHWORK IS DONE, THE CONTRACTOR SHALL STAKE OUT AND MARK THE LIMITS OF CONSTRUCTION AND OTHER ITEMS ESTABLISHED BY THE PLANS. THE CONTRACTOR SHALL PROTECT AND PRESERVE CONTROL POINTS
- FOR LINE AND GRADE CONTROL POINTS RELATED TO EARTHWORK. 10. CONTRACTOR STAGING AREA TO BE AGREED UPON BY OWNER PRIOR TO
- 11. THE CONTRACTOR MUST REVIEW AND MAINTAIN A COPY OF THE STORM REVIEW UPON REQUEST BY THE T.C.E.Q. OR THE GOVERNING CITY.

- PAYING ALL ASSOCIATED FEES.
- RESPONSIBLE FOR PROTECTING EXISTING UTILITIES (SHOWN OR NOT SHOWN) WITHIN SCOPE OF CONSTRUCTION. IF ANY EXISTING UTILITIES ARE DAMAGED,
- DRAINAGE ARE ENCOUNTERED.
- CONSTRUCTION.
- STRUCTURES AND CURB DEMOLITION SHALL BE REMOVED IN THEIR ENTIRETY DIRECTED BY THE OWNER OR ENGINEER.
- BE IN ACCORDANCE TO THE GEOTECHNICAL REPORT.
- 10.BEFORE PLACING PAVEMENT. CONTRACTOR SHALL VERIFY THAT SUITABLE HANDICAPPED ROUTES (PER A.D.A. & T.A.S) EXIST TO AND FROM EVERY DOOR. PERCENT. CONTRACTOR SHALL CONTACT ENGINEER PRIOR TO PAVING IF ANY EXCESSIVE SLOPES ARE ENCOUNTERED. NO CONTRACTOR CHANGE ORDERS

RING AY



SHEET

ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION ASSUMES NO RESPONSIBILITY FOR

ADEQUACY OR ACCURACY OF DESIGN.

GENERAL ITEMS

- All construction shall conform to the requirements set forth in the City of Rockwall's Engineering Department's "Standards of Design and Construction" and the "Standard Specifications for Public Works Construction" by the North Texas Central Council of Governments, 5th edition amended by the City of Rockwall. The CONTRACTOR shall reference the latest City of Rockwall standard details provided in the Rockwall Engineering Departments "Standards of Design and Construction" manual for details not provided in these plans. The CONTRACTOR shall possess one set of the NCTCOG Standard Specifications and Details and the City of Rockwall's "Standards of Design and Construction" manual on the project site at all times
- Where any conflicting notes, details or specifications occur in the plans the City of Rockwall General Construction Notes, Standards, Details and Specifications shall govern unless detail or specification is more
- The City of Rockwall Engineering Departments "Standards of Design and Construction" can be found online at: http://www.rockwall.com/engr.asp
- All communication between the City and the CONTRACTOR shall be through the Engineering Construction Inspector and City Engineer or designated representative only. It is the responsibility of the CONTRACTOR to contact the appropriate department for inspections that do not fall under this approved engineering plan set.
- Prior to construction, CONTRACTOR shall have in their possession all necessary permits, plans, licenses,
- The CONTRACTOR shall have at least one original stamped and signed set of approved engineering plans and specifications on-site and in their possession at all times. A stop work order will be issued if items are not on-site. Copies of the approved plans will not be substituted for the required original "approved plans to
- All material submittals, concrete batch designs and shop drawings required for City review and approval shall be submitted by the CONTRACTOR to the City sufficiently in advance of scheduled construction to allow no less than 10 business days for review and response by the City.
- All site dimensions are referenced to the face of curb or edge of pavement unless otherwise noted.
- The City requires ten (10%) percent-two (2) year maintenance bond for paving, paving improvements, water systems, wastewater systems, storm sewer systems including detention systems, and associated fixtures and structures which are located within the right-of-ways or defined easements. The two (2) year maintenance bond is to state "from date of City acceptance" as the starting time.
- 10. A review of the site shall be conducted at twenty (20) months into the two (2) year maintenance period. The design engineer or their designated representative and the CONTRACTOR shall be present to walk the site with the City of Rockwall Engineering Inspection personnel.

EROSION CONTROL & VEGETATION

- 1. The CONTRACTOR or developer shall be responsible, as the entity exercising operational control, for all permitting as required by the Environmental Protection Agency (EPA) and the Texas Commission on Environmental Quality (TCEQ). This includes, but is not limited to, preparation of the Storm Water Pollution Prevention Plan (SWPPP), the Construction Site Notice (CSN), the Notice of Intent (NOI), the Notice of Termination (NOT) and any Notice of Change (NOC) and is required to pay all associated fees
- Erosion control devices as shown on the erosion control plan for the project shall be installed prior to the start of land disturbing activities.
- All erosion control devices are to be installed in accordance with the approved plans, specifications and Storm Water Pollution Prevention Plan (SWPPP) for the project. Erosion control devices shall be placed and in working order prior to start of construction. Changes are to be reviewed and approved by the design engineer and the City of Rockwall prior to implementation.
- If the Erosion Control Plans and Storm Water Pollution Prevention Plan (SWPPP) as approved cannot appropriately control erosion and off-site sedimentation from the project, the erosion control plan and/or the SWPPP is required to be revised and any changes reported to the Texas Commission on Environmental Quality (TCEQ), when applicable
- All erosion control devices shall be inspected weekly by the CONTRACTOR and after all major rain events, or more frequently as dictated in the project Storm Water Pollution Prevention Plan (SWPPP). CONTRACTOR shall provide copies of inspection's reports to the engineering inspection after each
- The CONTRACTOR shall not dispose of waste and any materials into streams, waterways or floodplains. The CONTRACTOR shall secure all excavation at the end of each day and dispose of all excess materials.
- CONTRACTOR shall take all available precautions to control dust. CONTRACTOR shall control dust by sprinkling water or other means as approved by the City Engineer.
- CONTRACTOR shall establish grass and maintain the seeded area, including watering, until a "Permanent Stand of Grass" is obtained at which time the project will be accepted by the City. A "Stand of Grass" (not winter rye or weeds) shall consist of 75% to 80% coverage of all disturbed areas and a minimum of one-inch (1") in height as determined by the City. No bare spots will be allowed. Re-seeding will be required in all washed areas and areas that don't grow.
- All City right-of-ways shall be sodded if disturbed. No artificial grass is allowed in any City right-of-way and/or easements.
- 10. All adjacent streets/alleys shall be kept clean at all times
- 11. CONTRACTOR shall keep construction site clean at all times, immediately contain all debris and trash, all debris and trash shall be removed at the end of each work day, and all vegetation on the construction site 10inches or taller in height must be cut immediately.
- 12. Suspension of all construction activities for the project will be enforced by the City if any erosion control requirements are not meet. Work may commence after deficiency has been rectified.
- 13. During construction of the project, all soil stockpiles and borrow areas shall be stabilized or protected with sediment trapping measures. The CONTRACTOR is responsible for the temporary protection and permanent stabilization of all soil stockpiles on-site as well as borrow areas and soil intentionally transported from the
- 14. Where construction vehicles access routes intersect paved or public roads/alleys, construction entrances shall be installed to minimize the transport of sediment by vehicular tracking onto paved surfaces. Where sediment is transferred onto paved or public surfaces, the surface shall be immediately cleaned. Sediment shall be

- removed from the surface by shoveling or sweeping and transported to a sediment disposal area. Pavement washing shall be allowed only after sediment is removed in this manner.
- 15. All drainage inlets shall be protected from siltation, ineffective or unmaintained protection devices shall be immediately replaced and the inlet and storm system cleaned. Flushing is not an acceptable method of
- 16. During all dewatering operations, water shall be pumped into an approved filtering device prior to discharge into a receiving outlet.

TRAFFIC CONTROL

- . All new Detouring or Traffic Control Plans are required to be submitted to the City for review and approval a minimum of 21 calendar days prior to planned day of implementation.
- When the normal function of the roadway is suspended through closure of any portion of the right-of-way, temporary construction work zone traffic control devices shall be installed to effectively guide the motoring public through the area. Consideration for road user safety, worker safety, and the efficiency of road user flow is an integral element of every traffic control zone.
- All traffic control plans shall be prepared and submitted to the Engineering Department in accordance with the standards identified in Part VI of the most recent edition of the TMUTCD. Lane closures will not occur on roadways without an approval from the Rockwall Engineering Department and an approved traffic control plan. Traffic control plans shall be required on all roadways as determined by the City Engineer or the designated representative.
- All traffic control plans must be prepared, signed, and sealed by an individual that is licensed as a professional engineer in the State of Texas. All traffic control plans and copies of work zone certification must be submitted for review and approval a minimum of three (3) weeks prior to the anticipated temporary traffic control.
- The CONTRACTOR executing the traffic control plan shall notify all affected property owners two (2) weeks prior to any the closures in writing and verbally.
- 6. Any deviation from an approved traffic control plan must be reviewed by the City Engineer or the designated representative. If an approved traffic control plan is not adhered to, the CONTRACTOR will first receive a verbal warning and be required to correct the problem immediately. If the deviation is not corrected, all construction work will be suspended, the lane closure will be removed, and the roadway opened to traffic.
- All temporary traffic control devices shall be removed as soon as practical when they are no longer needed. When work is suspended for short periods of time at the end of the workday, all temporary traffic control devices that are no longer appropriate shall be removed or covered. The first violation of this provision will result in a verbal warning to the construction foreman. Subsequent violations will result in suspension of all work at the job site for a minimum of 48 hours. All contractors working on City funded projects will be charged one working day for each 24 hour closure.
- Lane closures on any major or minor arterial will not be permitted between the hours of 6:00 am to 9:00 am and 3:30 pm to 7:00 pm. Where lane closures are needed in a school area, they will not be permitted during peak hours of 7:00 am - 9:00 am and 3:00 pm to 5:00 pm. Closures may be adjusted according to the actual start-finish times of the actual school with approval by the City Engineer. The first violation of this provision will result in a verbal warning to the construction foreman. Subsequent violations will result in suspension of all work at the job site for a minimum of 48 hours. All contractors working on City funded projects will be charged one working day for each 24 hour closure of a roadway whether they are working or not.
- 9. No traffic signs shall be taken down without permission from the City.
- 10. No street/roadway will be allowed to be fully closed.

UTILITY LINE LOCATES

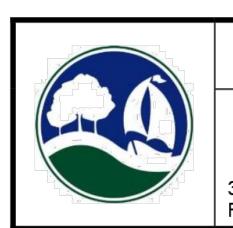
- It is the CONTRACTOR's responsibility to notify utility companies to arrange for utility locates at least 48 hours prior to beginning construction. The completeness and accuracy of the utility data shown on the plans is not guaranteed by the design engineer or the City. The CONTRACTOR is responsible for verifying the depth and location of existing underground utilities proper to excavating, trenching, or drilling and shall be required to take any precautionary measures to protect all lines shown and .or any other underground utilities not on record or not shown on the plans.
- 2. The CONTRACTOR shall be responsible for damages to utilities
- 3. CONTRACTOR shall adjust all City of Rockwall utilities to the final grades.
- 4. All utilities shall be placed underground.
- 5. CONTRACTOR shall be responsible for the protection of all existing main lines and service lines crossed or exposed by construction operations. Where existing mains or service lines are cut, broken or damaged, the CONTRACTOR shall immediately make repairs to or replace the entire service line with same type of original construction or better. The City of Rockwall can and will intervene to restore service if deemed necessary and charge the CONTRACTOR for labor, equipment, material and loss of water if repairs aren't made in a timely manner by the CONTRACTOR.
- 6. The City of Rockwall (City utilities) is not part of the Dig Tess or Texas one Call 811 line locate system. All City of Rockwall utility line locates are to be scheduled with the City of Rockwall Service Center. 972-771-7730. A 48-hour advance notice is required for all non-emergency line locates.
- Underground utility lines shall be installed in accordance with the following standards in addition to other
- applicable criteria: a. No more than 500 linear feet of trench may be opened at one time.
- b. Material used for backfilling trenches shall be properly compacted to 95% standard density in order to minimize erosion, settlement, and promote stabilization that the geotechnical engineer recommends.
- c. Applicable safety regulations shall be complied with.
- 11. This plan details pipes up to 5 feet from the building. Refer to the building plans for building connections. CONTRACTOR shall supply and install pipe adapters as necessary.
- 12. All underground lines shall be installed, inspected, and approved prior to backfilling.
- 13. All concrete encasement shall have a minimum of 28 days compressive strength at 3,000 psi (min. 5.5 sack mix).

WATER LINE NOTES

- The CONTRACTOR shall maintain existing water service at all times during construction.
- Proposed water lines shall be AWWA C900-16 PVC Pipe (blue in color) for all sizes, DR 14 (PC 305) for pipeline sizes 12-inch and smaller, and DR 18 (PC 235) for 14-inch and larger water pipelines unless otherwise shown on water plan and profiles sheets. Proposed water lines shall be constructed with minimum cover of 4 feet for 6-inch through 8-inch, 5 feet for 12-inch through 18-inch and 6 feet for 20-inch and larger.
- Proposed water line embedment shall be NCTCOG Class 'B-3' as amended by the City of Rockwall's engineering standards of design and construction manual.
- CONTRACTOR shall coordinate the shutting down of all water lines with the City of Rockwall Engineering Inspector and Water Department. The City shall operate all water valves. Allow 5 business days from the date of notice to allow City personnel time to schedule a shut down. Two additional days are required for the CONTRACTOR to notify residents in writing of the shut down after the impacted area has been identified. Water shut downs impacting businesses during their normal operation hours is not allowed. CONTRACTOR is required to coordinate with the Rockwall Fire Department regarding any fire watch requirements as well as any costs incurred when the loss of fire protection to a structure occurs.
- CONTRACTOR shall furnish and install gaskets on water lines between all dissimilar metals and at valves (both existing and proposed).
- 5. All fire hydrants and valves removed and salvaged shall be returned to the City of Rockwall Municipal
- Blue EMS pads shall be installed at every change in direction, valve, curb stop and service tap on the proposed water line and every 250'.
- 8. All water valve hardware and valve extensions, bolts, nuts and washers shall be 316 stainless steel.
- 9. All fire hydrants bolts, nuts and washers that are buried shall be 316 stainless steel.
- 10. Abandoned water lines to remain in place shall be cut and plugged and all void spaces within the abandoned line shall be filled with grout, flowable fill or an expandable permanent foam product. Valves to be abandoned in place shall have any extensions and the valve box removed and shall be capped in concrete.
- 11. All fire hydrants will have a minimum of 5 feet of clearance around the appurtenance including but not limited to parking spaces and landscaping.
- 12. All joints are to be megalug joints with thrust blocking.
- 13. Water and sewer mains shall be kept 10 feet apart (parallel) or when crossing 2 feet vertical clearance.
- 14. CONTRACTOR shall maintain a minimum of 4 feet of cover on all water lines.
- 15. All domestic and irrigation services are required to have a testable backflow device with a double check valve installed per the City of Rockwall regulations at the property line and shown on plans.

WASTEWATER LINE NOTES

- The CONTRACTOR shall maintain existing wastewater service at all times during construction.
- Wastewater line for 4-inch through 15-inch shall be Green PVC SDR 35 (ASTM D3034) [less 10 ft cover] and SDR 26 (ASTM D3034) [10 ft or more cover]. For 18-inch and lager wastewater line shall be Green PVC – PS 46 (ASTM F679) [less 10 ft cover] and PS 115 (ASTM F679) [10 ft or more cover]. No services will be allowed on a sanitary sewer line deeper than 10 feet.
- Proposed wastewater line embedment shall be NCTCOG Class 'H' as amended by the City of Rockwall's public works standard design and construction manual.
- Green EMS pads shall be installed at every 250', manhole, clean out and service lateral on proposed wastewater lines.
- 5. CONTRACTOR shall CCTV all existing wastewater lines that are to be abandoned to ensure that all laterals are accounted for and transferred to proposed wastewater lines prior to abandonment.
- 6. All abandoned wastewater and force main lines shall be cut and plugged and all void spaces within the abandoned line shall be filled with grout, flowable fill or an expandable permanent foam product.
- Existing manholes and cleanouts not specifically called to be relocated shall be adjusted to match final grades.
- All wastewater pipes and public services shall be inspected by photographic means (television and DVD) prior to final acceptance and after franchise utilities are installed. The CONTRACTOR shall furnish a DVD to the Engineering Construction Inspector for review. Pipes shall be cleaned prior to TV inspection of the pipes. Any sags, open joints, cracked pipes, etc. shall be repaired or removed by the CONTRACTOR at the CONTRACTOR's expense. A television survey will be performed as part of the final testing in the twentieth (20th) month of the maintenance period.
- All manholes (public or private) shall be fitted with inflow prevention. The inflow prevention shall conform to the measures called out in standard detail R-5031.
- 10. All new or existing manholes being modified shall have corrosion protection being Raven Liner 405 epoxy coating, ConShield, or approved equal.. Consheild must have terracotta color dye mixed in the precast and cast-in-place concrete. Where connections to existing manholes are made the CONTRACTOR shall rehab manhole as necessary and install a 125 mil thick coating of Raven Liner 405 or approved equal.
- 11. All new or existing manholes that are to be placed in pavement shall be fitted with a sealed (gasketed) rim and cover to prevent inflow.
- 12. If an existing wastewater main or trunk line is called out to be replaced in place a wastewater bypassing pump plan shall be required and submitted to the Engineering Construction Inspector and City Engineer for approva prior to implementation. Bypass pump shall be fitted with an auto dialer and conform to the City's Noise Ordinance. Plan shall be to the City sufficiently in advance of scheduled construction to allow no less than 10 business days for review and response by the City.
- 13. CONTRACTOR shall maintain a minimum of 4 feet of cover on all wastewater lines.



GENERAL CONSTRUCTION NOTES Sheet 1 of 2

October 2020

CITY OF ROCKWALL **ENGINEERING DEPARTMENT**

385 S. Goliad Rockwall, Texas 75087

P (972) 771-7746 F (972) 771-7748

REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OR ACCURACY OF DESIGN.





ROCKWALL CAD 814 JUSTIN ROAD ROCKWALL, TX 75087

SHEET

- DEMOLITION, REMOVAL, DISPOSAL AND EXCAVATION NOTES
- All pavements to be removed and replaced shall be saw cut to full depth along neat squared lines shown in
- Proposed concrete pavement shall be constructed with longitudinal butt construction joints at all connections to existing concrete pavement.
- All public concrete pavement to be removed and replaced shall be full panel replacement, 1-inch thicker and on top of 6-inch thick compacted flexbase.
- No excess excavated material shall be deposited in low areas or along natural drainage ways without written permission from the affected property owner and the City of Rockwall. No excess excavation shall be deposited in the City Limits without a permit from the City of Rockwall. If the CONTRACTOR places excess materials in these areas without written permission, the CONTRACTOR will be responsible for all damages resulting from such fill and shall remove the material at their own cost.

PAVING AND GRADING

- All detention systems are to be installed and verified for design compliance along with the associated storm sewer and outflow structures, prior to the start of any paving operations (including building foundations). Erosion protection shall be placed at the pond outflow structures, silt fence along the perimeter of the pond along with any of the associated erosion BMPs noted on the erosion control plan, and the sides and bottom of the detention system shall have either sod or anchored seeded curlex installed prior to any concrete placement.
- All paving roadway, driveways, fire lanes, drive-isles, parking, dumpster pads, etc. sections shall have a minimum thickness, strength, reinforcement, joint type, joint spacing and subgrade treatment shall at a minimum conform to the City standards of Design and Construction and table below.

Carrent/Personant Trans	Minimum	Streng th 28-	Minimum (sacks /	Cement		einforcement
Street/Pavement Type	Thickness (inches)	Day (psi)	Machine placed	Hand Placed	Bar#	Spacing (O.C.E.W.)
Arterial	10"	3,600	6.0	6.5	#4 bars	18"
Collector	8"	3,600	6.0	6.5	#4 bars	18"
Residential	6"	3,600	6.0	6.5	#3 bars	24"
Alley	7"-5"-7"	3,600	6.0	6.5	#3 bars	24"
Fire Lane	6"	3,600	6.0	6.5	#3 bars	24"
Driveways	6"	3,600	6.0	6.5	#3 bars	24"
Barrier Free Ramps	6"	3,600	N/A	6.5	#3 bars	24"
Sidewalks	4"	3,000	N/A	5.5	#3 bars	24"
Parking Lot/Drive Aisles	5"	3,000	5.0	5.5	#3 bars	24"
Dumpster Pads	7"	3,600	6.0	6.5	#3 bars	24"

- Reinforcing steel shall be tied (100%). Reinforcing steel shall be set on plastic chairs. Bar laps shall be minimum 30 diameters. Sawed transverse dummy joints shall be spaced every 15 feet or 1.25 time longitudinal butt joint spacing whichever is less. Sawing shall occur within 5 to 12 hours after the pour, including sealing. Otherwise, the section shall be removed and longitudinal butt joint constructed.
- No sand shall be allowed under any paving.
- All concrete mix design shall be submitted to the City for review and approval prior to placement.
- Fly ash may be used in concrete pavement locations provided that the maximum cement reduction does not exceed 20% by weight per C.Y. of concrete. The fly ash replacement shall be 1.25 lbs. per 1.0 lb. cement
- All curb and gutter shall be integral (monolithic) with the pavement.
- All fill shall be compacted by sheep's foot roller to a minimum 95% standard proctor. Maximum loose lift for compaction shall be 8 inches. All lifts shall be tested for density by an independent laboratory. All laboratory compaction reports shall be submitted to the City Engineering Construction Inspector once results are received. All reports will be required prior to final acceptance.
- All concrete compression tests and soil compaction/density tests are required to be submitted to the City's Engineering Inspector immediately upon results.
- 10. All proposed sidewalks shall include barrier free ramps at intersecting streets, alleys, etc. Barrier free ramps (truncated dome plate in Colonial or brick red color) shall meet current City and ADA requirements and be approved by the Texas Department of Licensing and Regulation (TDLR).
- 11. All public sidewalks shall be doweled into pavement where it abuts curbs and driveways. Expansion joint material shall be used at these locations.
- 12. All connection of proposed concrete pavement to existing concrete pavement shall include a longitudinal butt joint as the load transfer device. All longitudinal butt joints shall be clean, straight and smooth (not jagged in appearance)
- 13. Cracks formed in concrete pavement shall be repaired or removed by the CONTRACTOR at the City's discretion. CONTRACTOR shall replace existing concrete curbs, sidewalk, paving, a gutters as indicated on the plans and as necessary to connect to the existing infrastructure, including any damage caused by the CONTRACTOR.
- 14. All residential lots will require individual grading plans submitted during the building permit process that correspond with the engineered grading and drainage area plans.
- 15. Approval of this plan is not an authorization to grade adjacent properties when the plans or field conditions warrant off-site grading. Written permission must be obtained and signed from the affected property owner(s) and temporary construction easements may be required. The written permission shall be provided to the City as verification of approval by the adjacent property owner(s). Violation of this requirement will result in suspension of all work at the job site until issue has been rectified.
- 16. All cut or fill slopes of non-paved areas shall be a maximum of 4:1 and minimum of 1%.
- 17. CONTRACTOR agrees to repair any damage to property and the public right-of-way in accordance with the City Standards of Design and Construction.
- 18. CONTRACTOR shall protect all monuments, iron pins/rods, and property corners during construction.
- 19. CONTRACTOR shall ensure positive drainage so that runoff will drain by gravity flow to new or existing drainage inlets or sheet flow per these approved plans.

DRAINAGE / STORM SEWER NOTES

- 1. The CONTRACTOR shall maintain drainage at all times during construction. Ponding of water in streets, drives, trenches, etc. will not be allowed. Existing drainage ways shall not be blocked or removed unless explicitly stated in the plans or written approval is given by the City.
- All structural concrete shall be 4200 psi compressive strength at 28 days minimum 7.0 sack mix, air entrained, unless noted otherwise. Fly ash shall not be allowed in any structural concrete.
- Proposed storm sewer embedment shall be NCTCOG Class 'B' as amended by the City of Rockwall's Engineering Department Standards of Design and Construction Manual
- 4. All public storm pipe shall be a minimum of 18-inch reinforced concrete pipe (RCP), Class III, unless otherwise noted.
- 5. All storm pipe entering structures shall be grouted to assure connection at the structure is watertight.
- 6. All storm structures shall have a smooth uniform poured mortar invert from invert in to invert out.
- 7. All storm sewer manholes in paved areas shall be flush with the paving grade, and shall have traffic bearing ring and covers.
- 8. All storm sewer pipes and laterals shall be inspected by photographic means (television and DVD) prior to final acceptance and after franchise utilities are installed. The CONTRACTOR shall furnish a DVD to the Engineering Construction Inspector for review. Pipes shall be cleaned prior to TV inspection of the pipes. Any sags, open joints, cracked pipes, etc. shall be repaired or removed by the CONTRACTOR at the CONTRACTOR's expense. A television survey will be performed as part of the final testing in the twentieth (20th) month of the maintenance period.

RETAINING WALLS

- 1. All retaining walls, regardless of height, will be reviewed and approved by the City Engineering Department 2. All retaining walls (including foundation stem walls), regardless of height, will be constructed of rock/stone/brick or rock/stone/brick faced. No smooth concrete walls are allowed. Wall materials shall be the same for all walls on the project.
- 3. All portions, including footings, tie-backs, and drainage backfill, of the wall shall be on-site and not encroach into any public easements or right-of-way. The entire wall shall be in one lot and shall not be installed along a lot line.
- 4. All walls 3 feet and taller will be designed and signed/sealed by a registered professional engineer in the State of Texas. The wall design engineer is required to inspect the wall construction and supply a signed/sealed letter of wall construction compliance to the City of Rockwall along with wall as-builts prior to City Engineering acceptance.
- No walls are allowed in detention easements. A variance to allow retaining walls in a detention easement will require approval by the Planning and Zoning Commission with appeals being heard by the City Council.

FINAL ACCEPTANCE AND RECORD DRWINGS/AS-BUILTS

- Final Acceptance shall occur when all the items on the Checklist for Final Acceptance have been completed and signed-off by the City. An example of the checklist for final acceptance has been included in the Appendix of the Standards of Design and Construction. Items on the checklist for final acceptance will vary per project and additional items not shown on the check list may be required.
- After improvements have been constructed, the developer shall be responsible for providing to the City "As Built" or "Record Drawings". The Design Engineer shall furnish all digital files of the project formatted in Auto Cad 14, or 2000 format or newer and Adobe Acrobat (.pdf) format with a CD-ROM disk or flash drive. The disk or drive shall include a full set of plans along with any landscaping, wall plans, and details sheets.
- Submit 1-set of printed drawings of the "Record Drawings" containing copies of all sheets to the Engineering Construction Inspector for the project. The printed sheets will be reviewed by the inspector PRIOR to producing the "Record Drawing" digital files on disk or flash drive. This will allow any revisions to be addressed prior to producing the digital files.
- Record Drawing Disk drawings shall have the Design Engineers seal, signature and must be stamped and dated as "Record Drawings" or "As Built Drawings" on all sheets.
- 5. The City of Rockwall will not accept any Record Drawing disk drawings which include a disclaimer. A disclaimer shall not directly or indirectly state or indicate that the design engineer or the design engineer's surveyor/surveyors did not verify grades after construction, or that the Record Drawings were based solely on information provided by the construction contractor/contractors. Any Record Drawings which include like or similar disclaimer verbiage will not be accepted by the City of Rockwall.
- Example of Acceptable Disclaimer: "To the best of our knowledge ABC Engineering, Inc., hereby states that this plan is As-Built. This information provided is based on surveying at the site and information provided by the contractor."



GENERAL CONSTRUCTION NOTES Sheet 2 of 2

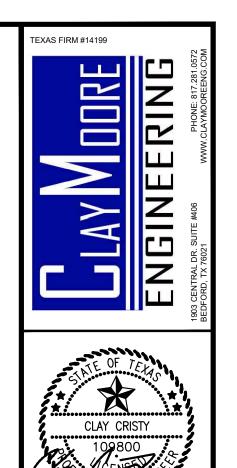
> CITY OF ROCKWALL **ENGINEERING DEPARTMENT**

October 2020

385 S. Goliad Rockwall, Texas 75087

P (972) 771-7746 F (972) 771-7748

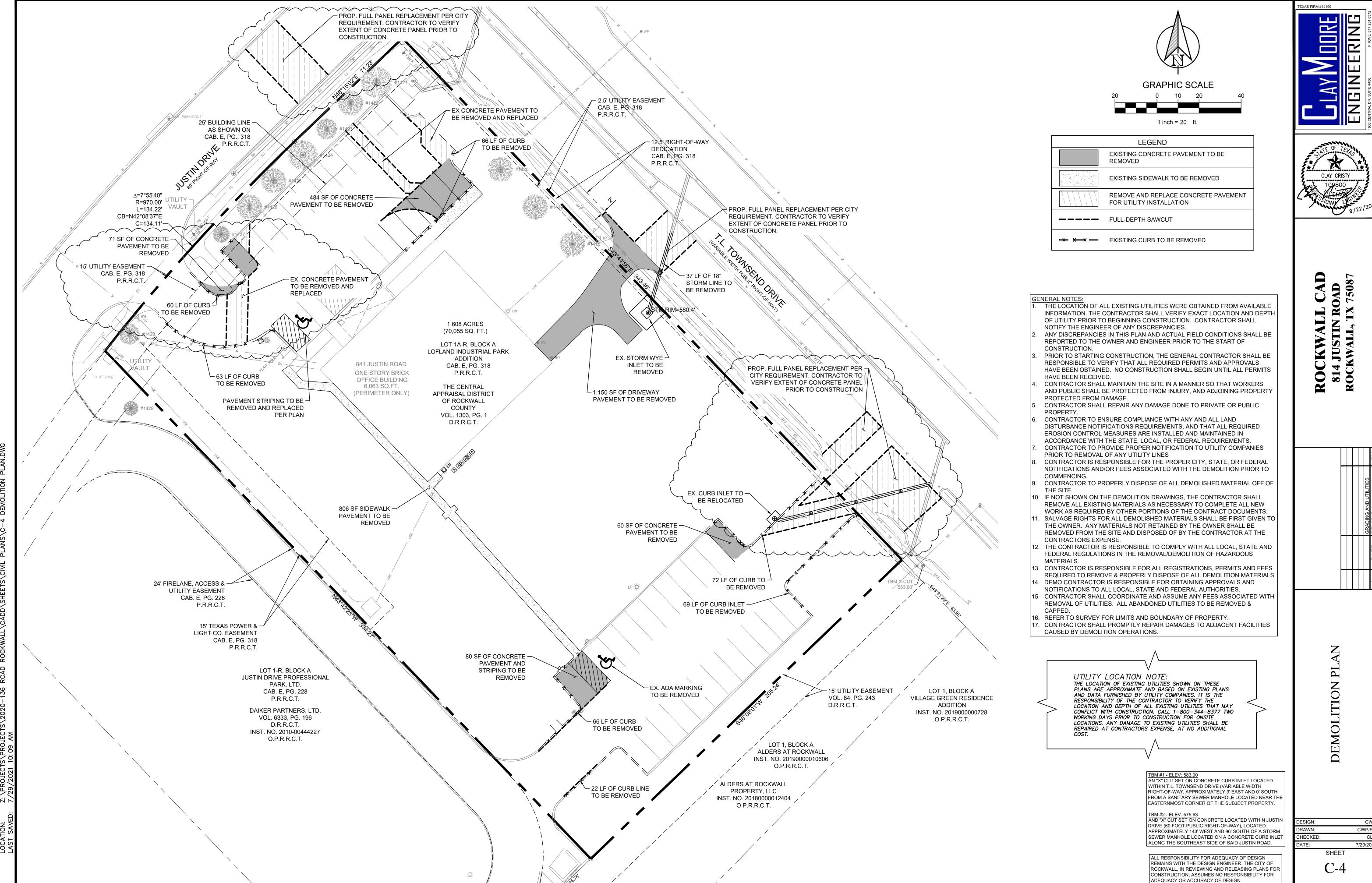
ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OR ACCURACY OF DESIGN.

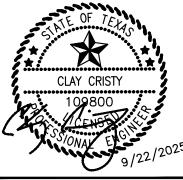


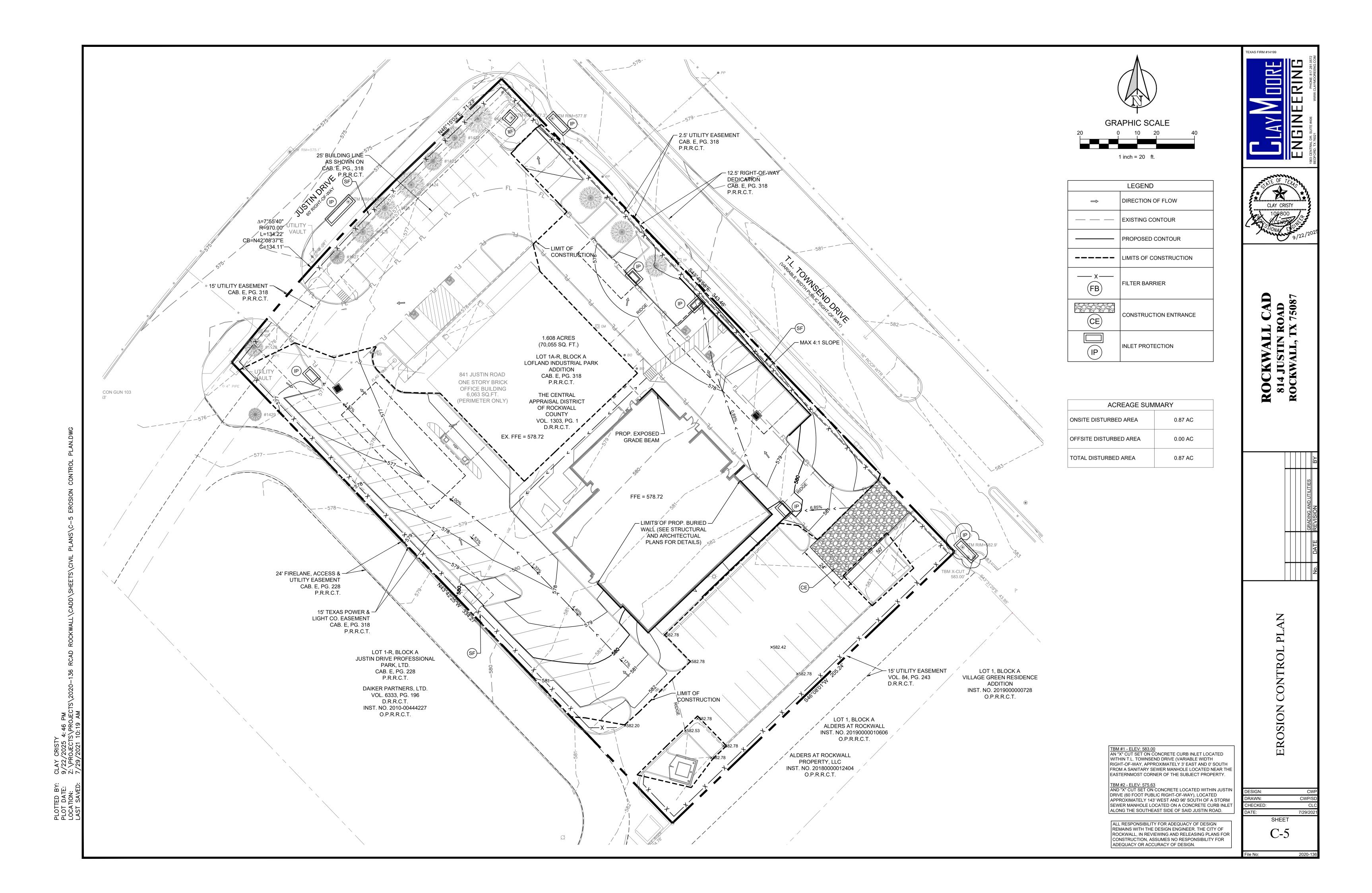
ROCKWALL CAD 814 JUSTIN ROAD ROCKWALL, TX 75087

SHEET

C-3







- EROSION CONTROL DEVICES AS SHOWN ON THE EROSION CONTROL PLAN FOR THE PROJECT SHALL BE INSTALLED PRIOR TO THE START OF LAND DISTURBING ACTIVITIES ON THE
- ALL EROSION CONTROL DEVICES ARE TO BE INSTALLED IN ACCORDANCE WITH THE APPROVED PLANS AND SPECIFICATIONS FOR THE PROJECT. CHANGES ARE TO BE APPROVED BEFORE CONSTRUCTION BY THE DESIGN ENGINEER AND THE CITY OF ROCKWALL.
- IF THE EROSION CONTROL PLAN AS APPROVED CANNOT CONTROL EROSION AND OFF-SITE SEDIMENTATION FROM THE PROJECT THE EROSION CONTROL PLAN WILL BE REQUIRED TO BE REVISED AND/OR ADDITIONAL EROSION CONTROL DEVICES WILL BE REQUIRED ON SITE.
- 4. IF OFF-SITE BORROW OR SPOILS SITES ARE USED IN CONJUNCTION WITH THIS PROJECT, THIS INFORMATION SHALL BE DISCLOSED AND SHOWN ON THE EROSION CONTROL PLAN. OFF-SITE BORROW AND SPOILS AREAS ARE CONSIDERED PART OF EROSION CONTROL REQUIREMENTS. THESE AREAS SHALL BE STABILIZED WITH GROUND COVER PRIOR TO FINAL APPROVAL OF THE PROJECT.
- INSPECTIONS SHALL BE MADE WEEKLY AND AFTER RAIN STORM EVENTS TO INSURE THAT THE DEVICES ARE FUNCTIONING PROPERLY. WHEN SEDIMENT OR MUD HAS CLOGGED THE VOID SPACES BETWEEN STONES OR MUD IS BEING TRACKED ONTO A PUBLIC ROADWAY THE AGGREGATE PAD MUST BE WASHED DOWN OR REPLACED. RUNOFF FROM THE WASH DOWN OPERATION HALL SHALL NOT BE ALLOWED TO DRAIN DIRECTLY OFF SITE WITHOUT FIRST FLOWING THROUGH ANOTHER BMP TO CONTROL OFF SITE SEDIMENTATION. PERIODIC RE-GRADING OR THE ADDITION OF NEW STONE MAY BE REQUIRED TO MAINTAIN THE EFFICIENCY OF THE INSTALLATION.
- CONTRACTOR SHALL HAVE A COPY OF THE SWPPP ON SITE AT ALL TIMES.
- CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTAL OF N.O.I., N.O.T. AND ANY ADDITIONAL INFORMATION REQUIRED BY THE E.P.A. CONTRACTOR SHALL COMPLY WITH ALL T.C.E.Q. STORM WATER POLLUTION PREVENTION REQUIREMENTS.

EROSION CONTROL SCHEDULE AND PHASING

THE PROJECT SHALL GENERALLY CONFORM TO THE FOLLOWING:

PHASE 1 - DEMOLITION/GRADING

- CONSTRUCT TEMPORARY CONSTRUCTION ENTRANCE, SILT FENCE, AND TREE PROTECTION FENCE ACCORDING TO THE APPROXIMATE LOCATION SHOWN ON GRADING AND EROSION CONTROL PLAN, NOTES, AND DETAIL SHEETS.
- BEGIN CLEARING AND GRADING OF SITE. SEED AND REVEGETATE SLOPES WHERE SHOWN.
- PHASE 2 UTILITIES A. KEEP ALL STORM WATER POLLUTION PREVENTION MEASURES IN
- INSTALL STORM DRAINS AS SPECIFIED ON PLAN SHEETS. INSTALL INLET PROTECTION.

PHASE 3 - PAVING

- A. KEEP ALL STORM WATER POLLUTION PREVENTION MEASURES IN PLACE. REMOVE AS NEEDED TO PAVE. STABILIZE SUBGRADE.
- C. PAVE PARKING LOT AND SIDEWALKS AS SPECIFIED ON PLAN
- REMOVE TEMPORARY CONSTRUCTION ENTRANCE. MAINTAIN INLET PROTECTION.

PHASE 4 - LANDSCAPING AND SOIL STABILIZATION A. REVEGETATE LOT AND PARKWAYS

- B. LANDSCAPE CONTRACTOR SHALL REVEGETATE ALL AREAS
- RESERVED FOR LANDSCAPE VEGETATIVE COVERS. REMOVE EROSION CONTROL DEVICES WHEN GROUND COVER
- ESTABLISHED.

