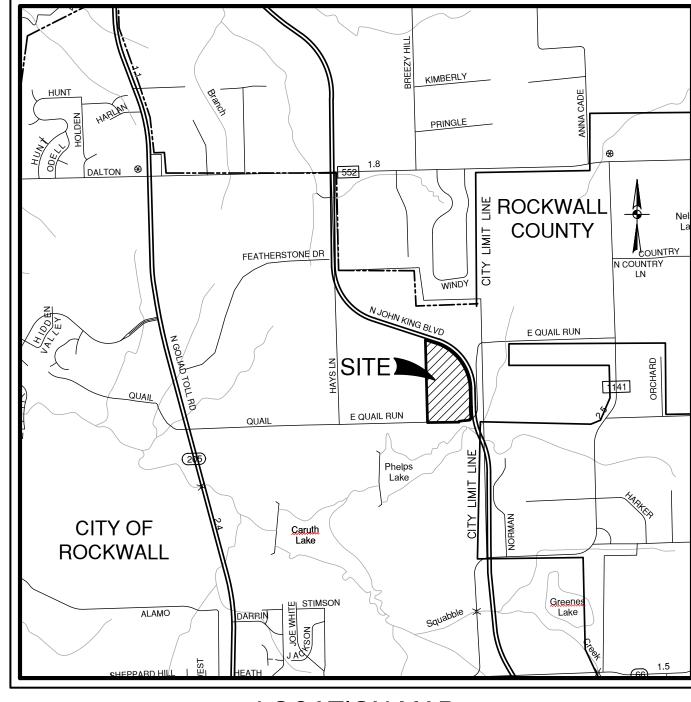
GIDEON GROVE - PHASE 2

CITY OF ROCKWALL, TEXAS CIVIL CONSTRUCTION PLANS



CONTAINING LOTS 1-12, 1X BLK A; LOTS 1-4, 1X, 3X, BLK B; LOTS 1-29, 1X, 18X, 23X, 27X, BLK C; LOTS 1-26, 1X, 14X BLK D; LOTS 1-13, 1X & 6X, BLK E FOR A TOTAL OF 84 SINGLE FAMILY LOTS AND 11 OPEN SPACE LOTS



LOCATION MAP MAPSCO GRID REF: 20D-A SCALE: 1" = 2000'

ENGINEER'S NOTES TO CONTRACTOR:

1. BY COMMENCING CONSTRUCTION, CONTRACTOR AFFIRMS THEY HAVE REVIEWED AND UNDERSTAND THE PLANS AND SPECIFICATIONS. ANY QUESTIONS AND DISCREPANCIES MUST BE ADDRESSED PRIOR TO COMMENCING CONSTRUCTION.

2. ALL CONSTRUCTION SPECIFICATIONS WITHIN THE CITY RIGHTS-OF WAY AND EASEMENTS SHOULD COMPLY WITH CITY OF ROCKWALL STANDARDS. PRIOR APPROVAL TO USE ANY NON-STANDRD MATERIAL

3. CONTRACTOR MUST VERIFY THE LOCATION OF ALL EXISTING UTILITIES PRIOR TO THE START OF CONSTRUCTION. 4. CONTRACTOR MUST INSURE THAT ALL AREAS ARE GRADED TO

5. CONTRACTOR MUST NOTIFY ENGINEER OF ANY DISCREPANCIES BETWEEN THESE PLANS AND ONSITE CONDITIONS.

DOCUMENT HAS BEEN PRODUCED FROM MATERIAL THAT WAS STORED AND/OR TRANSMITTED ELECTRONICALLY AND MAY HAVE BEEN INADVERTENTLY ALTERED. RELY ONLY ON FINAL HARDCOPY MATERIALS BEARING THE CONSULTANT'S ORIGINAL SIGNATURE AND SEAL. AERIAL IMAGERY PROVIDED BY GOOGLE® UNLESS OTHERWISE NOTED. Imagery © 2015, CAPCOG, Digital Globe, Texas Orthoimagery Program, USDA Farm Service Agency.

THE DETENTION POND SHALL BE CONSTRUCTED BEFORE ANY OTHER CONSTRUCTION CAN BEGIN

STANDARDS NOTE

WITHIN THIS PLAN SET, THE TERM "CITY OF ROCKWALL STANDARDS" IS MEANT TO INCLUDE THE CITY OF ROCKWALL STANDARDS OF DESIGN AND CONSTRUCTION MANUAL, DATED OCTOBER 2016, AND THE PUBLIC WORKS CONSTRUCTION STANDARDS AND SPECIFICATIONS, NORTH CENTRAL TEXAS, 5TH EDITION, DATED OCTOBER 2017 - PUBLISHED BY THE NORTH CENTRAL COUNCIL OF GOVERNMENTS. CONTRACTOR SHALL REFER TO BOTH MANUALS PRIOR TO BEGINNING CONSTRUCTION.

RESPONSIBILITY NOTE

ALL RESPONSIBILITY FOR THE ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR THE ADEQUACY OR ACCURACY OF DESIGN.