B.M.P. MAINTENANCE SCHEDULE

TEMPORARY STONE CONSTRUCTION ENTRANCE/EXIT:

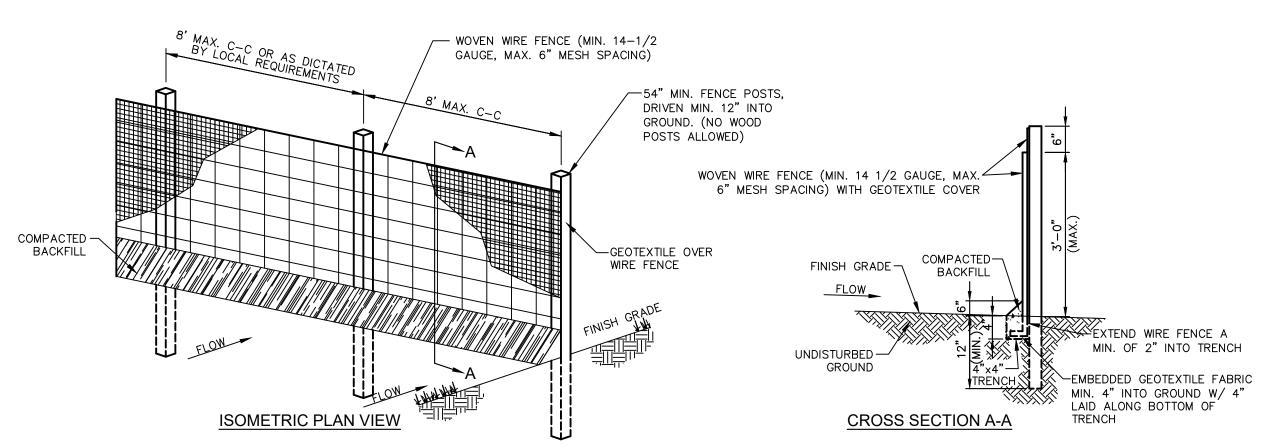
INSPECTIONS SHALL BE MADE WEEKLY AND AFTER RAIN STORM EVENTS TO ENSURE THAT THE FACILITY IS FUNCTIONING PROPERLY AGGREGATE PAD SHALL BE WASHED DOWN OR REPLACED WHEN SEDIMENT OR MUD HAS CLOGGED THE VOID SPACES BETWEEN THE STONES OR MUD IS BEING TRACKED ONTO THE PUBLIC ROADWAY. RUNOFF FROM WASH DOWN OPERATION SHALL BE FILTERED THROUGH ANOTHER B.M.P. PRIOR TO DRAINING OFF-SITE.

SILT FENCE:

INSPECTIONS SHALL BE MADE WEEKLY AND AFTER RAIN STORM EVENTS. SEDIMENT SHALL BE REMOVED FROM BEHIND THE FENCE WHEN THE DEPTH OF SEDIMENT HAS BUILT UP TO ONE-THIRD THE HEIGHT OF THE FENCE ABOVE GRADE. FENCE SHALL BE INSPECTED FOR GAPS AT BASE. INSPECT SUPPORTING POSTS AND FILTER FABRIC. REPLACE IF REQUIRED.

INLET PROTECTION:

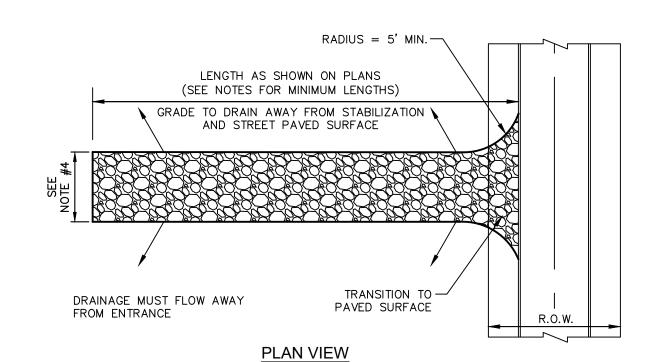
NSPECTIONS SHALL BE MADE WEEKLY AND AFTER RAIN STORM EVENTS TO ENSURE THAT THE DEVICE IS FUNCTIONING PROPERLY SEDIMENT SHALL BE REMOVED FROM THE STORAGE AREA WHEN SEDIMENT DEPTH HAS BUILT UP TO ONE-HALF THE DESIGN DEPTH. DE-WATERING OF THE STORAGE VOLUME IS NOT OCCURRING. CLEAN OR REPLACE THE FILTER STONE SURROUNDING THE INLET. CLEAN THE STONE SURFACE THE FIRST FEW TIMES BY RAKING. REPEATED SEDIMENT BUILD-UP WILL REQUIRE FILTER STONE REPLACEMENT.



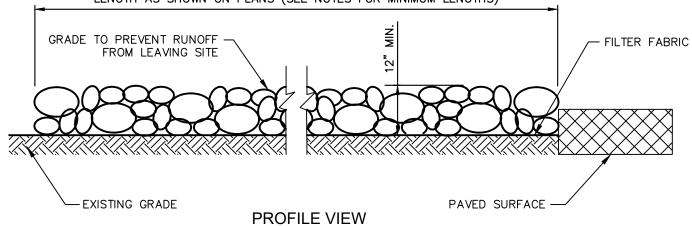
1. WOVEN WIRE FENCE TO BE FASTENED SECURELY TO FENCE POSTS WITH WIRE

- 2. GEOTEXTILE TO BE FASTENED SECURELY TO WOVEN WIRE FENCE WITH TIES SPACED EVERY 24" AT TOP AND MID SECTION.
- 3. WHEN TWO SECTIONS OF GEOTEXTILE ADJOIN EACH OTHER THEY SHALL BE OVERLAPPED BY SIX INCHES AND FOLDED.
- 4. MAINTENANCE SHALL BE PERFORMED AS NOTED IN THE EROSION CONTROL PLAN. 5. COLLECTED MATERIAL SHALL BE REMOVED WHEN "BULGES" DEVELOP IN THE SILT
- 6. ALL SILT FENCE SHALL INCLUDE WIRE SUPPORT UNLESS INDICATED OTHERWISE

SILT FENCE DETAIL



LENGTH AS SHOWN ON PLANS (SEE NOTES FOR MINIMUM LENGTHS)



- 1. STONE SHALL BE 4 TO 6 INCH DIAMETER CRUSHED ROCK, NO CRUSHED PORTLAND CEMENT CONCRETE ALLOWED.
- 2. LENGTH SHALL BE SHOWN ON PLANS, WITH A MINIMUM LENGTH OF 30 FEET FOR LOTS WHICH ARE LESS THAN 150 FEET FROM EDGE OF PAVEMENT. THE MINIMUM DEPTH IN ALL OTHER CASES SHALL BE 50 FEET.

POSTS: STEEL EITHER T OR U TYPE.

6" MAX. MESH OPENING

FENCE: WOVEN WIRE, 14-1/2 GA.

2. BELTECH 810

4. LING GTF 190

MIRAFI 130X

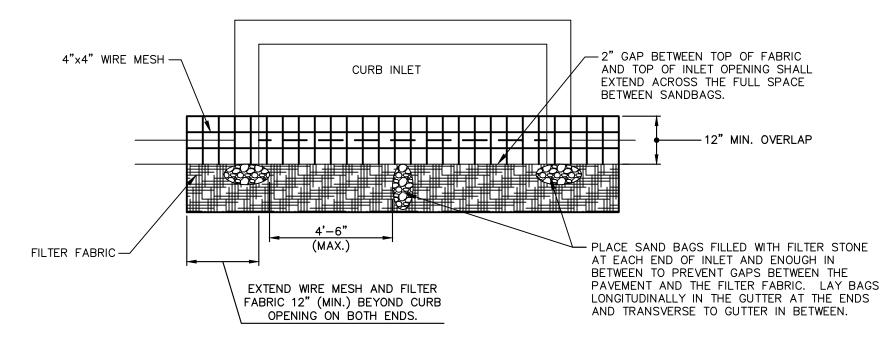
5. SI 915 SC

FABRIC:1. AMOCO 1198

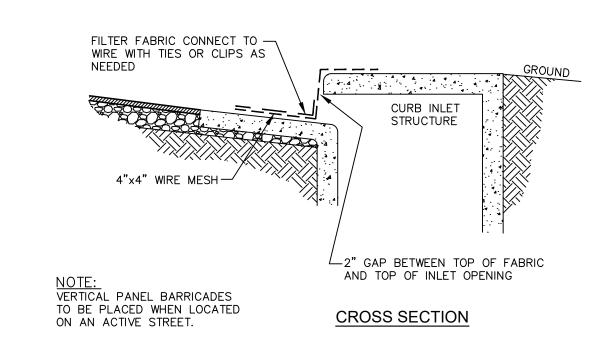
- 3. STONE LAYER THICKNESS SHALL NOT BE LESS THAN 12"
- 4. THE WIDTH SHALL BE NO LESS THAN THE FULL WIDTH OF ALL POINTS OF INGRESS OR EGRESS. INCHES.
- 5. WHEN NECESSARY, VEHICLES SHALL BE CLEANED TO REMOVE SEDIMENT PRIOR TO ENTRANCE ONTO A PUBLIC ROADWAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE WITH DRAINAGE FLOWING AWAY FROM BOTH THE STREET AND THE STABILIZED ENTRANCE. ALL SEDIMENT SHALL BE PREVENTED FROM ENTERING ANY STORM DRAIN, DITCH OR WATERCOURSE USING APPROVED METHODS.
- 6. THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PAVED SURFACES. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND. ALL SEDIMENT SPILLED, DROPPED, WASHED, OR TRACKED ONTO PAVED SURFACES MUST BE REMOVED IMMEDIATELY.
- 7. THE ENTRANCE MUST BE PROPERLY GRADED OR INCORPORATE A DRAINAGE

SWALE TO PREVENT RUNOFF FROM LEAVING THE CONSTRUCTION SITE.

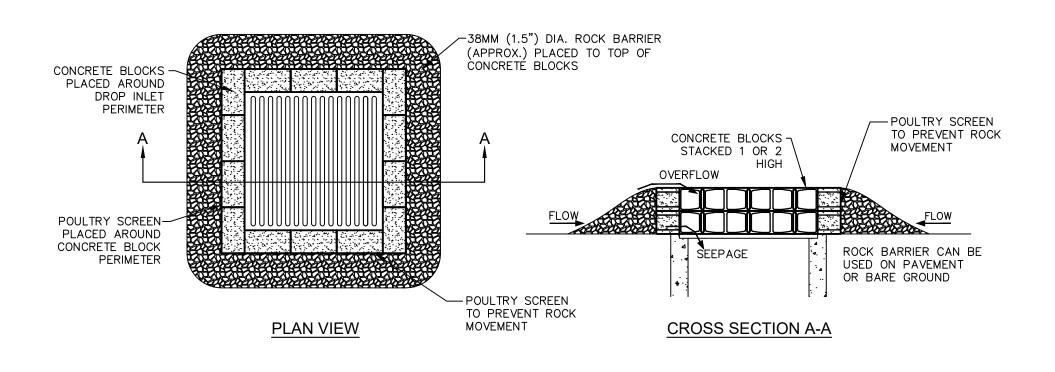
STABILIZED CONSTRUCTION ENTRANCE DETAIL (CE)



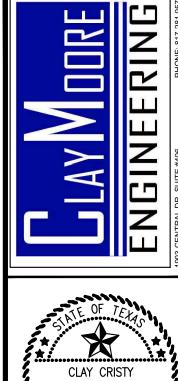
PLAN VIEW

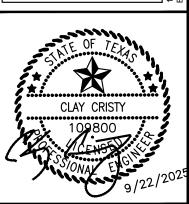


CURB INLET PROTECTION DETAIL N.T.S.



GRATE INLET PROTECTION DETAIL

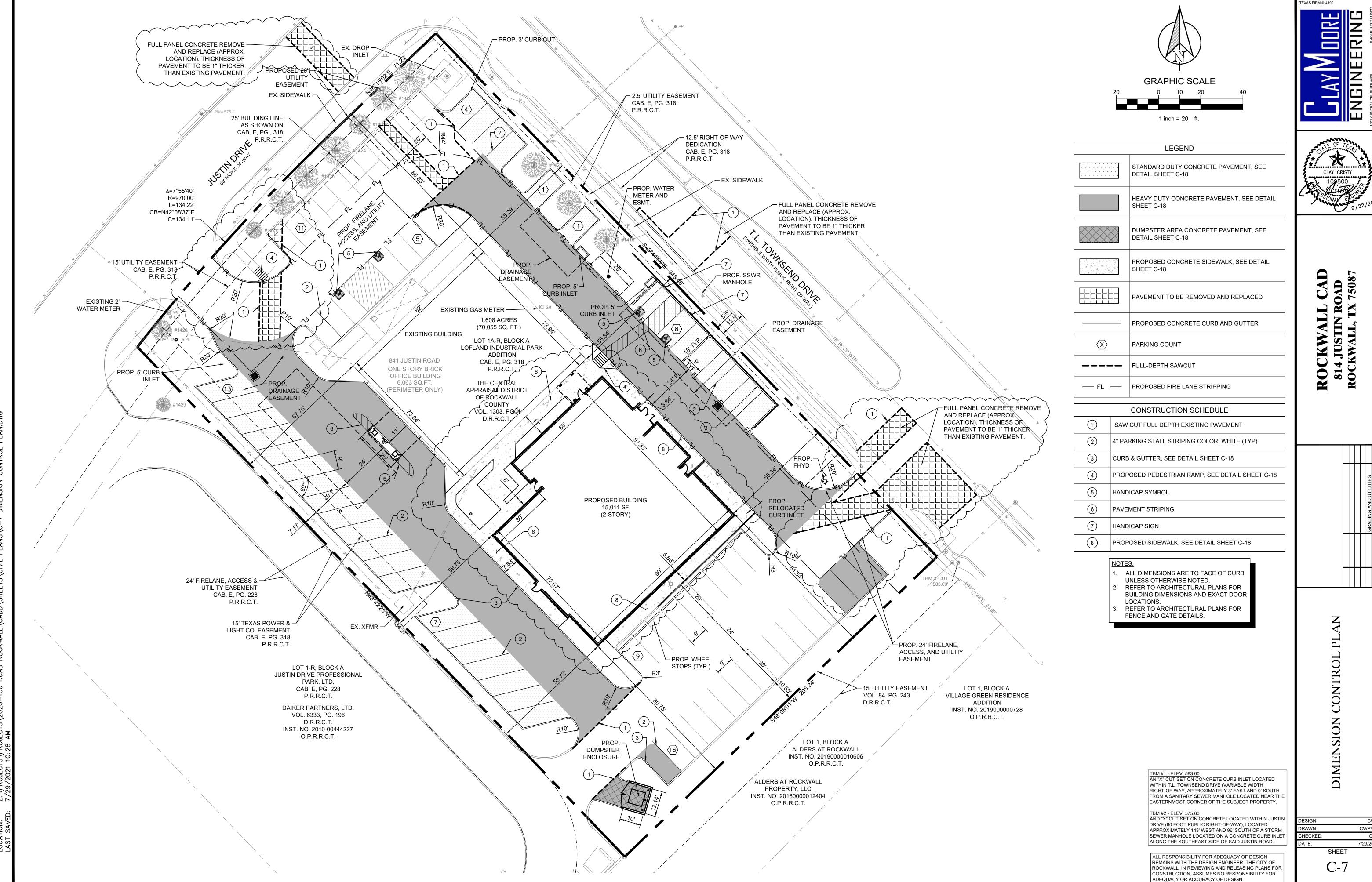


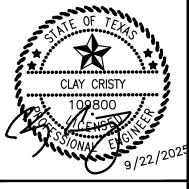


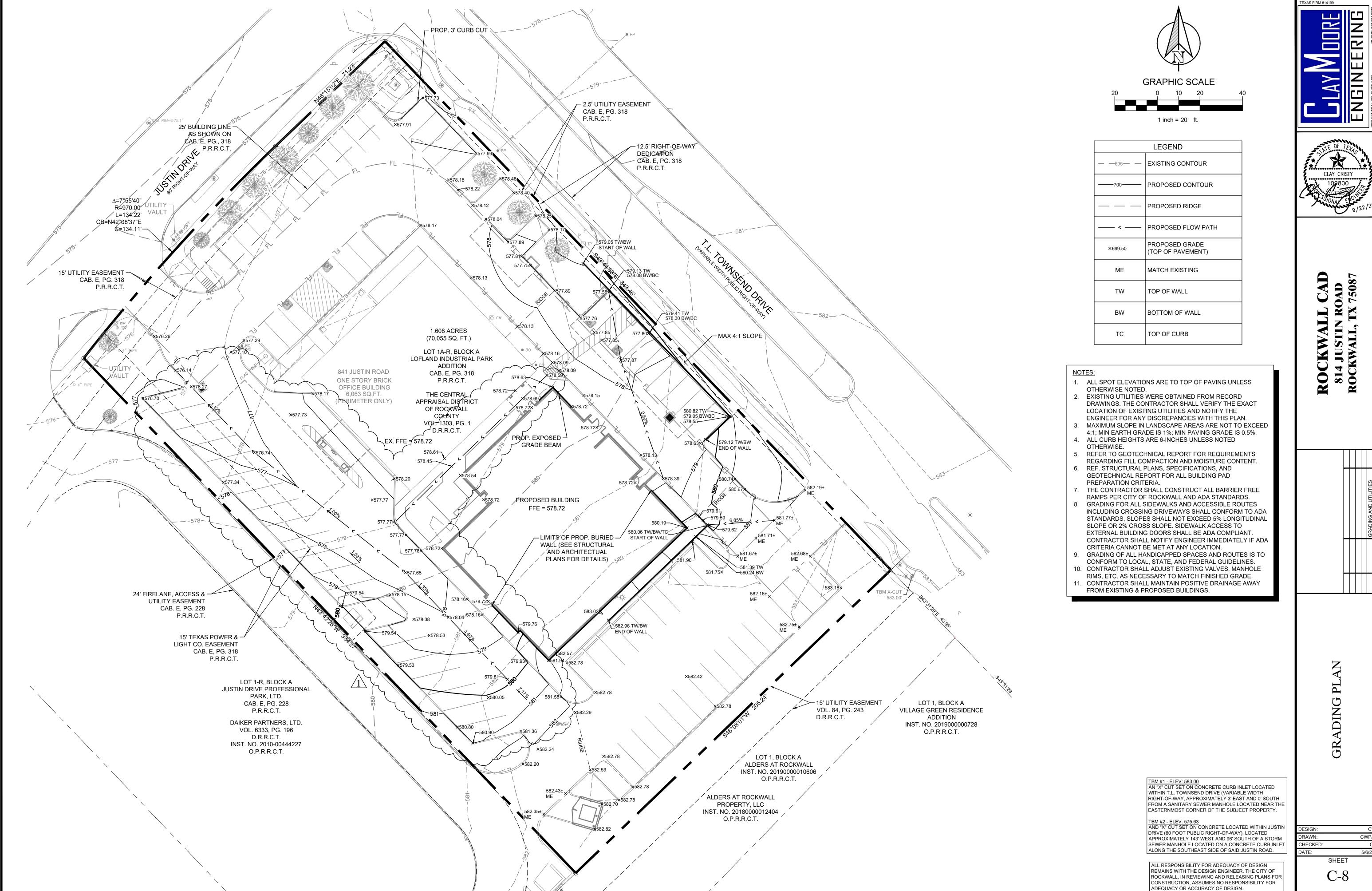
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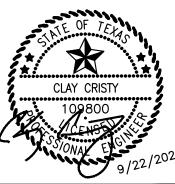
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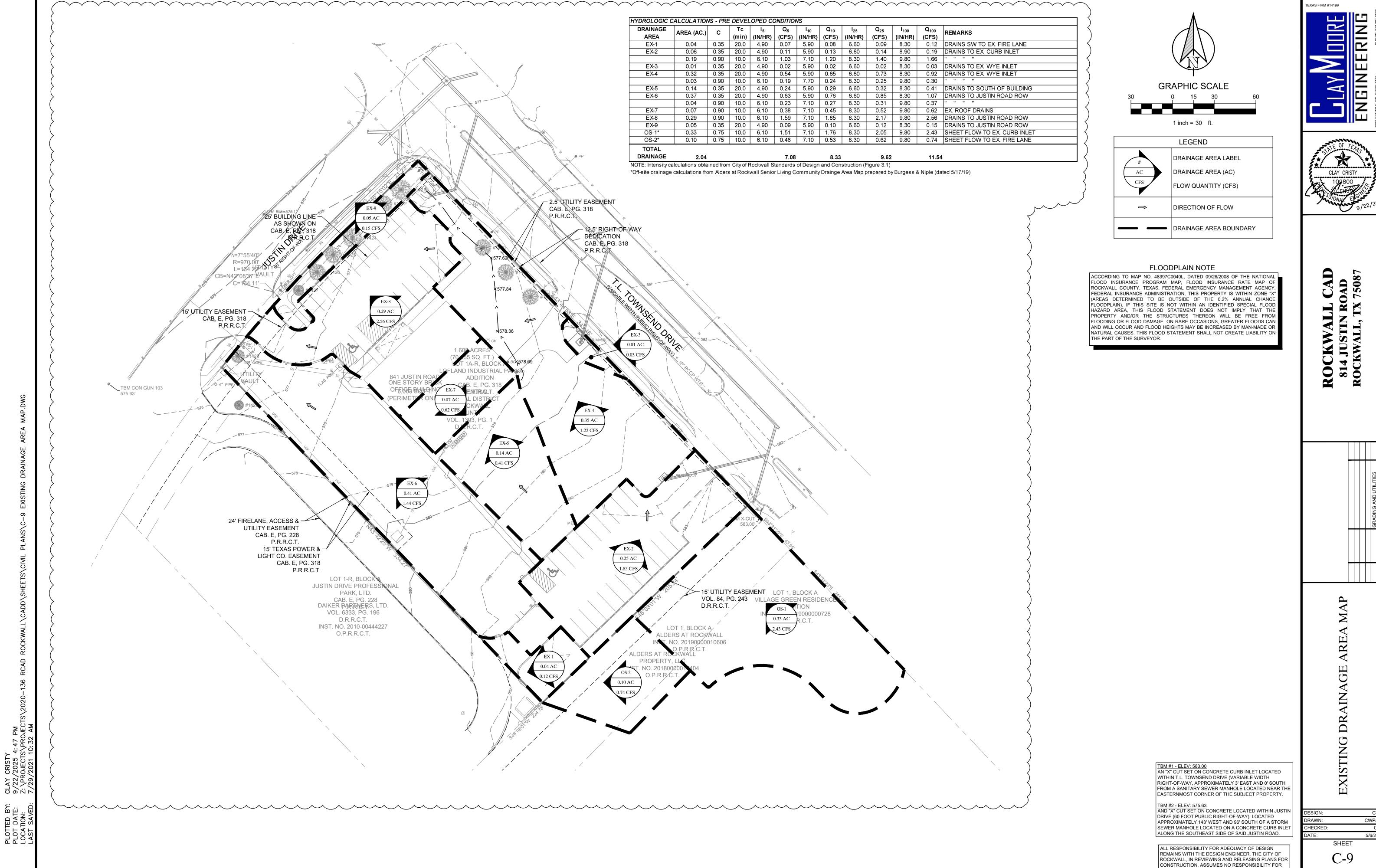
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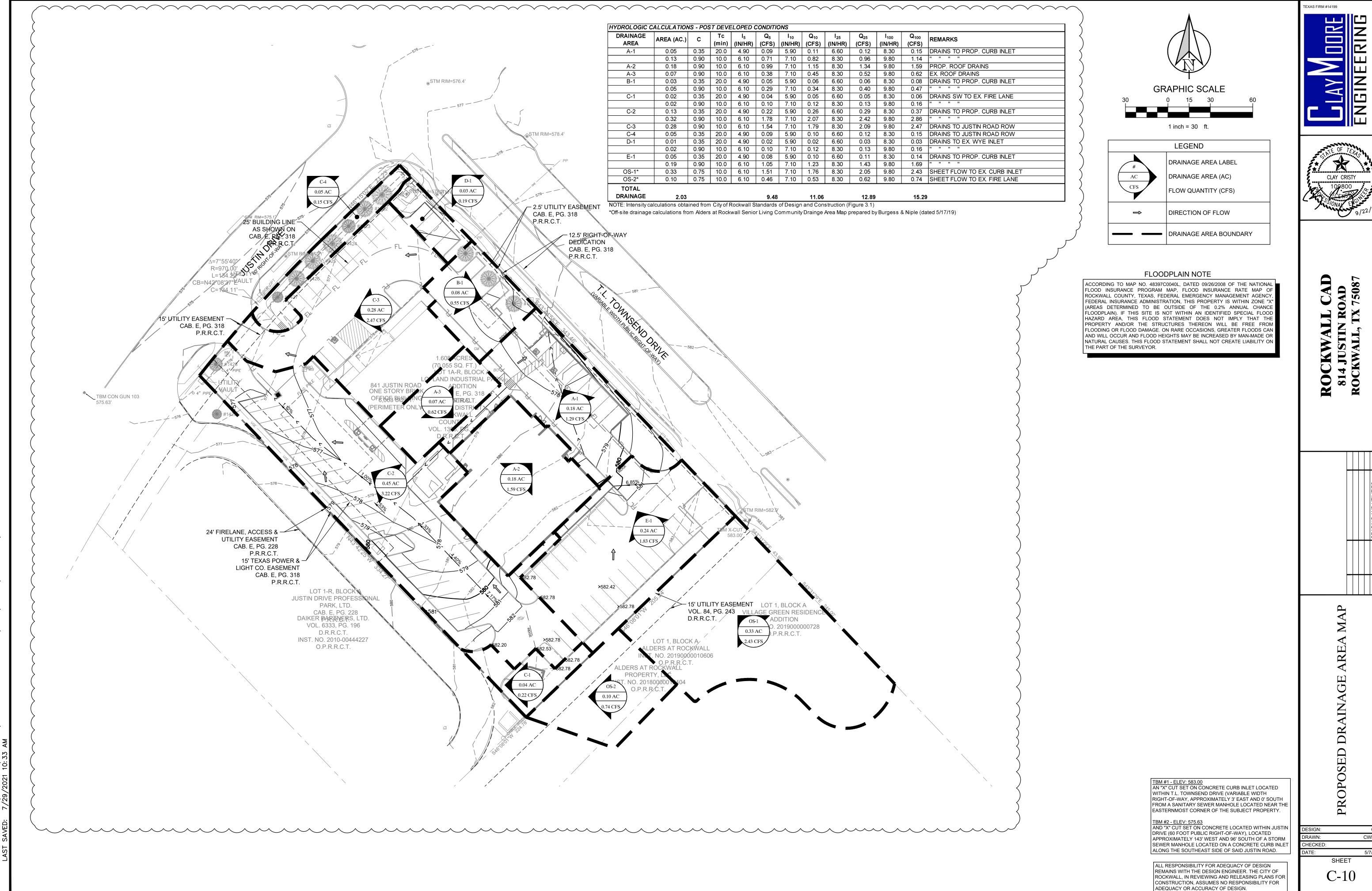








ADEQUACY OR ACCURACY OF DESIGN.



47 PM

: CLAY CRISTY 9/22/2025 4:47 PM Z:\PROJECTS\PROJECTS\2020—136 RCAD

PLOTTED BY: CLAY CR PLOT DATE: 9/22/20 LOCATION: Z:\PROJ LAST SAVED: 7/29/20

ENGINEERING ENGINEERING 10' SS TO BE ENCASED IN INSTALL RESTRICTOR PLATE PER DETAILS THIS SHEET. CONTRACTOR SHALL CONFIRM EXACT FLOW LINE ELEVATION PRIOR TO CONSTRUCTION. CONCRETE **GRAPHIC SCALE** 8 LF OF 18" RCP @ 0.30% 2.5' UTILITY EASEMENT CAB. E, PG. 318 P.R.R.C.T. INSTALL: 1 inch = 20 ft. 1 - 30° BEND 25' BUILDING LINE -FL 18": 573.33 AS SHOWN ON CAB. E, PG., 318 _, r.G., 3 , P.R.R.C.T. 12.5 RIGHT-OF-WAY — 30 LF OF 18" RCP @ 0.30% DEDICATION STA: 0+00 STORM LINE A
PROP. 12" STORM LINE CAB. E, PG. 318 P.R.R.C.T LEGEND CONNECT TO EX. CURB INLET CLAY CRISTY FL 12": 571.10

12 LF OF 12" RCP @ 0.50% 1 - 45° BEND PROPOSED STORM DRAIN FL 18": 573.42 — 4 LF OF 18" RCP @ 6.56% Δ=7°55'40"
R=970.00
10' SS TO BE
ENCASED IN STA: 0+12.25 STORM LINE A INSTALL: EXISTING STORM DRAIN ENCASED IN INSTALL: 1 - 5' CURB INLET L=134.22/ 1 - 12" 45° BEND CONCRETE RIM: 578.25 CB=N42°08'37"E FL 18": 573.75 PROPOSED CURB INLET C=134.11(STA: 0+21.16 STORM LINE A 9 LF OF 12" RCP @ 0.50% PROPOSED UNDERGROUND 1 - 5' CURB INLET CONTRACTOR TO CONNECT 1 - 18" 45° BEND DETENTION 1- 18"X12" REDUCER CURB INLET TO PROPOSED DETENTION SYSTEM FL 18": 571.21 FL 12": 571.21 PROPOSED CONCRETE PAVEMENT RIM: 578.18 REMOVE AND REPLACE **№ 15' UTILITY EASEMENT** CAB. E, PG. 318 P.R.R.C.T INSTALL 1- 45° WYE FL 18": 573.59 STA: 0+82.14 STORM LINE A FL 18": 573.59 INSTALL: FL 18": 573.52 1 - 45° BEND FL 18": 571.51 38 LF OF 18" RCP @ 0.30% — 1.608 ACRES (70,055 SQ. FT.) -- PROP. UNDERGROUND STORAGE
REFER TO SHEET C-14 FOR
STORAGE CALCULATIONS CONNECT TO PROP. UNDERGROUND DETENTION STRUCTURE STA: 0+85.64 STORM LINE A CONNECT TO PROP. DETENTION STRUCTURE 18 LF OF 18" RCP @ 2.75% — FL 18": 573.70 CONTRACTOR TO INSTALL RESTRICTOR PLATE CAB. E, PG. 318 ON OUTFLOW PIPE PER DETAIL. P.R.R.C.T. FL 36": 571.53 FL 18": 572.00 6,063 SQ.FT. THE CENTRAL STA: 0+94.04 STORM LINE A (PERIMETER ONLY) APPRAISAL DISTRICT 1 - 3'x3' GRATE INLET OF ROCKWALL RIM: 576.10 - PROP. UNDERGROUND STORAGE COUNTY REFER TO SHEET C-14 FOR STORAGE CALCULATIONS VOL. 1303, PG. 1 16 LF OF 18" RCP @ 0.50% D.R.R.C.T. END STORM LINE B STA: 0+88.01 STORM LINE A CONNECT TO PROP. UNDERGROUND PROP. ROOF DRAIN PICKUP INSTALL: 1 - 3'X3' GRATE INLET REFER TO MEP PLANS FOR DETAILS FL 8": 575.72 DETENTION STRUCTURE RIM: 577.70 FL 18": 572.00 FL 36": 571.55 CONNECT 18" STORM LINE TO GRATE INLET CONNECT TO ROOF DRAINS -PROP. ROOF DRAIN PICKUP REFER TO MEP PLANS FOR DETAILS FL 8": 575.72 24' FIRELANE, ACCESS & -UTILITY EASEMENT CAB. E, PG. 228 P.R.R.C.T. 15' TEXAS POWER & -LIGHT CO. EASEMENT CAB. E, PG. 318 P.R.R.C.T. LOT 1-R, BLOCK A JUSTIN DRIVE PROFESSIONAL PARK, LTD. - 15' UTILITY EASEMENT VOL. 84, PG. 243 LOT 1, BLOCK A CAB. E, PG. 228 VILLAGE GREEN RESIDENCE P.R.R.C.T. D.R.R.C.T. ADDITION DAIKER PARTNERS, LTD. INST. NO. 2019000000728 VOL. 6333, PG. 196 O.P.R.R.C.T. D.R.R.C.T. INST. NO. 2010-00444227 O.P.R.R.C.T. LOT 1, BLOCK A ALDERS AT ROCKWALL INST. NO. 20190000010606 O.P.R.R.C.T. TBM #1 - ELEV: 583.00
AN "X" CUT SET ON CONCRETE CURB INLET LOCATED ALDERS AT ROCKWALL WITHIN T.L. TOWNSEND DRIVE (VARIABLE WIDTH RIGHT-OF-WAY, APPROXIMATELY 3' EAST AND 0' SOUTH FROM A SANITARY SEWER MANHOLE LOCATED NEAR THE PROPERTY, LLC INST. NO. 20180000012404 EASTERNMOST CORNER OF THE SUBJECT PROPERTY. O.P.R.R.C.T. TBM #2 - ELEV: 575.63
AND "X" CUT SET ON CONCRETE LOCATED WITHIN JUSTIN DRIVE (60 FOOT PUBLIC RIGHT-OF-WAY), LOCATED APPROXIMATELY 143' WEST AND 96' SOUTH OF A STORM SEWER MANHOLE LOCATED ON A CONCRETE CURB INLET ALONG THE SOUTHEAST SIDE OF SAID JUSTIN ROAD. SHEET ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OR ACCURACY OF DESIGN.

LLAY MOORE ENGINEERING STORM LINE A 582 582 **GRAPHIC SCALE** 580 580 HORIZONTAL 1 inch = 20 ft. VERTICAL 1 inch = 2 ft. 578 578 CLAY CRISTY 576 576 16 LF OF 18" RCP @ 0.50%-PROP. UNDERGROUND DETENTION SYSTEM -574 574 18 LF OF 18" RCP @ 2.75%— 9 LF OF 12" RCP @ 0.50%— 12 LF OF 12" RCP @ 0.50% -61 LF OF 18" RCP @ 0.50% 572 572 FULL FLOW Q₁₀₀ = 3.79 CFS Q_{CAP} = 18.8 CFS FULL FLOW Q₁₀₀ = 3.79 CFS Q_{CAP} = 7.43 CFS 570 570 FULL FLOW —Q₁₀₀ = 3.79 CFS V = 2.25 FPS V = 2.25 FPS $S_F = 0.14\%$ $Q_{CAP} = 7.43 CFS$ $S_F = 0.14\%$ V = 2.25 FPS S_F = 0.14% ## SULL FLOW ## = 3.79 GFS ## = 47.25 FS FULL FLOW — 568 568 Q₁₀₀ = 3.79 CFS Q_{CAP} = 47.2 CFS V = 0.56 FPS $S_F = 0.00\%$ FULL FLOW Q₁₀₀ = 3.79 CFS Q_{CAP} = 7.43 CFS UV = 2.25 FPS Q_{SF} = 0.14% 566 566 564 564 STORM TBM #1 - ELEV: 583.00
AN "X" CUT SET ON CONCRETE CURB INLET LOCATED WITHIN T.L. TOWNSEND DRIVE (VARIABLE WIDTH RIGHT-OF-WAY, APPROXIMATELY 3' EAST AND 0' SOUTH FROM A SANITARY SEWER MANHOLE LOCATED NEAR THE EASTERNMOST CORNER OF THE SUBJECT PROPERTY. TBM #2 - ELEV: 575.63
AND "X" CUT SET ON CONCRETE LOCATED WITHIN JUSTIN DRIVE (60 FOOT PUBLIC RIGHT-OF-WAY), LOCATED APPROXIMATELY 143' WEST AND 96' SOUTH OF A STORM SEWER MANHOLE LOCATED ON A CONCRETE CURB INLET ALONG THE SOUTHEAST SIDE OF SAID JUSTIN ROAD. SHEET ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OR ACCURACY OF DESIGN.

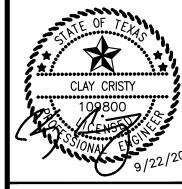
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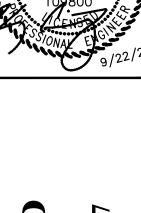
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													ľ	100 YR STORM	DRAIN CALCUL	ATIONS													HGL		/ERT
LINE. S	TA.		INCREMENTAL	CUMULATIVE	RUNOFF	INCREMENTAL	CUMULATIVE	INLET TIME			INTENSITY	DIST	TOTAL				ROUGH		PIPE						FRICTION			INCOMIN	G OUTGOING		
			AREA	AREA	COEFFICIENT	CA	CA		IN PIPE	CONCENTRATION			FLOW				NESS	SLOPE	CAPACITY				Flow		SLOPE		K_j H_j	PIPE	PIPE	PIPE	PIPE
											1 ₁₀₀		Q ₁₀₀	DIA. SPAN	RISE NUMBE	R A F	R n	So	Q _{cap}	V _{design} V	_{full} Q/Qfu	II V/Vfull d/	Depth	Vpartial	Sf	Hv	(MIN 0.1	.)			
			ACRES	ACRES						MIN	IN/HR	FT	CFS	IN FT	FT	Sq FT		%	CFS	Qdesign/A F	PS .		FT	FPS	FT/FT V	design^2/2g	FT.	FT	FT	FT	FT
INE A 0+0		ONNECT TO EX. STORM								13.24																		572.10	+	571.10	-
INC A O.	00.00	SITIAL OF THE LOCAL OF THE CANAL		0.45			0.41		0.22	10.24		12.25	0.74	12	1	0.79 0.2	250 0.013	0.50%	2.52	0.94 3	21 0 294	0.87 0.3	7 1.00	2.78	0.04%	0.01		072.10	+	+ 07 1.10	+
0+	12.25	45° BEND	0.00	0.40	0.90	0.00	0.41	10.00	0.22	13.02	9.80	12.20	0.14	12	'	0.70 0.2	200 0.010	0.0070	2.02	0.04	.21 0.204	0.07 0.0	1.00	2.70	0.0470		0.35 0.10	572.21	572.11	571.16	571.
	12.23	10 BEITE	0.00	0.45	0.00	0.00	0.41	10.00	0.16	10.02	0.00	8.91	0.74	12	1	0.79 0.2	250 0.013	0.50%	2.52	0.94 3	21 0 294	0.87 0.3	7 1.00	2.78	0.04%	0.01	5.00		- 072.11	+ 071.10	+ ***
0+:	21.16	45° BEND	0.00	5. 10	0.90	0.00		10.00	5.15	12.86	9.80	1	0		'	1 0.70	2.00	1 3.3378		1 0.01	00 .	0.07	1.00	2.70	0.0170		0.35 0.10	572.31	572.21	570.71	571.
				0.45	0.00	1	0.41	1	2.72	12.55		68.23	0.74	18	1.5	1.77 0.3	375 0.013	0.50%	7.43	0.42 4	.20 0.100	0.63 0.2	1 1.50	2.66	0.00%	0.00			+	1	+
0+8	32.14	45° BEND	0.00	57.75	0.90	0.00		10.00		10.39	9.80	1 33.23			1	11111	0.0.0					0,00			0.0070		0.35 0.10	573.28	572.31	571.78	571.
				0.45			0.41		0.12			3.00	0.74	18	1.5	1.77 0.3	375 0.013	0.50%	7.43	0.42 4	.20 0.100	0.63 0.2	1 1.50	2.66	0.00%	0.00					+
0+8	35.14 DE	ETENTION STRUCTURE	0.00		0.90	0.00		10.00		10.27	9.80																0.35 0.10	574.78	573.28	571.78	571.
				0.45			0.41		0.13			4.25	3.97	36	3	7.07 0.7	750 0.013	0.50%	47.16	0.56 6	.67 0.084	0.60 0.1	9 3.00	3.98	0.00%	0.00			+	<u> </u>	_
0+8	39.39 DE	ETENTION STRUCTURE	0.00		0.90	0.00		10.00		10.14	9.80																0.35 0.10	574.88	574.78	571.78	571.0
				0.45			0.41		0.14			18.39	3.97	18	1.5	1.77 0.3	375 0.013	3.21%	18.82	2.25 10	0.65 0.211	0.79 0.3	1 1.50	8.42	0.14%	0.08					
1+0	07.78	5' CURB INLET	0.45		0.90	0.41		10.00		10.00	9.80																1.25 0.10	575.01	574.91	573.87	572.3
1+0	07.78		0.45	0.45			0.41				9.80			18	1.5	1.77 0.3	375 0.013	3.21%	18.82	2.25 10	0.65 0.211	0.79 0.3	1 1.50	8.42	0.14%		1.25 0.10	575.01		573	3.87
<u> </u>	00.00	45° WYE			T	T	<u> </u>	1		14.61	<u> </u>	<u> </u>	<u> </u>			1 1			<u> </u>						<u> </u>			574.69	_	573.19	$\overline{}$
1EB 0+0	00.00					1		I	ĺ	1	1	40.40			1	1		1		1	1		1	1			1	1 37 1.00	ı	1 0.0.10	1

LINE C	0+00.00 E	K. STORM CONNECTION								10.38																				578.19	,	576.69	
				0.24			0.22		0.18			35.96	2.12	18	1.5	1.77	0.375	0.013	0.41%	6.73	1.20 3	81 0.31	5 0.88	0.38	0.57 3.35	0.04%	0.02			,			
	0+35.96	30° BEND	0.00		0.90	0.00		10.00		10.20	9.80																	0.50	0.10	578.30	578.20	576.80	576.84
				0.24			0.22		0.20			14.36	2.12	18	1.5	1.77	0.375	0.013	0.41%	6.73	1.20 3	81 0.31	5 0.88	0.38	1.50 3.35	0.04%	0.02			,	,		
	0+50.32	5' CURB INLET	0.24		0.90	0.22		10.00		10.00	9.80																	1.25	0.10	578.41	578.31	576.82	576.86

AS FIRM #14199





ROCKWALL CAD 814 JUSTIN ROAD ROCKWALL, TX 75087

STORM DRAIN HGL CALCULATIONS

SHEET C-13

2,032 cubic-fee Required Total Storage Volume 0.047 acre-feet 2,675 cubic-fee Provided Total Storage Volume 0.061 acre-feet

mpervious area delta (Overall Area "A") Time (Tc) 20 minutes C value 0.35 I-100yr 8.30 in/hr **∖** Q100yr (T 0.96 cfs Onsite Existing Flow Area Passing through Pond (Area E-1) Q100yr (T 0.00 cfs > Q100yr (T) Areas that are Bypassing Pond 0.00 cfs Q100yr (T Allowable Release Rate 0.96 cfs Q100yr (T) 0.96 cfs Proposed Release Rate

Onsite Proposed Conditions

100

110

110

BY:

120 0.96

3.40

3.20

0.90

0.90

2.44 cfs

Onsite Existing Conditions

0.28 acres mpervious area delta (Overall area "A") Time (Tc) 10 minutes C value 0.90 Developed Condition C-Value I-100yr 9.80 in/hr > Q100yr

Inflow per Storm Event Runoff per Storm Event - Developed Event Runoff Inflow (ft^3) (cfs) l-100yr C value 2.44 1,466 9.80 0.90 0.28 2.44 2,019 2.24 9.00 0.90 0.28 2.24 2.07 2.07 2,483 8.30 0.90 0.28 20 3,096 1.72 6.90 0.90 0.28 1.72 30 3,324 1.58 1.58 6.35 0.90 0.28 5.80 0.90 0.28 1.45 1.45 3,470 1.25 3,740 5.00 0.90 0.28 1.25 1.12 60 1.12 4,039 4.50 0.28 0.90 1.00 1.00 70 4,188 4.00 0.90 0.28 80 3.70 0.90 0.28 0.92 80 0.92 4,428 90 0.87 4,712 3.50 0.90 0.28 0.87

0.28

0.28

Developed Runoff

Max Allo	wable Out	flow per Sto	rm Event	Detention '	Volume Req	uired		
>			Outflow				Storage	Storage
Storm	Time	Release	(ft^3)	Storm	Inflow	Outflow	(ft^3)	(acre-ft)
10	20	0.96	575	10	1,466	575	891	0.020
15	25	0.96	719	15	2,019	719	1,300	0.030
20	30	0.96	863	20	2,483	863	1,620	0.037
30	40	0.96	1,150	30	3,096	1,150	1,946	0.045
35	45	0.96	1,294	35	3,324	1,294	2,030	0.047
40	50	0.96	1,438	40	3,470	1,438	2,032	0.047
50	60	0.96	1,726	50	3,740	1,726	2,014	0.046
60	70	0.96	2,013	60	4,039	2,013	2,025	0.046
70	80	0.96	2,301	70	4,188	2,301	1,887	0.043
80	90	0.96	2,588	80	4,428	2,588	1,839	0.042
90	100	0.96	2,876	90	4,712	2,876	1,836	0.042
100	110	0.96	3,164	100	5,086	3,164	1,922	0.044

110

0.85

0.80

100

110

5,265

3,451

0.85

0.80

Drainage / Detention Calculations Modified Rational Method

Required Total Storage Volume 1,651 cubic-feet 0.038 acre-feet 2,675 cubic-feet Provided Total Storage Volume 0.061 acre-feet

Onsite Existing Conditions 0.33 acres mpervious area delta (Overall Area "A") Time (Tc) 20 minutes C value 0.35 l-25yr 6.60 in/hr Q25yr (T) 0.76 cfs Onsite Existing Flow Area Passing through Pond (Area E-1) Areas that are Bypassing Pond Q25yr (T) 0.00 cfs Allowable Release Rate Q25yr (T 0.76 cfs Q25yr (T) 0.76 cfs Proposed Release Rate

Onsite Proposed Conditions 0.28 acres mpervious area delta (Overall area "A") Time (Tc) 10 minutes 0.90 C value Developed Condition C-Value 8.30 in/hr Q25yr 2.07 cfs Developed Runoff

Runoff per S	Storm Eve	ent - Develo	ped			Inflow per	Storm Eve	nt
-			•	Runoff	1	Storm		
Time (min.)	l-25yr	C value	Area (ac)	(cfs)		Event	Runoff	Inflow (ft^3)
10	8.30	0.90	0.28	2.07		10	2.07	1,242
15	7.50	0.90	0.28	1.87		15	1.87	1,683
20	6.60	0.90	0.28	1.65		20	1.65	1,974
30	5.50	0.90	0.28	1.37		30	1.37	2,468
35	5.05	0.90	0.28	1.26		35	1.26	2,644
40	4.60	0.90	0.28	1.15		40	1.15	2,752
50	4.00	0.90	0.28	1.00		50	1.00	2,992
60	3.50	0.90	0.28	0.87		60	0.87	3,141
70	3.30	0.90	0.28	0.82		70	0.82	3,455
80	3.10	0.90	0.28	0.77		80	0.77	3,710
90	2.90	0.90	0.28	0.72		90	0.72	3,904
100	2.70	0.90	0.28	0.67		100	0.67	4,039
110	2.50	0.90	0.28	0.62		110	0.62	4,113

B4 A11 -	ahla 0	£l 04a.	5		Datautian \	/alama Dan	!		
Max Allo	wable Out	flow per Sto		ļ	Detention v	/olume Req	uirea		
			Outflow					Storage	Storage
Storm	Time	Release	(ft^3)		Storm	Inflow	Outflow	(ft^3)	(acre-ft)
10	20	0.76	457		10	1,242	457	784	0.018
15	25	0.76	572		15	1,683	572	1,111	0.026
20	30	0.76	686		20	1,974	686	1,288	0.030
30	40	0.76	915		30	2,468	915	1,553	0.036
35	45	0.76	1,029		35	2,644	1,029	1,615	0.037
40	50	0.76	1,143		40	2,752	1,143	1,609	0.037
50	60	0.76	1,372		50	2,992	1,372	1,619	0.037
60	70	0.76	1,601		60	3,141	1,601	1,540	0.035
70	80	0.76	1,830		70	3,455	1,830	1,626	0.037
80	90	0.76	2,058		80	3,710	2,058	1,651	0.038
90	100	0.76	2,287		90	3,904	2,287	1,617	0.037
100	110	0.76	2,516		100	4,039	2,516	1,523	0.035
110	120	0.76	2,744		110	4,113	2,744	1,369	0.031

Drainage / Detention Calculations Modified Rational Method

Required Total Storage Volume 1,391 cubic-feet 0.032 acre-feet 2,675 cubic-feet Provided Total Storage Volume 0.061 acre-feet

Onsite Existing Conditions 0.33 acres mpervious area delta (Overall Area "A") Time (Tc) 20 minutes 0.35 C value l-10vr 5.90 in/hr Q10yr (1 0.68 cfs Onsite Existing Flow Q10yr (1 Area Passing through Pond (Area E-1) cfs 0.00 cfs Areas that are Bypassing Pond Allowable Release Rate 0.68 cfs Proposed Release Rate Q10yr (T) 0.68 cfs

Onsite Proposed Conditions 0.28 acres mpervious area delta (Overall area "A") Time (Tc) 10 minutes 0.90 C value Developed Condition C-Value I-10yr 7.10 in/hr Q10yr Developed Runoff 1.77 cfs

Runoff per S	Storm Ev	ent - Develo	ned		Infl	ow per	Storm Eve	nt
ranon por c		5116 507516	роч	Runoff	- ;	Storm		
Time (min.)	l-10yr	C value	Area (ac)	(cfs)		Event	Runoff	Inflow (ft^3
10	7.10	0.90	0.28	1.77		10	1.77	1,06
15	6.50	0.90	0.28	1.62		15	1.62	1,45
20	5.90	0.90	0.28	1.47		20	1.47	1,76
30	4.80	0.90	0.28	1.20		30	1.20	2,15
35	4.40	0.90	0.28	1.10		35	1.10	2,30
40	4.00	0.90	0.28	1.00		40	1.00	2,39
50	3.50	0.90	0.28	0.87		50	0.87	2,61
60	3.00	0.90	0.28	0.75		60	0.75	2,69
70	2.80	0.90	0.28	0.70		70	0.70	2,93
80	2.60	0.90	0.28	0.65		80	0.65	3,11
90	2.50	0.90	0.28	0.62		90	0.62	3,36
100	2.40	0.90	0.28	0.60		100	0.60	3,59
110	2.30	0.90	0.28	0.57		110	0.57	3,78

Max Allo	wable Out	flow per Sto	rm Event	Detention \	/olume Req	uired		
			Outflow				Storage	Storage
Storm	Time	Release	(ft^3)	Storm	Inflow	Outflow	(ft^3)	(acre-ft)
10	20	0.68	409	10	1,062	409	653	0.015
15	25	0.68	511	15	1,458	511	947	0.022
20	30	0.68	613	20	1,765	613	1,152	0.026
30	40	0.68	818	30	2,154	818	1,336	0.031
35	45	0.68	920	35	2,304	920	1,384	0.032
40	50	0.68	1,022	40	2,393	1,022	1,371	0.031
50	60	0.68	1,227	50	2,618	1,227	1,391	0.032
60	70	0.68	1,431	60	2,692	1,431	1,261	0.029
70	80	0.68	1,635	70	2,932	1,635	1,296	0.030
80	90	0.68	1,840	80	3,111	1,840	1,271	0.029
90	100	0.68	2,044	90	3,366	2,044	1,321	0.030
100	110	0.68	2,249	100	3,590	2,249	1,341	0.031
110	120	0.68	2,453	110	3,784	2,453	1,331	0.031

Drainage / Detention Calculations Modified Rational Method

Required Total Storage Volume 689 cubic-feet 0.016 acre-feet Provided Total Storage Volume 2,675 cubic-feet 0.061 acre-feet

Onsite Existii	ng Conditions	
Area	0.33 acres	Impervious area delta (Overall Area "A"
Time (Tc)	20 minutes	
C value	0.35	
l-5yr	8.30 in/hr	
Q5yr (T)	0.96 cfs	Onsite Existing Flow
Q5yr (T)	0.00 cfs	Area Passing through Pond (Area E
Q5yr (T)	0.00 cfs	Areas that are Bypassing Pond
Q5yr (T)	0.96 cfs	Allowable Release Rate
Q5yr (T)	0.96 cfs	Proposed Release Rate

I-5yr Q5yr	6.10 in/hr 1.52 cfs	Developed Runoff
C value	0.90	Developed Condition C-Value
Time (Tc)	10 minutes	
Area	0.28 acres	Impervious area delta (Overall area "A
Onsite Propo	sed Conditions	

Runoff per S	torm Ev	ent - Develo	ped		Inflow per	Storm Eve	nt
				Runoff	Storm		
Time (min.)	l-5yr	C value	Area (ac)	(cfs)	Event	Runoff	Inflow (ft^3
10	6.10	0.90	0.28	1.52	10	1.52	91
15	5.50	0.90	0.28	1.37	15	1.37	1,23
20	4.90	0.90	0.28	1.22	20	1.22	1,46
30	4.10	0.90	0.28	1.02	30	1.02	1,84
35	3.75	0.90	0.28	0.93	35	0.93	1,96
40	3.40	0.90	0.28	0.85	40	0.85	2,03
50	2.80	0.90	0.28	0.70	50	0.70	2,09
60	2.60	0.90	0.28	0.65	60	0.65	2,33
70	2.40	0.90	0.28	0.60	70	0.60	2,51
80	2.30	0.90	0.28	0.57	80	0.57	2,75
90	2.10	0.90	0.28	0.52	90	0.52	2,82
100	1.90	0.90	0.28	0.47	100	0.47	2,84
110	1.80	0.90	0.28	0.45	110	0.45	2,96

			Outflow				Storage	Storage
Storm	Time	Release	(ft^3)	Storm	Inflow	Outflow	(ft^3)	(acre-ft)
10	20	0.96	575	10	912	575	337	0.008
15	25	0.96	719	15	1,234	719	515	0.012
20	30	0.96	863	20	1,466	863	603	0.014
30	40	0.96	1,150	30	1,840	1,150	689	0.016
35	45	0.96	1,294	35	1,963	1,294	669	0.015
40	50	0.96	1,438	40	2,034	1,438	596	0.014
50	60	0.96	1,726	50	2,094	1,726	369	0.008
60	70	0.96	2,013	60	2,333	2,013	320	0.007
70	80	0.96	2,301	70	2,513	2,301	212	0.005
80	90	0.96	2,588	80	2,752	2,588	164	0.004
90	100	0.96	2,876	90	2,827	2,876	(49)	-0.001
100	110	0.96	3,164	100	2,842	3,164	(322)	-0.007
110	120	0.96	3,451	110	2,962	3,451	(489)	-0.011

OUTFALL RELEASE	SUMMARY	/
DETENTION POND	0.96	CFS
BYPASS FROM DA E-1	1.83	CFS
TOTAL RELEASE FROM		
DRAINAGE SYSTEM	2.79	CFS
RELEASE FROM ORIFICE	2.79	CFS

3,451

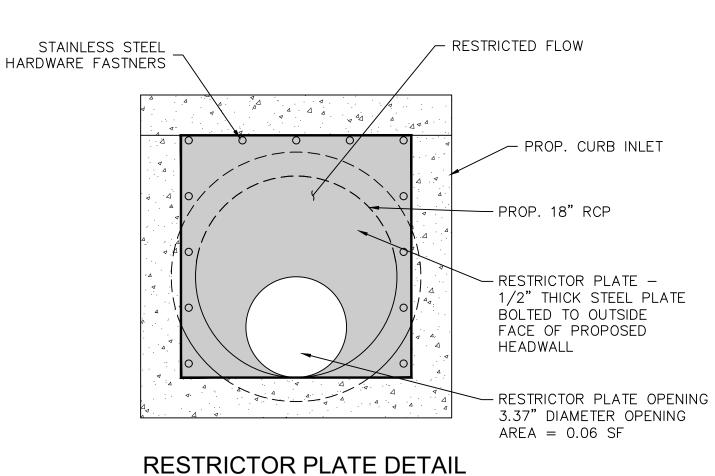
THE DESIGN CAPACITY FOR RUNOFF FROM THE SITE ENTERING EXISTING CURB INLET (STA 0+56 SD LAT B-9) IS 3.1 CFS. THE TOTAL FLOW OF 2.79 CFS IS LESS THAN

DETEN	TION POND A
100 YEAR OR	IFICE CALCULATION
Q =	C*A(2gh) ^{0.5}
PROPOS	ED 12" CULVERT
C =	3.33
Dia Orifice =	3.37 in
Open Area	0.06 ft ²
g =	32.2 ft/s ²
FL Outfall	0.00
WSE =	574.53
h=	2.86
O =	2.79 cfs

1,814

5,086

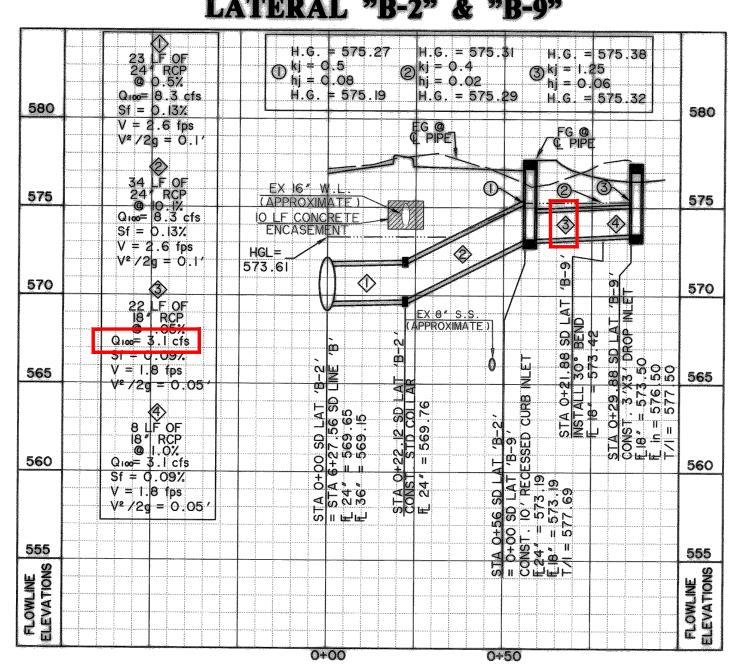
5,265



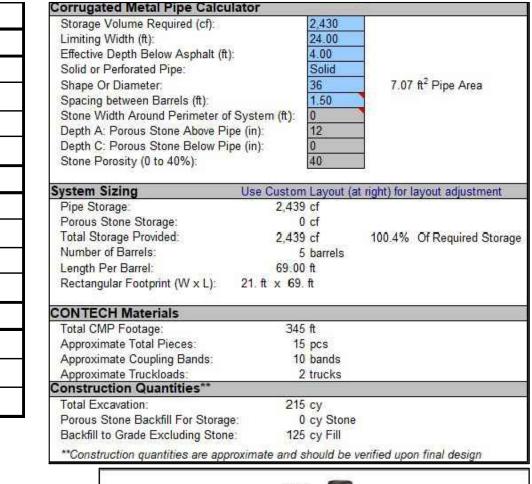
NOT TO SCALE

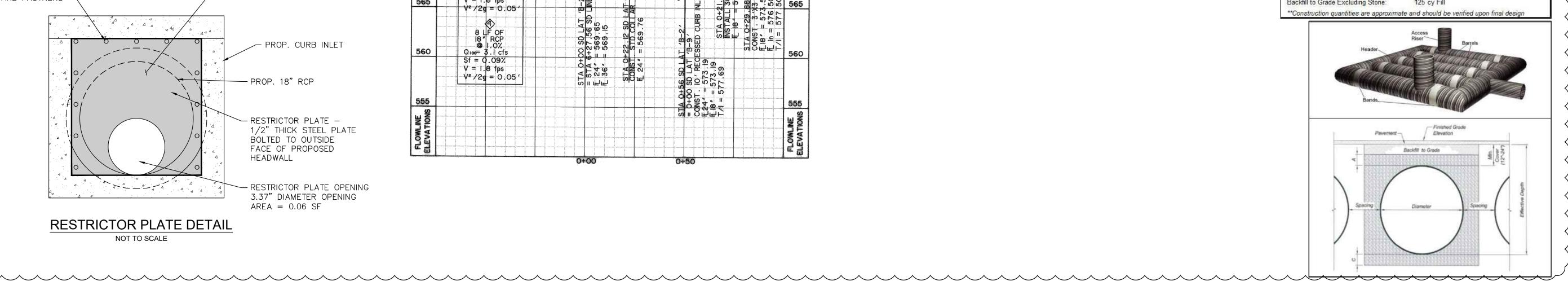
EXISTING PROFILE FOR STORM CONNECTION

LATERAL "B-2" & "B-9"



9 3	1,341 0.031 1,331 0.031		100 110		110 120	0.9 0.9
	DETENTION P		SHIMI	ΛARN		
	DENTENT	_				1
	PIPE SIZE	18"		IN		1
	AREA OF PIPE		1.767	FT ²		
	PIPE LENGTH		133.66]
	PIPE VOLUME	2	236.18	FT ³		
	UNDERGROU	ND C	ETEN	TION		
	PIPE SIZE		36	IN		
	AREA OF PIPE		7.07	FT ²		
	PIPE LENGTH		345]
	PIPE VOLUME		2439	FT ³		
	TOTAL DETE	OITV	N SYS	TEM		
	PIPE	2	236.18	FT ³		
	POND		2439	FT ³		
	TOTAL VOLUME	26	375.18	FT ³		
						•



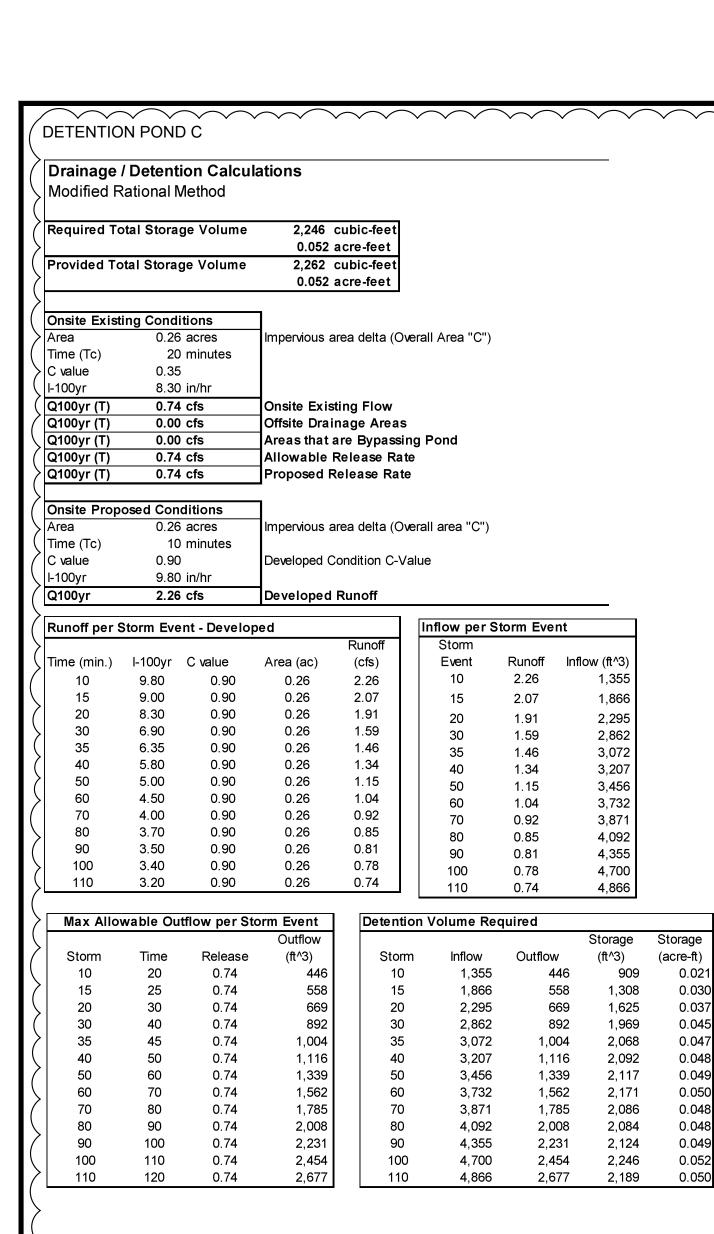


AY



CHECKED:

SHEET C-14



DETENTION POND C 100 YEAR ORIFICE CALCULATION

> $Q = C*A(2gh)^{0.5}$ PROPOSED 12" CULVERT C = 3.33

 $g = 32.2 \text{ ft/s}^2$

Dia Orifice = 1.72 in Open Area 0.02 ft²

FL Outfall 0.00 WSE = 576.58 h= 2.93 Q = 0.74 cfs

_	/ Detention Cald Rational Method	Julations		
Required To	otal Storage Volum	•	cubic-feet	
		0.042	acre-feet	
Provided To	otal Storage Volum	•	cubic-feet	
		0.052	acre-feet	
	ing Conditions	—		
Area	0.26 acres	Impervious	area delta (C	overall Area "C")
Time (Tc)	20 minutes			
C value	0.35			
l-25yr	6.60 in/hr			
Q25yr (T)	0.59 cfs	Onsite Exis	•	
Q25yr (T)	0.00 cfs		inage Area	
Q25yr (T)	0.00 cfs		are Bypass	•
Q25yr (T)	0.59 cfs		Release Ra	
Q25yr (T)	0.59 cfs	Proposed I	Release Ra	te
Onsite Pron	osed Conditions	\neg		
Area	0.26 acres		area delta (C	Overall area "C")
Time (Tc)	10 minutes			, ,
C value	0.90	Developed (Condition C-\	Value
l-25yr	8.30 in/hr			
Q25yr	1.91 cfs	Developed	l Runoff	
		-		
Runoff per S	torm Event - Devel	oped		Inflow per Storm
			Runoff	Storm
Time (min.)	l-25yr C value	Area (ac)	(cfs)	Event Rund
10	8.30 0.90	0.26	1.91	10 1.9

30	2.50	0.50	0.20	0.07	00	0.01	0,000	
100	2.70	0.90	0.26	0.62	100	0.62	3,732	
110	2.50	0.90	0.26	0.58	110	0.58	3,802	
					•			
Max Allo	wable Out	flow per Sto	rm Event	Detentio	n Volume Rec	uired		
			Outflow				Storage	Storage
Storm	Time	Release	(ft^3)	Storm	Inflow	Outflow	(ft^3)	(acre-ft)
10	20	0.59	355	10	1,147	355	793	0.018
15	25	0.59	444	15	1,555	444	1,112	0.026
20	30	0.59	532	20	1,825	532	1,293	0.030
30	40	0.59	710	30	2,281	710	1,571	0.036
35	45	0.59	798	35	2,443	798	1,645	0.038
40	50	0.59	887	40	2,544	887	1,657	0.038
50	60	0.59	1,064	50	2,765	1,064	1,700	0.039
60	70	0.59	1,242	60	2,903	1,242	1,661	0.038
70	80	0.59	1,419	70	3,193	1,419	1,774	0.041
80	90	0.59	1,597	80	3,428	1,597	1,832	0.042
90	100	0.59	1,774	90	3,608	1,774	1,834	0.042
				1				

Header Header Barrels Finished Grade Elevation Backfill to Grade	Bands Finished Grade Elevation	2 4 1 3	0.042 0.042 0.041 0.038	80 90 100 110	90 100 110 120	0.53 0.53 0.53 0.53	1,427 1,586 1,745 1,903
Pavement - Finished Grade Elevation	Pavement — Finished Grade Elevation Backfill to Grade ### 15 15 15 15 15 15 15 1		Hea		ccess	Barrels	
Pavement	Backfill to Grade		Bar	nds.			
	Spacing Diameter Spacing				200	will (Ib)	

Runoff per S	Storm Ev	ent - Develo _l	ped			Inflow per S	torm Even	it
				Run	off	Storm		
Time (min.)	l-25yr	C value	Area (ac)	(cfs		Event	Runoff	Inflow (ft^3)
10	8.30	0.90	0.26	1.9		10	1.91	1,147
15	7.50	0.90	0.26	1.7		15	1.73	1,555
20	6.60	0.90	0.26	1.5		20	1.52	1,825
30	5.50	0.90	0.26	1.2		30	1.27	2,281
35	5.05	0.90	0.26	1.1	6	35	1.16	2,443
40	4.60	0.90	0.26	1.0	6	40	1.06	2,544
50	4.00	0.90	0.26	0.9	2	50	0.92	2,765
60	3.50	0.90	0.26	0.8	1	60	0.81	2,903
70	3.30	0.90	0.26	0.7	6	70	0.76	3,193
80	3.10	0.90	0.26	0.7	1	80	0.71	3,428
90	2.90	0.90	0.26	0.6	7	90	0.67	3,608
100	2.70	0.90	0.26	0.6	2	100	0.62	3,732
110	2.50	0.90	0.26	0.5	8	110	0.58	3,802
					<u> </u>			
Max Allov	vable Οι	utflow per St	orm Event		Detention	ı Volume Red	uired	
			Outflow					Storage
Storm	Time	Release	(ft^3)		Storm	Inflow	Outflow	(ft^3)
10	20	0.59	355		10	1,147	355	793
15	25	0.59	444		15	1,555	444	1,112
20	30	0.59	532		20	1,825	532	1,293
30	40	0.59	710		30	2,281	710	1,571
35	45	0.59	798		35	2,443	798	1,645
40	50	0.59	887		40	2,544	887	7 1,657
50	60	0.59	1,064		50	2,765	1,064	1,700

Max Allowable Outflow per Storm Event			rm Event	Detention \	√olume Req	uired		
			Outflow				Storage	Storage
Storm	Time	Release	(ft^3)	Storm	Inflow	Outflow	(ft^3)	(acre-ft)
10	20	0.59	355	10	1,147	355	793	0.018
15	25	0.59	444	15	1,555	444	1,112	0.026
20	30	0.59	532	20	1,825	532	1,293	0.030
30	40	0.59	710	30	2,281	710	1,571	0.036
35	45	0.59	798	35	2,443	798	1,645	0.038
40	50	0.59	887	40	2,544	887	1,657	0.038
50	60	0.59	1,064	50	2,765	1,064	1,700	0.039
60	70	0.59	1,242	60	2,903	1,242	1,661	0.038
70	80	0.59	1,419	70	3,193	1,419	1,774	0.04
80	90	0.59	1,597	80	3,428	1,597	1,832	0.042
90	100	0.59	1,774	90	3,608	1,774	1,834	0.042
100	110	0.59	1,951	100	3,732	1,951	1,781	0.04
110	120	0.59	2,129	110	3,802	2,129	1,673	0.038

Storage Volume Required (cf): Limiting Width (ft): Effective Depth Below Asphalt (ft): Solid or Perforated Pipe: Shape Or Diameter: Spacing between Barrels (ft): Stone Width Around Perimeter of Stone Width Around Perimeter of Stone Porous Stone Below Pipe Stone Porosity (0 to 40%):	System (ft): e (in):	2,246 24.00 4.00 Solid 36 1.50 0 12 0 40	7.07	ft ² Pípe Area
System Sizing	Use Custom	Layout (at	right) for I	ayout adjustment
Pipe Storage:	2,262	cf		
Porous Stone Storage:	0	cf		
Total Storage Provided:	2,262	cf	100.7%	Of Required Storag
Number of Barrels:	5	barrels		
Length Per Barrel:	64.00	ft		
Rectangular Footprint (W x L):	21. ft x 64.	ft		
CONTECH Materials				
Total CMP Footage:	320	ft		
Approximate Total Pieces:	15	pcs		
Approximate Coupling Bands:	10	bands		
Approximate Truckloads:	2	trucks		
Construction Quantities**				
Total Excavation:	200	су		
Porous Stone Backfill For Storage:	0	cy Stone		
Backfill to Grade Excluding Stone:		cy Fill		
**Construction quantities are appro	751 2			A COLD

DERGROUND STORAGE DESIGN	

Calculations	Drainage / Detention Calcula

(acre-ft)

0.021

0.030

0.037

0.045

0.047

0.050

0.049

0.052

909

1,308

1,625

1,969

2,068

2,092

2,117

QZ5YI	1.3	T CIS	Developed	a Rulloll			
Runoff per S	Storm Ev	ent - Develo	ped		Inflow per	Storm Eve	nt
Time (min.)	l-25yr	C value	Area (ac)	Runoff (cfs)	Storm Event	Runoff	Inflow (ft^3)
10	8.30	0.90	0.26	1.91	10	1.91	1,147
15	7.50	0.90	0.26	1.73	15	1.73	1,555
20	6.60	0.90	0.26	1.52	20	1.52	1,825
30	5.50	0.90	0.26	1.27	30	1.27	2,281
35	5.05	0.90	0.26	1.16	35	1.16	2,443
40	4.60	0.90	0.26	1.06	40	1.06	2,544
50	4.00	0.90	0.26	0.92	50	0.92	2,765
60	3.50	0.90	0.26	0.81	60	0.81	2,903
70	3.30	0.90	0.26	0.76	70	0.76	3,193
80	3.10	0.90	0.26	0.71	80	0.71	3,428
90	2.90	0.90	0.26	0.67	90	0.67	3,608
30	2.90	0.90	0.20	0.07	1 30	0.07	3,000

Time (min.) I-10yr C value Area (ac) (cfs) Event Runoff 10 7.10 0.90 0.26 1.64 15 6.50 0.90 0.26 1.50 15 1.50 20 5.90 0.90 0.26 1.36 30 4.80 0.90 0.26 1.11 30 1.11 35 4.40 0.90 0.26 1.01 35 1.01 40 4.00 0.90 0.26 0.92 40 0.92 50 3.50 0.90 0.26 0.81 50 0.81 60 3.00 0.90 0.26 0.69	Inflow (ft^3) 982 1,348 1,631 1,991 2,129
10 7.10 0.90 0.26 1.64 15 6.50 0.90 0.26 1.50 20 5.90 0.90 0.26 1.36 30 4.80 0.90 0.26 1.11 35 4.40 0.90 0.26 1.01 40 4.00 0.90 0.26 0.92 50 3.50 0.90 0.26 0.81 50	1,348 1,631 1,991
15 6.50 0.90 0.26 1.50 20 5.90 0.90 0.26 1.36 30 4.80 0.90 0.26 1.11 35 4.40 0.90 0.26 1.01 40 4.00 0.90 0.26 0.92 50 3.50 0.90 0.26 0.81	1,631 1,991
20 5.90 0.90 0.26 1.36 30 4.80 0.90 0.26 1.11 35 4.40 0.90 0.26 1.01 40 4.00 0.90 0.26 0.92 50 3.50 0.90 0.26 0.81 50 0.81	1,991
30 4.80 0.90 0.26 1.11 30 1.11 35 4.40 0.90 0.26 1.01 35 1.01 40 4.00 0.90 0.26 0.92 40 0.92 50 3.50 0.90 0.26 0.81 50 0.81	1,991
40 4.00 0.90 0.26 0.92 40 0.92 50 3.50 0.90 0.26 0.81 50 0.81	2 129
50 3.50 0.90 0.26 0.81 50 0.81	2,120
	2,212
60 3.00 0.90 0.26 0.69 60 0.69	2,419
	2,488
70 2.80 0.90 0.26 0.65 70 0.65	2,710
80 2.60 0.90 0.26 0.60 80 0.60	2,875
90 2.50 0.90 0.26 0.58 90 0.58	3,110
100 2.40 0.90 0.26 0.55 100 0.55	3,318
110 2.30 0.90 0.26 0.53 110 0.53	3,497

llo	wable Out	flow per Sto	rm Event	Detention \	/olume Red	uired		
			Outflow		'		Storage	Storage
	Time	Release	(ft^3)	Storm	Inflow	Outflow	(ft^3)	(acre-ft)
	20	0.53	317	10	982	317	664	0.01
	25	0.53	396	15	1,348	396	951	0.022
	30	0.53	476	20	1,631	476	1,155	0.027
	40	0.53	634	30	1,991	634	1,356	0.03
	45	0.53	714	35	2,129	714	1,415	0.032
	50	0.53	793	40	2,123	793	1,419	0.033
	60	0.53	952	50	2,419	952	1,468	0.034
	70	0.53	1,110	60	2,488	1,110	1,378	0.032
	80	0.53	1,269	70	2,710	1,110	1,441	0.032
	90	0.53	1,427	80	2,710	1,209	1, 44 1 1,448	0.03
	100	0.53	1,586		•			
	110	0.53	1,745	90	3,110	1,586	1,524	0.03
	120	0.53	1,903	100 110	3,318 3,497	1,745 1,903	1,573 1,594	0.036 0.037

Drainage / Detention Calculations
Modified Rational Method

Required Total Storage Volume	1,358 cubic-feet
	0.031 acre-feet
Provided Total Storage Volume	2,262 cubic-feet
	0.052 acre-feet

Onsite Existin	ng Conditions	
Area	0.26 acres	Impervious area delta (Overall Area "C")
Time (Tc)	20 minutes	
C value	0.35	
l-5yr	4.90 in/hr	
Q5yr (T)	0.44 cfs	Onsite Existing Flow
Q5yr (T)	0.00 cfs	Offsite Drainage Areas
Q5yr (T)	0.00 cfs	Areas that are Bypassing Pond
Q5yr (T)	0.44 cfs	Allowable Release Rate
Q5yr (T)	0.44 cfs	Proposed Release Rate

Q5vr	1.41 cfs	Developed Runoff
l-5yr	6.10 in/hr	
C value	0.90	Developed Condition C-Value
Time (Tc)	10 minutes	
Area	0.26 acres	Impervious area delta (Overall area "C"
Onsite Propo	sed Conditions	

Runoff per S	torm Ev	ent - Develo	ped		Inflow per	Storm Eve	nt
•				Runoff	Storm	Dunoff	Inflow (#A2
Гime (min.)	l-5yr	C value	Area (ac)	(cfs)	Event	Runoff	Inflow (ft^3
10	6.10	0.90	0.26	1.41	10	1.41	843
15	5.50	0.90	0.26	1.27	15	1.27	1,14
20	4.90	0.90	0.26	1.13	20	1.13	1,35
30	4.10	0.90	0.26	0.94	30	0.94	1,70
35	3.75	0.90	0.26	0.86	35	0.86	1,81
40	3.40	0.90	0.26	0.78	40	0.78	1,88
50	2.80	0.90	0.26	0.65	50	0.65	1,93
60	2.60	0.90	0.26	0.60	60	0.60	2,15
70	2.40	0.90	0.26	0.55	70	0.55	2,32
80	2.30	0.90	0.26	0.53	80	0.53	2,54
90	2.10	0.90	0.26	0.48	90	0.48	2,61
100	1.90	0.90	0.26	0.44	100	0.44	2,62
110	1.80	0.90	0.26	0.41	110	0.41	2,73

Max Allowable Outflow per Storm Event				Detention Volume Required					
			Outflow				Storage	Storage	
Storm	Time	Release	(ft^3)	Storm	Inflow	Outflow	(ft^3)	(acre-ft)	
10	20	0.44	263	10	843	263	580	0.013	
15	25	0.44	329	15	1,140	329	811	0.019	
20	30	0.44	395	20	1,355	395	960	0.022	
30	40	0.44	527	30	1,700	527	1,174	0.027	
35	45	0.44	593	35	1,814	593	1,222	0.028	
40	50	0.44	659	40	1,880	659	1,222	0.028	
50	60	0.44	790	50	1,935	790	1,145	0.026	
60	70	0.44	922	60	2,157	922	1,235	0.028	
70	80	0.44	1,054	70	2,322	1,054	1,269	0.029	
80	90	0.44	1,185	80	2,544	1,185	1,358	0.031	
90	100	0.44	1,317	90	2,613	1,317	1,296	0.030	
100	110	0.44	1,449	100	2,627	1,449	1,178	0.027	
110	120	0.44	1,581	110	2,737	1,581	1,157	0.027	

STAINLESS STEEL	RESTRICTED FLOW
	PROP. CURB INLET
	PROP. 18" RCP
	RESTRICTOR PLATE — 1/2" THICK STEEL PLATE BOLTED TO OUTSIDE FACE OF PROPOSED HEADWALL
	RESTRICTOR PLATE OPENING 0.74" DIAMETER OPENING AREA = 0.02 SF

RESTRICTOR PLATE DETAIL

OORE

AY

CLAY CRISTY

ENGINEERING

SHEET C-16

1,594 cubic-feet

0.037 acre-feet

2,262 cubic-feet

0.052 acre-feet

Onsite Existing Conditions 0.26 acres Impervious area delta (Overall Area "C") 20 minutes 5.90 in/hr Onsite Existing Flow Offsite Drainage Areas Areas that are Bypassing Pond

Allowable Release Rate

Q10yr (T)	0.53 cfs	Proposed Release Rate
Onsite Propo	sed Conditions	٦
Area	0.26 acres	Impervious area delta (Overall area "C")
Time (Tc)	10 minutes	
C value	0.90	Developed Condition C-Value

Drainage / Detention Calculations

Modified Rational Method

Required Total Storage Volume

Provided Total Storage Volume

0.35

0.53 cfs

0.00 cfs

0.00 cfs

0.53 cfs

7.10 in/hr

Time (Tc)

C value

l-10yr

Q10yr (T)

Q10yr (T

l-10yr

/r	1.64 cfs	Developed Runoff					
off per Storm Event - Developed							

					-			
off per S	torm Eve	ent - Develo	ped		Inflow per	Storm Eve	nt	
(min.)	l-10yr	C value	Area (ac)	Runoff (cfs)		Storm Event	Runoff	Inflow (ft^3)
10 15	7.10 6.50	0.90 0.90	0.26 0.26	1.64 1.50		10 15	1.64 1.50	982 1,348
20 30	5.90 4.80	0.90 0.90	0.26 0.26	1.36 1.11		20 30	1.36 1.11	1,631 1,991
35	4.40	0.90	0.26	1.01		35	1.01	2,129
40 50	4.00 3.50	0.90 0.90	0.26 0.26	0.92 0.81		40 50	0.92 0.81	2,212 2,419
60 70	3.00 2.80	0.90 0.90	0.26 0.26	0.69 0.65		60 70	0.69 0.65	2,488 2,710
80	2.60	0.90	0.26	0.60		80	0.60	2,875
90 100	2.50 2.40	0.90 0.90	0.26 0.26	0.58 0.55		90 100	0.58 0.55	3,110 3,318
110	2.30	0.90	0.26	0.53		110	0.53	3,497

Max Allov	vable Out	flow per Sto		Detention \	Detention Volume Rec	Detention Volume Required	Detention Volume Required
_			Outflow			·	Storage
Storm	Time	Release	(ft^3)	Storm	Storm Inflow	Storm Inflow Outflow	Storm Inflow Outflow (ft^3)
10	20	0.53	317	10	10 982	10 982 317	10 982 317 664
15	25	0.53	396	15	15 1,348	15 1,348 396	15 1,348 396 951
20	30	0.53	476	20	20 1,631	20 1,631 476	20 1,631 476 1,155
30	40	0.53	634	30	30 1,991	30 1,991 634	30 1,991 634 1,356
35	45	0.53	714	35	35 2,129	35 2,129 714	35 2,129 714 1,415
40	50	0.53	793	40	40 2,212	40 2,212 793	40 2,212 793 1,419
50	60	0.53	952	50	50 2,419	50 2,419 952	50 2,419 952 1,468
60	70	0.53	1,110	60	,	,	,
70	80	0.53	1,269	70	1		· · · · · · · · · · · · · · · · · · ·
80	90	0.53	1,427	80	1	, , , , , , , , , , , , , , , , , , , ,	, , , , , , , , , , , , , , , , , , , ,
90	100	0.53	1,586	90	•	·	
100	110	0.53	1,745	100	,	•	
110	120	0.53	1,903	110	,	•	·



TEXAS FIRM #14199



OAD (75087

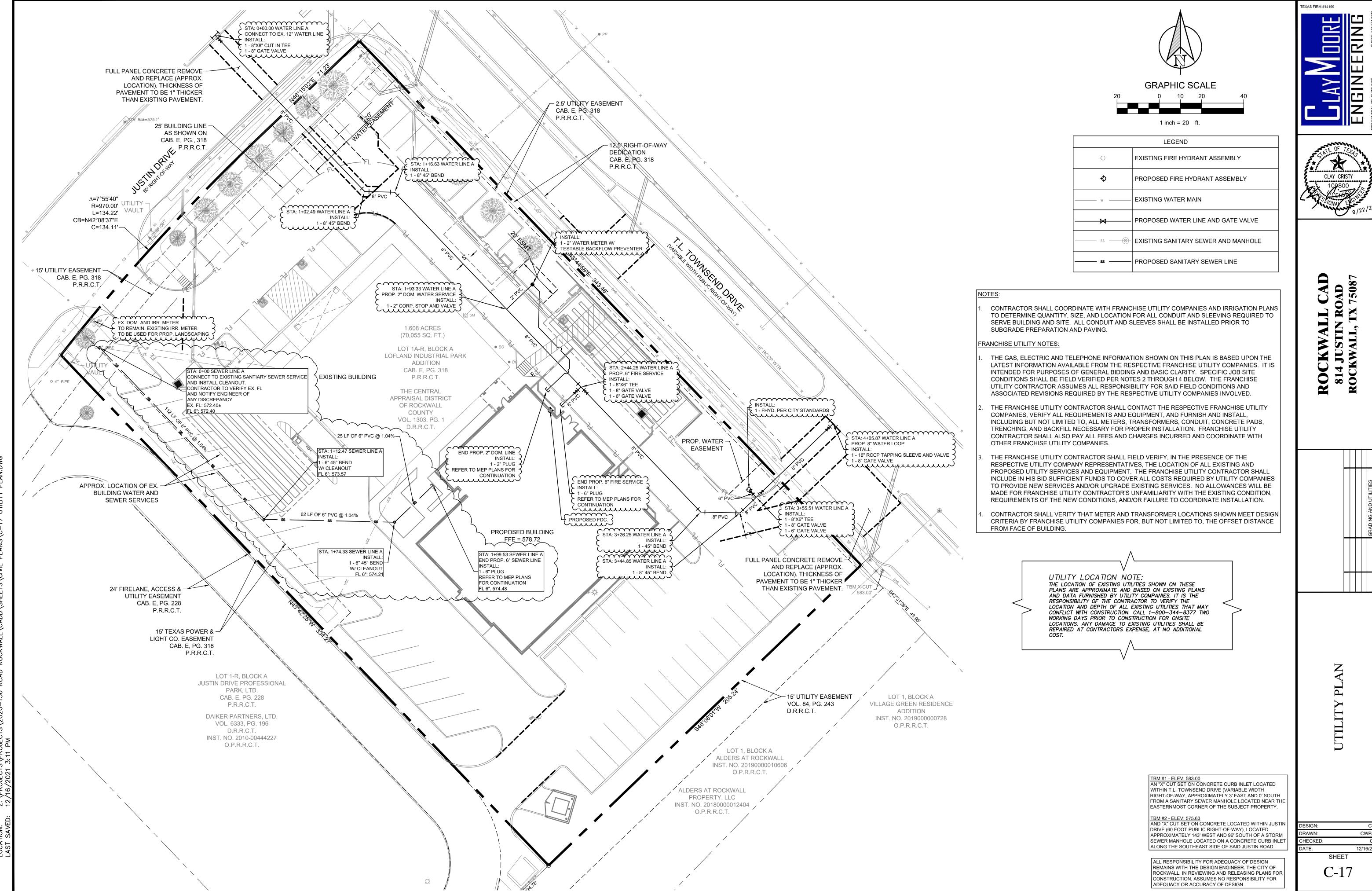
ROCKWALL CA 814 JUSTIN ROAD ROCKWALL, TX 7508

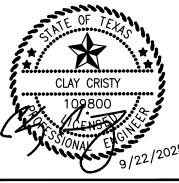
GRADING AND UTILITIES

IMPERVIOUS AREA DELTA CALCULATIONS

BIGN: C AWN: CWP ECKED: (TE: 7/29/2 SHEET

C-16.01





COMPACTED SUBGRADE LIME STABILIZED MEDIUM DUTY

CONCRETE PAVING

DUMPSTER AREA

GENERAL PAVING NOTES

AND REQUIREMENTS.