PREPARED FOR:

QUALICO COMMUNITIES 6950 TCP DRIVE, SUITE 350 McKINNEY, TEXAS 75070

> **CONTACT: JOHN VICK** PHONE: 469-659-6150 EMAIL: john.vick@qualico.com

> > June 2021



FORT WORTH I SAN ANTONIO I AUSTIN I HOUSTON I DALLAS 6500 W FWY, STE 700 I FT. WORTH, TX 76102 I 817.870.3668

TEXAS BOARD OF PROFESSIONAL ENGINEERS, FIRM REGISTRATION #470

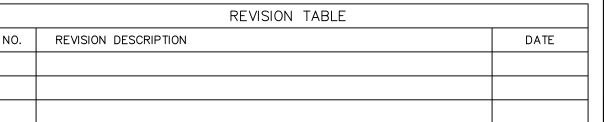
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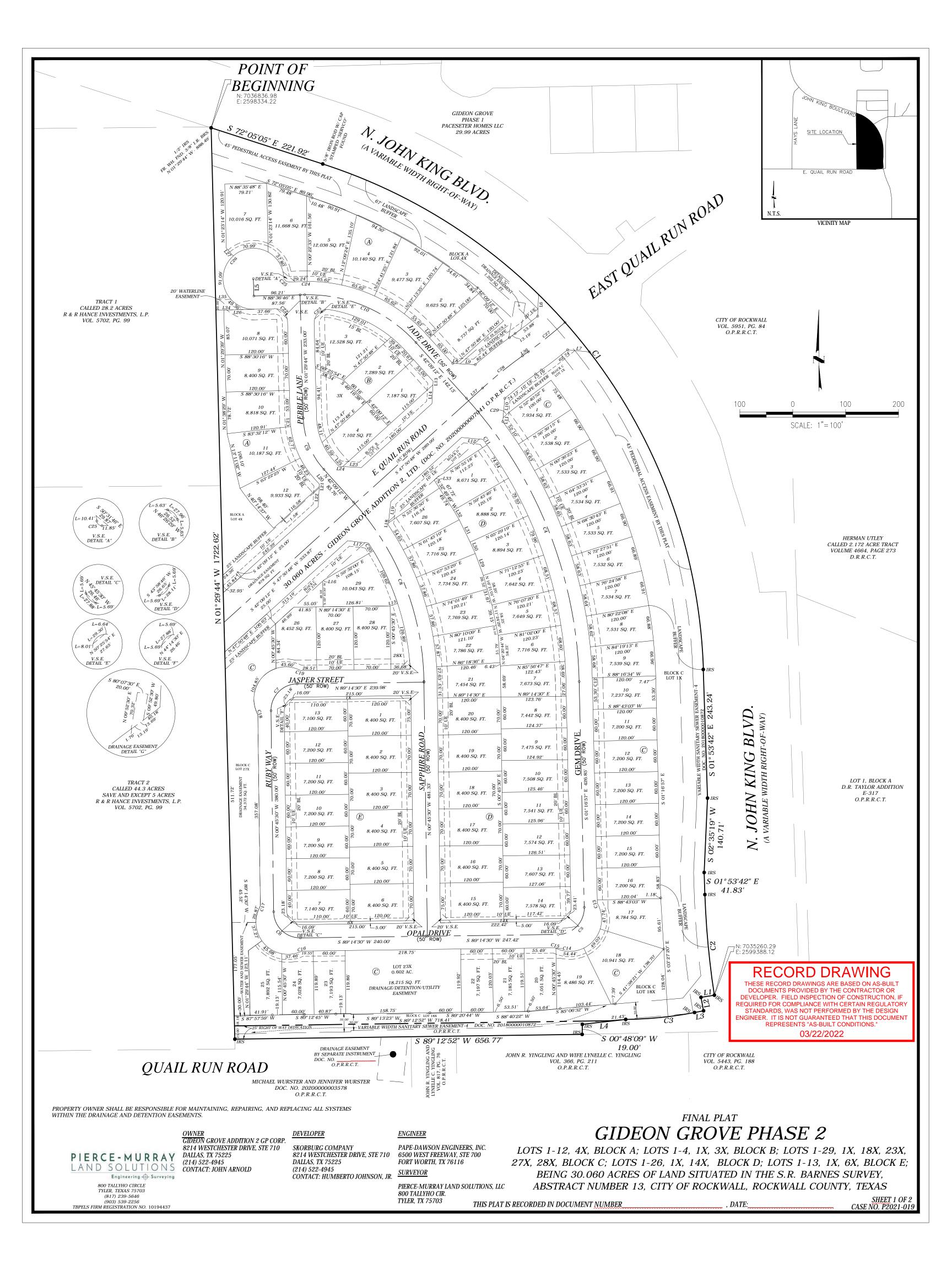
Sheet No Sheet Desription **COVER SHEET** FINAL PLAT **CONSTRUCTION NOTES DEMOLITION PLAN** DRAINAGE AREA MAP - EXISTING DRAINAGE AREA MAP - PROPOSED HYDRAULIC CALCULATIONS STORM DRAIN LINE A - PLAN AND PROFILE STORM DRAIN LINE A - LATERAL PROFILES STORM DRAIN LINE B - PLAN AND PROFILE 1 OF 2 STORM DRAIN LINE B - PLAN AND PROFILE 2 OF 2 STORM DRAIN LINE B1 & B2 - PLAN AND PROFILE STORM DRAIN B - LATERAL PROFILES STORM DRAIN LINE C - PLAN & PROFILE STORM DRAIN LINE C2 - PLAN & PROFILE STORM DRAIN LINE C1 - PLAN & PROFILE (ULTIMATE CONDITION) DETENTION POND **DETENTION POND OUTFALL** STORM DRAINAGE DETAILS (1) STORM DRAINAGE DETAILS (2) STORM DRAINAGE DETAILS (3) E QUAIL RUN ROAD - PLAN AND PROFILE RUBY WAY - PLAN AND PROFILE SAPPHIRE ROAD - PLAN & PROFILE OPAL DRIVE - PLAN AND PROFILE GEM DRIVE - PLAN AND PROFILE GEM DRIVE & JASPER STREET- PLAN AND PROFILE PEBBLE LANE - PLAN AND PROFILE JADE DRIVE - PLAN AND PROFILE PAVING DETAILS PAVING DETAILS STREET LIGHT AND SIGNAGE PLAN PAVEMENT MARKING DETAILS WATER DISTRIBUTION PLAN 1 OF 2 WATER DISTRIBUTION PLAN 2 OF 2 WATER PLAN AND PROFILE SANITARY SEWER PLAN 1 OF 2 SANITARY SEWER PLAN 2 OF 2 SANITARY SEWER PROFILES SS-1, SS-2, SS-3 SANITARY SEWER PROFILE SS-4 DOCUMENTS PROVIDED BY THE CONTRACTOR OR SANITARY SEWER PROFILE SS-5 & SS-6 DEVELOPER. FIELD INSPECTION OF CONSTRUCTION, IF QUIRED FOR COMPLIANCE WITH CERTAIN REGULATOR GRADING PLAN - 1 OF 2 STANDARDS, WAS NOT PERFORMED BY THE DESIGN SINEER. IT IS NOT GUARANTEED THAT THIS DOCUMEN GRADING PLAN - 2 0F 2 REPRESENTS "AS-BUILT CONDITIONS." OVERALL EROSION CONTROL PLAN