CONTENT AT LEAST 3% ABOVE OPTIMUM.

PER PROJECT SPECIFICATIONS.

CONCRETE PAVING

1. REFER TO GEOTECHNICAL REPORT NO. 21.032 PREPARED BY HOOPER

2. FOR PREPARATION OF PAVEMENT SUBGRADE, FILL PLACED BELOW

GROUP, INC. (DATED 02/18/21) FOR ALL SUBGRADE SPECIFICATIONS

FINISHED SUBGRADE ELEVATION IN FILL AREAS IN ALL AREAS TO BE

ASTM D698 (STANDARD PROCTOR) MAXIMUM DENSITY AT A MOISTURE

FOR HEAVY DUTY AND DUMPSTER AREA AND 3,600 PSI FOR STANDARD

DUTY AT 28 DAYS. JOINTS IN CONCRETE PAVING SHALL BE FORMED AT

A MAXIMUM OF 15 FEET. CONCRETE SHALL INCLUDE AIR ENTRAINMENT

OF 5.0±1.0 PERCENT. ALL OTHER JOINT SPACING SHALL BE INSTALLED

3. CONCRETE SHALL HAVE A MINIMUM 3,600 PSI COMPRESSIVE STRENGTH

4. JOINTS IN CONCRETE PAVEMENT SHALL NOT EXCEED 15 FOOT SPACING.

PAVING DETAILS

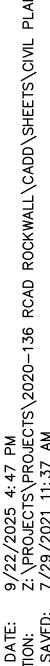
SCALE: NONE

PAVED SHALL BE COMPACTED TO THE RANGE OF 92 TO 98 PERCENT OF

#3 BARS AT 24" O.C.E.W. GRADE 60, ASTM A615

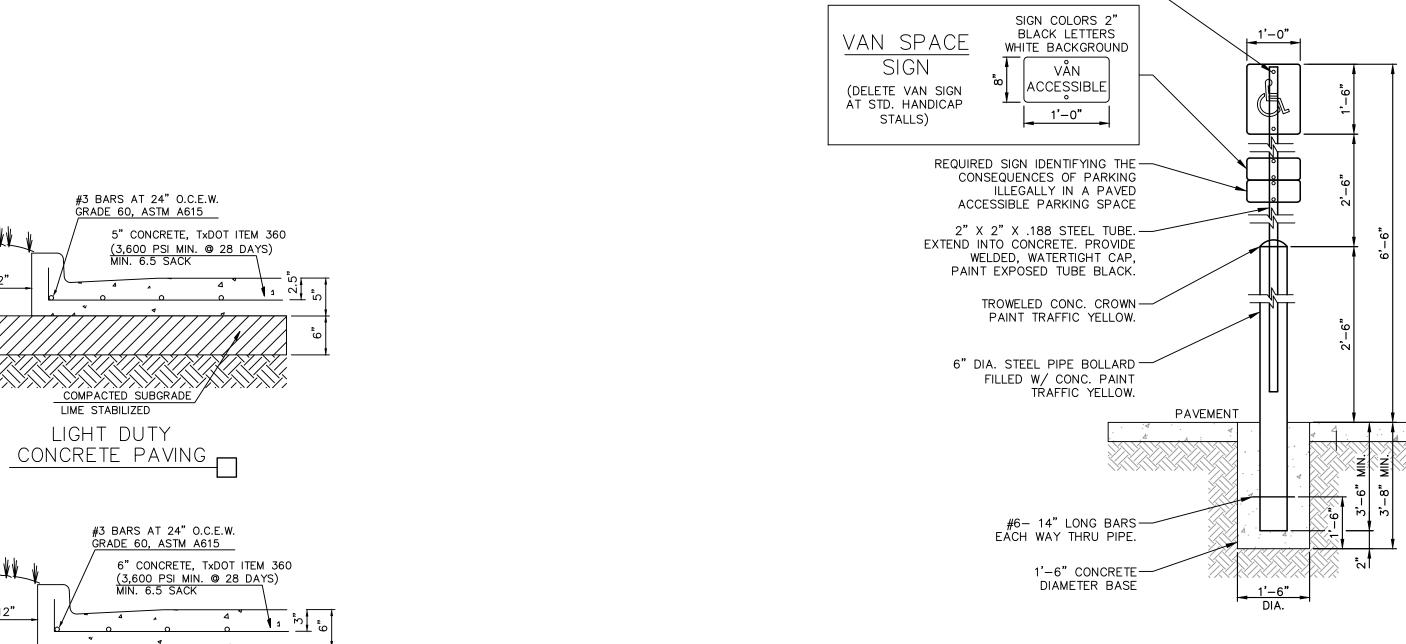
6" CONCRETE, TxDOT ITEM 360

(3,600 PSI MIN. @ 28 DAYS) MIN. 6.5 SACK







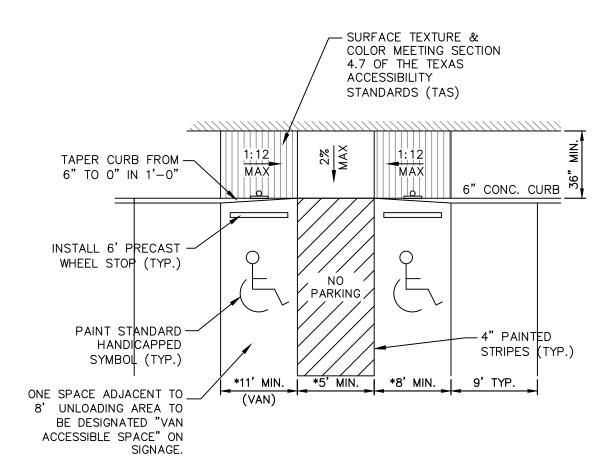


1. SIGN COLORS
1.1. BACKGROUND: WHITE 1.2. SYMBOL:

BOLT SIGN TO POST (TYP.) -

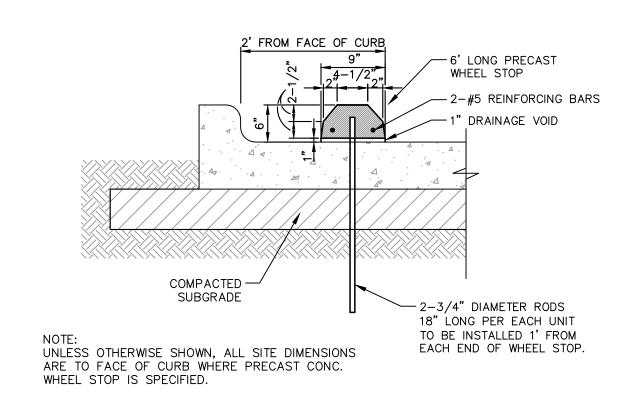
- 2. PROVIDE SIGNAGE AT END OF STALL AT LOCATIONS W/ ACCESSIBLE DESIGNATION TO ACT AS BUMPER STOP.
- 3. 1'-0"x1'-6"x .080" ALUM. HANDICAPPED PARKING SIGN. SIGN TO READ "RESERVED PARKING" W/ IDENTIFICATION SYMBOL, BOLT TO STEEL TUBE W/ 3/8" CADMIUM PLATED BOLTS, NUTS & WASHERS.
- 4. HANDICAP SIGNAGE TO BE IN ACCORDANCE WITH TEXAS DEPARTMENT OF LICENSING AND REGULATION TEXAS ACCESSIBILITY STANDARDS (TAS) PER

"HANDICAPPED PARKING" SIGN POST DETAIL N.T.S.

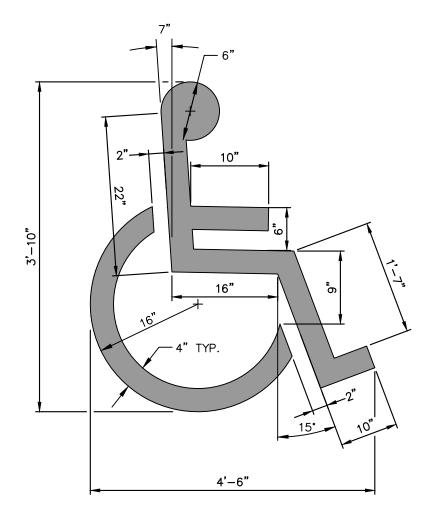


- 1. * DIMENSIONS MAY VARY REFER TO DIMENSIONAL CONTROL PLAN
- 2. SIGNAGE AND MARKINGS TO BE IN ACCORDANCE WITH FEDERAL STATE AND LOCAL
- 3. MAXIMUM SLOPE FOR HANDICAPPED ACCESSIBLE PATHS ARE 5% WITH A MAXIMUM CROSS FALL SLOPE OF 2%. THE FIRST FIVE FEET FROM THE DOOR IS NOT TO EXCEED 2% IN ANY DIRECTION.
- 4. HANDICAPPED SIGNS, INSTALL 2' FROM BACK OF CURB (TYP. EACH SPACE). SIGNAGE TO BE IN ACCORDANCE WITH TEXAS DEPARTMENT OF LICENSING AND REGULATION TEXAS ACCESSIBILITY STANDARDS (TAS) PER 4.6.4
- 5. THE WORDS "NO PARKING" ARE REQUIRED TO BE PAINTED ON ANY ACCESSIBLE AISLES ADJACENT TO THE PARKING SPACE. MINIMUM LETTER HEIGHT OF 12" AND MINIMUM STROKE WIDTH IS 2". CENTERED IN AISLES.

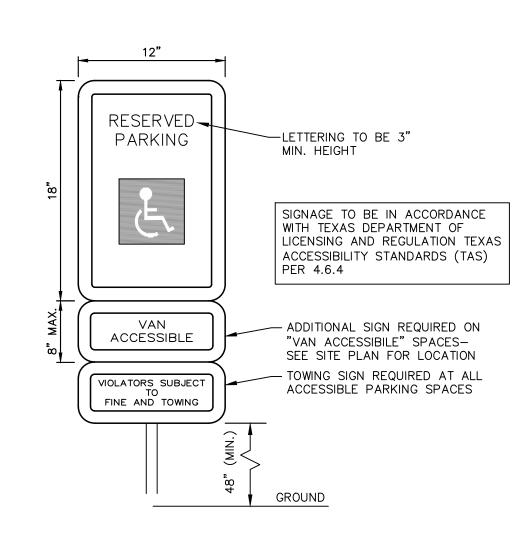
HANDICAPPED PARKING DETAIL N.T.S.



PRECAST CONCRETE WHEEL STOP DETAIL



1. ALL TRAFFIC MARKINGS TO BE THERMOPLASTIC AS PER DIMENSIONS SHOWN. HANDICAPPED PARKING SYMBOL DETAIL

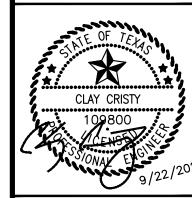


- A SIGN IDENTIFYING THE CONSEQUENCES OF PARKING ILLEGALLY IN A PAVED ACCESSIBLE PARKING SPACE MUST AT A MINIMUM:
- 1.1. STATE "VIOLATORS SUBJECT TO FINE AND TOWING" IN A LETTER HEIGHT OF AT LEAST 1 INCH;
- 1.2. BE MOUNTED ON A POLE, POST, WALL OR FREESTANDING BOARD;
- 1.3. BE NO MORE THAN EIGHT (8) INCHES BELOW A SIGN REQUIRED BY TEXAS ACCESSIBILITY STANDARDS, 506.6;
- 1.4. AND BE INSTALLED SO THAT THE BOTTOM EDGE OF THE SIGN IS NO LOWER THAN 48 INCHES AND NO HIGHER THAN 80 INCHES ABOVE GROUND LEVEL.

DISABLED SIGN DETAIL

N.T.S.

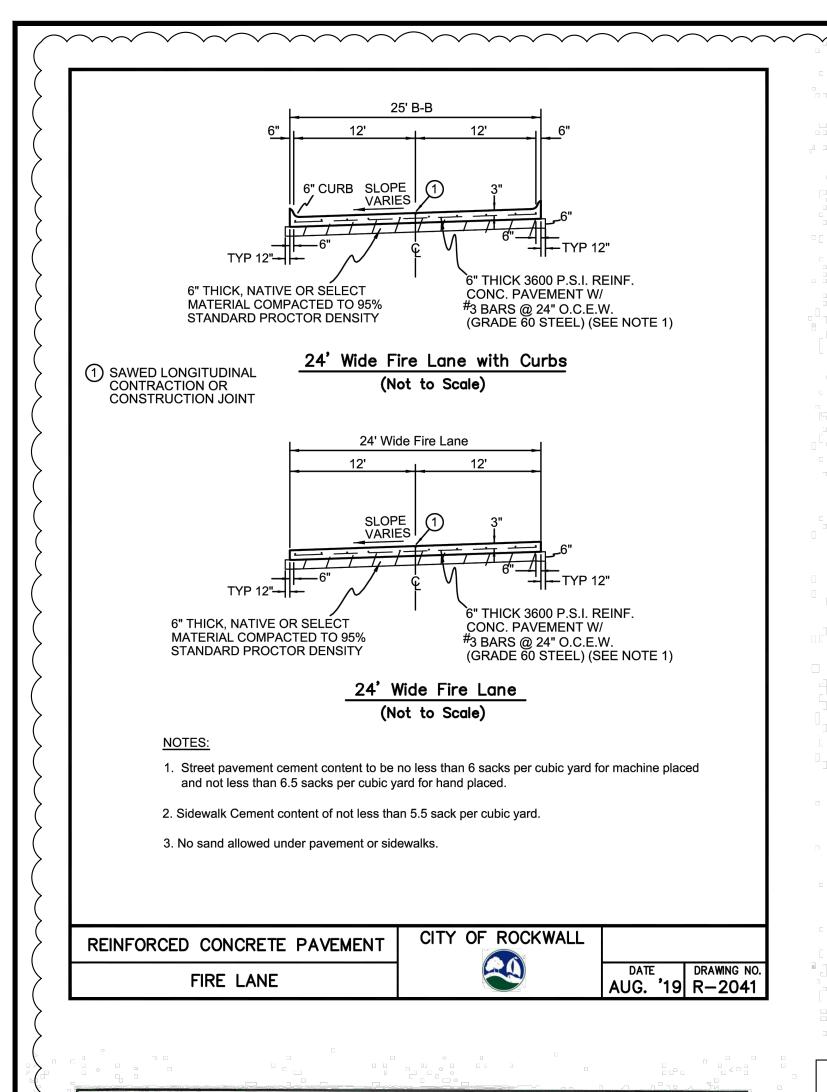


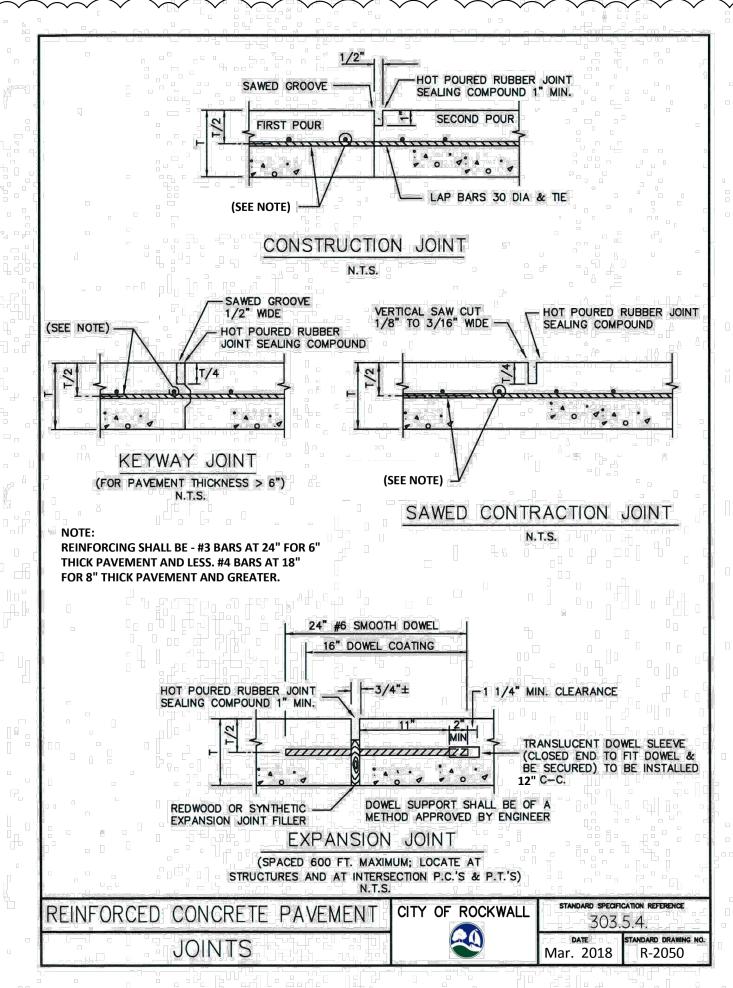


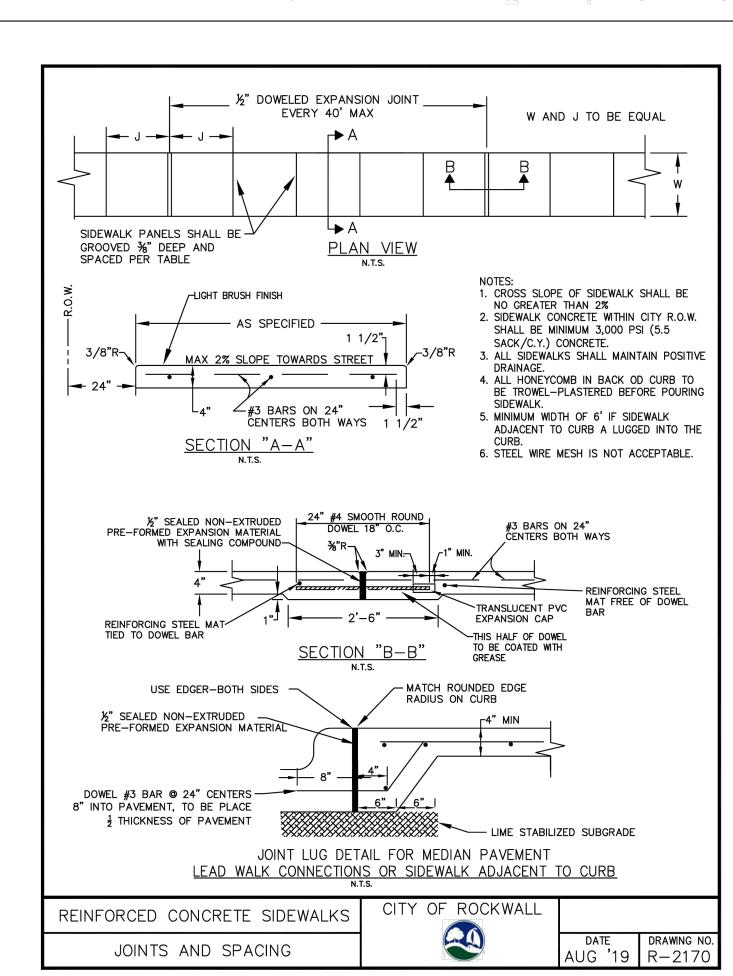
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VILS			

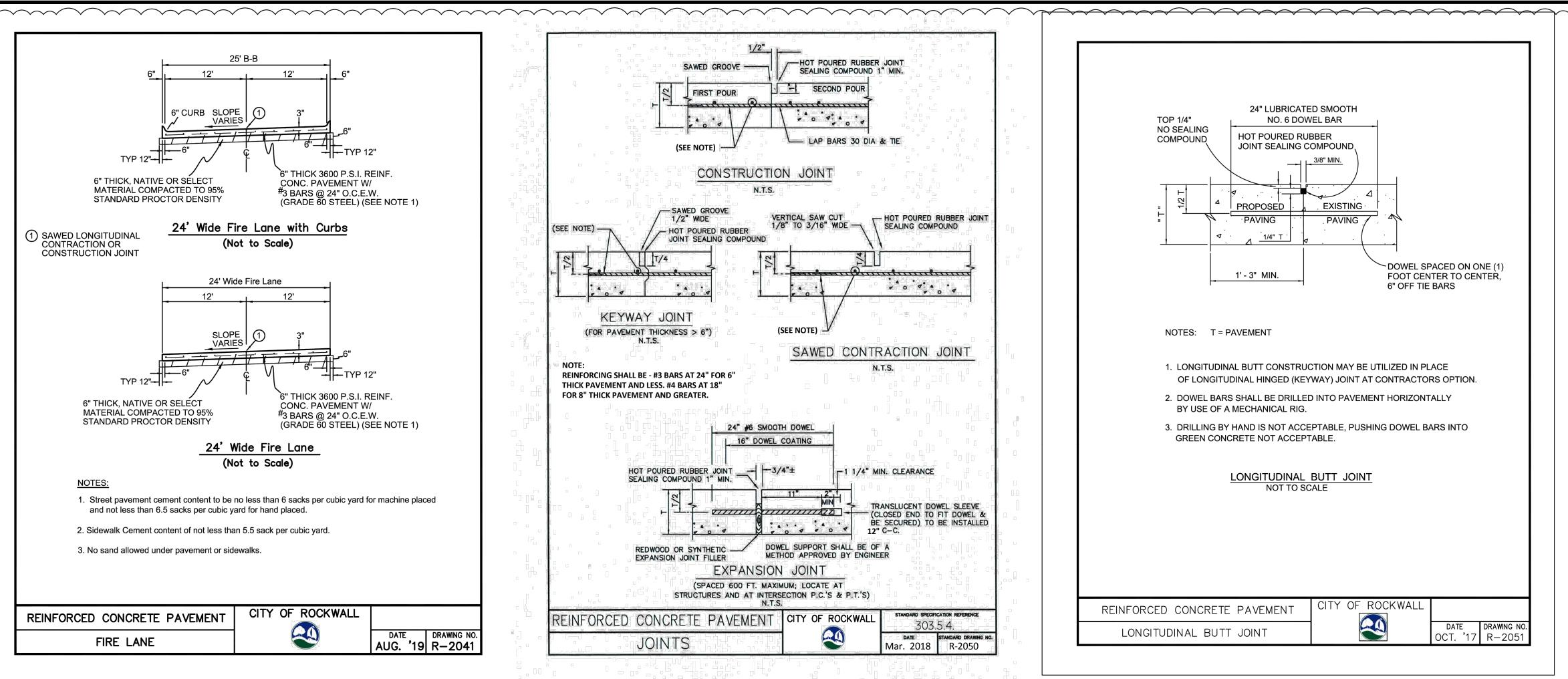
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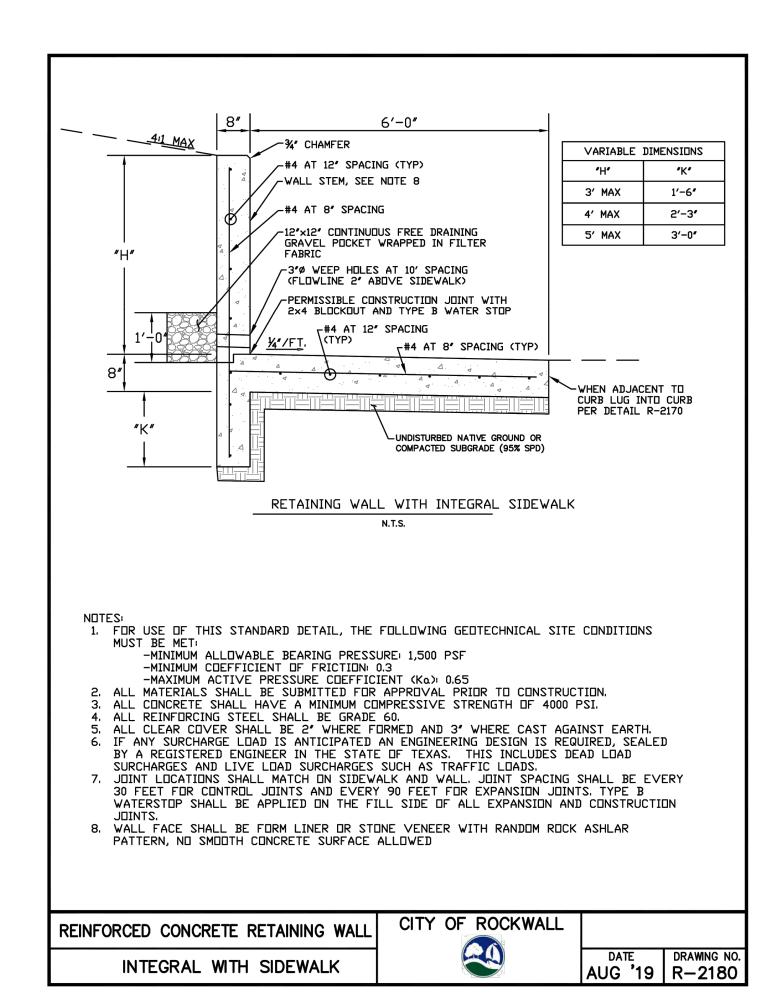
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DATE:		7/29/2
	SHEET	

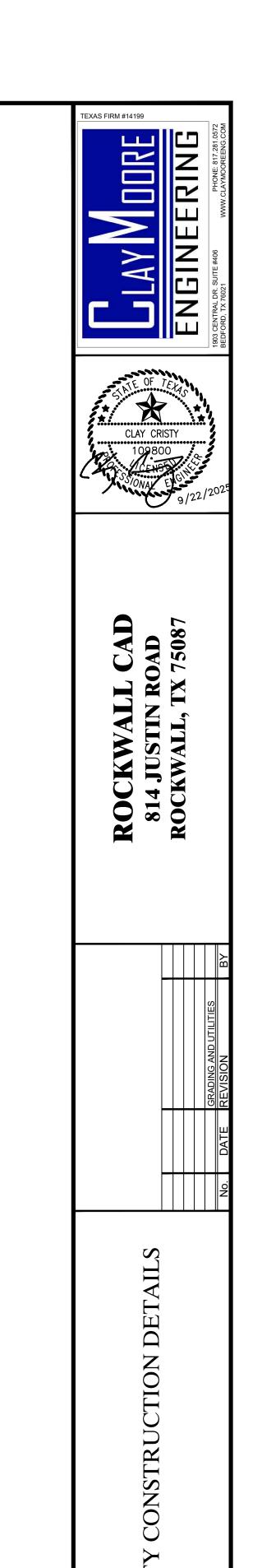




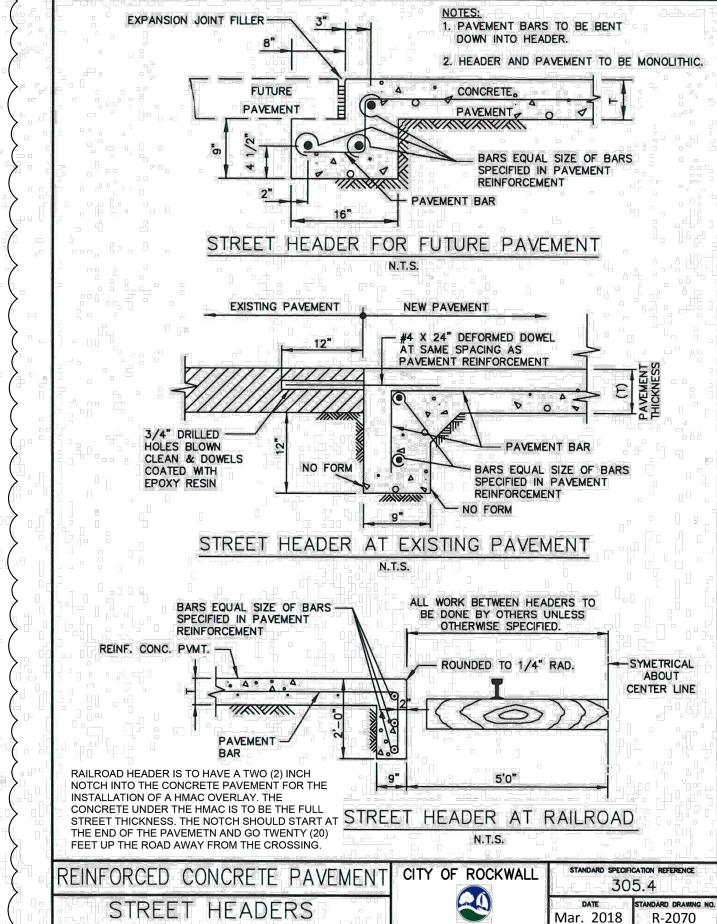








SHEET





PROJECT SUMMARY

CALCULATION DETAILS

- LOADING = HS20 & HS25
- APPROX. LINEAR FOOTAGE = 344 lf.

STORAGE SUMMARY

- STORAGE VOLUME REQUIRED = 2,430 cf.
- PIPE STORAGE VOLUME = 2,432 cf.
- BACKFILL STORAGE VOLUME = 0 cf.
- TOTAL STORAGE PROVIDED = 2,493 cf.

PIPE DETAILS

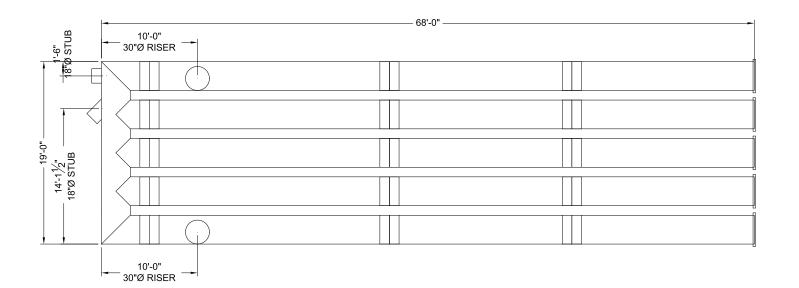
- DIAMETER = 36 IN.
- CORRUGATION = 2 2/3x1/2
- GAGE = 16
- COATING = ALT2
- WALL TYPE = Solid
- BARRELL SPACING = 12 IN.

BACKFILL DETAILS

- WIDTH AT ENDS = 0 IN.
- ABOVE PIPE = 0 IN.
- WIDTH AT SIDES = 0 IN.
- BELOW PIPE = 0 IN.

<u>NOTES</u>

- ALL RISER AND STUB DIMENSIONS ARE TO CENTERLINE. ALL ELEVATIONS, DIMENSIONS, AND LOCATIONS OF RISERS AND INLETS, SHALL BE VERIFIED BY THE ENGINEER OF RECORD PRIOR TO RELEASING FOR FABRICATION.
- ALL FITTINGS AND REINFORCEMENT COMPLY WITH ASTM A998.
- ALL RISERS AND STUBS ARE $2\frac{2}{3}$ " x $\frac{1}{2}$ " CORRUGATION AND 16 GAGE UNLESS OTHERWISE NOTED.
- RISERS TO BE FIELD TRIMMED TO GRADE.
- QUANTITY OF PIPE SHOWN DOES NOT PROVIDE EXTRA PIPE FOR CONNECTING THE SYSTEM TO EXISTING PIPE OR DRAINAGE STRUCTURES. OUR SYSTEM AS DETAILED PROVIDES NOMINAL INLET AND/OR OUTLET PIPE STUB FOR CONNECTION TO EXISTING DRAINAGE FACILITIES. IF ADDITIONAL PIPE IS NEEDED IT IS THE RESPONSIBILITY OF THE CONTRACTOR.
- BAND TYPE TO BE DETERMINED UPON FINAL DESIGN.
- THE PROJECT SUMMARY IS REFLECTIVE OF THE DYODS DESIGN, QUANTITIES ARE APPROX. AND SHOULD BE VERIFIED UPON FINAL DESIGN AND APPROVAL. FOR EXAMPLE, TOTAL EXCAVATION DOES NOT CONSIDER ALL VARIABLES SUCH AS SHORING AND ONLY ACCOUNTS FOR MATERIAL WITHIN THE ESTIMATED EXCAVATION FOOTPRINT.
- THESE DRAWINGS ARE FOR CONCEPTUAL PURPOSES AND DO NOT REFLECT ANY LOCAL PREFERENCES OR REGULATIONS. PLEASE CONTACT YOUR LOCAL CONTECH REP FOR MODIFICATIONS.



ASSEMBLY SCALE: 1" = 10'

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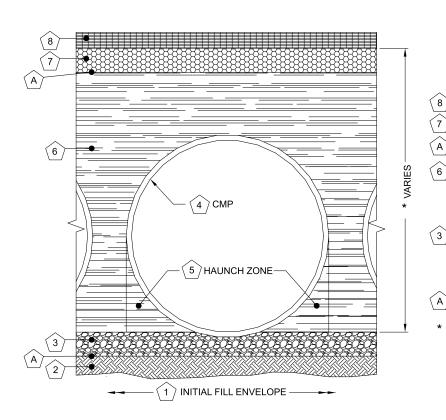


DRAWING

DYO7911 Rockwall CAD Rockwall, TX **DETENTION SYSTEM**

109800 7/29/2021

PROJECT No.:	SEQ. I	No.:	DATE:
4927	7911		5/20/2021
DESIGNED:		DRAW	/N:
DYO			DYO
CHECKED:		APPR	OVED:
DYO			DYO
SHEET NO.:	D	1	



Material Location	Description	Material Designation	Designation
Rigid or Flexible Pa (if applicable)	vement		
Road Base (if applicable)			
Geotextile Layer	Non-Woven Geotextile	CONTECH C-40 or C-45	Engineer Decision for consideration to prevent so migration into varying soil types
Backfill	Well graded granular material which may contain small amounts of silt or clay.	AASHTO M 145- A-1, A-2, A-3	Placed in 8" +/- loose lifts and compacted to 90% Standard Proctor Per AASHTO T 99
Bedding Stone	Well graded granular bedding material w/maximum particle size of 3"	AASHTO M43 - 3,357,4,467, 5, 56, 57	Engineer to determine if bedding is required. Pipe may be placed on the trench bottom of a relatively loose, native suitable well graded & granular material. For Arch pipes it is recommended to be shaped to a relatively flat bottom or fine-grade the foundation to a slight v-shape. Unsuitable material should be over-excavated and re-placed with a 4"-6" layer of well graded & granular stone per the material designation. See AASHTO 26.3.8.1 / 26.5.3 Bedding info.
Geotextile Layer	Non-Woven Geotextile	CONTECH C-40 or C-45	Engineer Decision for consideration to prevent so migration into varying soil types

MINIMUM WIDTH DEPENDS ON SITE CONDITIONS AND ENGINEERING JUDGEMENT

FOUNDATION/BEDDING PREPARATION

- PRIOR TO PLACING THE BEDDING, THE FOUNDATION MUST BE CONSTRUCTED TO A UNIFORM AND STABLE GRADE. IN THE EVENT THAT UNSUITABLE FOUNDATION MATERIALS ARE ENCOUNTERED DURING EXCAVATION, THEY SHALL BE REMOVED AND BROUGHT BACK TO THE GRADE WITH A FILL MATERIAL AS APPROVED BY THE ENGINEER.
- HAUNCH ZONE MATERIAL SHALL BE PLACED AND UNIFORMALLY COMPACTED WITHOUT

BACKFILL

WHEN PLACING THE FIRST LIFTS OF BACKFILL IT IS IMPORTANT TO MAKE SURE THAT THE BACKFILL IS PROPERLY COMPACTED UNDER AND AROUND THE PIPE HAUNCHES. BACKFILL SHALL BE PLACED SUCH THAT THERE IS NO MORE THAN A TWO LIFT (16") DIFFERENTIAL BETWEEN ANY OF THE PIPES AT ANY TIME DURING THE BACKFILL PROCESS. THE BACKFILL SHALL BE ADVANCED ALONG THE LENGTH OF THE DETENTION SYSTEM AT THE SAME RATE TO AVOID DIFFERENTIAL LOADING ON THE PIPE.

OTHER ALTERNATE BACKFILL MATERIAL MAY BE ALLOWED DEPENDING ON SITE SPECIFIC CONDITIONS, AS APPROVED BY SITE ENGINEER.



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not allow for placement and adequate compaction of the backfill.

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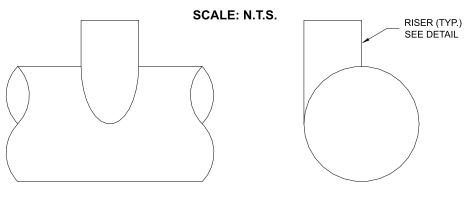
DRAWING

DYO7911 Rockwall CAD Rockwall, TX **DYODS**

FRONT

MANWAY DETAIL APPLICABLE FOR CMP SYSTEMS WITH DIAMETERS 48" AND LARGER. MANWAYS MAY BE REQUIRED ON SMALLER SYSTEMS DEPENDING ON ACTUAL SITE SPECIFIC CONDITIONS.

TYPICAL MANWAY DETAIL



ELEVATION

2'-6" -

Ø PIPF

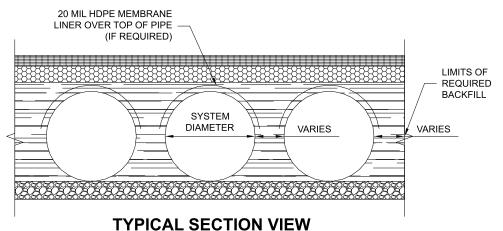
PLAN

TYPICAL RISER DETAIL

SCALE: N.T.S.

END

LADDERS ARE OPTIONAL AND ARE NOT REQUIRED FOR ALL SYSTEMS.



LINER OVER ROWS

SCALE: N.T.S.

NOTE: IF SALTING AGENTS FOR SNOW AND ICE REMOVAL ARE USED ON OR NEAR THE PROJECT, AN HDPE MEMBRANE LINER IS RECOMMENDED WITH THE SYSTEM. THE IMPERMEABLE LINER IS INTENDED TO HELP PROTECT THE SYSTEM FROM THE POTENTIAL ADVERSE EFFECTS THAT MAY RESULT FROM A CHANGE IN THE SURROUNDING ENVIRONMENT OVER A PERIOD OF TIME. PLEASE REFER TO THE CORRUGATED METAL PIPE DETENTION DESIGN GUIDE FOR ADDITIONAL INFORMATION.

> PROJECT No. SEQ. No.: 4927 5/20/2021 7911 DESIGNED: DYO DYO CHECKED: DYO DYO SHEET NO.: D2

DETENTION SYSTEM

CONSTRUCTION LOADS

FOR TEMPORARY CONSTRUCTION VEHICLE LOADS, AN EXTRA AMOUNT OF COMPACTED COVER MAY BE REQUIRED OVER THE TOP OF THE PIPE. THE HEIGHT-OF-COVER SHALL MEET THE MINIMUM REQUIREMENTS SHOWN IN THE TABLE BELOW. THE USE OF HEAVY CONSTRUCTION EQUIPMENT NECESSITATES GREATER PROTECTION FOR THE PIPE THAN FINISHED GRADE COVER MINIMUMS FOR NORMAL HIGHWAY TRAFFIC.

PIPE SPAN, INCHES	AXLE LOADS (kips)					
INCHES	18-50	50-75	75-110	110-150		
	MINIMUM COVER (FT)					
12-42	2.0	2.5	3.0	3.0		
48-72	3.0	3.0	3.5	4.0		
78-120	3.0	3.5	4.0	4.0		
126-144	3.5	4.0	4.5	4.5		

*MINIMUM COVER MAY VARY, DEPENDING ON LOCAL CONDITIONS. THE CONTRACTOR MUST PROVIDE THE ADDITIONAL COVER REQUIRED TO AVOID DAMAGE TO THE PIPE. MINIMUM COVER IS MEASURED FROM THE TOP OF THE PIPE TO THE TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE.

CONSTRUCTION LOADING DIAGRAM

SCALE: N.T.S.

SPECIFICATION FOR DESIGNED DETENTION SYSTEM:

THIS SPECIFICATION COVERS THE MANUFACTURE AND INSTALLATION OF THE DESIGNED DETENTION SYSTEM DETAILED IN THE PROJECT PLANS.

THE MATERIAL SHALL CONFORM TO THE APPLICABLE REQUIREMENTS LISTED BELOW

ALUMINIZED TYPE 2 STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-274 OR ASTM A-92.

THE GALVANIZED STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-218 OR ASTM A-929

THE POLYMER COATED STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-246 OR ASTM A-742.

THE ALUMINUM COILS SHALL CONFORM TO THE APPLICABLE OF AASHTO M-197 OR ASTM B-744.

CONSTRUCTION LOADS

MODIFICATIONS

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CONSTRUCTION LOADS MAY BE HIGHER THAN FINAL LOADS. FOLLOW THE MANUFACTURER'S OR NCSPA GUIDELINES.

DATE

THE PIPE SHALL BE MANUFACTURED IN ACCORDANCE TO THE APPLICABLE REQUIREMENTS LISTED BELOW:

ALUMINIZED TYPE 2: AASHTO M-36 OR ASTM A-760

GALVANIZED: AASHTO M-36 OR ASTM A-760

APPOLYMETE COATED: AASHTO M-245 OR ASTM A-762

ALUMINUM: AASHTO M-196 OR ASTM B-745 APPLICABLE

HANDLING AND ASSEMBLY

SHALL BE IN ACCORDANCE WITH NCSP'S (NATIONAL CORRUGATED STEEL APPRECABSECIATION) FOR ALUMINIZED TYPE 2. GALVANIZED OR POLYMER COATED STEEL. SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR ALUMINUM PIPE.

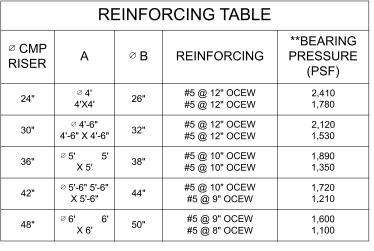
REQUIREMENTS

SHALL BE IN ACCORDANCE WITH AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SECTION 26, DIVISION II DIVISION II OR ASTM A-798 (FOR ALUMINIZED TYPE 2, GALVANIZED OR POLYMER COATED STEEL) OR ASTM B-788 (FOR ALUMINUM PIPE) AND IN CONFORMANCE WITH THE PROJECT PLANS AND SPECIFICATIONS. IF THERE ARE ANY INCONSISTENCIES OR CONFLICTS THE CONTRACTOR SHOULD DISCUSS AND RESOLVE WITH THE SITE ENGINEER.

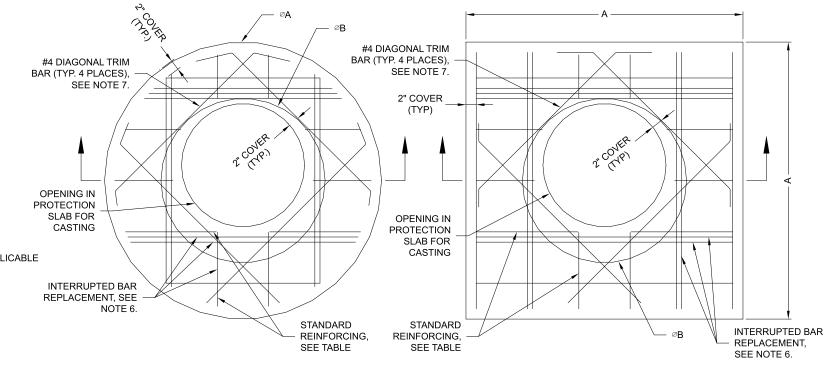
IT IS ALWAYS THE RESPONSIBILITY OF THE CONTRACTOR TO FOLLOW OSHA GUIDELINES FOR SAFE PRACTICES.

ACCESS CASTING TO BE PROVIDED AND INSTALLED BY CONTRACTOR. ∅ CMP RISER GASKET MATERIAL SUFFICIENT TO PREVENT SLAB FROM BEARING ON RISER TO BE PROVIDED BY CONTRACTOR.

SECTION VIEW



** ASSUMED SOIL BEARING CAPACITY



ROUND OPTION PLAN VIEW

NOTES:

- 1. DESIGN IN ACCORDANCE WITH AASHTO, 17th EDITION.
- 2. DESIGN LOAD HS25.
- 3. EARTH COVER = 1' MAX.
- 4. CONCRETE STRENGTH = 3,500 psi
- 5. REINFORCING STEEL = ASTM A615. GRADE 60.
- 6. PROVIDE ADDITIONAL REINFORCING AROUND OPENINGS EQUAL TO THE BARS INTERRUPTED, HALF EACH SIDE. ADDITIONAL BARS TO BE IN THE SAME PLANE.

SQUARE OPTION PLAN VIEW

- 7. TRIM OPENING WITH DIAGONAL #4 BARS, EXTEND BARS A MINIMUM OF 12" BEYOND OPENING, BEND BARS AS REQUIRED TO MAINTAIN BAR COVER.
- 8. PROTECTION SLAB AND ALL MATERIALS TO BE PROVIDED AND INSTALLED BY CONTRACTOR.
- 9. DETAIL DESIGN BY DELTA ENGINEERING, BINGHAMTON, NY.

MANHOLE CAP DETAIL

SCALE: N.T.S.

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REVISION DESCRIPTION

BY

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DRAWING

DYO7911 Rockwall CAD Rockwall, TX DETENTION SYSTEM (2)



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	SHEET NO.:	D3					

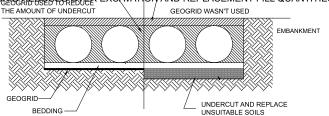
CMP DETENTION INSTALLATION GUIDE

PROPER INSTALLATION OF A FLEXIBLE UNDERGROUND DETENTION SYSTEM WILL ENSURE LONG-TERM PERFORMANCE. THE CONFIGURATION OF THESE SYSTEMS OFTEN REQUIRES SPECIAL CONSTRUCTION PRACTICES THAT DIFFER FROM CONVENTIONAL FLEXIBLE PIPE CONSTRUCTION. CONTECH ENGINEERED SOLUTIONS STRONGLY SUGGESTS SCHEDULING A PRE-CONSTRUCTION MEETING WITH YOUR LOCAL SALES ENGINEER TO DETERMINE IF ADDITIONAL MEASURES, NOT COVERED IN THIS GUIDE, ARE APPROPRIATE FOR YOUR SITE.

FOUNDATION

CONSTRUCT A FOUNDATION THAT CAN SUPPORT THE DESIGN LOADING APPLIED BY THE PIPE AND ADJACENT BACKFILL WEIGHT AS WELL AS MAINTAIN ITS INTEGRITY DURING CONSTRUCTION.

IF SOFT OR UNSUITABLE SOILS ARE ENCOUNTERED, REMOVE THE POOR SOILS DOWN TO A SUITABLE DEPTH AND THEN BUILD UP TO THE APPROPRIATE ELEVATION WITH A COMPETENT BACKFILL MATERIAL. THE STRUCTURAL FILL MATERIAL GRADATION SHOULD NOT ALLOW THE MIGRATION OF FINES, WHICH CAN CAUSE SETTLEMENT OF THE DETENTION SYSTEM OR PAVEMENT ABOVE. IF THE STRUCTURAL FILL MATERIAL IS NOT COMPATIBLE WITH THE UNDERLYING SOILS AN ENGINEERING FABRIC SHOULD BE USED AS A SEPARATION. IN SOME CASES, USING A STIFF REINFORCING GEOGRID REDUCTES BY ER EXCAVATION AND PEPLACEMENT FILL QUANTITIES.

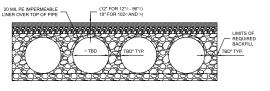


GRADE THE FOUNDATION SUBGRADE TO A UNIFORM OR SLIGHTLY SLOPING GRADE. IF THE SUBGRADE IS CLAY OR RELATIVELY NON-POROUS AND THE CONSTRUCTION SEQUENCE WILL LAST FOR AN EXTENDED PERIOD OF TIME, IT IS BEST TO SLOPE THE GRADE TO ONE END OF THE SYSTEM. THIS WILL ALLOW EXCESS WATER TO DRAIN QUICKLY, PREVENTING SATURATION OF THE SUBGRADE.

GEOMEMBRANE BARRIER

A SITE'S RESISTIVITY MAY CHANGE OVER TIME WHEN VARIOUS TYPES OF SALTING AGENTS ARE USED, SUCH AS ROAD SALTS FOR DEICING AGENTS. IF SALTING AGENTS ARE USED ON OR NEAR THE PROJECT SITE, A GEOMEMBRANE BARRIER IS RECOMMENDED WITH THE SYSTEM. THE GEOMEMBRANE LINER IS INTENDED TO HELP PROTECT THE SYSTEM FROM THE POTENTIAL ADVERSE EFFECTS THAT MAY RESULT FROM THE USE OF SUCH AGENTS INCLUDING PREMATURE CORROSION AND REDUCED ACTUAL

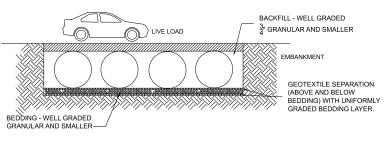
等用と学Rも地でで、SENGINEER OF RECORD IS TO EVALUATE WHETHER SALTING AGENTS WILL BE USED ON OR NEAR THE PROJECT SITE, AND USE HIS/HER BEST JUDGEMENT TO DETERMINE IF ANY ADDITIONAL PROTECTIVE MEASURES ARE REQUIRED. BELOW IS A TYPICAL DETAIL SHOWING THE PLACEMENT OF A GEOMEMBRANE BARRIER FOR PROJECTS WHERE SALTING AGENTS ARE USED ON OR NEAR THE PROJECT SITE.



IN-SITU TRENCH WALL

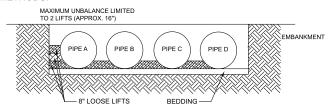
IF EXCAVATION IS REQUIRED, THE TRENCH WALL NEEDS TO BE CAPABLE OF SUPPORTING THE LOAD THAT THE PIPE SHEDS AS THE SYSTEM IS LOADED. IF SOILS ARE NOT CAPABLE OF SUPPORTING THESE LOADS, THE PIPE CAN DEFLECT PERFORM A SIMPLE SOIL PRESSURE CHECK USING THE APPLIED LOADS TO DETERMINE THE LIMITS OF EXCAVATION BEYOND THE SPRING LINE OF THE OUTER MOST PIPES.

IN MOST CASES THE REQUIREMENTS FOR A SAFE WORK ENVIRONMENT AND PROPER BACKFILL PLACEMENT AND COMPACTION TAKE CARE OF THIS CONCERN.



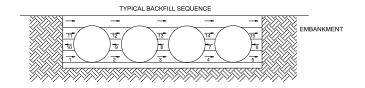
BACKFILL PLACEMENT

MATERIAL SHALL BE WORKED INTO THE PIPE HAUNCHES BY MEANS OF SHOVEL-SLICING, RODDING, AIR TAMPER, VIBRATORY ROD, OR OTHER EFFECTIVE METHODS.

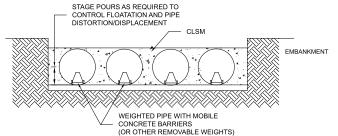


IF AASHTO T99 PROCEDURES ARE DETERMINED INFEASIBLE BY THE GEOTECHNICAL ENGINEER OF RECORD, COMPACTION IS CONSIDERED ADEQUATE WHEN NO FURTHER YIELDING OF THE MATERIAL IS OBSERVED UNDER THE COMPACTOR, OR UNDER FOOT, AND THE GEOTECHNICAL ENGINEER OF RECORD (OR REPRESENTATIVE THEREOF) IS SATISFIED WITH THE LEVEL OF COMPACTION.

FOR LARGE SYSTEMS, CONVEYOR SYSTEMS, BACKHOES WITH LONG REACHES OR DRAGLINES WITH STONE BUCKETS MAY BE USED TO PLACE BACKFILL. ONCE MINIMUM COVER FOR CONSTRUCTION LOADING ACROSS THE ENTIRE WIDTH OF THE SYSTEM IS REACHED, ADVANCE THE EQUIPMENT TO THE END OF THE RECENTLY PLACED FILL, AND BEGIN THE SEQUENCE AGAIN UNTIL THE SYSTEM IS COMPLETELY BACKFILLED. THIS TYPE OF CONSTRUCTION SEQUENCE PROVIDES ROOM FOR STOCKPILED BACKFILL DIRECTLY BEHIND THE BACKHOE, AS WELL AS THE MOVEMENT OF CONSTRUCTION TRAFFIC. MATERIAL STOCKPILES ON TOP OF THE BACKFILLED DETENTION SYSTEM SHOULD BE LIMITED TO 8- TO 10-FEET HIGH AND MUST PROVIDE BALANCED LOADING ACROSS ALL BARRELS. TO DETERMINE THE PROPER COVER OVER THE PIPES TO ALLOW THE MOVEMENT OF CONSTRUCTION EQUIPMENT SEE TABLE 1, OR CONTACT YOUR LOCAL CONTECH SALES ENGINEER.



WHEN FLOWABLE FILL IS USED, YOU MUST PREVENT PIPE FLOATATION. TYPICALLY, SMALL LIFTS ARE PLACED BETWEEN THE PIPES AND THEN ALLOWED TO SET-UP PRIOR TO THE PLACEMENT OF THE NEXT LIFT. THE ALLOWABLE THICKNESS OF THE CLSM LIFT IS A FUNCTION OF A PROPER BALANCE BETWEEN THE UPLIFT FORCE OF THE CLSM, THE OPPOSING WEIGHT OF THE PIPE, AND THE EFFECT OF OTHER RESTRAINING MEASURES. THE PIPE CAN CARRY LIMITED FLUID PRESSURE WITHOUT PIPE DISTORTION OR DISPLACEMENT, WHICH ALSO AFFECTS THE CLSM LIFT THICKNESS. YOUR LOCAL CONTECH SALES ENGINEER CAN HELP DETERMINE THE PROPER LIFT THICKNESS.

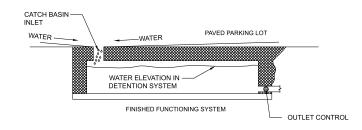


CONSTRUCTION LOADING

TYPICALLY, THE MINIMUM COVER SPECIFIED FOR A PROJECT ASSUMES H-20 LIVE LOAD. BECAUSE CONSTRUCTION LOADS OFTEN EXCEED DESIGN LIVE LOADS, INCREASED TEMPORARY MINIMUM COVER REQUIREMENTS ARE NECESSARY. SINCE CONSTRUCTION EQUIPMENT VARIES FROM JOB TO JOB, IT IS BEST TO ADDRESS EQUIPMENT SPECIFIC MINIMUM COVER REQUIREMENTS WITH YOUR LOCAL CONTECH SALES ENGINEER DURING YOUR PRE-CONSTRUCTION MEETING.

ADDITIONAL CONSIDERATIONS

BECAUSE MOST SYSTEMS ARE CONSTRUCTED BELOW-GRADE, RAINFALL CAN RAPIDLY FILL THE EXCAVATION; POTENTIALLY CAUSING FLOATATION AND MOVEMENT OF THE PREVIOUSLY PLACED PIPES. TO HELP MITIGATE POTENTIAL PROBLEMS, IT IS BEST TO START THE INSTALLATION AT THE DOWNSTREAM END WITH THE OUTLET ALREADY CONSTRUCTED TO ALLOW A ROUTE FOR THE WATER TO ESCAPE. TEMPORARY DIVERSION MEASURES MAY BE REQUIRED FOR HIGH FLOWS DUE TO THE RESTRICTED NATURE OF THE OUTLET PIPE.



CMP DETENTION SYSTEM INSPECTION AND MAINTENANCE

UNDERGROUND STORMWATER DETENTION AND INFILTRATION SYSTEMS MUST BE INSPECTED AND MAINTAINED AT REGULAR INTERVALS FOR PURPOSES OF PERFORMANCE AND LONGEVITY.

INSPECTION

INSPECTION IS THE KEY TO EFFECTIVE MAINTENANCE OF CMP DETENTION SYSTEMS AND IS EASILY PERFORMED. CONTECH RECOMMENDS ONGOING, ANNUAL INSPECTIONS. SITES WITH HIGH TRASH LOAD OR SMALL OUTLET CONTROL ORIFICES MAY NEED MORE FREQUENT INSPECTIONS. THE RATE AT WHICH THE SYSTEM COLLECTS POLLUTANTS WILL DEPEND MORE ON SITE SPECIFIC ACTIVITIES RATHER THAN THE SIZE OR CONFIGURATION OF THE SYSTEM

INSPECTIONS SHOULD BE PERFORMED MORE OFTEN IN EQUIPMENT WASHDOWN AREAS, IN CLIMATES WHERE SANDING AND/OR SALTING OPERATIONS TAKE PLACE, AND IN OTHER VARIOUS INSTANCES IN WHICH ONE WOULD EXPECT HIGHER ACCUMULATIONS OF SEDIMENT OR ABRASIVE/CORROSIVE CONDITIONS. A RECORD OF EACH INSPECTION IS TO BE MAINTAINED FOR THE LIFE OF THE SYSTEM

MAINTENANCE

CMP DETENTION SYSTEMS SHOULD BE CLEANED WHEN AN INSPECTION REVEALS ACCUMULATED SEDIMENT OR TRASH IS CLOGGING THE DISCHARGE ORIFICE

ACCUMULATED SEDIMENT AND TRASH CAN TYPICALLY BE EVACUATED THROUGH THE MANHOLE OVER THE OUTLET ORIFICE. IF MAINTENANCE IS NOT PERFORMED AS RECOMMENDED, SEDIMENT AND TRASH MAY ACCUMULATE IN FRONT OF THE OUTLET ORIFICE. MANHOLE COVERS SHOULD BE SECURELY SEATED FOLLOWING CLEANING ACTIVITIES. CONTECH SUGGESTS THAT ALL SYSTEMS BE DESIGNED WITH AN ACCESS/INSPECTION MANHOLE SITUATED AT OR NEAR THE INLET AND THE OUTLET ORIFICE. SHOULD IT BE NECESSARY TO GET INSIDE THE SYSTEM TO PERFORM MAINTENANCE ACTIVITIES, ALL APPROPRIATE PRECAUTIONS REGARDING CONFINED SPACE ENTRY AND OSHA REGULATIONS SHOULD BE FOLLOWED.

ANNUAL INSPECTIONS ARE BEST PRACTICE FOR ALL UNDERGROUND SYSTEMS. DURING THIS INSPECTION, IF EVIDENCE OF SALTING/DE-ICING AGENTS IS OBSERVED WITHIN THE SYSTEM, IT IS BEST PRACTICE FOR THE SYSTEM TO BE RINSED, INCLUDING ABOVE THE SPRING LINE SOON AFTER THE SPRING THAW AS PART OF THE MAINTENANCE PROGRAM FOR THE SYSTEM.

MAINTAINING AN UNDERGROUND DETENTION OR INFILTRATION SYSTEM IS EASIEST WHEN THERE IS NO FLOW ENTERING THE SYSTEM. FOR THIS REASON, IT IS A GOOD IDEA TO SCHEDULE THE CLEANOUT DURING DRY WEATHER

THE FOREGOING INSPECTION AND MAINTENANCE EFFORTS HELP ENSURE UNDERGROUND PIPE SYSTEMS USED FOR STORMWATER STORAGE CONTINUE TO FUNCTION AS INTENDED BY IDENTIFYING RECOMMENDED REGULAR INSPECTION AND MAINTENANCE PRACTICES. INSPECTION AND MAINTENANCE RELATED TO THE STRUCTURAL INTEGRITY OF THE PIPE OR THE SOUNDNESS OF PIPE JOINT CONNECTIONS IS BEYOND THE SCOPE OF THIS GUIDE.



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DRAWING

DYO7911 Rockwall CAD A Rockwall, TX DETENTION SYSTEM

PROJECT No.:	SEQ. I	No.:	DATE:
4927	79	11	5/20/2021
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PROJECT SUMMARY

CALCULATION DETAILS

- LOADING = HS20 & HS25
- APPROX. LINEAR FOOTAGE = 59 lf.

STORAGE SUMMARY

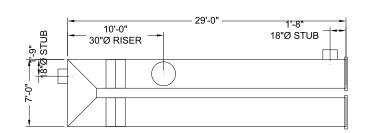
- STORAGE VOLUME REQUIRED = 404 cf.
- PIPE STORAGE VOLUME = 417 cf.
- BACKFILL STORAGE VOLUME = 0 cf.
- TOTAL STORAGE PROVIDED = 442 cf.

PIPE DETAILS

- DIAMETER = 36 IN.
- CORRUGATION = 2 2/3x1/2
- GAGE = 16
- COATING = ALT2
- WALL TYPE = Solid
- BARRELL SPACING = 12 IN.

BACKFILL DETAILS

- WIDTH AT ENDS = 0 IN.
- ABOVE PIPE = 0 IN.
- WIDTH AT SIDES = 0 IN.
- BELOW PIPE = 0 IN.



ASSEMBLY

SCALE: 1" = 10'

NOTES

- ALL RISER AND STUB DIMENSIONS ARE TO CENTERLINE. ALL ELEVATIONS, DIMENSIONS, AND LOCATIONS OF RISERS AND INLETS, SHALL BE VERIFIED BY THE ENGINEER OF RECORD PRIOR TO RELEASING FOR FABRICATION.
- ALL FITTINGS AND REINFORCEMENT COMPLY WITH ASTM A998.
- ALL RISERS AND STUBS ARE $2\frac{2}{3}$ " x $\frac{1}{2}$ " CORRUGATION AND 16 GAGE UNLESS OTHERWISE NOTED.
- RISERS TO BE FIELD TRIMMED TO GRADE.
- QUANTITY OF PIPE SHOWN DOES NOT PROVIDE EXTRA PIPE FOR CONNECTING THE SYSTEM TO EXISTING PIPE OR DRAINAGE STRUCTURES. OUR SYSTEM AS DETAILED PROVIDES NOMINAL INLET AND/OR OUTLET PIPE STUB FOR CONNECTION TO EXISTING DRAINAGE FACILITIES. IF ADDITIONAL PIPE IS NEEDED IT IS THE RESPONSIBILITY OF THE CONTRACTOR.
- BAND TYPE TO BE DETERMINED UPON FINAL DESIGN.
- THE PROJECT SUMMARY IS REFLECTIVE OF THE DYODS DESIGN, QUANTITIES ARE APPROX. AND SHOULD BE VERIFIED UPON FINAL DESIGN AND APPROVAL. FOR EXAMPLE, TOTAL EXCAVATION DOES NOT CONSIDER ALL VARIABLES SUCH AS SHORING AND ONLY ACCOUNTS FOR MATERIAL WITHIN THE ESTIMATED EXCAVATION FOOTPRINT.
- THESE DRAWINGS ARE FOR CONCEPTUAL PURPOSES AND DO NOT REFLECT ANY LOCAL PREFERENCES OR REGULATIONS. PLEASE CONTACT YOUR LOCAL CONTECH REP FOR



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CANTECH® CMP DETENTION SYSTEMS CONTECH

DYODS

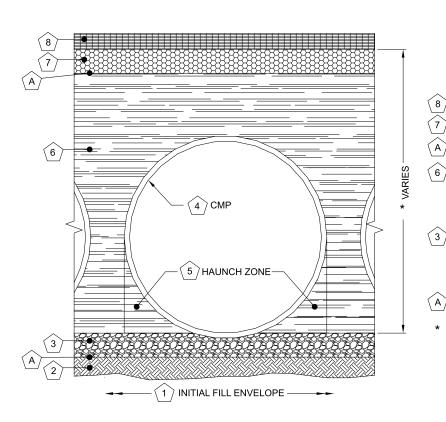
DRAWING

DYO7912 Rockwall CAD Rockwall, TX **DETENTION SYSTEM**

109800 7/29/2021

PROJECT No.:	SEQ. No.:		DATE:	
4927	7912		5/20/2021	
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	DETENTION SYSTEM	MS - CMP DETENTIC	ON / CMP DRAINAGE
Material Location	Description	Material Designation	Designation
Rigid or Flexible Pa (if applicable)	vement		
Road Base (if applied	cable)		
Geotextile Layer	Non-Woven Geotextile	CONTECH C-40 or C-45	Engineer Decision for consideration to prevent soil migration into varying soil types
Backfill	Well graded granular material which may contain small amounts of silt or clay.	AASHTO M 145- A-1, A-2, A-3	Placed in 8" +/- loose lifts and compacted to 90% Standard Proctor Per AASHTO T 99
Bedding Stone	Well graded granular bedding material w/maximum particle size of 3"	AASHTO M43 - 3,357,4,467, 5, 56, 57	Engineer to determine if bedding is required. Pipe may be placed on the trench bottom of a relatively loose, native suitable well graded & granular material. For Arch pipes it is recommended to be shaped to a relatively flat bottom or fine-grade the foundation to a slight v-shape. Unsuitable material should be over-excavated and re-placed with a 4"-6" layer of well graded & granular stone per the material designation. See AASHTO 26.3.8.1 / 26.5.3 Bedding info.
Geotextile Layer	Non-Woven Geotextile	CONTECH C-40 or C-45	Engineer Decision for consideration to prevent soil migration into varying soil types

Note: Backfill using controlled low-strength material (CLSM, "flash fill" or "flowable fill") when the spacing between the pipes will

not allow for placement and adequate compaction of the backfill.

MINIMUM WIDTH DEPENDS ON SITE CONDITIONS AND ENGINEERING JUDGEMENT

FOUNDATION/BEDDING PREPARATION

- PRIOR TO PLACING THE BEDDING, THE FOUNDATION MUST BE CONSTRUCTED TO A UNIFORM AND STABLE GRADE. IN THE EVENT THAT UNSUITABLE FOUNDATION MATERIALS ARE ENCOUNTERED DURING EXCAVATION, THEY SHALL BE REMOVED AND BROUGHT BACK TO THE GRADE WITH A FILL MATERIAL AS APPROVED BY THE ENGINEER.
- HAUNCH ZONE MATERIAL SHALL BE PLACED AND UNIFORMALLY COMPACTED WITHOUT

BACKFILL

WHEN PLACING THE FIRST LIFTS OF BACKFILL IT IS IMPORTANT TO MAKE SURE THAT THE BACKFILL IS PROPERLY COMPACTED UNDER AND AROUND THE PIPE HAUNCHES. BACKFILL SHALL BE PLACED SUCH THAT THERE IS NO MORE THAN A TWO LIFT (16") DIFFERENTIAL BETWEEN ANY OF THE PIPES AT ANY TIME DURING THE BACKFILL PROCESS. THE BACKFILL SHALL BE ADVANCED ALONG THE LENGTH OF THE DETENTION SYSTEM AT THE SAME RATE TO AVOID DIFFERENTIAL LOADING ON THE PIPE.

OTHER ALTERNATE BACKFILL MATERIAL MAY BE ALLOWED DEPENDING ON SITE SPECIFIC CONDITIONS, AS APPROVED BY SITE ENGINEER.



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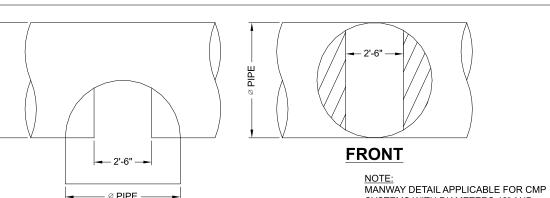
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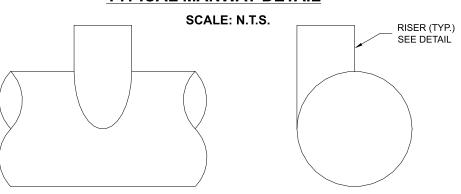
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DRAWING



PLAN

TYPICAL MANWAY DETAIL



ELEVATION

TYPICAL RISER DETAIL

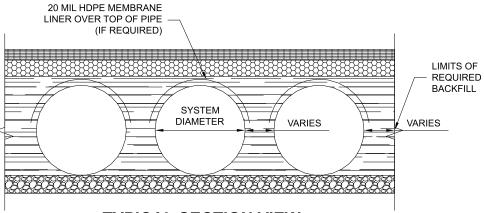
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END

LADDERS ARE OPTIONAL AND ARE NOT REQUIRED FOR ALL SYSTEMS.

SYSTEMS WITH DIAMETERS 48" AND LARGER. MANWAYS MAY BE REQUIRED

ON SMALLER SYSTEMS DEPENDING ON ACTUAL SITE SPECIFIC CONDITIONS.



TYPICAL SECTION VIEW

LINER OVER ROWS SCALE: N.T.S.

NOTE: IF SALTING AGENTS FOR SNOW AND ICE REMOVAL ARE USED ON OR NEAR THE PROJECT, AN HDPE MEMBRANE LINER IS RECOMMENDED WITH THE SYSTEM. THE IMPERMEABLE LINER IS INTENDED TO HELP PROTECT THE SYSTEM FROM THE POTENTIAL ADVERSE EFFECTS THAT MAY RESULT FROM A CHANGE IN THE SURROUNDING ENVIRONMENT OVER A PERIOD OF TIME. PLEASE REFER TO THE CORRUGATED METAL PIPE DETENTION DESIGN GUIDE FOR ADDITIONAL INFORMATION.

DYO7912 Rockwall CAD Rockwall, TX **DETENTION SYSTEM**

PROJECT No.:	SEQ. I	No.:	DATE:
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CONSTRUCTION LOADS

FOR TEMPORARY CONSTRUCTION VEHICLE LOADS, AN EXTRA AMOUNT OF COMPACTED COVER MAY BE REQUIRED OVER THE TOP OF THE PIPE. THE HEIGHT-OF-COVER SHALL MEET THE MINIMUM REQUIREMENTS SHOWN IN THE TABLE BELOW. THE USE OF HEAVY CONSTRUCTION EQUIPMENT NECESSITATES GREATER PROTECTION FOR THE PIPE THAN FINISHED GRADE COVER MINIMUMS FOR NORMAL HIGHWAY TRAFFIC.

PIPE SPAN, INCHES	A	XLE LO	ADS (kips	s)
INCLIES	18-50	50-75	75-110	110-150
	MI	NIMUM C	OVER (F	- T)
12-42	2.0	2.5	3.0	3.0
48-72	3.0	3.0	3.5	4.0
78-120	3.0	3.5	4.0	4.0
126-144	3.5	4.0	4.5	4.5

*MINIMUM COVER MAY VARY, DEPENDING ON LOCAL CONDITIONS. THE CONTRACTOR MUST PROVIDE THE ADDITIONAL COVER REQUIRED TO AVOID DAMAGE TO THE PIPE. MINIMUM COVER IS MEASURED FROM THE TOP OF THE PIPE TO THE TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE.

CONSTRUCTION LOADING DIAGRAM

SCALE: N.T.S.

SPECIFICATION FOR DESIGNED DETENTION SYSTEM:

SCOPE

THIS SPECIFICATION COVERS THE MANUFACTURE AND INSTALLATION OF THE DESIGNED DETENTION SYSTEM DETAILED IN THE PROJECT PLANS.

MATERIA

THE MATERIAL SHALL CONFORM TO THE APPLICABLE REQUIREMENTS LISTED BELOW:

ALUMINIZED TYPE 2 STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-274 OR ASTM A-92.

THE GALVANIZED STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-218 OR ASTM A-929.

THE POLYMER COATED STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-246 OR ASTM A-742.

THE ALUMINUM COILS SHALL CONFORM TO THE APPLICABLE OF AASHTO M-197 OR ASTM B-744.

CONSTRUCTION LOADS

THESE DRAWINGS ARE FOR CONCEPTUAL PURPOSES AND DO NOT REFLECT ANY LOCAL

PREFERENCES OR REGULATIONS. PLEASE

CONSTRUCTION LOADS MAY BE HIGHER THAN FINAL LOADS. FOLLOW THE MANUFACTURER'S OR NCSPA GUIDELINES.

DIDE

THE PIPE SHALL BE MANUFACTURED IN ACCORDANCE TO THE APPLICABLE REQUIREMENTS LISTED BELOW:

ALUMINIZED TYPE 2: AASHTO M-36 OR ASTM A-760

GALVANIZED: AASHTO M-36 OR ASTM A-760

AFPOLYMBLE COATED: AASHTO M-245 OR ASTM A-762

ALUMINUM: AASHTO M-196 OR ASTM B-745 APPLICABLE

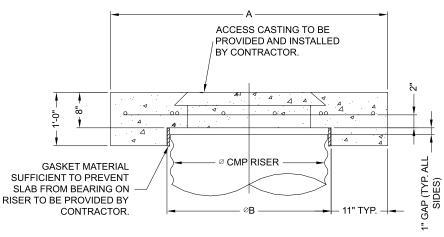
HANDLING AND ASSEMBLY

SHALL BE IN ACCORDANCE WITH NCSP'S (NATIONAL CORRUGATED STEEL APPRECABSDCIATION) FOR ALUMINIZED TYPE 2, GALVANIZED OR POLYMER COATED STEEL. SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR ALUMINUM PIPE.

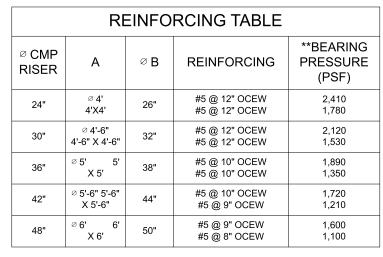
REQUIREMENTS INSTALLATION

SHALL BE IN ACCORDANCE WITH AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SECTION 26, DIVISION II DIVISION II OR ASTM A-798 (FOR ALUMINIZED TYPE 2, GALVANIZED OR POLYMER COATED STEEL) OR ASTM B-788 (FOR ALUMINUM PIPE) AND IN CONFORMANCE WITH THE PROJECT PLANS AND SPECIFICATIONS. IF THERE ARE ANY INCONSISTENCIES OR CONFLICTS THE CONTRACTOR SHOULD DISCUSS AND RESOLVE WITH THE SITE FINGINFER

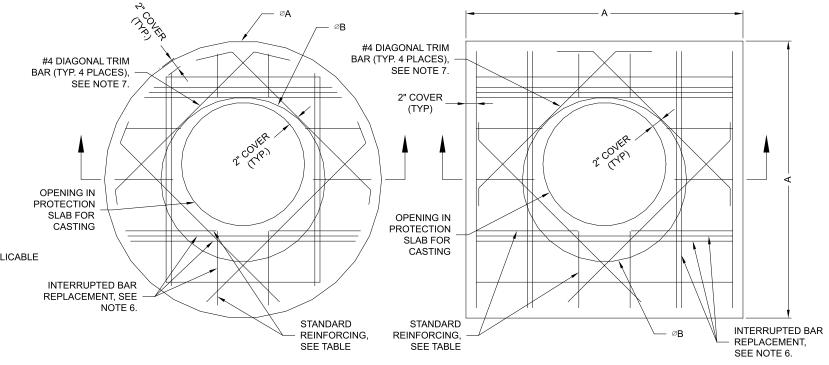
IT IS ALWAYS THE RESPONSIBILITY OF THE CONTRACTOR TO FOLLOW OSHA GUIDELINES FOR SAFE PRACTICES.



SECTION VIEW



** ASSUMED SOIL BEARING CAPACITY



ROUND OPTION PLAN VIEW

NOTES:

- 1. DESIGN IN ACCORDANCE WITH AASHTO, 17th EDITION.
- 2. DESIGN LOAD HS25.

THE SAME PLANE.

- 3. EARTH COVER = 1' MAX.
- 4. CONCRETE STRENGTH = 3,500 psi
- PROVIDE ADDITIONAL REINFORCING AROUND OPENINGS EQUAL TO THE BARS INTERRUPTED, HALF EACH SIDE. ADDITIONAL BARS TO BE IN

5. REINFORCING STEEL = ASTM A615. GRADE 60.

SQUARE OPTION PLAN VIEW

- 7. TRIM OPENING WITH DIAGONAL #4 BARS, EXTEND BARS A MINIMUM OF 12" BEYOND OPENING, BEND BARS AS REQUIRED TO MAINTAIN BAR COVER.
- 8. PROTECTION SLAB AND ALL MATERIALS TO BE PROVIDED AND INSTALLED BY CONTRACTOR.
- 9. DETAIL DESIGN BY DELTA ENGINEERING, BINGHAMTON, NY.

MANHOLE CAP DETAIL

SCALE: N.T.S.

CONTACT YOUR LOCAL CONTECH REP FOR MODIFICATIONS.

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CMP DETENTION SYSTEMS

DYODS

DRAWING

B
Rockwall, TX
DETENTION SYSTEM

DYO7912 Rockwall CAD



	PROJECT No.:	SEQ. I	No.:	DATE:
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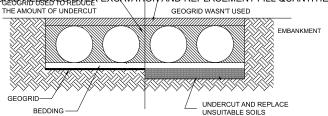
CMP DETENTION INSTALLATION GUIDE

PROPER INSTALLATION OF A FLEXIBLE UNDERGROUND DETENTION SYSTEM WILL ENSURE LONG-TERM PERFORMANCE. THE CONFIGURATION OF THESE SYSTEMS OFTEN REQUIRES SPECIAL CONSTRUCTION PRACTICES THAT DIFFER FROM CONVENTIONAL FLEXIBLE PIPE CONSTRUCTION. CONTECH ENGINEERED SOLUTIONS STRONGLY SUGGESTS SCHEDULING A PRE-CONSTRUCTION MEETING WITH YOUR LOCAL SALES ENGINEER TO DETERMINE IF ADDITIONAL MEASURES, NOT COVERED IN THIS GUIDE, ARE APPROPRIATE FOR YOUR SITE.

FOUNDATION

CONSTRUCT A FOUNDATION THAT CAN SUPPORT THE DESIGN LOADING APPLIED BY THE PIPE AND ADJACENT BACKFILL WEIGHT AS WELL AS MAINTAIN ITS INTEGRITY DURING CONSTRUCTION.