EROSION CONTROL DETAILS





GENERAL ITEMS All construction shall conform to the requirements set forth in the City of Rockwall's Engineering Department's "Standards of Design and Construction" and the "Standard Specifications for Public Works Construction" by the North Texas Central Council of Governments, 5th edition amended by the City of Rockwall. The CONTRACTOR shall reference the latest City of Rockwall standard details provided in the Rockwall Engineering Departments "Standards of Design and Construction" manual for details not provided in these plans. The CONTRACTOR shall possess one set of the NCTCOG Standard Specifications and Details and the City of Rockwall's "Standards of Design and Construction" manual on the project site at all times Where any conflicting notes, details or specifications occur in the plans the City of Rockwall General Construction Notes, Standards, Details and Specifications shall govern unless detail or specification is more

The City of Rockwall Engineering Departments "Standards of Design and Construction" can be found online at: http://www.rockwall.com/engr.asp

All communication between the City and the CONTRACTOR shall be through the Engineering Construction Inspector and City Engineer or designated representative only. It is the responsibility of the CONTRACTOR to contact the appropriate department for inspections that do not fall under this approved engineering plan set. Prior to construction, CONTRACTOR shall have in their possession all necessary permits, plans, licenses,

The CONTRACTOR shall have at least one original stamped and signed set of approved engineering plans and specifications on-site and in their possession at all times. A stop work order will be issued if items are not on-site. Copies of the approved plans will not be substituted for the required original "approved plans to

All material submittals, concrete batch designs and shop drawings required for City review and approval shall be submitted by the CONTRACTOR to the City sufficiently in advance of scheduled construction to allow

no less than 10 business days for review and response by the City. All site dimensions are referenced to the face of curb or edge of pavement unless otherwise noted. The City requires ten (10%) percent-two (2) year maintenance bond for paving, paving improvements, water systems, wastewater systems, storm sewer systems including detention systems, and associated fixtures and

structures which are located within the right-of-ways or defined easements. The two (2) year maintenance

bond is to state "from date of City acceptance" as the starting time. A review of the site shall be conducted at twenty (20) months into the two (2) year maintenance period. The design engineer or their designated representative and the CONTRACTOR shall be present to walk the site with the City of Rockwall Engineering Inspection personnel.

EROSION CONTROL & VEGETATION

The CONTRACTOR or developer shall be responsible, as the entity exercising operational control, for all permitting as required by the Environmental Protection Agency (EPA) and the Texas Commission on Environmental Quality (TCEQ). This includes, but is not limited to, preparation of the Storm Water Pollution Prevention Plan (SWPPP), the Construction Site Notice (CSN), the Notice of Intent (NOI), the Notice of Termination (NOT) and any Notice of Change (NOC) and is required to pay all associated fees

Erosion control devices as shown on the erosion control plan for the project shall be installed prior to the start of land disturbing activities. All erosion control devices are to be installed in accordance with the approved plans, specifications and Storm Water Pollution Prevention Plan (SWPPP) for the project. Erosion control devices shall be placed and in working order prior to start of construction. Changes are to be reviewed and approved by the design engineer

and the City of Rockwall prior to implementation If the Erosion Control Plans and Storm Water Pollution Prevention Plan (SWPPP) as approved cannot appropriately control erosion and off-site sedimentation from the project, the erosion control plan and/or the

SWPPP is required to be revised and any changes reported to the Texas Commission on Environmental Quality (TCEQ), when applicable. All erosion control devices shall be inspected weekly by the CONTRACTOR and after all major rain events, or more frequently as dictated in the project Storm Water Pollution Prevention Plan (SWPPP). CONTRACTOR shall provide copies of inspection's reports to the engineering inspection after each

The CONTRACTOR shall not dispose of waste and any materials into streams, waterways or floodplains. The CONTRACTOR shall secure all excavation at the end of each day and dispose of all excess materials. CONTRACTOR shall take all available precautions to control dust. CONTRACTOR shall control dust by sprinkling water or other means as approved by the City Engineer.

CONTRACTOR shall establish grass and maintain the seeded area, including watering, until a "Permanent Stand of Grass" is obtained at which time the project will be accepted by the City. A "Stand of Grass" (not winter rye or weeds) shall consist of 75% to 80% coverage of all disturbed areas and a minimum of one-inch (1") in height as determined by the City. No bare spots will be allowed. Re-seeding will be required in all

washed areas and areas that don't grow. All City right-of-ways shall be sodded if disturbed. No artificial grass is allowed in any City right-of-way and/or easements.

. All adjacent streets/alleys shall be kept clean at all times . CONTRACTOR shall keep construction site clean at all times, immediately contain all debris and trash, all debris and trash shall be removed at the end of each work day, and all vegetation on the construction site 10inches or taller in height must be cut immediately. . Suspension of all construction activities for the project will be enforced by the City if any erosion control

requirements are not meet. Work may commence after deficiency has been rectified. . During construction of the project, all soil stockpiles and borrow areas shall be stabilized or protected with sediment trapping measures. The CONTRACTOR is responsible for the temporary protection and permanent stabilization of all soil stockpiles on-site as well as borrow areas and soil intentionally transported from the

. Where construction vehicles access routes intersect paved or public roads/alleys, construction entrances shall be installed to minimize the transport of sediment by vehicular tracking onto paved surfaces. Where sediment is transferred onto paved or public surfaces, the surface shall be immediately cleaned. Sediment shall be

moved from the surface by shoveling or sweeping and transported to a sediment disposal area, Pavemen washing shall be allowed only after sediment is removed in this manner.

. All drainage inlets shall be protected from siltation, ineffective or unmaintained protection devices shall be immediately replaced and the inlet and storm system cleaned. Flushing is not an acceptable method of

16. During all dewatering operations, water shall be pumped into an approved filtering device prior to discharge into a receiving outlet.

All new Detouring or Traffic Control Plans are required to be submitted to the City for review and approval a minimum of 21 calendar days prior to planned day of implementation When the normal function of the roadway is suspended through closure of any portion of the right-of-way

is an integral element of every traffic control zone. All traffic control plans shall be prepared and submitted to the Engineering Department in accordance with the standards identified in Part VI of the most recent edition of the TMUTCD. Lane closures will not occur on roadways without an approval from the Rockwall Engineering Department and an approved traffic control plan. Traffic control plans shall be required on all roadways as determined by the City Engineer or the designated representative.

All traffic control plans must be prepared, signed, and sealed by an individual that is licensed as a professional engineer in the State of Texas. All traffic control plans and copies of work zone certification must be submitted for review and approval a minimum of three (3) weeks prior to the anticipated temporary traffic control. The CONTRACTOR executing the traffic control plan shall notify all affected property owners two (2) weeks prior to any the closures in writing and verbally.

Any deviation from an approved traffic control plan must be reviewed by the City Engineer or the designated representative. If an approved traffic control plan is not adhered to, the CONTRACTOR will first receive a verbal warning and be required to correct the problem immediately. If the deviation is not corrected, all construction work will be suspended, the lane closure will be removed, and the roadway opened to traffic. All temporary traffic control devices shall be removed as soon as practical when they are no longer needed When work is suspended for short periods of time at the end of the workday, all temporary traffic control devices that are no longer appropriate shall be removed or covered. The first violation of this provision will result in a verbal warning to the construction foreman. Subsequent violations will result in suspension of all work at the job site for a minimum of 48 hours. All contractors working on City funded projects will be

Lane closures on any major or minor arterial will not be permitted between the hours of 6:00 am to 9:00 am and 3:30 pm to 7:00 pm. Where lane closures are needed in a school area, they will not be permitted during peak hours of 7:00 am – 9:00 am and 3:00 pm to 5:00 pm. Closures may be adjusted according to the actual start-finish times of the actual school with approval by the City Engineer. The first violation of this provision will result in a verbal warning to the construction foreman. Subsequent violations will result in suspension of all work at the job site for a minimum of 48 hours. All contractors working on City funded projects will be charged one working day for each 24 hour closure of a roadway whether they are working or not. No traffic signs shall be taken down without permission from the City.

No street/roadway will be allowed to be fully closed.

charged one working day for each 24 hour closure.

It is the CONTRACTOR's responsibility to notify utility companies to arrange for utility locates at least 48 hours prior to beginning construction. The completeness and accuracy of the utility data shown on the plans is not guaranteed by the design engineer or the City. The CONTRACTOR is responsible for verifying the depth and location of existing underground utilities proper to excavating, trenching, or drilling and shall be required to take any precautionary measures to protect all lines shown and .or any other underground utilities not on record or not shown on the plans.

The CONTRACTOR shall be responsible for damages to utilities CONTRACTOR shall adjust all City of Rockwall utilities to the final grades.

All utilities shall be placed underground. CONTRACTOR shall be responsible for the protection of all existing main lines and service lines crossed or exposed by construction operations. Where existing mains or service lines are cut, broken or damaged, the CONTRACTOR shall immediately make repairs to or replace the entire service line with same type of original construction or better. The City of Rockwall can and will intervene to restore service if deemed necessary and charge the CONTRACTOR for labor, equipment, material and loss of water if repairs aren't made in a

timely manner by the CONTRACTOR. The City of Rockwall (City utilities) is not part of the Dig Tess or Texas one Call – 811 – line locate system. All City of Rockwall utility line locates are to be scheduled with the City of Rockwall Service Center. 972 771-7730. A 48-hour advance notice is required for all non-emergency line locates. Underground utility lines shall be installed in accordance with the following standards in addition to other

applicable criteria: a. No more than 500 linear feet of trench may be opened at one time. b. Material used for backfilling trenches shall be properly compacted to 95% standard density in order to minimize erosion, settlement, and promote stabilization that the geotechnical engineer recommends.

c. Applicable safety regulations shall be complied with. This plan details pipes up to 5 feet from the building. Refer to the building plans for building connections CONTRACTOR shall supply and install pipe adapters as necessary. . All underground lines shall be installed, inspected, and approved prior to backfilling.