IF SOFT OR UNSUITABLE SOILS ARE ENCOUNTERED, REMOVE THE POOR SOILS DOWN TO A SUITABLE DEPTH AND THEN BUILD UP TO THE APPROPRIATE ELEVATION WITH A COMPETENT BACKFILL MATERIAL. THE STRUCTURAL FILL MATERIAL GRADATION SHOULD NOT ALLOW THE MIGRATION OF FINES, WHICH CAN CAUSE SETTLEMENT OF THE DETENTION SYSTEM OR PAVEMENT ABOVE. IF THE STRUCTURAL FILL MATERIAL IS NOT COMPATIBLE WITH THE UNDERLYING SOILS AN ENGINEERING FABRIC SHOULD BE USED AS A SEPARATION. IN SOME CASES, USING A STIFF REINFORCING GEOGRID REDUCTOR OVER EXCAVATION AND PEPLACEMENT FILL QUANTITIES.

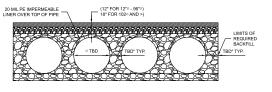


GRADE THE FOUNDATION SUBGRADE TO A UNIFORM OR SLIGHTLY SLOPING GRADE. IF THE SUBGRADE IS CLAY OR RELATIVELY NON-POROUS AND THE CONSTRUCTION SEQUENCE WILL LAST FOR AN EXTENDED PERIOD OF TIME, IT IS BEST TO SLOPE THE GRADE TO ONE END OF THE SYSTEM. THIS WILL ALLOW EXCESS WATER TO DRAIN QUICKLY, PREVENTING SATURATION OF THE SUBGRADE.

GEOMEMBRANE BARRIER

A SITE'S RESISTIVITY MAY CHANGE OVER TIME WHEN VARIOUS TYPES OF SALTING AGENTS ARE USED, SUCH AS ROAD SALTS FOR DEICING AGENTS. IF SALTING AGENTS ARE USED ON OR NEAR THE PROJECT SITE, A GEOMEMBRANE BARRIER IS RECOMMENDED WITH THE SYSTEM. THE GEOMEMBRANE LINER IS INTENDED TO HELP PROTECT THE SYSTEM FROM THE POTENTIAL ADVERSE EFFECTS THAT MAY RESULT FROM THE USE OF SUCH AGENTS INCLUDING PREMATURE CORROSION AND REDUCED ACTUAL

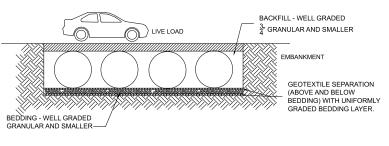
新尼学尼步氏: S ENGINEER OF RECORD IS TO EVALUATE WHETHER SALTING AGENTS WILL BE USED ON OR NEAR THE PROJECT SITE, AND USE HIS/HER BEST JUDGEMENT TO DETERMINE IF ANY ADDITIONAL PROTECTIVE MEASURES ARE REQUIRED. BELOW IS A TYPICAL DETAIL SHOWING THE PLACEMENT OF A GEOMEMBRANE BARRIER FOR PROJECTS WHERE SALTING AGENTS ARE USED ON OR NEAR THE PROJECT SITE.



IN-SITU TRENCH WALL

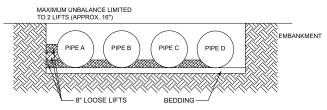
IF EXCAVATION IS REQUIRED, THE TRENCH WALL NEEDS TO BE CAPABLE OF SUPPORTING THE LOAD THAT THE PIPE SHEDS AS THE SYSTEM IS LOADED. IF SOILS ARE NOT CAPABLE OF SUPPORTING THESE LOADS, THE PIPE CAN DEFLECT PERFORM A SIMPLE SOIL PRESSURE CHECK USING THE APPLIED LOADS TO DETERMINE THE LIMITS OF EXCAVATION BEYOND THE SPRING LINE OF THE OUTER MOST PIPES.

IN MOST CASES THE REQUIREMENTS FOR A SAFE WORK ENVIRONMENT AND PROPER BACKFILL PLACEMENT AND COMPACTION TAKE CARE OF THIS CONCERN.



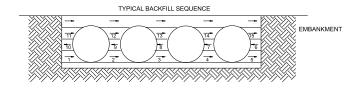
BACKFILL PLACEMENT

MATERIAL SHALL BE WORKED INTO THE PIPE HAUNCHES BY MEANS OF SHOVEL-SLICING, RODDING, AIR TAMPER, VIBRATORY ROD, OR OTHER EFFECTIVE METHODS.

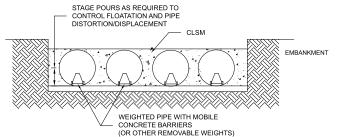


IF AASHTO T99 PROCEDURES ARE DETERMINED INFEASIBLE BY THE GEOTECHNICAL ENGINEER OF RECORD, COMPACTION IS CONSIDERED ADEQUATE WHEN NO FURTHER YIELDING OF THE MATERIAL IS OBSERVED UNDER THE COMPACTOR, OR UNDER FOOT, AND THE GEOTECHNICAL ENGINEER OF RECORD (OR REPRESENTATIVE THEREOF) IS SATISFIED WITH THE LEVEL OF COMPACTION.

FOR LARGE SYSTEMS, CONVEYOR SYSTEMS, BACKHOES WITH LONG REACHES OR DRAGLINES WITH STONE BUCKETS MAY BE USED TO PLACE BACKFILL. ONCE MINIMUM COVER FOR CONSTRUCTION LOADING ACROSS THE ENTIRE WIDTH OF THE SYSTEM IS REACHED, ADVANCE THE EQUIPMENT TO THE END OF THE RECENTLY PLACED FILL, AND BEGIN THE SEQUENCE AGAIN UNTIL THE SYSTEM IS COMPLETELY BACKFILLED. THIS TYPE OF CONSTRUCTION SEQUENCE PROVIDES ROOM FOR STOCKPILED BACKFILL DIRECTLY BEHIND THE BACKHOE, AS WELL AS THE MOVEMENT OF CONSTRUCTION TRAFFIC. MATERIAL STOCKPILES ON TOP OF THE BACKFILLED DETENTION SYSTEM SHOULD BE LIMITED TO 8- TO 10-FEET HIGH AND MUST PROVIDE BALANCED LOADING ACROSS ALL BARRELS. TO DETERMINE THE PROPER COVER OVER THE PIPES TO ALLOW THE MOVEMENT OF CONSTRUCTION EQUIPMENT SEE TABLE 1, OR CONTACT YOUR LOCAL CONTECH SALES ENGINEER.



WHEN FLOWABLE FILL IS USED, YOU MUST PREVENT PIPE FLOATATION. TYPICALLY, SMALL LIFTS ARE PLACED BETWEEN THE PIPES AND THEN ALLOWED TO SET-UP PRIOR TO THE PLACEMENT OF THE NEXT LIFT. THE ALLOWABLE THICKNESS OF THE CLSM LIFT IS A FUNCTION OF A PROPER BALANCE BETWEEN THE UPLIFT FORCE OF THE CLSM, THE OPPOSING WEIGHT OF THE PIPE, AND THE EFFECT OF OTHER RESTRAINING MEASURES. THE PIPE CAN CARRY LIMITED FLUID PRESSURE WITHOUT PIPE DISTORTION OR DISPLACEMENT, WHICH ALSO AFFECTS THE CLSM LIFT THICKNESS. YOUR LOCAL CONTECH SALES ENGINEER CAN HELP DETERMINE THE PROPER LIFT THICKNESS.

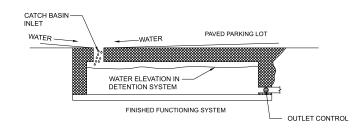


CONSTRUCTION LOADING

TYPICALLY, THE MINIMUM COVER SPECIFIED FOR A PROJECT ASSUMES H-20 LIVE LOAD. BECAUSE CONSTRUCTION LOADS OFTEN EXCEED DESIGN LIVE LOADS, INCREASED TEMPORARY MINIMUM COVER REQUIREMENTS ARE NECESSARY. SINCE CONSTRUCTION EQUIPMENT VARIES FROM JOB TO JOB, IT IS BEST TO ADDRESS EQUIPMENT SPECIFIC MINIMUM COVER REQUIREMENTS WITH YOUR LOCAL CONTECH SALES ENGINEER DURING YOUR PRE-CONSTRUCTION MEETING.

ADDITIONAL CONSIDERATIONS

BECAUSE MOST SYSTEMS ARE CONSTRUCTED BELOW-GRADE, RAINFALL CAN RAPIDLY FILL THE EXCAVATION; POTENTIALLY CAUSING FLOATATION AND MOVEMENT OF THE PREVIOUSLY PLACED PIPES. TO HELP MITIGATE POTENTIAL PROBLEMS, IT IS BEST TO START THE INSTALLATION AT THE DOWNSTREAM END WITH THE OUTLET ALREADY CONSTRUCTED TO ALLOW A ROUTE FOR THE WATER TO ESCAPE. TEMPORARY DIVERSION MEASURES MAY BE REQUIRED FOR HIGH FLOWS DUE TO THE RESTRICTED NATURE OF THE OUTLET PIPE.



CMP DETENTION SYSTEM INSPECTION AND MAINTENANCE

UNDERGROUND STORMWATER DETENTION AND INFILTRATION SYSTEMS MUST BE INSPECTED AND MAINTAINED AT REGULAR INTERVALS FOR PURPOSES OF PERFORMANCE AND LONGEVITY.

INSPECTION

INSPECTION IS THE KEY TO EFFECTIVE MAINTENANCE OF CMP DETENTION SYSTEMS AND IS EASILY PERFORMED. CONTECH RECOMMENDS ONGOING, ANNUAL INSPECTIONS. SITES WITH HIGH TRASH LOAD OR SMALL OUTLET CONTROL ORIFICES MAY NEED MORE FREQUENT INSPECTIONS. THE RATE AT WHICH THE SYSTEM COLLECTS POLLUTANTS WILL DEPEND MORE ON SITE SPECIFIC ACTIVITIES RATHER THAN THE SIZE OR CONFIGURATION OF THE SYSTEM

INSPECTIONS SHOULD BE PERFORMED MORE OFTEN IN EQUIPMENT WASHDOWN AREAS, IN CLIMATES WHERE SANDING AND/OR SALTING OPERATIONS TAKE PLACE, AND IN OTHER VARIOUS INSTANCES IN WHICH ONE WOULD EXPECT HIGHER ACCUMULATIONS OF SEDIMENT OR ABRASIVE/CORROSIVE CONDITIONS. A RECORD OF EACH INSPECTION IS TO BE MAINTAINED FOR THE LIFE OF THE SYSTEM

MAINTENANCE

CMP DETENTION SYSTEMS SHOULD BE CLEANED WHEN AN INSPECTION REVEALS ACCUMULATED SEDIMENT OR TRASH IS CLOGGING THE DISCHARGE ORIFICE

ACCUMULATED SEDIMENT AND TRASH CAN TYPICALLY BE EVACUATED THROUGH THE MANHOLE OVER THE OUTLET ORIFICE. IF MAINTENANCE IS NOT PERFORMED AS RECOMMENDED, SEDIMENT AND TRASH MAY ACCUMULATE IN FRONT OF THE OUTLET ORIFICE. MANHOLE COVERS SHOULD BE SECURELY SEATED FOLLOWING CLEANING ACTIVITIES. CONTECH SUGGESTS THAT ALL SYSTEMS BE DESIGNED WITH AN ACCESS/INSPECTION MANHOLE SITUATED AT OR NEAR THE INLET AND THE OUTLET ORIFICE. SHOULD IT BE NECESSARY TO GET INSIDE THE SYSTEM TO PERFORM MAINTENANCE ACTIVITIES, ALL APPROPRIATE PRECAUTIONS REGARDING CONFINED SPACE ENTRY AND OSHA REGULATIONS SHOULD BE FOLLOWED.

ANNUAL INSPECTIONS ARE BEST PRACTICE FOR ALL UNDERGROUND SYSTEMS. DURING THIS INSPECTION, IF EVIDENCE OF SALTING/DE-ICING AGENTS IS OBSERVED WITHIN THE SYSTEM, IT IS BEST PRACTICE FOR THE SYSTEM TO BE RINSED, INCLUDING ABOVE THE SPRING LINE SOON AFTER THE SPRING THAW AS PART OF THE MAINTENANCE PROGRAM FOR THE SYSTEM.

MAINTAINING AN UNDERGROUND DETENTION OR INFILTRATION SYSTEM IS EASIEST WHEN THERE IS NO FLOW ENTERING THE SYSTEM. FOR THIS REASON, IT IS A GOOD IDEA TO SCHEDULE THE CLEANOUT DURING DRY WEATHER

THE FOREGOING INSPECTION AND MAINTENANCE EFFORTS HELP ENSURE UNDERGROUND PIPE SYSTEMS USED FOR STORMWATER STORAGE CONTINUE TO FUNCTION AS INTENDED BY IDENTIFYING RECOMMENDED REGULAR INSPECTION AND MAINTENANCE PRACTICES. INSPECTION AND MAINTENANCE RELATED TO THE STRUCTURAL INTEGRITY OF THE PIPE OR THE SOUNDNESS OF PIPE JOINT CONNECTIONS IS BEYOND THE SCOPE OF THIS GUIDE.



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DYO7912 Rockwall CAD B Rockwall, TX DETENTION SYSTEM

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PROJECT SUMMARY

CALCULATION DETAILS

- LOADING = HS20 & HS25
- APPROX. LINEAR FOOTAGE = 318 lf.

STORAGE SUMMARY

- STORAGE VOLUME REQUIRED = 2,246 cf.
- PIPE STORAGE VOLUME = 2,248 cf.
- BACKFILL STORAGE VOLUME = 0 cf.
- TOTAL STORAGE PROVIDED = 2,285 cf.

PIPE DETAILS

- DIAMETER = 36 IN.
- CORRUGATION = 2 2/3x1/2
- GAGE = 16
- COATING = ALT2
- WALL TYPE = Solid
- BARRELL SPACING = 12 IN.

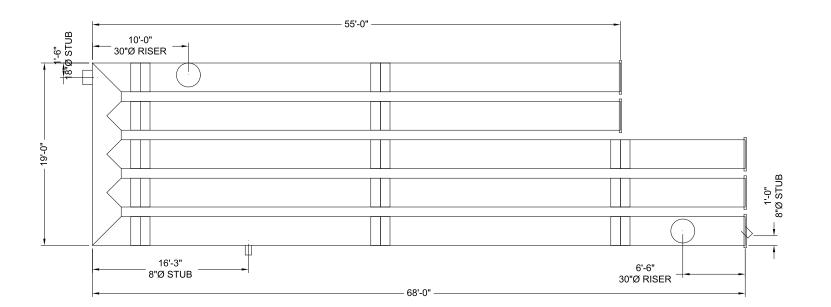
BACKFILL DETAILS

- WIDTH AT ENDS = 0 IN.
- ABOVE PIPE = 0 IN.
- WIDTH AT SIDES = 0 IN.
- BELOW PIPE = 0 IN.

<u>NOTES</u>

- ALL RISER AND STUB DIMENSIONS ARE TO CENTERLINE. ALL ELEVATIONS, DIMENSIONS, AND LOCATIONS OF RISERS AND INLETS, SHALL BE VERIFIED BY THE ENGINEER OF RECORD PRIOR TO RELEASING FOR FABRICATION.
- ALL FITTINGS AND REINFORCEMENT COMPLY WITH ASTM A998.
- ALL RISERS AND STUBS ARE $2\frac{2}{3}$ " x $\frac{1}{2}$ " CORRUGATION AND 16 GAGE UNLESS OTHERWISE NOTED.
- RISERS TO BE FIELD TRIMMED TO GRADE.
- QUANTITY OF PIPE SHOWN DOES NOT PROVIDE EXTRA PIPE FOR CONNECTING THE SYSTEM TO EXISTING PIPE OR DRAINAGE STRUCTURES. OUR SYSTEM AS DETAILED PROVIDES NOMINAL INLET AND/OR OUTLET PIPE STUB FOR CONNECTION TO EXISTING DRAINAGE FACILITIES. IF ADDITIONAL PIPE IS NEEDED IT IS THE RESPONSIBILITY OF THE CONTRACTOR.
- BAND TYPE TO BE DETERMINED UPON FINAL DESIGN.
- THE PROJECT SUMMARY IS REFLECTIVE OF THE DYODS DESIGN, QUANTITIES ARE APPROX. AND SHOULD BE VERIFIED UPON FINAL DESIGN AND APPROVAL. FOR EXAMPLE, TOTAL EXCAVATION DOES NOT CONSIDER ALL VARIABLES SUCH AS SHORING AND ONLY ACCOUNTS FOR MATERIAL WITHIN THE ESTIMATED EXCAVATION FOOTPRINT.
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ASSEMBLY SCALE: 1" = 10'





Rockwall, TX **DETENTION SYSTEM**

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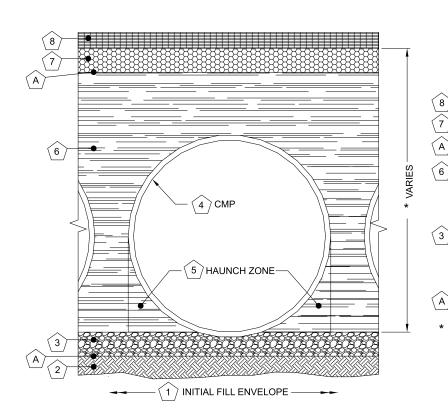
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REVISION DESCRIPTION

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DYO7913 Rockwall CAD



Material Location	Description	Material Designation	Designation
Rigid or Flexible Pa (if applicable)	vement		
Road Base (if applic	cable)		
Geotextile Layer	Non-Woven Geotextile	CONTECH C-40 or C-45	Engineer Decision for consideration to prevent s migration into varying soil types
Backfill	Well graded granular material which may contain small amounts of silt or clay.	AASHTO M 145- A-1, A-2, A-3	Placed in 8" +/- loose lifts and compacted to 909 Standard Proctor Per AASHTO T 99
Bedding Stone	Well graded granular bedding material w/maximum particle size of 3"	AASHTO M43 - 3,357,4,467, 5, 56, 57	Engineer to determine if bedding is required. Pipmay be placed on the trench bottom of a relative loose, native suitable well graded & granular material. For Arch pipes it is recommended to be shaped to a relatively flat bottom or fine-grade the foundation to a slight v-shape. Unsuitable material should be over-excavated and re-place with a 4"-6" layer of well graded & granular ston per the material designation. See AASHTO 26.3.8.1 / 26.5.3 Bedding info.
Geotextile Layer	Non-Woven Geotextile	CONTECH C-40 or C-45	Engineer Decision for consideration to prevent s migration into varying soil types

MINIMUM WIDTH DEPENDS ON SITE CONDITIONS AND ENGINEERING JUDGEMENT

FOUNDATION/BEDDING PREPARATION

- PRIOR TO PLACING THE BEDDING, THE FOUNDATION MUST BE CONSTRUCTED TO A UNIFORM AND STABLE GRADE. IN THE EVENT THAT UNSUITABLE FOUNDATION MATERIALS ARE ENCOUNTERED DURING EXCAVATION, THEY SHALL BE REMOVED AND BROUGHT BACK TO THE GRADE WITH A FILL MATERIAL AS APPROVED BY THE ENGINEER.
- HAUNCH ZONE MATERIAL SHALL BE PLACED AND UNIFORMALLY COMPACTED WITHOUT

BACKFILL

WHEN PLACING THE FIRST LIFTS OF BACKFILL IT IS IMPORTANT TO MAKE SURE THAT THE BACKFILL IS PROPERLY COMPACTED UNDER AND AROUND THE PIPE HAUNCHES. BACKFILL SHALL BE PLACED SUCH THAT THERE IS NO MORE THAN A TWO LIFT (16") DIFFERENTIAL BETWEEN ANY OF THE PIPES AT ANY TIME DURING THE BACKFILL PROCESS. THE BACKFILL SHALL BE ADVANCED ALONG THE LENGTH OF THE DETENTION SYSTEM AT THE SAME RATE TO AVOID DIFFERENTIAL LOADING ON THE PIPE.

OTHER ALTERNATE BACKFILL MATERIAL MAY BE ALLOWED DEPENDING ON SITE SPECIFIC CONDITIONS, AS APPROVED BY SITE ENGINEER.



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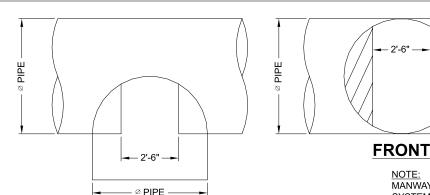
not allow for placement and adequate compaction of the backfill.

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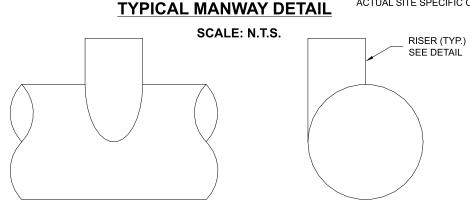
DRAWING

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PLAN

MANWAY DETAIL APPLICABLE FOR CMP SYSTEMS WITH DIAMETERS 48" AND LARGER. MANWAYS MAY BE REQUIRED ON SMALLER SYSTEMS DEPENDING ON ACTUAL SITE SPECIFIC CONDITIONS.



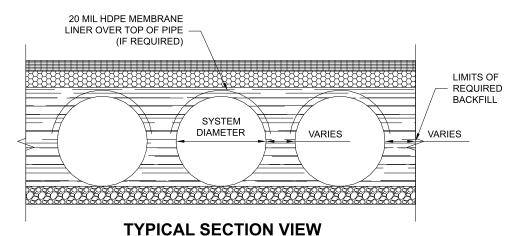
ELEVATION

TYPICAL RISER DETAIL

SCALE: N.T.S.

LADDERS ARE OPTIONAL AND ARE NOT REQUIRED FOR ALL SYSTEMS.

END



LINER OVER ROWS SCALE: N.T.S.

NOTE: IF SALTING AGENTS FOR SNOW AND ICE REMOVAL ARE USED ON OR NEAR THE PROJECT, AN HDPE MEMBRANE LINER IS RECOMMENDED WITH THE SYSTEM. THE IMPERMEABLE LINER IS INTENDED TO HELP PROTECT THE SYSTEM FROM THE POTENTIAL ADVERSE EFFECTS THAT MAY RESULT FROM A CHANGE IN THE SURROUNDING ENVIRONMENT OVER A PERIOD OF TIME. PLEASE REFER TO THE CORRUGATED METAL PIPE DETENTION DESIGN GUIDE FOR ADDITIONAL INFORMATION.

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CONSTRUCTION LOADS

FOR TEMPORARY CONSTRUCTION VEHICLE LOADS, AN EXTRA AMOUNT OF COMPACTED COVER MAY BE REQUIRED OVER THE TOP OF THE PIPE. THE HEIGHT-OF-COVER SHALL MEET THE MINIMUM REQUIREMENTS SHOWN IN THE TABLE BELOW. THE USE OF HEAVY CONSTRUCTION EQUIPMENT NECESSITATES GREATER PROTECTION FOR THE PIPE THAN FINISHED GRADE COVER MINIMUMS FOR NORMAL HIGHWAY TRAFFIC.

PIPE SPAN, INCHES	AXLE LOADS (kips)					
INCITES	18-50	50-75	75-110	110-150		
	MINIMUM COVER (FT)					
12-42	2.0	2.5	3.0	3.0		
48-72	3.0	3.0	3.5	4.0		
78-120	3.0	3.5	4.0	4.0		
126-144	3.5	4.0	4.5	4.5		

*MINIMUM COVER MAY VARY, DEPENDING ON LOCAL CONDITIONS. THE CONTRACTOR MUST PROVIDE THE ADDITIONAL COVER REQUIRED TO AVOID DAMAGE TO THE PIPE. MINIMUM COVER IS MEASURED FROM THE TOP OF THE PIPE TO THE TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE.

CONSTRUCTION LOADING DIAGRAM

SCALE: N.T.S.

SPECIFICATION FOR DESIGNED DETENTION SYSTEM:

SCOPE

THIS SPECIFICATION COVERS THE MANUFACTURE AND INSTALLATION OF THE DESIGNED DETENTION SYSTEM DETAILED IN THE PROJECT PLANS.

MATERIA

THE MATERIAL SHALL CONFORM TO THE APPLICABLE REQUIREMENTS LISTED BELOW:

ALUMINIZED TYPE 2 STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-274 OR ASTM A-92.

THE GALVANIZED STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-218 OR ASTM A-929.

THE POLYMER COATED STEEL COILS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M-246 OR ASTM A-742.

THE ALUMINUM COILS SHALL CONFORM TO THE APPLICABLE OF AASHTO M-197 OR ASTM B-744.

CONSTRUCTION LOADS

THESE DRAWINGS ARE FOR CONCEPTUAL PURPOSES AND DO NOT REFLECT ANY LOCAL

PREFERENCES OR REGULATIONS. PLEASE

CONSTRUCTION LOADS MAY BE HIGHER THAN FINAL LOADS. FOLLOW THE MANUFACTURER'S OR NCSPA GUIDELINES.

DIDE

THE PIPE SHALL BE MANUFACTURED IN ACCORDANCE TO THE APPLICABLE REQUIREMENTS LISTED BELOW:

ALUMINIZED TYPE 2: AASHTO M-36 OR ASTM A-760

GALVANIZED: AASHTO M-36 OR ASTM A-760

AFPOLYMBLE COATED: AASHTO M-245 OR ASTM A-762

ALUMINUM: AASHTO M-196 OR ASTM B-745 APPLICABLE

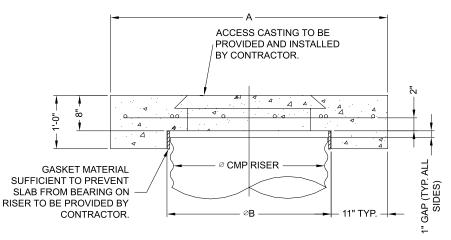
HANDLING AND ASSEMBLY

SHALL BE IN ACCORDANCE WITH NCSP'S (NATIONAL CORRUGATED STEEL APPRECABSDCIATION) FOR ALUMINIZED TYPE 2, GALVANIZED OR POLYMER COATED STEEL. SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR ALUMINUM PIPE.

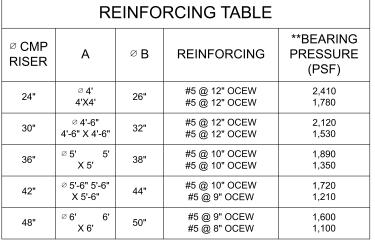
REQUIREMENTS INSTALLATION

SHALL BE IN ACCORDANCE WITH AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SECTION 26, DIVISION II DIVISION II OR ASTM A-798 (FOR ALUMINIZED TYPE 2, GALVANIZED OR POLYMER COATED STEEL) OR ASTM B-788 (FOR ALUMINUM PIPE) AND IN CONFORMANCE WITH THE PROJECT PLANS AND SPECIFICATIONS. IF THERE ARE ANY INCONSISTENCIES OR CONFLICTS THE CONTRACTOR SHOULD DISCUSS AND RESOLVE WITH THE SITE FINGINFER

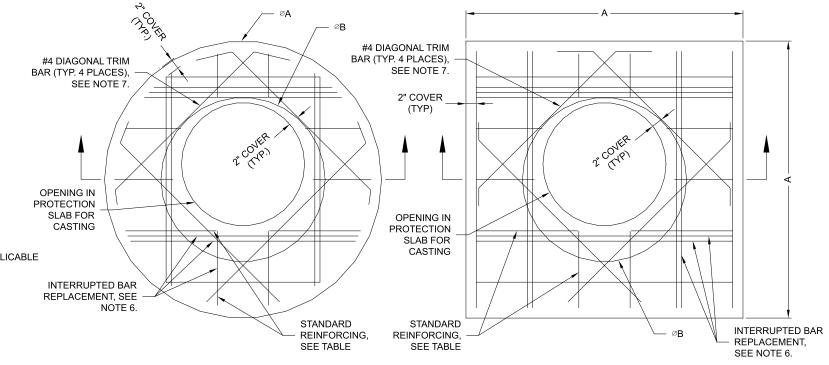
IT IS ALWAYS THE RESPONSIBILITY OF THE CONTRACTOR TO FOLLOW OSHA GUIDELINES FOR SAFE PRACTICES.



SECTION VIEW



** ASSUMED SOIL BEARING CAPACITY



ROUND OPTION PLAN VIEW

NOTES:

- 1. DESIGN IN ACCORDANCE WITH AASHTO, 17th EDITION.
- 2. DESIGN LOAD HS25.
- 3. EARTH COVER = 1' MAX.
- 4. CONCRETE STRENGTH = 3,500 psi
- 5. REINFORCING STEEL = ASTM A615, GRADE 60.
- PROVIDE ADDITIONAL REINFORCING AROUND OPENINGS EQUAL TO THE BARS INTERRUPTED, HALF EACH SIDE. ADDITIONAL BARS TO BE IN THE SAME PLANE.

SQUARE OPTION PLAN VIEW

- 7. TRIM OPENING WITH DIAGONAL #4 BARS, EXTEND BARS A MINIMUM OF 12" BEYOND OPENING, BEND BARS AS REQUIRED TO MAINTAIN BAR COVER.
- 8. PROTECTION SLAB AND ALL MATERIALS TO BE PROVIDED AND INSTALLED BY CONTRACTOR.
- 9. DETAIL DESIGN BY DELTA ENGINEERING, BINGHAMTON, NY.

MANHOLE CAP DETAIL

SCALE: N.T.S.

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DYODS

DRAWING

Rockwall, TX
DETENTION SYSTEM

DYO7913 Rockwall CAD



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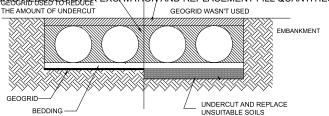
CMP DETENTION INSTALLATION GUIDE

PROPER INSTALLATION OF A FLEXIBLE UNDERGROUND DETENTION SYSTEM WILL ENSURE LONG-TERM PERFORMANCE. THE CONFIGURATION OF THESE SYSTEMS OFTEN REQUIRES SPECIAL CONSTRUCTION PRACTICES THAT DIFFER FROM CONVENTIONAL FLEXIBLE PIPE CONSTRUCTION. CONTECH ENGINEERED SOLUTIONS STRONGLY SUGGESTS SCHEDULING A PRE-CONSTRUCTION MEETING WITH YOUR LOCAL SALES ENGINEER TO DETERMINE IF ADDITIONAL MEASURES, NOT COVERED IN THIS GUIDE, ARE APPROPRIATE FOR YOUR SITE.

FOUNDATION

CONSTRUCT A FOUNDATION THAT CAN SUPPORT THE DESIGN LOADING APPLIED BY THE PIPE AND ADJACENT BACKFILL WEIGHT AS WELL AS MAINTAIN ITS INTEGRITY DURING CONSTRUCTION.

IF SOFT OR UNSUITABLE SOILS ARE ENCOUNTERED, REMOVE THE POOR SOILS DOWN TO A SUITABLE DEPTH AND THEN BUILD UP TO THE APPROPRIATE ELEVATION WITH A COMPETENT BACKFILL MATERIAL. THE STRUCTURAL FILL MATERIAL GRADATION SHOULD NOT ALLOW THE MIGRATION OF FINES, WHICH CAN CAUSE SETTLEMENT OF THE DETENTION SYSTEM OR PAVEMENT ABOVE. IF THE STRUCTURAL FILL MATERIAL IS NOT COMPATIBLE WITH THE UNDERLYING SOILS AN ENGINEERING FABRIC SHOULD BE USED AS A SEPARATION. IN SOME CASES, USING A STIFF REINFORCING GEOGRID REDUCTOR OVER EXCAVATION AND PEPLACEMENT FILL QUANTITIES.

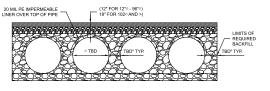


GRADE THE FOUNDATION SUBGRADE TO A UNIFORM OR SLIGHTLY SLOPING GRADE. IF THE SUBGRADE IS CLAY OR RELATIVELY NON-POROUS AND THE CONSTRUCTION SEQUENCE WILL LAST FOR AN EXTENDED PERIOD OF TIME, IT IS BEST TO SLOPE THE GRADE TO ONE END OF THE SYSTEM. THIS WILL ALLOW EXCESS WATER TO DRAIN QUICKLY, PREVENTING SATURATION OF THE SUBGRADE.

GEOMEMBRANE BARRIER

A SITE'S RESISTIVITY MAY CHANGE OVER TIME WHEN VARIOUS TYPES OF SALTING AGENTS ARE USED, SUCH AS ROAD SALTS FOR DEICING AGENTS. IF SALTING AGENTS ARE USED ON OR NEAR THE PROJECT SITE, A GEOMEMBRANE BARRIER IS RECOMMENDED WITH THE SYSTEM. THE GEOMEMBRANE LINER IS INTENDED TO HELP PROTECT THE SYSTEM FROM THE POTENTIAL ADVERSE EFFECTS THAT MAY RESULT FROM THE USE OF SUCH AGENTS INCLUDING PREMATURE CORROSION AND REDUCED ACTUAL

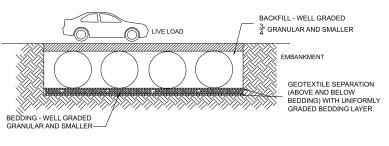
新尼学尼步氏: S ENGINEER OF RECORD IS TO EVALUATE WHETHER SALTING AGENTS WILL BE USED ON OR NEAR THE PROJECT SITE, AND USE HIS/HER BEST JUDGEMENT TO DETERMINE IF ANY ADDITIONAL PROTECTIVE MEASURES ARE REQUIRED. BELOW IS A TYPICAL DETAIL SHOWING THE PLACEMENT OF A GEOMEMBRANE BARRIER FOR PROJECTS WHERE SALTING AGENTS ARE USED ON OR NEAR THE PROJECT SITE.



IN-SITU TRENCH WALL

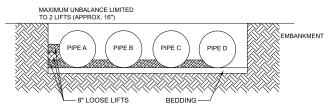
IF EXCAVATION IS REQUIRED, THE TRENCH WALL NEEDS TO BE CAPABLE OF SUPPORTING THE LOAD THAT THE PIPE SHEDS AS THE SYSTEM IS LOADED. IF SOILS ARE NOT CAPABLE OF SUPPORTING THESE LOADS, THE PIPE CAN DEFLECT PERFORM A SIMPLE SOIL PRESSURE CHECK USING THE APPLIED LOADS TO DETERMINE THE LIMITS OF EXCAVATION BEYOND THE SPRING LINE OF THE OUTER MOST PIPES.

IN MOST CASES THE REQUIREMENTS FOR A SAFE WORK ENVIRONMENT AND PROPER BACKFILL PLACEMENT AND COMPACTION TAKE CARE OF THIS CONCERN.



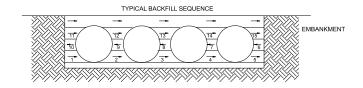
BACKFILL PLACEMENT

MATERIAL SHALL BE WORKED INTO THE PIPE HAUNCHES BY MEANS OF SHOVEL-SLICING, RODDING, AIR TAMPER, VIBRATORY ROD, OR OTHER EFFECTIVE METHODS.

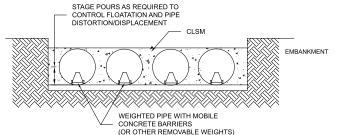


IF AASHTO T99 PROCEDURES ARE DETERMINED INFEASIBLE BY THE GEOTECHNICAL ENGINEER OF RECORD, COMPACTION IS CONSIDERED ADEQUATE WHEN NO FURTHER YIELDING OF THE MATERIAL IS OBSERVED UNDER THE COMPACTOR, OR UNDER FOOT, AND THE GEOTECHNICAL ENGINEER OF RECORD (OR REPRESENTATIVE THEREOF) IS SATISFIED WITH THE LEVEL OF COMPACTION.

FOR LARGE SYSTEMS, CONVEYOR SYSTEMS, BACKHOES WITH LONG REACHES OR DRAGLINES WITH STONE BUCKETS MAY BE USED TO PLACE BACKFILL. ONCE MINIMUM COVER FOR CONSTRUCTION LOADING ACROSS THE ENTIRE WIDTH OF THE SYSTEM IS REACHED, ADVANCE THE EQUIPMENT TO THE END OF THE RECENTLY PLACED FILL, AND BEGIN THE SEQUENCE AGAIN UNTIL THE SYSTEM IS COMPLETELY BACKFILLED. THIS TYPE OF CONSTRUCTION SEQUENCE PROVIDES ROOM FOR STOCKPILED BACKFILL DIRECTLY BEHIND THE BACKHOE, AS WELL AS THE MOVEMENT OF CONSTRUCTION TRAFFIC. MATERIAL STOCKPILES ON TOP OF THE BACKFILLED DETENTION SYSTEM SHOULD BE LIMITED TO 8- TO 10-FEET HIGH AND MUST PROVIDE BALANCED LOADING ACROSS ALL BARRELS. TO DETERMINE THE PROPER COVER OVER THE PIPES TO ALLOW THE MOVEMENT OF CONSTRUCTION EQUIPMENT SEE TABLE 1, OR CONTACT YOUR LOCAL CONTECH SALES ENGINEER.



WHEN FLOWABLE FILL IS USED, YOU MUST PREVENT PIPE FLOATATION. TYPICALLY, SMALL LIFTS ARE PLACED BETWEEN THE PIPES AND THEN ALLOWED TO SET-UP PRIOR TO THE PLACEMENT OF THE NEXT LIFT. THE ALLOWABLE THICKNESS OF THE CLSM LIFT IS A FUNCTION OF A PROPER BALANCE BETWEEN THE UPLIFT FORCE OF THE CLSM, THE OPPOSING WEIGHT OF THE PIPE, AND THE EFFECT OF OTHER RESTRAINING MEASURES. THE PIPE CAN CARRY LIMITED FLUID PRESSURE WITHOUT PIPE DISTORTION OR DISPLACEMENT, WHICH ALSO AFFECTS THE CLSM LIFT THICKNESS. YOUR LOCAL CONTECH SALES ENGINEER CAN HELP DETERMINE THE PROPER LIFT THICKNESS.

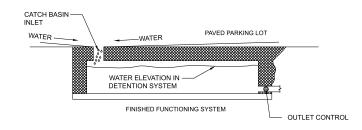


CONSTRUCTION LOADING

TYPICALLY, THE MINIMUM COVER SPECIFIED FOR A PROJECT ASSUMES H-20 LIVE LOAD. BECAUSE CONSTRUCTION LOADS OFTEN EXCEED DESIGN LIVE LOADS, INCREASED TEMPORARY MINIMUM COVER REQUIREMENTS ARE NECESSARY. SINCE CONSTRUCTION EQUIPMENT VARIES FROM JOB TO JOB, IT IS BEST TO ADDRESS EQUIPMENT SPECIFIC MINIMUM COVER REQUIREMENTS WITH YOUR LOCAL CONTECH SALES ENGINEER DURING YOUR PRE-CONSTRUCTION MEETING.

ADDITIONAL CONSIDERATIONS

BECAUSE MOST SYSTEMS ARE CONSTRUCTED BELOW-GRADE, RAINFALL CAN RAPIDLY FILL THE EXCAVATION; POTENTIALLY CAUSING FLOATATION AND MOVEMENT OF THE PREVIOUSLY PLACED PIPES. TO HELP MITIGATE POTENTIAL PROBLEMS, IT IS BEST TO START THE INSTALLATION AT THE DOWNSTREAM END WITH THE OUTLET ALREADY CONSTRUCTED TO ALLOW A ROUTE FOR THE WATER TO ESCAPE. TEMPORARY DIVERSION MEASURES MAY BE REQUIRED FOR HIGH FLOWS DUE TO THE RESTRICTED NATURE OF THE OUTLET PIPE.



CMP DETENTION SYSTEM INSPECTION AND MAINTENANCE

UNDERGROUND STORMWATER DETENTION AND INFILTRATION SYSTEMS MUST BE INSPECTED AND MAINTAINED AT REGULAR INTERVALS FOR PURPOSES OF PERFORMANCE AND LONGEVITY.

INSPECTION

INSPECTION IS THE KEY TO EFFECTIVE MAINTENANCE OF CMP DETENTION SYSTEMS AND IS EASILY PERFORMED. CONTECH RECOMMENDS ONGOING, ANNUAL INSPECTIONS. SITES WITH HIGH TRASH LOAD OR SMALL OUTLET CONTROL ORIFICES MAY NEED MORE FREQUENT INSPECTIONS. THE RATE AT WHICH THE SYSTEM COLLECTS POLLUTANTS WILL DEPEND MORE ON SITE SPECIFIC ACTIVITIES RATHER THAN THE SIZE OR CONFIGURATION OF THE SYSTEM

INSPECTIONS SHOULD BE PERFORMED MORE OFTEN IN EQUIPMENT WASHDOWN AREAS, IN CLIMATES WHERE SANDING AND/OR SALTING OPERATIONS TAKE PLACE, AND IN OTHER VARIOUS INSTANCES IN WHICH ONE WOULD EXPECT HIGHER ACCUMULATIONS OF SEDIMENT OR ABRASIVE/CORROSIVE CONDITIONS. A RECORD OF EACH INSPECTION IS TO BE MAINTAINED FOR THE LIFE OF THE SYSTEM

MAINTENANCE

CMP DETENTION SYSTEMS SHOULD BE CLEANED WHEN AN INSPECTION REVEALS ACCUMULATED SEDIMENT OR TRASH IS CLOGGING THE DISCHARGE ORIFICE.