3. All concrete encasement shall have a minimum of 28 days compressive strength at 3,000 psi (min. 5.5 sack

he CONTRACTOR shall maintain existing water service at all times during construction

Proposed water lines shall be AWWA C900-16 PVC Pipe (blue in color) for all sizes, DR 14 (PC 305) for pipeline sizes 12-inch and smaller, and DR 18 (PC 235) for 14-inch and larger water pipelines unless otherwi shown on water plan and profiles sheets. Proposed water lines shall be constructed with minimum cover of feet for 6-inch through 8-inch, 5 feet for 12-inch through 18-inch and 6 feet for 20-inch and larger. Proposed water line embedment shall be NCTCOG Class 'B-3' as amended by the City of Rockwall

engineering standards of design and construction manual CONTRACTOR shall coordinate the shutting down of all water lines with the City of Rockwall Engineer Inspector and Water Department. The City shall operate all water valves. Allow 5 business days from the date of notice to allow City personnel time to schedule a shut down. Two additional days are required for the CONTRACTOR to notify residents in writing of the shut down after the impacted area has been identified temporary construction work zone traffic control devices shall be installed to effectively guide the motoring Water shut downs impacting businesses during their normal operation hours is not allowed. CONTRACTOR public through the area. Consideration for road user safety, worker safety, and the efficiency of road user flow is required to coordinate with the Rockwall Fire Department regarding any fire watch requirements as well as any costs incurred when the loss of fire protection to a structure occurs.

CONTRACTOR shall furnish and install gaskets on water lines between all dissimilar metals and at valve (both existing and proposed). All fire hydrants and valves removed and salvaged shall be returned to the City of Rockwall Municip

Blue EMS pads shall be installed at every change in direction, valve, curb stop and service tap on the propose water line and every 250'.

All water valve hardware and valve extensions, bolts, nuts and washers shall be 316 stainless steel. All fire hydrants bolts, nuts and washers that are buried shall be 316 stainless steel.

. Abandoned water lines to remain in place shall be cut and plugged and all void spaces within the abandone line shall be filled with grout, flowable fill or an expandable permanent foam product. Valves to be abandonin place shall have any extensions and the valve box removed and shall be capped in concrete. All fire hydrants will have a minimum of 5 feet of clearance around the appurtenance including but not limit

to parking spaces and landscaping. All joints are to be megalug joints with thrust blocking.

3. Water and sewer mains shall be kept 10 feet apart (parallel) or when crossing 2 feet vertical clearance. 4. CONTRACTOR shall maintain a minimum of 4 feet of cover on all water lines.

 All domestic and irrigation services are required to have a testable backflow device with a double check valve installed per the City of Rockwall regulations at the property line and shown on plans.

WASTEWATER LINE NOTES

The CONTRACTOR shall maintain existing wastewater service at all times during construction. Wastewater line for 4-inch through 15-inch shall be Green PVC - SDR 35 (ASTM D3034) [less 10 ft cover and SDR 26 (ASTM D3034) [10 ft or more cover]. For 18-inch and lager wastewater line shall be Green PVC - PS 46 (ASTM F679) [less 10 ft cover] and PS 115 (ASTM F679) [10 ft or more cover]. No service will be allowed on a sanitary sewer line deeper than 10 feet Proposed wastewater line embedment shall be NCTCOG Class 'H' as amended by the City of Rockwall

public works standard design and construction manual. Green EMS pads shall be installed at every 250', manhole, clean out and service lateral on propose wastewater lines.

CONTRACTOR shall CCTV all existing wastewater lines that are to be abandoned to ensure that all latera are accounted for and transferred to proposed wastewater lines prior to abandonment. All abandoned wastewater and force main lines shall be cut and plugged and all void spaces within ti

abandoned line shall be filled with grout, flowable fill or an expandable permanent foam product.

Existing manholes and cleanouts not specifically called to be relocated shall be adjusted to match final grade All wastewater pipes and public services shall be inspected by photographic means (television and DVI prior to final acceptance and after franchise utilities are installed. The CONTRACTOR shall furnish a DVI to the Engineering Construction Inspector for review. Pipes shall be cleaned prior to TV inspection of th pipes. Any sags, open joints, cracked pipes, etc. shall be repaired or removed by the CONTRACTOR at the CONTRACTOR's expense. A television survey will be performed as part of the final testing in the twentiet

(20th) month of the maintenance period. All manholes (public or private) shall be fitted with inflow prevention. The inflow prevention shall conform to the measures called out in standard detail R-5031. All new or existing manholes being modified shall have corrosion protection being Raven Liner 405 epox

coating, ConShield, or approved equal.. Consheild must have terracotta color dye mixed in the precast and cast-in-place concrete. Where connections to existing manholes are made the CONTRACTOR shall reha manhole as necessary and install a 125 mil thick coating of Rayen Liner 405 or approved equal. . All new or existing manholes that are to be placed in pavement shall be fitted with a sealed (gasketed) ri

and cover to prevent inflow. 2. If an existing wastewater main or trunk line is called out to be replaced in place a wastewater bypassing pump plan shall be required and submitted to the Engineering Construction Inspector and City Engineer for approv prior to implementation. Bypass pump shall be fitted with an auto dialer and conform to the City's Noi Ordinance. Plan shall be to the City sufficiently in advance of scheduled construction to allow no less that 10 business days for review and response by the City.

3. CONTRACTOR shall maintain a minimum of 4 feet of cover on all wastewater lines.



ENERAL CONSTRUCTION NOTES Sheet 1 of 2 CITY OF ROCKWALL

ENGINEERING DEPARTMENT

P (972) 771-7746 385 S. Goliad Rockwall, Texas 75087 F (972) 771-7748

DEMOLITION, REMOVAL, DISPOSAL AND EXCAVATION NOTES All pavements to be removed and replaced shall be saw cut to full depth along neat squared lines shown in

Proposed concrete pavement shall be constructed with longitudinal butt construction joints at all connections to existing concrete pavement. All public concrete pavement to be removed and replaced shall be full panel replacement, 1-inch thicker and

on top of 6-inch thick compacted flexbase. No excess excavated material shall be deposited in low areas or along natural drainage ways without written permission from the affected property owner and the City of Rockwall. No excess excavation shall be deposited in the City Limits without a permit from the City of Rockwall. If the CONTRACTOR places excess materials in these areas without written permission, the CONTRACTOR will be responsible for all damages resulting from such fill and shall remove the material at their own cost.

PAVING AND GRADING

DOCUMENT HAS BEEN PRODUCED FROM MATERIAL THAT WAS STORED AND/OR TRANSMITTED ELECTRONICALLY AND MAY HAVE BEEN INADVERTENTLY ALTERED. RELY ONLY ON FINAL HARDCOPY MATERIALS BEARING THE CONSULTANT'S ORIGINAL SIGNATURE AND SEAL. AERIAL IMAGERY PROVIDED BY GOOGLE® UNLESS OTHERWISE NOTED. Imagery © 2016,CAPCOG,Digital Globe,Texas Orthoimagery Program, USDA Farm Service Agency.

All detention systems are to be installed and verified for design compliance along with the associated storm sewer and outflow structures, prior to the start of any paving operations (including building foundations). Erosion protection shall be placed at the pond outflow structures, silt fence along the perimeter of the pond along with any of the associated erosion BMPs noted on the erosion control plan, and the sides and bottom of the detention system shall have either sod or anchored seeded curlex installed prior to any concrete placement. All paving roadway, driveways, fire lanes, drive-isles, parking, dumpster pads, etc. sections shall have a minimum thickness, strength, reinforcement, joint type, joint spacing and subgrade treatment shall at a minimum conform to the City standards of Design and Cons

minimum conform to the City	standards of 1	Jesign and	Construction	and table b	elow.	
Street/Personent True	Minimum Thickness	Streng th 28-	Minimum (sacks /		Steel Re	einforcement
Street/Pavement Type	(inches)	Day (psi)	Machine placed	Hand Placed	Bar#	Spacing (O.C.E.W.)
Arterial	10"	3,600	6.0	6.5	#4 bars	18"
Collector	8"	3,600	6.0	6.5	#4 bars	18"
Residential	6"	3,600	6.0	6.5	#3 bars	24"
Alley	7"-5"-7"	3,600	6.0	6.5	#3 bars	24"
Fire Lane	6"	3,600	6.0	6.5	#3 bars	24"
Driveways	6"	3,600	6.0	6.5	#3 bars	24"
Barrier Free Ramps	6"	3,600	N/A	6.5	#3 bars	24"
Sidewalks	4"	3,000	N/A	5.5	#3 bars	24"
Parking Lot/Drive Aisles	5"	3,000	5.0	5.5	#3 bars	24"
Dumpster Pads	7"	3,600	6.0	6.5	#3 bars	24"

Reinforcing steel shall be tied (100%). Reinforcing steel shall be set on plastic chairs. Bar laps shall be minimum 30 diameters. Sawed transverse dummy joints shall be spaced every 15 feet or 1.25 time longitudinal butt joint spacing whichever is less. Sawing shall occur within 5 to 12 hours after the pour, including sealing. Otherwise, the section shall be removed and longitudinal butt joint constructed. No sand shall be allowed under any paving.

All concrete mix design shall be submitted to the City for review and approval prior to placement. Fly ash may be used in concrete pavement locations provided that the maximum cement reduction does not exceed 20% by weight per C.Y. of concrete. The fly ash replacement shall be 1.25 lbs. per 1.0 lb. cement

All curb and gutter shall be integral (monolithic) with the pavement. All fill shall be compacted by sheep's foot roller to a minimum 95% standard proctor. Maximum loose lift for compaction shall be 8 inches. All lifts shall be tested for density by an independent laboratory. All laboratory compaction reports shall be submitted to the City Engineering Construction Inspector once results are

received. All reports will be required prior to final acceptance. All concrete compression tests and soil compaction/density tests are required to be submitted to the City's Engineering Inspector immediately upon results. . All proposed sidewalks shall include barrier free ramps at intersecting streets, alleys, etc. Barrier free ramps

(truncated dome plate in Colonial or brick red color) shall meet current City and ADA requirements and be approved by the Texas Department of Licensing and Regulation (TDLR). . All public sidewalks shall be doweled into pavement where it abuts curbs and driveways. Expansion joint material shall be used at these locations.

. All connection of proposed concrete pavement to existing concrete pavement shall include a longitudinal butt joint as the load transfer device. All longitudinal butt joints shall be clean, straight and smooth (not jagged in . Cracks formed in concrete pavement shall be repaired or removed by the CONTRACTOR at the City's discretion. CONTRACTOR shall replace existing concrete curbs, sidewalk, paving, a gutters as indicated on

. All residential lots will require individual grading plans submitted during the building permit process that correspond with the engineered grading and drainage area plans. Approval of this plan is not an authorization to grade adjacent properties when the plans or field conditions warrant off-site grading. Written permission must be obtained and signed from the affected property owner(s) and temporary construction easements may be required. The written permission shall be provided to the City as verification of approval by the adjacent property owner(s). Violation of this requirement will result in

the plans and as necessary to connect to the existing infrastructure, including any damage caused by the

suspension of all work at the job site until issue has been rectified. . All cut or fill slopes of non-paved areas shall be a maximum of 4:1 and minimum of 1%. . CONTRACTOR agrees to repair any damage to property and the public right-of-way in accordance with the

City Standards of Design and Construction. . CONTRACTOR shall protect all monuments, iron pins/rods, and property corners during construction. CONTRACTOR shall ensure positive drainage so that runoff will drain by gravity flow to new or existing drainage inlets or sheet flow per these approved plans.

DRAINAGE / STORM SEWER NOTES

The CONTRACTOR shall maintain drainage at all times during construction. Ponding of water in streets, drives, trenches, etc. will not be allowed. Existing drainage ways shall not be blocked or removed unless explicitly stated in the plans or written approval is given by the City.