ACCUMULATED SEDIMENT AND TRASH CAN TYPICALLY BE EVACUATED THROUGH THE MANHOLE OVER THE OUTLET ORIFICE. IF MAINTENANCE IS NOT PERFORMED AS RECOMMENDED, SEDIMENT AND TRASH MAY ACCUMULATE IN FRONT OF THE OUTLET ORIFICE. MANHOLE COVERS SHOULD BE SECURELY SEATED FOLLOWING CLEANING ACTIVITIES. CONTECH SUGGESTS THAT ALL SYSTEMS BE DESIGNED WITH AN ACCESS/INSPECTION MANHOLE SITUATED AT OR NEAR THE INLET AND THE OUTLET ORIFICE. SHOULD IT BE NECESSARY TO GET INSIDE THE SYSTEM TO PERFORM MAINTENANCE ACTIVITIES, ALL APPROPRIATE PRECAUTIONS REGARDING CONFINED SPACE ENTRY AND OSHA REGULATIONS SHOULD BE FOLLOWED.

ANNUAL INSPECTIONS ARE BEST PRACTICE FOR ALL UNDERGROUND SYSTEMS. DURING THIS INSPECTION, IF EVIDENCE OF SALTING/DE-ICING AGENTS IS OBSERVED WITHIN THE SYSTEM, IT IS BEST PRACTICE FOR THE SYSTEM TO BE RINSED, INCLUDING ABOVE THE SPRING LINE SOON AFTER THE SPRING THAW AS PART OF THE MAINTENANCE PROGRAM FOR THE SYSTEM.

MAINTAINING AN UNDERGROUND DETENTION OR INFILTRATION SYSTEM IS EASIEST WHEN THERE IS NO FLOW ENTERING THE SYSTEM. FOR THIS REASON, IT IS A GOOD IDEA TO SCHEDULE THE CLEANOUT DURING DRY WEATHER

THE FOREGOING INSPECTION AND MAINTENANCE EFFORTS HELP ENSURE UNDERGROUND PIPE SYSTEMS USED FOR STORMWATER STORAGE CONTINUE TO FUNCTION AS INTENDED BY IDENTIFYING RECOMMENDED REGULAR INSPECTION AND MAINTENANCE PRACTICES. INSPECTION AND MAINTENANCE RELATED TO THE STRUCTURAL INTEGRITY OF THE PIPE OR THE SOUNDNESS OF PIPE JOINT CONNECTIONS IS BEYOND THE SCOPE OF THIS GUIDE.



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DATE REVISION DESCRIPTION BY



9025 Centre Pointe Dr., Suite 400, West Chester, OH 45069 800-338-1122 513-645-7000 513-645-7993 FAX



DRAWING

DYO7913 Rockwall CAD C Rockwall, TX DETENTION SYSTEM

PROJECT No.:	SEQ. I	No.:	DATE:
4927	7913		5/20/2021
DESIGNED:		DRAW	N:
DYO			DYO
CHECKED:		APPR	OVED:
DYO			DYO
SHEET NO.:	D	4	



SPACING QUANTITY REMARKS

12" O.C.





C Ś AND

CASE #: SP2021-001

CENTRAL APPRAISAL DISTRICT OF ROCKWALL 841 JUSTIN ROAD ROCKWALL, TX 75087

PH: 972-771-2034 CLAYMOORE ENGINEERING, INC. 1903 CENTRAL DRIVE, SUITE #406

CONTACT NAME: CLAY CRIST LEGAL DESCRIPTION: LOT 1A-R, BLOCK A, LOFLAND INDUSTRIAL

PARK ADDITION ROCKWALL **TEXAS** ROCKWAL B.J.T. LEWIS 255

SYMBOL BOTANIC NAME

PLANTING LEGEND

TREES						
CECA	Cercis canadensis 'Texana'	Texas Redbud	2" cal., 8'-10' high	per plan	6	Accent Tree
QUTX	Quercus texana	Texas Red Oak	4" cal., 16'-18' high	per plan	8	Canopy Tree
	ROOT QUALITY.	-GROWN, CONTAINER SIZE AS AF	PROPRIATE FOR T	HE CALIPER 5	PECIFIED. SE	E SPECIFICATIONS FOR
ILBN	Ilex cornuta 'Burfordii 'Nana'	Dwarf Burford Holly	5 Gallon	per plan	128	Evergreen Screening Shrub
TURF AND	O GROUND COVER					•

MIN. SIZE

LANDSCAPE STANDARDS

05.01 LANDSCAPE BUFFERS - NON-RESIDENTIAL

REQ. ABUTTING A PUBLIC RIGHT-OF-WAY:

Ruellia brittoniana 'Katie'

BERM, AND SHRUBBERY 30" HIGH + 1 CANOPY TREE & 1 ACCENT TREE PER 50 LIN. FEET OF FRONTAGE GROUND COVER, SHRUBS, 6 OAKS, 6 REDBUDS

10' WIDE LANDSCAPE BUFFER W/ GROUND COVER,

05.02 LANDSCAPE SCREENING REQ. HEADLIGHT SCREENING

PROVIDED 10' BUFFER - T.L. TOWNSEND DR.:

HEAD-IN PARKING ADJ. TO STREET SHALL INCORP. MIN. 2' BERM W/ MATURE EVERGREEN SHRUBS ALONG ENTIRE PARKING AREAS

PROVIDED SCREENING EVERGREEN HOLLY SHRUBS PROVIDED IN FRONT OF PARKING SPACES LOCATED ALONG STREET

COMMON NAME

Dwarf Katie Ruellia

FRONTAGES

05.03 LANDSCAPE REQUIREMENTS - COMMERCIAL (C) DISTRICT TOTAL SITE AREA FOR NEW DEVELOPMENT: 38,486 SF

LANDSCAPE AREA REQUIRED (20%): LANDSCAPE AREA PROVIDED:

13,616 SF (35% OF ENTIRE SITE AREA)

MIN. 50% OF REQ. LANDSCAPING SHALL BE LOCATED LOCATION OF LANDSCAPING: IN THE FRONT OF & ALONG THE SIDE OF BUILDINGS

7.697 SF

W/ STREET FRONTAGE.

MIN. SIZE OF AREAS ALL REQ. LANDSCAPING SHALL BE NO LESS THAN 5' WIDE AND A MIN. OF 25 SF IN AREA

DETENTION BASINS NONE PROPOSED

PARKING LOT LANDSCAPING MIN. 5% OR 200 SF OF LANDSCAPING, WHICHEVER IS GREATER, IN THE INTERIOR OF THE PARKING LOT

PROPOSED PARKING AREA: 32,111 SF (ENTIRE SITE) **REQ. PARKING LOT LANDSCAPING:** 1,606 SF (ENTIRE SITE) PROPOSED PARKING LOT LANDSCAPING; ±3,610 SF / 11% (ENTIRE SITE)

REQ. PARKING SPACES MUST BE WITHIN 80' OF A CANOPY TREE TRUNK

- NO TREES TO BE PLANTED WITHIN 5' OF A CITY UTILITY.
- ALL RIGHTS-OF WAY DISTURBED DURING CONSTRUCTION SHALL BE SODDED.
- ALL SHRUBS OR TREES PLANTED ADJACENT TO 18' PARKING SPACES SHALL BE PLANTED 4' FROM THE BACK OF CURB.

THE CONTRACTOR SHALL INSTALL ROOT BARRIERS NEAR ALL NEWLY-PLANTED TREES THAT ARE LOCATED WITHIN FIVE (5) FEET OF PAVING OR CURBS. ROOT BARRIERS SHALL BE "CENTURY" OR "DEEP-ROOT" 24" DEEP PANELS (OR EQUAL). BARRIERS SHALL BE LOCATED IMMEDIATELY ADJACENT TO HARDSCAPE. INSTALL PANELS PER MANUFACTURER'S RECOMMENDATIONS. UNDER NO CIRCUMSTANCES SHALL THE CONTRACTOR USE ROOT BARRIERS OF A TYPE THAT COMPLETELY ENCIRCLE THE ROOTBALL.

MULCHES

AFTER ALL PLANTING IS COMPLETE, CONTRACTOR SHALL INSTALL 3" THICK LAYER OF 1-1/2" SHREDDED WOOD MULCH, NATURAL (UNDYED), IN ALL PLANTING AREAS (EXCEPT FOR TURF AND SEEDED AREAS). CONTRACTOR SHALL SUBMIT SAMPLES OF ALL MULCHES TO LANDSCAPE ARCHITECT AND OWNER FOR APPROVAL PRIOR TO CONSTRUCTION. ABSOLUTELY NO EXPOSED GROUND SHALL BE LEFT SHOWING ANYWHERE ON THE PROJECT AFTER MULCH HAS BEEN INSTALLED (SUBJECT TO THE CONDITIONS AND REQUIREMENTS OF THE "GENERAL GRADING AND PLANTING NOTES" AND SPECIFICATIONS).

GENERAL GRADING AND PLANTING NOTES

ON TURF AREA AND PLANTING BED PREPARATION.

- 1. BY SUBMITTING A PROPOSAL FOR THE LANDSCAPE PLANTING SCOPE OF WORK, THE CONTRACTOR CONFIRMS THAT HE HAS READ, AND WILL COMPLY WITH, THE ASSOCIATED NOTES, SPECIFICATIONS, AND DETAILS WITH THIS PROJECT.
- THE GENERAL CONTRACTOR IS RESPONSIBLE FOR REMOVING ALL EXISTING VEGETATION (EXCEPT WHERE NOTED TO 3. IN THE CONTEXT OF THESE PLANS, NOTES, AND SPECIFICATIONS, "FINISH GRADE" REFERS TO THE FINAL ELEVATION OF THE SOIL SURFACE (NOT TOP OF MULCH) AS INDICATED ON THE GRADING PLANS. a. BEFORE STARTING WORK, THE LANDSCAPE CONTRACTOR SHALL VERIFY THAT THE ROUGH GRADES OF ALL
- b. CONSTRUCT AND MAINTAIN FINISH GRADES AS SHOWN ON GRADING PLANS, AND CONSTRUCT AND MAINTAIN SLOPES AS RECOMMENDED BY THE GEOTECHNICAL REPORT. ALL LANDSCAPE AREAS SHALL HAVE POSITIVE DRAINAGE AWAY FROM STRUCTURES AT THE MINIMUM SLOPE SPECIFIED IN THE REPORT AND ON THE GRADING PLANS, AND AREAS OF POTENTIAL PONDING SHALL BE REGRADED TO BLEND IN WITH THE SURROUNDING GRADES AND ELIMINATE PONDING POTENTIAL.

LANDSCAPE AREAS ARE WITHIN +/-0.1' OF FINISH GRADE. SEE SPECIFICATIONS FOR MORE DETAILED INSTRUCTION

c. THE LANDSCAPE CONTRACTOR SHALL DETERMINE WHETHER OR NOT THE EXPORT OF ANY SOIL WILL BE NEEDED. TAKING INTO ACCOUNT THE ROUGH GRADE PROVIDED, THE AMOUNT OF SOIL AMENDMENTS TO BE ADDED (BASED

ON A SOIL TEST, PER SPECIFICATIONS), AND THE FINISH GRADES TO BE ESTABLISHED.

- ENSURE THAT THE FINISH GRADE IN SHRUB AREAS IMMEDIATELY ADJACENT TO WALKS AND OTHER WALKING SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 3" BELOW THE ADJACENT FINISH SURFACE, IN ORDER TO ALLOW FOR PROPER MULCH DEPTH. TAPER THE SOIL SURFACE TO MEET FINISH GRADE, AS SPECIFIED ON THE GRADING PLANS. AT APPROXIMATELY 18" AWAY FROM THE WALKS.
- e. ENSURE THAT THE FINISH GRADE IN TURF AREAS IMMEDIATELY ADJACENT TO WALKS AND OTHER WALKING SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 1" BELOW THE FINISH SURFACE OF THE WALKS. TAPER THE SOIL SURFACE TO MEET FINISH GRADE, AS SPECIFIED ON THE GRADING PLANS, AT APPROXIMATELY 18" AWAY FROM THE WALKS.
- SHOULD ANY CONFLICTS AND/OR DISCREPANCIES ARISE BETWEEN THE GRADING PLANS, GEOTECHNICAL REPORT, THESE NOTES AND PLANS, AND ACTUAL CONDITIONS, THE CONTRACTOR SHALL IMMEDIATELY BRING SUCH ITEMS TO THE ATTENTION OF THE LANDSCAPE ARCHITECT, GENERAL CONTRACTOR, AND OWNER. ALL PLANT LOCATIONS ARE DIAGRAMMATIC. ACTUAL LOCATIONS SHALL BE VERIFIED WITH THE LANDSCAPE ARCHITECT
- a. THE LANDSCAPE CONTRACTOR IS RESPONSIBLE FOR DETERMINING PLANT QUANTITIES; PLANT QUANTITIES SHOWN ON LEGENDS AND CALLOUTS ARE FOR GENERAL INFORMATION ONLY. IN THE EVENT OF A DISCREPANCY BETWEEN THE PLAN AND THE PLANT LEGEND, THE PLANT QUANTITY AS SHOWN ON THE PLAN (FOR INDIVIDUAL SYMBOLS) OR

OR DESIGNER PRIOR TO PLANTING. THE LANDSCAPE CONTRACTOR SHALL ENSURE THAT ALL REQUIREMENTS OF THE

PERMITTING AUTHORITY ARE MET (I.E., MINIMUM PLANT QUANTITIES, PLANTING METHODS, TREE PROTECTION METHODS,

- CALLOUT (FOR GROUNDCOVER PATTERNS) SHALL TAKE PRECEDENCE. b. NO SUBSTITUTIONS OF PLANT MATERIALS SHALL BE ALLOWED WITHOUT THE WRITTEN PERMISSION OF THE LANDSCAPE ARCHITECT. IF SOME OF THE PLANTS ARE NOT AVAILABLE, THE LANDSCAPE CONTRACTOR SHALL
- NOTIFY THE LANDSCAPE ARCHITECT IN WRITING (VIA PROPER CHANNELS). c. THE CONTRACTOR SHALL, AT A MINIMUM, PROVIDE REPRESENTATIVE PHÓTOS OF ALL PLANTS PROPOSED FOR THE PROJECT. THE CONTRACTOR SHALL ALLOW THE LANDSCAPE ARCHITECT AND THE OWNER/OWNER'S REPRESENTATIVE TO INSPECT, AND APPROVE OR REJECT, ALL PLANTS DELIVERED TO THE JOBSITE. REFER TO SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS FOR SUBMITTALS.
- THE CONTRACTOR SHALL MAINTAIN THE LANDSCAPE IN A HEALTHY CONDITION FOR 90 DAYS AFTER ACCEPTANCE BY THE OWNER. REFER TO SPECIFICATIONS FOR CONDITIONS OF ACCEPTANCE FOR THE START OF THE MAINTENANCE PERIOD, AND FOR FINAL ACCEPTANCE AT THE END OF THE MAINTENANCE PERIOD.
- 6. SEE SPECIFICATIONS AND DETAILS FOR FURTHER REQUIREMENTS.

IRRIGATION CONCEPT

DIRECTOR OF PLANNING & ZONING

- 1. AN AUTOMATIC IRRIGATION SYSTEM SHALL BE INSTALLED AND OPERATIONAL BY THE TIME OF FINAL INSPECTION. THE ENTIRE IRRIGATION SYSTEM SHALL BE INSTALLED BY A LICENSED AND QUALIFIED IRRIGATION CONTRACTOR.
- 2. THE IRRIGATION SYSTEM WILL OPERATE ON POTABLE WATER, AND THE SYSTEM WILL HAVE APPROPRIATE BACKFLOW PREVENTION DEVICES INSTALLED TO PREVENT CONTAMINATION OF THE POTABLE SOURCE.
- 3. ALL NON-TURF PLANTED AREAS SHALL BE DRIP IRRIGATED. SODDED AND SEEDED AREAS SHALL BE IRRIGATED WITH SPRAY OR ROTOR HEADS AT 100% HEAD-TO-HEAD COVERAGE.
- 4. ALL PLANTS SHARING SIMILAR HYDROZONE CHARACTERISTICS SHALL BE PLACED ON A VALVE DEDICATED TO PROVIDE THE NECESSARY WATER REQUIREMENTS SPECIFIC TO THAT HYDROZONE.
- 5. THE IRRIGATION SYSTEM SHALL BE DESIGNED AND INSTALLED, TO THE MAXIMUM EXTENT POSSIBLE, TO CONSERVE WATER BY USING THE FOLLOWING DEVICES AND SYSTEMS: MATCHED PRECIPITATION RATE TECHNOLOGY ON ROTOR AND SPRAY HEADS (WHEREVER POSSIBLE), RAIN SENSORS, AND MULTI-PROGRAM COMPUTERIZED

IRRIGATION CONTROLLERS FEATURING SENSORY INPUT CAPABILITIES.

I HEREBY CERTIFY THAT THE ABOVE AND FOREGOING SITE PLAN FOR A DEVELOPMENT IN THE CITY OF ROCKWALL, TEXAS, WAS APPROVED BY THE PLANNING & ZONING COMMISSION OF THE CITY OF ROCKWALL ON THE _____ DAY OF _ WITNESS OUR HANDS, THIS _____ DAY OF _____, ____

BEDFORD, TX 76021 PH: 817.281.0572

HECKED: SHEET

construction process reported by the contractor to ClayMoore Engineering, Inc. and considered to be significant. This drawing is not guaranteed to be "As Built" but is based on the information made

This drawing has been revised to show those changes during the

- SPECIALIZING IN LANDSCAPE PLANTING. 2. A LIST OF SUCCESSFULLY COMPLETED PROJECTS OF THIS TYPE, SIZE AND NATURE MAY BE
- REQUESTED BY THE OWNER FOR FURTHER QUALIFICATION MEASURES. THE LANDSCAPE CONTRACTOR SHALL HOLD A VALID NURSERY AND FLORAL CERTIFICATE ISSUED BY THE TEXAS DEPARTMENT OF AGRICULTURE, AS WELL AS OPERATE UNDER A COMMERCIAL PESTICIDE APPLICATOR LICENSE ISSUED BY EITHER THE TEXAS DEPARTMENT OF AGRICULTURE OR THE TEXAS STRUCTURAL PEST CONTROL BOARD.
- B. SCOPE OF WORK WORK COVERED BY THESE SECTIONS INCLUDES THE FURNISHING AND PAYMENT OF ALL MATERIALS LABOR, SERVICES, EQUIPMENT, LICENSES, TAXES AND ANY OTHER ITEMS THAT ARE NECESSARY FOR
- THE EXECUTION, INSTALLATION AND COMPLETION OF ALL WORK, SPECIFIED HEREIN AND / OR SHOWN ON THE LANDSCAPE PLANS, NOTES, AND DETAILS. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE LAWS. CODES AND REGULATIONS REQUIRED BY AUTHORITIES HAVING JURISDICTION OVER SUCH WORK, INCLUDING ALL
- TRANSPORTATION AND INSTALLATION OF MATERIALS. THE LANDSCAPE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL UNDERGROUND UTILITY LINES (WATER, SEWER, ELECTRICAL, TELEPHONE, GAS, CABLE, TELEVISION, ETC.) PRIOR TO THE START OF

INSPECTIONS AND PERMITS REQUIRED BY FEDERAL, STATE AND LOCAL AUTHORITIES IN SUPPLY,

PRODUCTS

A. ALL MANUFACTURED PRODUCTS SHALL BE NEW.

- B. CONTAINER AND BALLED-AND-BURLAPPED PLANTS: FURNISH NURSERY-GROWN PLANTS COMPLYING WITH ANSI Z60.1-2014. PROVIDE WELL-SHAPED, FULLY BRANCHED, HEALTHY, VIGOROUS STOCK FREE OF DISEASE, INSECTS, EGGS, LARVAE, AND DEFECTS SUCH AS KNOTS, SUN SCALD, INJURIES, ABRASIONS, AND DISFIGUREMENT. ALL PLANTS WITHIN A SPECIES SHALL HAVE SIMILAR SIZE, AND SHALL BE OF A FORM TYPICAL FOR THE SPECIES. ALL TREES SHALL BE OBTAINED FROM SOURCES WITHIN 200 MILES OF THE PROJECT SITE, AND WITH SIMILAR CLIMACTIC CONDITIONS.
- 2. ROOT SYSTEMS SHALL BE HEALTHY, DENSELY BRANCHED ROOT SYSTEMS, NON-POT-BOUND, FREE FROM ENCIRCLING AND/OR GIRDLING ROOTS, AND FREE FROM ANY OTHER ROOT DEFECTS (SUCH AS 3. TREES MAY BE PLANTED FROM CONTAINERS OR BALLED-AND-BURLAPPED (B&B), UNLESS SPECIFIED
- ON THE PLANTING LEGEND. BARE-ROOT TREES ARE NOT ACCEPTABLE. 4. ANY PLANT DEEMED UNACCEPTABLE BY THE LANDSCAPE ARCHITECT OR OWNER SHALL BE IMMEDIATELY REMOVED FROM THE SITE AND SHALL BE REPLACED WITH AN ACCEPTBLE PLANT OF LIKE TYPE AND SIZE AT THE CONTRACTOR'S OWN EXPENSE. ANY PLANTS APPEARING TO BE UNHEALTHY, EVEN IF DETERMINED TO STILL BE ALIVE, SHALL NOT BE ACCEPTED. THE LANDSCAPE ARCHITECT AND
- OWNER SHALL BE THE SOLE JUDGES AS TO THE ACCEPTABILITY OF PLANT MATERIAL ALL TREES SHALL BE STANDARD IN FORM, UNLESS OTHERWISE SPECIFIED. TREES WITH CENTRAL LEADERS WILL NOT BE ACCEPTED IF LEADER IS DAMAGED OR REMOVED. PRUNE ALL DAMAGED TWIGS
- 6. CALIPER MEASUREMENTS FOR STANDARD (SINGLE TRUNK) TREES SHALL BE AS FOLLOWS: SIX INCHES ABOVE THE ROOT FLARE FOR TREES UP TO AND INCLUDING FOUR INCHES IN CALIPER, AND TWELVE INCHES ABOVE THE ROOT FLARE FOR TREES EXCEEDING FOUR INCHES IN CALIPER. 7. MULTI-TRUNK TREES SHALL BE MEASURED BY THEIR OVERALL HEIGHT, MEASURED FROM THE TOP OF
- THE ROOT BALL. WHERE CALIPER MEASUREMENTS ARE USED, THE CALIPER SHALL BE CALCULATED AS ONE-HALF OF THE SUM OF THE CALIPER OF THE THREE LARGEST TRUNKS ANY TREE OR SHRUB SHOWN TO HAVE EXCESS SOIL PLACED ON TOP OF THE ROOT BALL, SO THAT THE ROOT FLARE HAS BEEN COMPLETELY COVERED, SHALL BE REJECTED. SOD: PROVIDE WELL-ROOTED SOD OF THE VARIETY NOTED ON THE PLANS. SOD SHALL BE CUT FROM
- HEALTHY, MATURE TURF WITH SOIL THICKNESS OF 3/4" TO 1". EACH PALLET OF SOD SHALL BE ACCOMPANIED BY A CERTIFICATE FROM SUPPLIER STATING THE COMPOSITION OF THE SOD. TOPSOIL: SANDY TO CLAY LOAM TOPSOIL, FREE OF STONES LARGER THAN ½ INCH, FOREIGN MATTER PLANTS, ROOTS, AND SEEDS.
- E. COMPOST: WELL-COMPOSTED, STABLE, AND WEED-FREE ORGANIC MATTER, pH RANGE OF 5.5 TO 8; MOISTURE CONTENT 35 TO 55 PERCENT BY WEIGHT; 100 PERCENT PASSING THROUGH 3/4-INCH SIEVE; SOLUBLE SALT CONTENT OF 5 TO 10 DECISIEMENS/M; NOT EXCEEDING 0.5 PERCENT INERT CONTAMINANTS AND FREE OF SUBSTANCES TOXIC TO PLANTINGS. NO MANURE OR ANIMAL-BASED PRODUCTS SHALL BE F. FERTILIZER: GRANULAR FERTILIZER CONSISTING OF NITROGEN, PHOSPHORUS, POTASSIUM, AND OTHER
- NUTRIENTS IN PROPORTIONS, AMOUNTS, AND RELEASE RATES RECOMMENDED IN A SOIL REPORT FROM A QUALIFIED SOIL-TESTING AGENCY (SEE BELOW). G. MULCH: SIZE AND TYPE AS INDICATED ON PLANS, FREE FROM DELETERIOUS MATERIALS AND SUITABLE AS A TOP DRESSING OF TREES AND SHRUBS.
- STAKES: 6' LONG GREEN METAL T-POSTS. GUY AND TIE WIRE: ASTM A 641, CLASS 1, GALVANIZED-STEEL WIRE, 2-STRAND, TWISTED, 0.106 INCH
- 3. STRAP CHAFING GUARD: REINFORCED NYLON OR CANVAS AT LEAST 1-1/2 INCH WIDE, WITH GROMMETS TO PROTECT TREE TRUNKS FROM DAMAGE.
- STEEL EDGING: PROFESSIONAL STEEL EDGING, 14 GAUGE THICK X 4 INCHES WIDE, FACTORY PAINTED DARK GREEN. ACCEPTABLE MANUFACTURERS INCLUDE COL-MET OR APPROVED EQUAL. PRE-EMERGENT HERBICIDES: ANY GRANULAR, NON-STAINING PRE-EMERGENT HERBICIDE THAT IS LABELED FOR THE SPECIFIC ORNAMENTALS OR TURF ON WHICH IT WILL BE UTILIZED. PRE-EMERGENT HERBICIDES SHALL BE APPLIED PER THE MANUFACTURER'S LABELED RATES.

H. TREE STAKING AND GUYING

- BEFORE STARTING WORK, THE LANDSCAPE CONTRACTOR SHALL VERIFY THAT THE GRADE OF ALL LANDSCAPE AREAS ARE WITHIN +/-0.1' OF FINISH GRADE. THE CONTRACTOR SHALL NOTIFY THE OWNER IMMEDIATELY SHOULD ANY DISCREPANCIES EXIST.
- AFTER FINISH GRADES HAVE BEEN ESTABLISHED, CONTRACTOR SHALL HAVE SOIL SAMPLES FROM THE PROJECT'S LANDSCAPE AREAS TESTED BY AN ESTABLISHED SOIL TESTING LABORATORY. EACH SAMPLE SUBMITTED TO THE LAB SHALL CONTAIN NO LESS THAN ONE QUART OF SOIL, TAKEN FROM BETWEEN THE SOIL SURFACE AND 6" DEPTH. IF NO SAMPLE LOCATIONS ARE INDICATED ON THE PLANS, THE CONTRACTOR SHALL TAKE A MINIMUM OF THREE SAMPLES FROM VARIOUS REPRESENTATIVE LOCATIONS FOR TESTING.
- THE CONTRACTOR SHALL HAVE THE SOIL TESTING LABORATORY PROVIDE RESULTS FOR THE FOLLOWING: SOIL TEXTURAL CLASS, GENERAL SOIL FERTILITY, pH, ORGANIC MATTER CONTENT, SALT (CEC), LIME, SODIUM ADSORPTION RATIO (SAR) AND BORON CONTENT. THE CONTRACTOR SHALL ALSO SUBMIT THE PROJECT'S PLANT LIST TO THE LABORATORY ALONG WITH THE SOIL SAMPLES. THE SOIL REPORT PRODUCED BY THE LABORATORY SHALL CONTAIN RECOMMENDATIONS FOR
- THE FOLLOWING (AS APPROPRIATE): SEPARATE SOIL PREPARATION AND BACKFILL MIX RECOMMENDATIONS FOR GENERAL ORNAMENTAL PLANTS, XERIC PLANTS, TURE, AND NATIVE SEED, AS WELL AS PRE-PLANT FERTILIZER APPLICATIONS AND RECOMMENDATIONS FOR ANY OTHER SOIL RELATED ISSUES. THE REPORT SHALL ALSO PROVIDE A FERTILIZER PROGRAM FOR THE ESTABLISHMENT PERIOD AND FOR LONG-TERM MAINTENANCE.
- THE CONTRACTOR SHALL INSTALL SOIL AMENDMENTS AND FERTILIZERS PER THE SOILS REPORT RECOMMENDATIONS. ANY CHANGE IN COST DUE TO THE SOIL REPORT RECOMMENDATIONS, EITHER INCREASE OR DECREASE. SHALL BE SUBMITTED TO THE OWNER WITH THE REPORT. 4. FOR BIDDING PURPOSES ONLY, THE SOIL PREPARATION SHALL CONSIST OF THE FOLLOWING:
- ROTOTILLING AFTER CROSS-RIPPING: NITROGEN STABILIZED ORGANIC AMENDMENT - 4 CU. YDS. PER 1,000 S.F.
- PREPLANT TURF FERTILIZER (10-20-10 OR SIMILAR, SLOW RELEASE, ORGANIC) 15 LBS PER 1,000 "CLAY BUSTER" OR EQUAL - USE MANUFACTURER'S RECOMMENDED RATE

TREES, SHRUBS, AND PERENNIALS: INCORPORATE THE FOLLOWING AMENDMENTS INTO THE TOP

TURF: INCORPORATE THE FOLLOWING AMENDMENTS INTO THE TOP 8" OF SOIL BY MEANS OF

- 8" OF SOIL BY MEANS OF ROTOTILLING AFTER CROSS-RIPPING NITROGEN STABILIZED ORGANIC AMENDMENT - 4 CU. YDS. PER 1,000 S.F.
- 12-12-12 FERTILIZER (OR SIMILAR, ORGANIC, SLOW RELEASE) 10 LBS. PER CU. YD. "CLAY BUSTER" OR EQUAL - USE MANUFACTURER'S RECOMMENDED RATE
- IRON SULPHATE 2 LBS. PER CU. YD. 5. IN THE CONTEXT OF THESE PLANS, NOTES, AND SPECIFICATIONS, "FINISH GRADE" REFERS TO THE FINAL ELEVATION OF THE SOIL SURFACE (NOT TOP OF MULCH) AS INDICATED ON THE GRADING PLANS. BEFORE STARTING WORK, THE LANDSCAPE CONTRACTOR SHALL VERIFY THAT THE ROUGH GRADES OF ALL LANDSCAPE AREAS ARE WITHIN +/-0.1' OF FINISH GRADE. SEE SPECIFICATIONS
- FOR MORE DETAILED INSTRUCTION ON TURF AREA AND PLANTING BED PREPARATION. CONSTRUCT AND MAINTAIN FINISH GRADES AS SHOWN ON GRADING PLANS, AND CONSTRUCT AND MAINTAIN SLOPES AS RECOMMENDED BY THE GEOTECHNICAL REPORT. ALL LANDSCAPE AREAS SHALL HAVE POSITIVE DRAINAGE AWAY FROM STRUCTURES AT THE MINIMUM SLOPE SPECIFIED IN THE REPORT AND ON THE GRADING PLANS, AND AREAS OF POTENTIAL PONDING SHALL BE REGRADED TO BLEND IN WITH THE SURROUNDING GRADES AND ELIMINATE PONDING
- THE LANDSCAPE CONTRACTOR SHALL DETERMINE WHETHER OR NOT THE EXPORT OF ANY SOIL WILL BE NEEDED, TAKING INTO ACCOUNT THE ROUGH GRADE PROVIDED, THE AMOUNT OF SOIL AMENDMENTS TO BE ADDED (**BASED ON A SOIL TEST**, PER SPECIFICATIONS), AND THE FINISH GRADES TO BE ESTABLISHED
- ENSURE THAT THE FINISH GRADE IN SHRUB AREAS IMMEDIATELY ADJACENT TO WALKS AND OTHER WALKING SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 3" BELOW THE ADJACENT FINISH SURFACE, IN ORDER TO ALLOW FOR PROPER MULCH DEPTH. TAPER THE SOIL SURFACE TO MEET FINISH GRADE, AS SPECIFIED ON THE GRADING PLANS, AT APPROXIMATELY 18" AWAY FROM THE WALKS
- ENSURE THAT THE FINISH GRADE IN TURF AREAS IMMEDIATELY ADJACENT TO WALKS AND OTHER WALKING SURFACES, AFTER INSTALLING SOIL AMENDMENTS, IS 1" BELOW THE FINISH SURFACE OF THE WALKS. TAPER THE SOIL SURFACE TO MEET FINISH GRADE, AS SPECIFIED ON
- THE GRADING PLANS, AT APPROXIMATELY 18" AWAY FROM THE WALKS. SHOULD ANY CONFLICTS AND/OR DISCREPANCIES ARISE BETWEEN THE GRADING PLANS, GEOTECHNICAL REPORT, THESE NOTES AND PLANS, AND ACTUAL CONDITIONS, THE CONTRACTOR SHALL IMMEDIATELY BRING SUCH ITEMS TO THE ATTENTION OF THE LANDSCAPE
- ARCHITECT, GENERAL CONTRACTOR, AND OWNER. ONCE SOIL PREPARATION IS COMPLETE, THE LANDSCAPE CONTRACTOR SHALL ENSURE THAT THERE ARE NO DEBRIS, TRASH, OR STONES LARGER THAN 1" REMAINING IN THE TOP 6" OF SOIL.

- THE CONTRACTOR SHALL PROVIDE SUBMITTALS AND SAMPLES IF REQUIRED TO THE LANDSCAPE ARCHITECT, AND RECEIVE APPROVAL IN WRITING FOR SLICH SUBMITTALS BEFORE WORK COMMENCES. SUBMITTALS SHALL INCLUDE PHOTOS OF PLANTS WITH A RULER OR MEASURING STICK FOR SCALE PHOTOS OR SAMPLES OF ANY REQUIRED MULCHES, AND SOIL TEST RESULTS AND PREPARATION
- TYPES, AND OTHER AMENDMENTS FOR TREE/SHRUB, TURF, AND SEED AREAS AS MAY BE SUBMITTALS SHALL ALSO INCLUDE MANUFACTURER CUT SHEETS FOR PLANTING ACCESSORIES SUCH

RECOMMENDATIONS FROM THE TESTING LAB (INCLUDING COMPOST AND FERTILIZER RATES AND

- AS TREE STAKES AND TIES, EDGING, AND LANDSCAPE FABRICS (IF ANY). WHERE MULTIPLE ITEMS ARE SHOWN ON A PAGE, THE CONTRACTOR SHALL CLEARLY INDICATE THE ITEM BEING CONSIDERED.
- GENERAL PLANTING REMOVE ALL NURSERY TAGS AND STAKES FROM PLANTS.

B. SUBMITTALS

- EXCEPT IN AREAS TO BE PLANTED WITH ORNAMENTAL GRASSES, APPLY PRE-EMERGENT HERBICIDES AT THE MANUFACTURER'S RECOMMENDED RATE.
- TRENCHING NEAR EXISTING TREES: a. CONTRACTOR SHALL NOT DISTURB ROOTS 1-1/2" AND LARGER IN DIAMETER WITHIN THE CRITICAL ROOT ZONE (CRZ) OF EXISTING TREES, AND SHALL EXERCISE ALL POSSIBLE CARE AND PRECAUTIONS TO AVOID INJURY TO TREE ROOTS, TRUNKS, AND BRANCHES. THE CRZ IS DEFINED AS A CIRCULAR AREA EXTENDING OUTWARD FROM THE TREE TRUNK, WITH A RADIUS EQUAL TO 1' FOR EVERY 1" OF TRUNK DIAMETER-AT-BREAST-HEIGHT (4.5' ABOVE THE AVERAGE
- ALL EXCAVATION WITHIN THE CRZ SHALL BE PERFORMED USING HAND TOOLS. NO MACHINE EXCAVATION OR TRENCHING OF ANY KIND SHALL BE ALLOWED WITHIN THE CRZ. ALTER ALIGNMENT OF PIPE TO AVOID TREE ROOTS 1-1/2" AND LARGER IN DIAMETER. WHERE TREE ROOTS 1-1/2" AND LARGER IN DIAMETER ARE ENCOUNTERED IN THE FIELD, TUNNEL UNDER SUCH ROOTS. WRAP EXPOSED ROOTS WITH SEVERAL LAYERS OF BURLAP AND KEEP MOIST. CLOSE ALL TRENCHES WITHIN THE CANOPY DRIP LINES WITHIN 24 HOURS. d. ALL SEVERED ROOTS SHALL BE HAND PRUNED WITH SHARP TOOLS AND ALLOWED TO AIR-DRY. DO NOT USE ANY SORT OF SEALERS OR WOUND PAINTS.
- TREE PLANTING TREE PLANTING HOLES SHALL BE EXCAVATED TO MINIMUM WIDTH OF TWO TIMES THE WIDTH OF THE ROOTBALL, AND TO A DEPTH EQUAL TO THE DEPTH OF THE ROOTBALL LESS TWO TO FOUR INCHES. SCARIEY THE SIDES AND BOTTOM OF THE PLANTING HOLF PRIOR TO THE PLACEMENT OF THE TREE REMOVE ANY GLAZING THAT MAY HAVE BEEN CAUSED DURING THE EXCAVATION OF THE HOLE. FOR CONTAINER AND BOX TREES. TO REMOVE ANY POTENTIALLY GIRDLING ROOTS AND OTHER ROOT DEFECTS, THE CONTRACTOR SHALL SHAVE A 1" LAYER OFF OF THE SIDES AND BOTTOM OF THE ROOTBALL OF ALL TREES JUST BEFORE PLACING INTO THE PLANTING PIT. DO NOT "TEASE" ROOTS
- OUT FROM THE ROOTBALL INSTALL THE TREE ON UNDISTURBED SUBGRADE SO THAT THE TOP OF THE ROOTBALL IS TWO TO
- FOUR INCHES ABOVE THE SURROUNDING GRADE. BACKFILL THE TREE HOLE UTILIZING THE EXISTING TOPSOIL FROM ON-SITE. ROCKS LARGER THAN 1" DIA. AND ALL OTHER DEBRIS SHALL BE REMOVED FROM THE SOIL PRIOR TO THE BACKFILL. SHOULD ADDITIONAL SOIL BE REQUIRED TO ACCOMPLISH THIS TASK, USE STORED TOPSOIL FROM ON-SITE OR IMPORT ADDITIONAL TOPSOIL FROM OFF-SITE AT NO ADDITIONAL COST TO THE OWNER. IMPORTED TOPSOIL SHALL BE OF SIMILAR TEXTURAL CLASS AND COMPOSITION IN THE ON-SITE SOIL.
- TREES SHALL NOT BE STAKED UNLESS LOCAL CONDITIONS (SUCH AS HEAVY WINDS OR SLOPES) REQUIRE STAKES TO KEEP TREES UPRIGHT. SHOULD STAKING BE REQUIRED, THE TOTAL NUMBER OF TREE STAKES (BEYOND THE MINIMUMS LISTED BELOW) WILL BE LEFT TO THE LANDSCAPE CONTRACTOR'S DISCRETION. SHOULD ANY TREES FALL OR LEAN, THE LANDSCAPE CONTRACTOR SHALL STRAIGHTEN THE TREE, OR REPLACE IT SHOULD IT BECOME DAMAGED. TREE STAKING SHALL ADHERE TO THE FOLLOWING GUIDELINES:
- 1"-2" TREES TWO STAKES PER TREE 2-1/2"-4" TREES THREE STAKES PER TREE
- TREES OVER 4" CALIPER GUY AS NEEDED MULTI-TRUNK TREES THREE STAKES PER TREE MINIMUM, QUANTITY AND POSITIONS AS NEEDED TO STABILIZE THE TREE THREE STAKES PER TREE MINIMUM, QUANTITY AND POSITIONS AS MULTI-TRUNK TREES
- NEEDED TO STABILIZE THE TREE UPON COMPLETION OF PLANTING, CONSTRUCT AN EARTH WATERING BASIN AROUND THE TREE. COVER THE INTERIOR OF THE TREE RING WITH THE WEED BARRIER CLOTH AND TOPDRESS WITH
- MULCH (TYPE AND DEPTH PER PLANS) SHRUB, PERENNIAL, AND GROUNDCOVER PLANTING DIG THE PLANTING HOLES TWICE AS WIDE AND 2" LESS DEEP THAN EACH PLANT'S ROOTBALL. INSTALL
- THE PLANT IN THE HOLE. BACKFILL AROUND THE PLANT WITH SOIL AMENDED PER SOIL TEST INSTALL THE WEED BARRIER CLOTH, OVERLAPPING IT AT THE ENDS. UTILIZE STEEL STAPLES TO KEEP
- THE WEED BARRIER CLOTH IN PLACE. WHEN PLANTING IS COMPLETE, INSTALL MULCH (TYPE AND DEPTH PER PLANS) OVER ALL PLANTING BEDS, COVERING THE ENTIRE PLANTING AREA.
- SOD VARIETY TO BE AS SPECIFIED ON THE LANDSCAPE PLAN.

GUARANTEE PERIODS WILL COMMENCE.