All structural concrete shall be 4200 psi compressive strength at 28 days minimum 7.0 sack mix, air entrained, unless noted otherwise. Fly ash shall not be allowed in any structural concrete. Proposed storm sewer embedment shall be NCTCOG Class 'B' as amended by the City of Rockwall's Engineering Department Standards of Design and Construction Manual. All public storm pipe shall be a minimum of 18-inch reinforced concrete pipe (RCP), Class III, unless

otherwise noted. All storm pipe entering structures shall be grouted to assure connection at the structure is watertight.

All storm structures shall have a smooth uniform poured mortar invert from invert in to invert out. All storm sewer manholes in paved areas shall be flush with the paving grade, and shall have traffic bearing

All storm sewer pipes and laterals shall be inspected by photographic means (television and DVD) prior to final acceptance and after franchise utilities are installed. The CONTRACTOR shall furnish a DVD to the Engineering Construction Inspector for review. Pipes shall be cleaned prior to TV inspection of the pipes. Any sags, open joints, cracked pipes, etc. shall be repaired or removed by the CONTRACTOR at the CONTRACTOR's expense. A television survey will be performed as part of the final testing in the twentieth (20th) month of the maintenance period.

RETAINING WALLS

addressed prior to producing the digital files.

All retaining walls, regardless of height, will be reviewed and approved by the City Engineering Department All retaining walls (including foundation stem walls), regardless of height, will be constructed of rock/stone/brick or rock/stone/brick faced. No smooth concrete walls are allowed. Wall materials shall be the same for all walls on the project.

All portions, including footings, tie-backs, and drainage backfill, of the wall shall be on-site and not encroach into any public easements or right-of-way. The entire wall shall be in one lot and shall not be installed along . All walls 3 feet and taller will be designed and signed/sealed by a registered professional engineer in the State

of Texas. The wall design engineer is required to inspect the wall construction and supply a signed/sealed letter of wall construction compliance to the City of Rockwall along with wall as-builts prior to City Engineering acceptance. No walls are allowed in detention easements. A variance to allow retaining walls in a detention easement will

require approval by the Planning and Zoning Commission with appeals being heard by the City Council. FINAL ACCEPTANCE AND RECORD DRWINGS/AS-BUILTS Final Acceptance shall occur when all the items on the Checklist for Final Acceptance have been completed

and signed-off by the City. An example of the checklist for final acceptance has been included in the

Appendix of the Standards of Design and Construction. Items on the checklist for final acceptance will vary

per project and additional items not shown on the check list may be required. After improvements have been constructed, the developer shall be responsible for providing to the City "As Built" or "Record Drawings". The Design Engineer shall furnish all digital files of the project formatted in Auto Cad 14, or 2000 format or newer and Adobe Acrobat (.pdf) format with a CD-ROM disk or flash drive. The disk or drive shall include a full set of plans along with any landscaping, wall plans, and details sheets. Submit 1-set of printed drawings of the "Record Drawings" containing copies of all sheets to the Engineering Construction Inspector for the project. The printed sheets will be reviewed by the inspector PRIOR to producing the "Record Drawing" digital files on disk or flash drive. This will allow any revisions to be

Record Drawing Disk drawings shall have the Design Engineers seal, signature and must be stamped and dated as "Record Drawings" or "As Built Drawings" on all sheets. The City of Rockwall will not accept any Record Drawing disk drawings which include a disclaimer. A disclaimer shall not directly or indirectly state or indicate that the design engineer or the design engineer's surveyor/surveyors did not verify grades after construction, or that the Record Drawings were based solely on information provided by the construction contractor/contractors. Any Record Drawings which include like or similar disclaimer verbiage will not be accepted by the City of Rockwall

this plan is As-Built. This information provided is based on surveying at the site and information provided by the contractor."

Example of Acceptable Disclaimer: "To the best of our knowledge ABC Engineering, Inc., hereby states that



ENERAL CONSTRUCTION NOTES Sheet 2 of 2 CITY OF ROCKWALL ENGINEERING DEPARTMENT

385 S. Goliad P (972) 771-7746 Rockwall, Texas 75087 F (972) 771-7748



6126300 ESIGNER

HECKED

DRAWN

THESE RECORD DRAWINGS ARE BASED ON AS-BUILT

DOCUMENTS PROVIDED BY THE CONTRACTOR OR

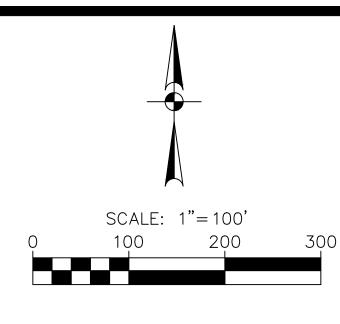
DEVELOPER. FIELD INSPECTION OF CONSTRUCTION, IF

EQUIRED FOR COMPLIANCE WITH CERTAIN REGULATORY

STANDARDS, WAS NOT PERFORMED BY THE DESIGN

NGINEER. IT IS NOT GUARANTEED THAT THIS DOCUMEN

REPRESENTS "AS-BUILT CONDITIONS."



TBM NO. 1

X-CUT SET IN CONCRETE LOCATED IN THE CENTER OF A CURB INLET IN THE CENTER OF A CURB INLET IN THE CENTER OF A CURB LINE OF NORTH CURB LINE OF NORTH CURB LINE OF NORTH CORRESPONDENCE OF A CURB INLET IN THE WEST CURB LINE OF NORTH JOHN KING SOUTHWESTERLY CURB LINE OF NORTH

JOHN KING BOULEVARD AND BEING +/235' SOUTHEST OF THE NORTHWEST

CORNER OF THE SUBJECT TRACT AND +/-BOULEVARD AND WEST QUAL RUN ROAD 535' NORTHWEST OF THE INTERSECTION OF AND +/- 723' SOUTHEAST OF THE N. JOHN KING BOULEVARD AND EAST QUAIL RUN ROAD BOULEVARD AND EAST QUAIL RUN ROAD. ELEV: 546.16' ELEV: 530.38'



DOCUMENTS PROVIDED BY THE CONTRACTOR OR DEVELOPER. FIELD INSPECTION OF CONSTRUCTION, IF REQUIRED FOR COMPLIANCE WITH CERTAIN REGULATORY STANDARDS, WAS NOT PERFORMED BY THE DESIGN ENGINEER. IT IS NOT GUARANTEED THAT THIS DOCUMENT REPRESENTS "AS-BUILT CONDITIONS." 03/22/2022

NOTES

CONTRACTOR SHALL PROTECT ALL EXISTING TREES, FENCES, RETAINING WALLS AND STRUCTURES UNLESS OTHERWISE NOTED.