- LAY SOD WITHIN 24 HOURS FROM THE TIME OF STRIPPING. DO NOT LAY IF THE GROUND IS FROZEN. LAY THE SOD TO FORM A SOLID MASS WITH TIGHTLY FITTED JOINTS. BUTT ENDS AND SIDES OF SOD STRIPS - DO NOT OVERLAP. STAGGER STRIPS TO OFFSET JOINTS IN ADJACENT COURSES. ROLL THE SOD TO ENSURE GOOD CONTACT OF THE SOD'S ROOT SYSTEM WITH THE SOIL
- WATER THE SOD THOROUGHLY WITH A FINE SPRAY IMMEDIATELY AFTER PLANTING TO OBTAIN AT LEAST SIX INCHES OF PENETRATION INTO THE SOIL BELOW THE SOD.
- INSTALL MULCH TOPDRESSING, TYPE AND DEPTH PER MULCH NOTE, IN ALL PLANTING AREAS AND DO NOT INSTALL MULCH WITHIN 6" OF TREE ROOT FLARE AND WITHIN 24" OF HABITABLE STRUCTURES, EXCEPT AS MAY BE NOTED ON THESE PLANS. MULCH COVER WITHIN 6" OF CONCRETE WALKS AND
- COVER WITHIN 12" OF WALLS SHALL BE AT LEAST 3" LOWER THAN THE TOP OF WALL. DURING LANDSCAPE PREPARATION AND PLANTING, KEEP ALL PAVEMENT CLEAN AND ALL WORK AREAS IN A NEAT, ORDERLY CONDITION

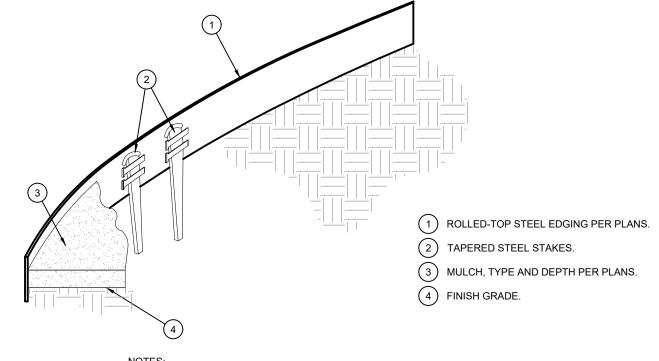
CURBS SHALL NOT PROTRUDE ABOVE THE FINISH SURFACE OF THE WALKS AND CURBS. MULCH

- DISPOSED LEGALLY OF ALL EXCAVATED MATERIALS OFF THE PROJECT SITE. INSPECTION AND ACCEPTANCE UPON COMPLETION OF THE WORK, THE LANDSCAPE CONTRACTOR SHALL PROVIDE THE SITE CLEAN, FREE OF DEBRIS AND TRASH, AND SUITABLE FOR USE AS INTENDED. THE LANDSCAPE CONTRACTOR SHALL THEN REQUEST AN INSPECTION BY THE OWNER TO DETERMINE FINAL ACCEPTABILITY.
- WHEN THE INSPECTED PLANTING WORK DOES NOT COMPLY WITH THE CONTRACT DOCUMENTS. THE LANDSCAPE CONTRACTOR SHALL REPLACE AND/OR REPAIR THE REJECTED WORK TO THE OWNER'S SATISFACTION WITHIN 24 HOURS THE LANDSCAPE MAINTENANCE PERIOD WILL NOT COMMENCE UNTIL THE LANDSCAPE WORK HAS BEEN RE-INSPECTED BY THE OWNER AND FOUND TO BE ACCEPTABLE. AT THAT TIME, A WRITTEN NOTICE OF FINAL ACCEPTANCE WILL BE ISSUED BY THE OWNER, AND THE MAINTENANCE AND
- J. LANDSCAPE MAINTENANCE THE LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF ALL WORK SHOWN ON THESE PLANS FOR 90 DAYS BEYOND FINAL ACCEPTANCE OF ALL LANDSCAPE WORK BY THE OWNER. LANDSCAPE MAINTENANCE SHALL INCLUDE WEEKLY SITE VISITS FOR THE FOLLOWING ACTIONS (AS APPROPRIATE): PROPER PRUNING, RESTAKING OF TREES, RESETTING OF PLANTS THAT HAVE SETTLED, MOWING AND AERATION OF LAWNS, WEEDING, TREATING FOR INSECTS AND DISEASES, REPLACEMENT OF MULCH, REMOVAL OF LITTER, REPAIRS TO THE IRRIGATION SYSTEM DUE TO FAULTY PARTS AND/OR WORKMANSHIP, AND THE APPROPRIATE WATERING OF ALL PLANTINGS. THE LANDSCAPE CONTRACTOR SHALL MAINTAIN THE IRRIGATION SYSTEM IN PROPER WORKING ORDER, WITH SCHEDULING ADJUSTMENTS BY SEASON TO MAXIMIZE WATER CONSERVATION.
- SHOULD SEEDED AND/OR SODDED AREAS NOT BE COVERED BY AN AUTOMATIC IRRIGATION SYSTEM, THE LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR WATERING THESE AREAS AND OBTAINING
- TO ACHIEVE FINAL ACCEPTANCE AT THE END OF THE MAINTENANCE PERIOD, ALL OF THE FOLLOWING THE LANDSCAPE SHALL SHOW ACTIVE, HEALTHY GROWTH (WITH EXCEPTIONS MADE FOR SEASONAL DORMANCY). ALL PLANTS NOT MEETING THIS CONDITION SHALL BE REJECTED AND

A FULL, HEALTHY STAND OF PLANTS AT NO ADDITIONAL COST TO THE OWNER.

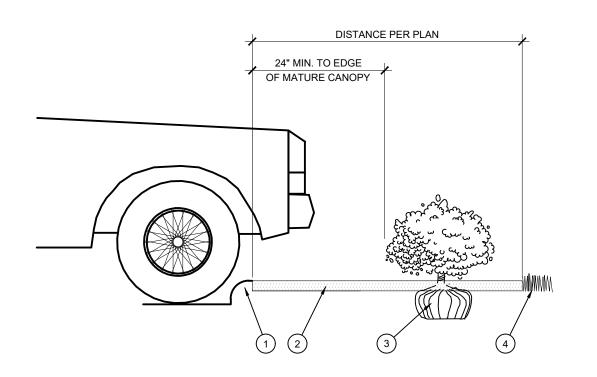
- REPLACED BY HEALTHY PLANT MATERIAL PRIOR TO FINAL ACCEPTANCE. ALL HARDSCAPE SHALL BE CLEANED PRIOR TO FINAL ACCEPTANCE. SODDED AREAS MUST BE ACTIVELY GROWING AND MUST REACH A MINIMUM HEIGHT OF 1 1/2
- INCHES BEFORE FIRST MOWING. BARE AREAS LARGER THAN TWELVE SQUARE INCHES MUST BE RESODDED (AS APPROPRIATE) PRIOR TO FINAL ACCEPTANCE. ALL SODDED TURF SHALL BE NEATLY MOWED. WARRANTY PERIOD, PLANT GUARANTEE AND REPLACEMENTS
- THE LANDSCAPE CONTRACTOR SHALL GUARANTEE ALL TREES, SHRUBS, PERENNIALS, SOD, AND IRRIGATION SYSTEMS FOR A PERIOD OF ONE YEAR FROM THE DATE OF THE OWNER'S FINAL ACCEPTANCE (90 DAYS FOR ANNUAL PLANTS). THE CONTRACTOR SHALL REPLACE, AT HIS OWN EXPENSE AND TO THE SATISFACTION OF THE OWNER ANY PLANTS WHICH DIE IN THAT TIME OR REPAIR ANY PORTIONS OF THE IRRIGATION SYSTEM WHICH OPERATE IMPROPERLY. AFTER THE INITIAL MAINTENANCE PERIOD AND DURING THE GUARANTEE PERIOD. THE LANDSCAPE
- CONTRACTOR SHALL ONLY BE RESPONSIBLE FOR REPLACEMENT OF PLANTS WHEN PLANT DEATH CANNOT BE ATTRIBUTED DIRECTLY TO OVERWATERING OR OTHER DAMAGE BY HUMAN ACTIONS. PROVIDE A MINIMUM OF (2) COPIES OF RECORD DRAWINGS TO THE OWNER UPON COMPLETION OF WORK. A RECORD DRAWING IS A RECORD OF ALL CHANGES THAT OCCURRED IN THE FIELD AND THAT ARE

DOCUMENTED THROUGH CHANGE ORDERS, ADDENDA, OR CONTRACTOR/CONSULTANT DRAWING MARKUPS.



1) INSTALL EDGING SO THAT STAKES WILL BE ON INSIDE OF PLANTING BED. BOTTOM OF EDGING SHALL BE BURIED A MINIMUM OF 1" BELOW FINISH GRADE. B) TOP OF MULCH SHALL BE 1" LOWER THAN TOP OF EDGING.

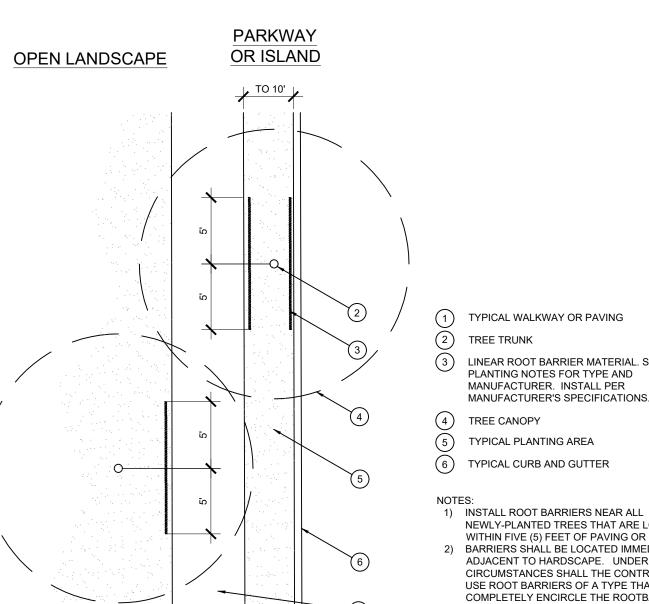
TEEL EDGING



- (2) MULCH LAYER. (3) PLANT.
- (4) TURF (WHERE SHOWN ON PLAN)



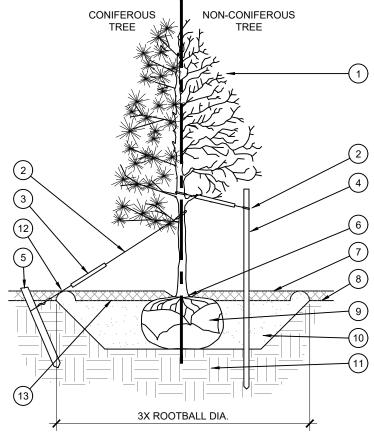
ROOT BARRIER - PLAN VIEW

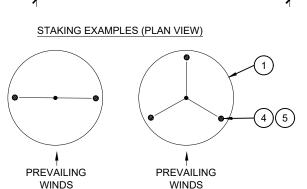


1) TYPICAL WALKWAY OR PAVING (3) LINEAR ROOT BARRIER MATERIAL. SEE PLANTING NOTES FOR TYPE AND MANUFACTURER. INSTALL PER

- (6) TYPICAL CURB AND GUTTER
- 1) INSTALL ROOT BARRIERS NEAR ALL NEWLY-PLANTED TREES THAT ARE LOCATED WITHIN FIVE (5) FEET OF PAVING OR CURBS.) BARRIERS SHALL BE LOCATED IMMEDIATELY ADJACENT TO HARDSCAPE. UNDER NO
- CIRCUMSTANCES SHALL THE CONTRACTOR USE ROOT BARRIERS OF A TYPE THAT COMPLETELY ENCIRCLE THE ROOTBALL.

DIRECTOR OF PLANNING & ZONING



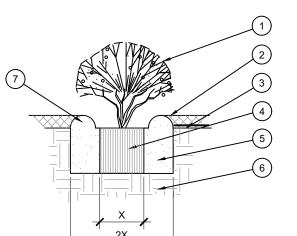




- (1) TREE CANOPY.
- 2) CINCH-TIES (24" BOX/2" CAL, TREES AND SMALLER) OR 12 GAUGE GALVANIZED WIRE WITH NYLON TREE STRAPS AT TREE AND STAKE (36" BOX/2.5" CAL. TREES AND LARGER). SECURE TIES OR STRAPS TO TRUNK JUST ABOVE LOWEST MAJOR BRANCHES.
- (3) 24" X 3/4" P.V.C. MARKERS OVER WIRES.
- (4) GREEN STEEL T-POSTS. EXTEND POSTS 12" MIN. INTO
- UNDISTURBED SOIL. (5) PRESSURE-TREATED WOOD DEADMAN, TWO PER
- TREE (MIN.). BURY OUTSIDE OF PLANTING PIT AND 18" MIN. INTO UNDISTURBED SOIL.
- (7) MULCH, TYPE AND DEPTH PER PLANS. DO NOT PLACE MULCH WITHIN 6" OF TRUNK.
- (8) FINISH GRADE.

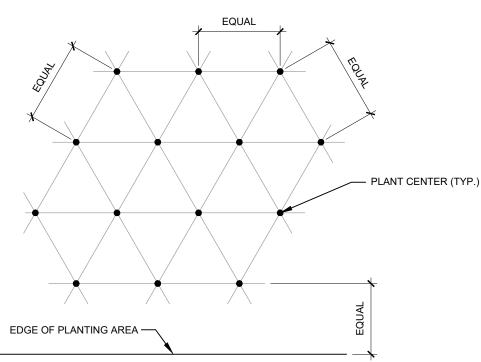
(6) TRUNK FLARE.

- (9) ROOT BALL.
- 10) BACKFILL, AMEND AND FERTILIZE ONLY AS RECOMMENDED IN SOIL FERTILITY ANALYSIS
- (11) UNDISTURBED NATIVE SOIL
- (12) 4" HIGH EARTHEN WATERING BASIN (13) FINISH GRADE.
- SCARIFY SIDES OF PLANTING PIT PRIOR TO SETTING TREE REMOVE EXCESS SOIL APPLIED ON TOP OF THE ROOTBALL THAT COVERS THE ROOT FLARE. THE PLANTING HOLE DEPTH SHALL BE SUCH THAT THE ROOTBALL RESTS ON UNDISTURBED SOIL, AND THE
- ROOT FLARE IS 2"-4" ABOVE FINISH GRADE. FOR B&B TREES, CUT OFF BOTTOM 1/3 OF WIRE BASKET BEFORE PLACING TREE IN HOLE, CUT OFF AND REMOVE REMAINDER OF BASKET AFTER TREE IS SET IN HOLE, REMOVE ALL NYLON TIES, TWINE, ROPE, AND OTHER PACKING MATERIAL. REMOVE AS MUCH BURLAP FROM AROUND ROOTBALL AS IS PRACTICAL.
- 4. REMOVE ALL NURSERY STAKES AFTER PLANTING. FOR TREES 36" BOX/2.5" CAL. AND LARGER, USE THREE STAKES OR DEADMEN (AS APPROPRIATE), SPACED EVENLY AROUND TREE
- 6. STAKING SHALL BE TIGHT ENOUGH TO PREVENT TRUNK FROM BENDING, BUT LOOSE ENOUGH TO ALLOW SOME TRUNK MOVEMENT



- (1) SHRUB, PERENNIAL, OR ORNAMENTAL GRASS. 2) MULCH TYPE AND DEPTH PER PLANS. PLACE NO
- MORE THAN 1" OF MULCH WITHIN 6" OF PLANT CENTER
- (3) FINISH GRADE.
- (4) ROOT BALL.
- (5) BACKFILL. AMEND AND FERTILIZE ONLY AS RECOMMENDED IN SOIL FERTILITY ANALYSIS.
- (6) UNDISTURBED NATIVE SOIL.
- (7) 3" HIGH EARTHEN WATERING BASIN.

SHRUB AND PERENNIAL PLANTING



NOTE: ALL PLANTS SHALL BE PLANTED AT EQUAL TRIANGULAR SPACING (EXCEPT WHERE SHOWN ON PLANS AS INFORMAL GROUPINGS). REFER TO PLANT LEGEND FOR SPACING DISTANCE BETWEEN PLANTS

THE FOLLOWING FORMULA: TOTAL PERIMETER LENGTH / PLANT SPACING = TOTAL PLANT SUBTRACTION

1) STEP 1: DETERMINE TOTAL PLANTS FOR THE AREA WITH THE FOLLOWING FORMULA: TOTAL AREA / AREA DIVIDER = TOTAL PLANTS

0.39 3.46 0.60 5 41 0.87 7.79 2) STEP 2: SUBTRACT THE ROW (S) OF PLANTS THAT WOULD OCCUR AT THE EDGE OF THE PLANTED AREA WITH

EXAMPLE: PLANTS AT 18" O.C. IN 100 SF PLANTING AREA, 40 LF PERIMETER STEP 1: 100 SF/1.95 = 51 PLANTS STEP 2: 51 PLANTS - (40 LF / 1.95 = 21 PLANTS) = 30 PLANTS TOTAL

PLANT SPACING

This drawing has been revised to show those changes during the construction process reported by the contractor to ClayMoore Engineering, Inc. and considered to be significant. This drawing is not guaranteed to be "As Built" but is based on the information made

I HEREBY CERTIFY THAT THE ABOVE AND FOREGOING SITE PLAN FOR A DEVELOPMENT IN THE CITY OF ROCKWALL, TEXAS, WAS APPROVED BY THE PLANNING & ZONING COMMISSION OF THE CITY OF ROCKWALL ON THE _____ DAY OF _ WITNESS OUR HANDS, THIS _____ DAY OF _____, ____

LEGAL DESCRIPTION: LOT 1A-R, BLOCK A, LOFLAND INDUSTRIAL PARK ADDITION **ROCKWALL TEXAS**

B.J.T. LEWIS

CASE #: SP2021-001

CENTRAL APPRAISAL DISTRICT OF ROCKWALL

841 JUSTIN ROAD

PH: 972-771-2034

ROCKWALL, TX 75087

BEDFORD, TX 76021

PH: 817.281.0572

ROCKWAL

CLAYMOORE ENGINEERING, INC. 1903 CENTRAL DRIVE. SUITE #406 **EVERGREEN** (800) 680-6630 15455 Dallas Pkwy., Ste 600 Addison, TX 75001 www.EvergreenDesignGroup.com

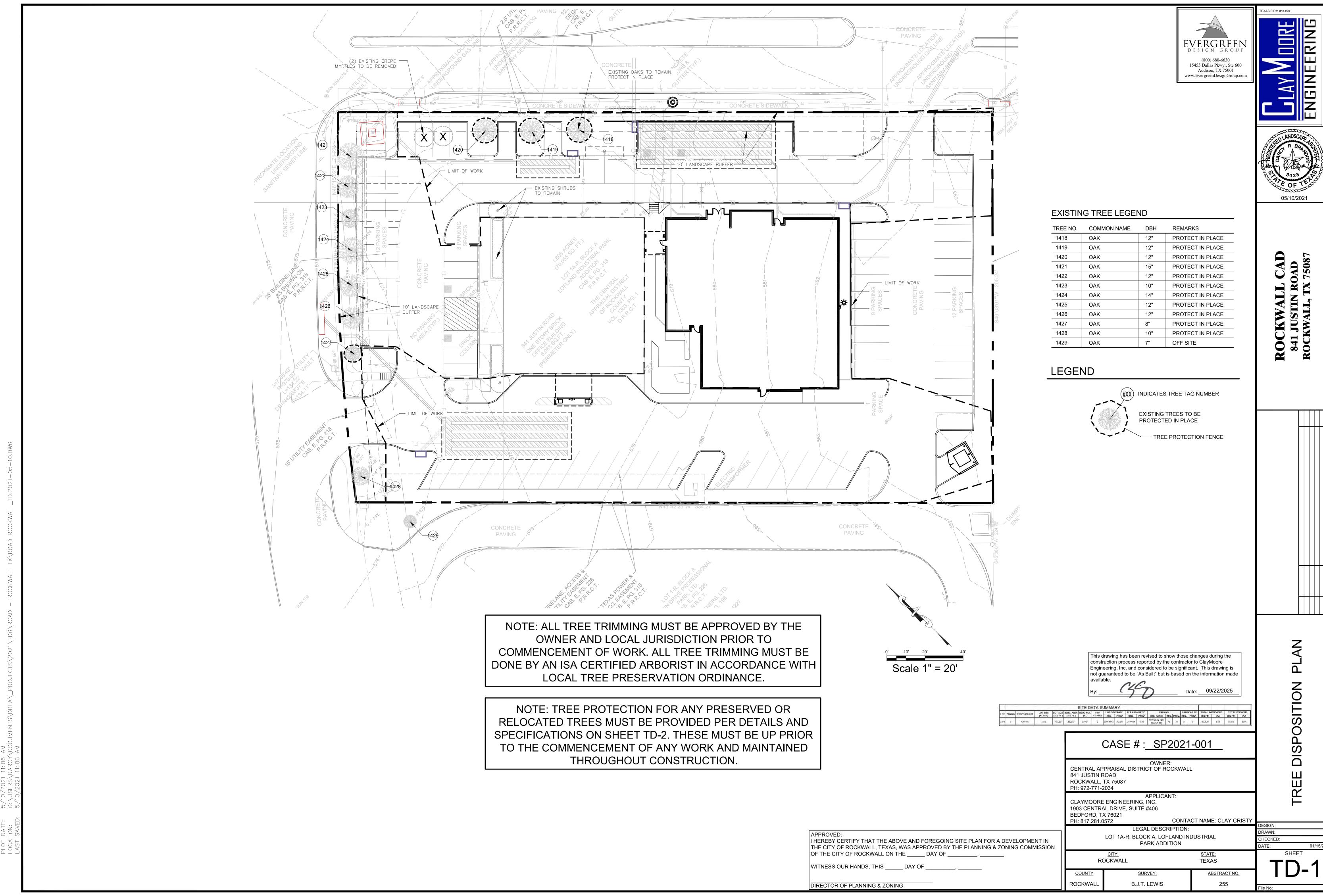


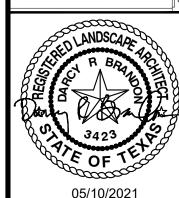


CONTACT NAME: CLAY CRIST HECKED: SHEET

ABSTRACT NO

255





- 2. POSTS: POSTS SHALL BE A MINIMUM OF 72 INCHES LONG AND STEEL 'T' SHAPED WITH A MINIMUM WEIGHT OF 1.3 POUNDS PER LINEAR FOOT.
- 3. TIE WIRE: WIRE FOR ATTACHING THE FABRIC TO THE T-POSTS SHALL BE NOT LESS THAN NO. 12 GAUGE GALVANIZED WIRE,
- 4. USED MATERIALS: PREVIOUSLY-USED MATERIALS, MEETING THE ABOVE

REQUIREMENTS AND WHEN APPROVED BY THE OWNER, MAY BE USED.

CONSTRUCTION METHODS

ALL TREES AND SHRUBS SHOWN TO REMAIN WITHIN THE PROXIMITY OF THE CONSTRUCTION SITE SHALL BE PROTECTED PRIOR TO BEGINNING ANY DEVELOPMENT ACTIVITY.

- 2. EMPLOY THE SERVICES OF AN ISA (INTERNATIONAL SOCIETY OF ARBORICULTURE) CERTIFIED ARBORIST AND OBTAIN ALL REQUIRED PERMITS TO PRUNE THE EXISTING TREES FOR CLEANING, RAISING AND THINNING, AS MAY BE REQUIRED.
- 3. PROTECTIVE FENCING SHALL BE ERECTED OUTSIDE THE CRITICAL ROOT ZONE (CRZ, EQUAL TO 1' FROM THE TRUNK FOR EVERY 1" OF DBH) AT LOCATIONS SHOWN IN THE PLANS OR AS DIRECTED BY THE LANDSCAPE CONSULTANT AND/OR CITY ARBORIST, AND IN ACCORDANCE WITH THE DETAILS SHOWN ON THE PLANS. FENCING SHALL BE MAINTAINED AND REPAIRED BY THE CONTRACTOR DURING SITE CONSTRUCTION. TREES IN CLOSE PROXIMITY SHALL BE FENCED TOGETHER, RATHER THAN INDIVIDUALLY.
- 4. PROTECTIVE FENCE LOCATIONS IN CLOSE PROXIMITY TO STREET INTERSECTIONS OR DRIVES SHALL ADHERE TO THE APPLICABLE JURISDICTION'S SIGHT DISTANCE CRITERIA.
- 5. THE PROTECTIVE FENCING SHALL BE ERECTED BEFORE SITE WORK COMMENCES AND SHALL REMAIN IN PLACE DURING THE ENTIRE CONSTRUCTION PHASE.
- 6. THE INSTALLATION POSTS SHALL BE PLACED EVERY 6 FEET ON CENTER AND EMBEDDED TO 18 INCHES DEEP. MESH FABRIC SHALL BE ATTACHED TO THE INSTALLATION POSTS BY THE USE OF SUFFICIENT WIRE TIES TO SECURELY FASTEN THE FABRIC TO THE T-POSTS TO HOLD THE FABRIC IN A STABLE AND UPRIGHT POSITION.

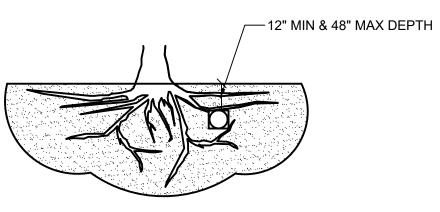
7. WITHIN THE CRZ:

- a. DO NOT CLEAR, FILL OR GRADE IN THE CRZ OF ANY TREE.
- b. DO NOT STORE, STOCKPILE OR DUMP ANY JOB MATERIAL, SOIL OR RUBBISH UNDER THE SPREAD OF THE TREE BRANCHES. c. DO NOT PARK OR STORE ANY EQUIPMENT OR SUPPLIES UNDER THE
- d. DO NOT SET UP ANY CONSTRUCTION OPERATIONS UNDER THE TREE CANOPY (SUCH AS PIPE CUTTING AND THREADING, MORTAR MIXING,
- PAINTING OR LUMBER CUTTING). e. DO NOT NAIL OR ATTACH TEMPORARY SIGNS METERS, SWITCHES,
- WIRES, BRACING OR ANY OTHER ITEM TO THE TREES. f. DO NOT PERMIT RUNOFF FROM WASTE MATERIALS INCLUDING SOLVENTS, CONCRETE WASHOUTS, ASPHALT TACK COATS (MC-30 OIL), ETC. TO ENTER THE CRZ. BARRIERS ARE TO BE PROVIDED TO PREVENT SUCH RUNOFF SUBSTANCES FROM ENTERING THE CRZ WHENEVER POSSIBLE, INCLUDING IN AN AREA WHERE RAIN OR SURFACE WATER COULD CARRY SUCH MATERIALS TO THE ROOT SYSTEM OF THE TREE.
- 8. ROUTE UNDERGROUND UTILITIES TO AVOID THE CRZ. IF DIGGING IS UNAVOIDABLE, BORE UNDER THE ROOTS, OR HAND DIG TO AVOID SEVERING THEM.

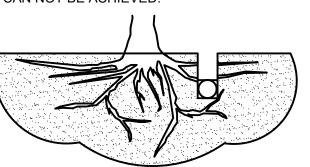
- 9. WHERE EXCAVATION IN THE VICINITY OF TREES MUST OCCUR, SUCH AS FOR IRRIGATION INSTALLATION, PROCEED WITH CAUTION, AND USING HAND TOOLS ONLY
- 10. THE CONTRACTOR SHALL NOT CUT ROOTS LARGER THAN ONE INCH IN DIAMETER WHEN EXCAVATION OCCURS NEAR EXISTING TREES. ALL ROOTS LARGER THAN ONE INCH IN DIAMETER ARE TO BE CUT CLEANLY. FOR OAKS ONLY, ALL WOUNDS SHALL BE PAINTED WITH WOUND SEALER
- 11. REMOVE ALL TREES, SHRUBS OR BUSHES TO BE CLEARED FROM PROTECTED ROOT ZONE AREAS BY HAND.
- 12. TREES DAMAGED OR KILLED DUE TO CONTRACTOR'S NEGLIGENCE DURING CONSTRUCTION SHALL BE MITIGATED AT THE CONTRACTOR'S EXPENSE AND TO THE PROJECT OWNER'S AND LOCAL JURISDICTION'S SATISFACTION.
- 13. ANY TREE REMOVAL SHALL BE APPROVED BY THE OWNER AND LOCAL JURISDICTION PRIOR TO ITS REMOVAL, AND THE CONTRACTOR SHALL HAVE ALL REQUIRED PERMITS FOR SUCH ACTIVITIES.
- 14. COVER EXPOSED ROOTS AT THE END OF EACH DAY WITH SOIL, MULCH OR WET BURLAP.
- 15. IN CRITICAL ROOT ZONE AREAS THAT CANNOT BE PROTECTED DUING CONSTRUCTION AND WHERE HEAVY TRAFFIC IS ANTICIPATED, COVER THE SOIL WITH EIGHT INCHES OF ORGANIC MULCH TO MINIMIZE SOIL COMPACTION. THIS EIGHT INCH DEPTH OF MULCH SHALL BE MAINTAINED THROUGHOUT CONSTRUCTION.
- 16. WATER ALL TREES IMPACTED BY CONSTRUCTION ACTIVITIES, DEEPLY ONCE A WEEK DURING PERIODS OF HOT DRY WEATHER. SPRAY TREE CROWNS WITH WATER PERIODICALLY TO REDUCE DUST ACCUMULATION ON THE LEAVES.
- 17. WHEN INSTALLING CONCRETE ADJACENT TO THE ROOT ZONE OF A TREE, USE A PLASTIC VAPOR BARRIER BEHIND THE CONCRETE TO PROHIBIT LEACHING OF LIME INTO THE SOIL.
- 18. CONTRACTOR SHALL REMOVE AND DISPOSE OF ALL TREE PROTECTION FENCING WHEN ALL THREATS TO THE EXISTING TREES FROM CONSTRUCTION-RELATED ACTIVITIES HAVE BEEN REMOVED.

TREES THAT ARE MARKED TO BE PRESERVED ON A SITE PLAN AND FOR WHICH UTILITIES MUST PASS TROUGH THEIR ROOT PROTECTION ZONES MAY REQUIRE TUNNELING AS OPPOSED TO OPEN TRENCHES. THE DECISION TO TUNNEL WILL BE DETERMINED ON A CASE BY CASE BASIS BY THE ENGINEER.

TUNNELS SHALL BE DUG THROUGH THE ROOT PROTECTION ZONE IN ORDER TO MINIMIZE ROOT DAMAGE.



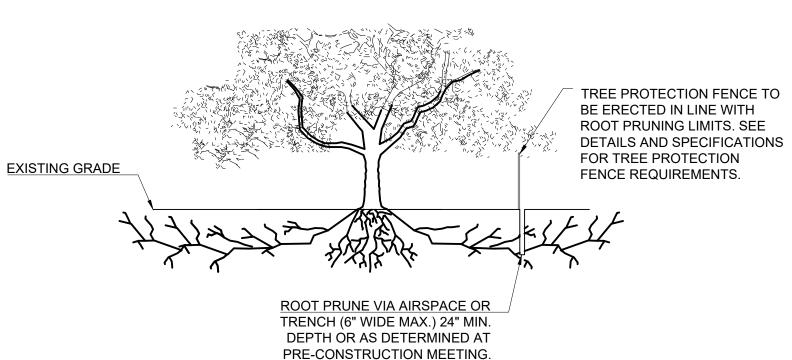
TUNNEL TO MINIMIZE ROOT DAMAGE (TOP) AS OPPOSED TO SURFACE-DUG TRENCHES IN ROOT PROTECTION ZONE WHEN THE 5' MINIMUM DISTANCE FROM TRUNK CAN NOT BE ACHIEVED.



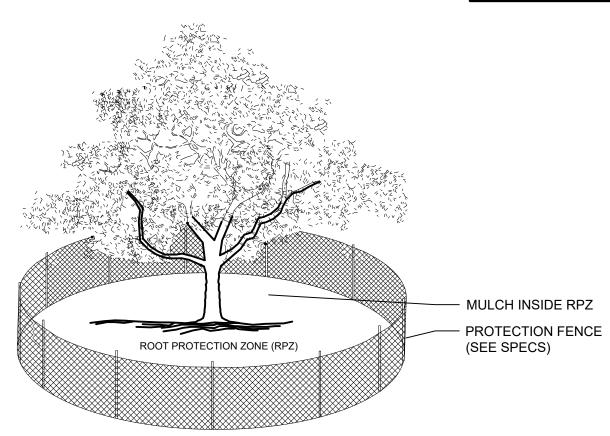
OPEN TRENCHING MAY BE USED IF EXPOSED TREE ROOTS DO NOT EXCEED 3" OR ROOTS CAN BE BENT BACK.

BORING THROUGH ROOT PROTECTION ZONE

- RETENTION AREAS WILL BE SET AS PART OF THE REVIEW PROCESS AND PRE-CONSTRUCTION MEETING.
- BOUNDARIES OF RETENTION AREAS MUST BE STAKED AT THE PRE-CONSTRUCTION MEETING AND FLAGGED PRIOR TO ROOT PRUNING.
- EXACT LOCATION OF ROOT PRUNING SHALL BE DETERMINED IN THE FIELD IN
- COORDINATION WITH THE FORESTRY INSPECTOR. TRENCH SHOULD BE IMMEDIATELY BACKFILLED WITH EXCAVATED SOIL OR OTHER
- ORGANIC SOIL AS SPECIFIED PER PLAN OR BY THE FORESTRY INSPECTOR. ROOTS SHALL BE CLEANLY CUT USING VIBRATORY KNIFE OR OTHER ACCEPTABLE EQUIPMENT. ROT PRUNING METHODS AND MEANS MUST BE IN ACCORDANCE WITH
- ANSI STANDARD A3000. 6. ALL PRUNING MUST BE EXECUTED AT LOD SHOWN ON PLANS OR AS AUTHORIZED
- IN WRITING BY THE FORESTRY INSPECTOR. SUPPLEMENTAL WATERING MAY BE REQUIRED FOR ROOT PRUNED TREES THROUGHOUT THE GROWING SEASON DURING CONSTRUCTION AND SUBSEQUENT WARRANTY AND MAINTENANCE PERIOD.



ROOT PRUNING DETAIL



1. THE FENCING LOCATION SHOWN ABOVE IS DIAGRAMATIC ONLY AND WILL CONFORM TO THE DRIP LINE AND BE LIMITED TO PROJECT BOUNDARY. WHERE MULTIPLE ADJACENT TREES WILL BE ENCLOSED BY FENCING, THE FENCING SHALL BE CONTINUOUS AROUND ALL TREES. 2. FOR ACCEPTABLE FENCING MATERIALS SEE

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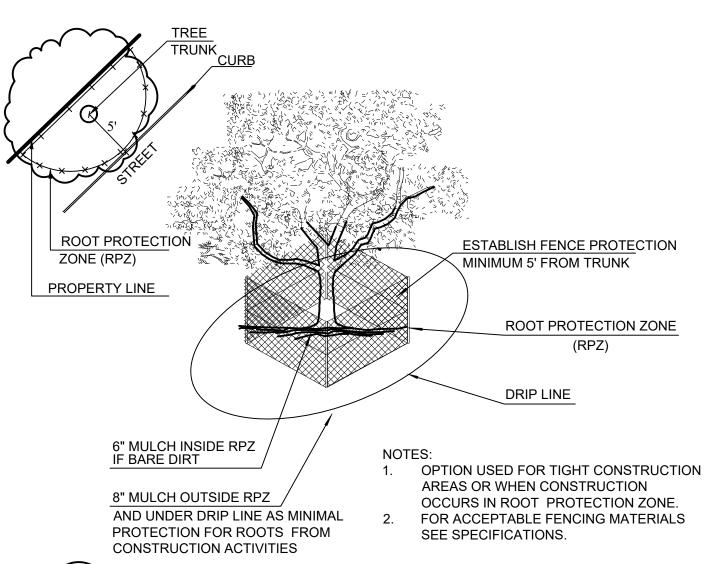
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05/102021

SPECIFICATIONS. TREE PROTECTION FENCE



TREE PROTECTION FENCE - TIGHT CONSTRUCTION

ROCKWAL

CASE #: SP2021-001 CENTRAL APPRAISAL DISTRICT OF ROCKWALL 841 JUSTIN ROAD ROCKWALL, TX 75087

PH: 972-771-2034 CLAYMOORE ENGINEERING, INC. 1903 CENTRAL DRIVE, SUITE #406 BEDFORD, TX 76021 CONTACT NAME: CLAY CRIST PH: 817.281.0572

> LEGAL DESCRIPTION LOT 1A-R, BLOCK A, LOFLAND INDUSTRIAL PARK ADDITION ROCKWALL **TEXAS**

> > 255

B.J.T. LEWIS

 \Box \odot TREE SPE

HECKED: SHEET

TREE REMOVAL PERMIT HAS NOT BEEN ISSUED AND SHALL ERECT BARRIERS FOR THE PROTECTION OF THE TREES ACCORDING TO THE

(1) AROUND AN AREA AT OR GREATER THAN A SIX-FOOT RADIUS OF ALL SPECIES OF MANGROVES AND PROTECTED CABBAGE PALMS; AROUND AN AREA AT OR GREATER THAN THE FULL DRIPLINE OF ALL PROTECTED NATIVE PINES;

(B) NO PERSON SHALL ATTACH ANY SIGN, NOTICE OR OTHER OBJECT TO ANY PROTECTED TREE OR FASTEN ANY WIRES, CABLES, NAILS OR SCREWS TO ANY PROTECTED TREE IN ANY MANNER THAT COULD PROVE HARMFUL TO THE PROTECTED TREE. EXCEPT AS NECESSARY IN CONJUNCTION WITH ACTIVITIES IN THE PUBLIC INTEREST.

OR MATERIAL WITHIN THE OUTSIDE PERIMETER OF THE CROWN (DRIPLINE) OR ON THE NEARBY GROUND OF ANY TREE OR GROUP OF THE CONTRACTOR SHALL NOT CAUSE OR PERMIT STORAGE OF BUILDING MATERIAL AND/OR EQUIPMENT, OR DISPOSAL OF WASTE

(F) PRIOR TO ISSUING A CERTIFICATE OF OCCUPANCY OR COMPLIANCE FOR ANY DEVELOPMENT, BUILDING OR STRUCTURE, ALL TREES

DESIGNATED TO BE PRESERVED THAT WERE DESTROYED DURING CONSTRUCTION SHALL BE REPLACED BY THE CONTRACTOR WITH TREES OF EQUIVALENT DIAMETER AT BREAST HEIGHT TREE CALIPER AND OF THE SAME SPECIES AS SPECIFIED BY THE CITY ADMINISTRATOR. BEFORE OCCUPANCY OR USE, UNLESS APPROVAL FOR THEIR REMOVAL HAS BEEN GRANTED UNDER PERMIT.

(H) IF, IN THE OPINION OF THE CITY ADMINISTRATOR, DEVELOPMENT ACTIVITIES WILL SO SEVERELY STRESS SLASH PINES OR ANY OTHER PROTECTED TREE SUCH THAT THEY ARE MADE SUSCEPTIBLE TO INSECT ATTACK, PREVENTATIVE SPRAYING OF THESE TREES BY THE CONTRACTOR MAY BE REQUIRED.

TREE PROTECTION GENERAL NOTES

AROUND AN AREA AT OR GREATER THAN TWO-THIRDS OF THE DRIPLINE OF ALL OTHER PROTECTED SPECIES.

TREES WHICH IS TO BE PRESERVED. WITHIN THE OUTSIDE PERIMETER OF THE CROWN (DRIPLINE) OF ANY TREE OR ON NEARBY GROUND, MATERIAL SUCH AS PAINTS, OIL, SOLVENTS, ASPHALT, CONCRETE, MORTAR OR ANY OTHER MATERIAL HARMFUL TO THE LIFE OF THE TREE

(A) PRIOR TO THE LAND CLEARING STAGE OF DEVELOPMENT, THE CONTRACTOR SHALL CLEARLY MARK ALL PROTECTED TREES FOR WHICH A

(C) DURING THE CONSTRUCTION STAGE OF DEVELOPMENT, THE CONTRACTOR SHALL NOT CAUSE OR PERMIT THE CLEANING OF EQUIPMENT

(D) NO PERSON SHALL PERMIT ANY UNNECESSARY FIRE OR BURNING WITHIN 30 FEET OF THE DRIPLINE OF A PROTECTED TREE. (E) ANY LANDSCAPING ACTIVITIES WITHIN THE BARRIER AREA SHALL BE ACCOMPLISHED WITH HAND LABOR.

(G) THE CITY ADMINISTRATOR MAY CONDUCT PERIODIC INSPECTIONS OF THE SITE DURING LAND CLEARANCE AND CONSTRUCTION.

This drawing has been revised to show those changes during the construction process reported by the contractor to ClayMoore Engineering, Inc. and considered to be significant. This drawing is not guaranteed to be "As Built" but is based on the information made

Date: __ 09/22/2025

HEREBY CERTIFY THAT THE ABOVE AND FOREGOING SITE PLAN FOR A DEVELOPMENT IN THE CITY OF ROCKWALL, TEXAS, WAS APPROVED BY THE PLANNING & ZONING COMMISSION OF THE CITY OF ROCKWALL ON THE _____ DAY OF _

DIRECTOR OF PLANNING & ZONING

WITNESS OUR HANDS, THIS _____ DAY OF _____, ____