UTILITY NOTE

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RESPONSIBILITY NOTE

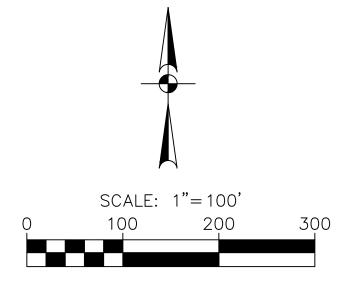
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N N N N N N N N N N N N N N N N N N N	JOHN KING BLVD				
OVERLAND SHEET FLOW		M.9j.	542	539 M. 91.X3	
-549 -548	2,46	EX 70" 88		n 30 acr 530.93	
547	546	, copy		536	
TRACT 1 CALLED 28.2 ACRES R & R HANCE INVESTMENTS, L.P. VOL. 5702, PG. 99	542	582			235
542	540 540	, s.	540		534
		539	539 538		533
	538	SHALLOW CONC. FLOW			530
537 - 536	535		B1 0.78		525
535	A 25:50				
	534-		535		
533	233	533	533 0		520
532 ~	532—1		533 — 532 —	B2 0.60	525
TRACT 2 CALLED 44.3 ACRES SAVE AND EXCEPT 5 ACRES R & R HANCE TINVESTMENTS, L.P. VOL. 5702, PG. 99	531				
528	530	CHANNELIZED FLOW	B4 0.36	B3 1.11	
		52>	529	528	510
-52>	525-	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		C 524	
522 523 EX-12" SS	523	522	EX.W	.72	508
	MICHAEL WURSTER AND JENNIFER WURSTER DOC. NO. 20260000003578	JOHN R. YINGLING AND WIFE	EASSINE TO THE PARTY OF THE PAR	5096	510 509
	EXST 36" CMP	JOHN R. YINGLING AND WIFE LYNELLE C. YINGLING VOL. 817, PG. 76 O.P.R. R. C.T.	JOHN'R. YINGLING AND WE SLYNELLE C. YINGLING NO. 366, PG. 214 O.P.R.R.C.T.		CTY OF ROCKWALL VOL. 5443, PG. 188 (O.P.R.R.C.T.

THIS DOCUMENT HAS BEEN PRODUCED FROM MATERIAL THAT WAS STORED AND/OR TRANSMITTED ELECTRONICALLY AND MAY HAVE BEEN INADVERTENTLY ALTERED. RELY ONLY ON FINAL HARDCOPY MATERIALS BEARING THE CONSULTANT'S ORIGINAL SIGNATURE AND SEAL. AERIAL IMAGERY PROVIDED BY GOOGLE® UNLESS OTHERWISE NOTED. Imagery © 2016, CAPCOG, Digital Globe, Texas Orthorimagery Program, USDA Farm Service Agency.

				PRE-DEVE	LOPMENT DRA	INAGE AREA	CALCULA	TIONS			
	5A 5	Areas Draine	ed	20.	Weighted				Intensity	Q100	
Area	Total	Parks or	Residential	Commercial	Runoff	CA	Tc	Design	100		Drains
ID.	Drainage	Open Area			Coefficient		(min)	Storm	Year	Year	To/
	Area	(C=0.35)	(C=0.5)	(C=0.9)	С			Frequency	(in/hr)	(cfs)	Remarks
	(acres)	(acres)	(acres)	(acres)							
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)			(12)
Onsite		0.35	0.5	0.9			4 0 00 00				
Α	25.50	25.50			0.35	8.93	20	100	8.30	74.08	Drain to existing 36" Pipe
B1	0.78	0.78			0.35	0.27	20	100	8.30	2.27	Existing Inlet in John King
B2	0.60	0.60			0.35	0.21	20	100	8.30	1.74	Existing Inlet in John King
В3	1.11	1.11	***************************************		0.35	0.39	20	100	8.30	3.22	Existing Inlet in John King
В4	0.36	0.36			0.35	0.13	20	100	8.30	1.05	Existing Inlet in John King
С	1.72	1.72			0.35	0.60	20	100	8.30	5.00	Existing Wye Inlet
TOTAL	30.07									81.31	



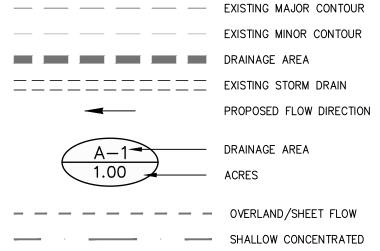
BENCHMARKS

ELEV: 546.16'

TBM NO. 1 X-CUT SET IN CONCRETE LOCATED IN THE X-CUT SET IN CONCRETE LOCATED IN THE CENTER OF A CURB INLET IN THE CENTER OF A CURB INLET IN THE WEST SOUTHWESTERLY CURB LINE OF NORTH CURB LINE OF NORTH JOHN KING JOHN KING BOULEVARD AND BEING +/- BOULEVARD AND BEING +/- 662' NORTH 235' SOUTHEST OF THE NORTHWEST OF THE INTERSECTION NORTH JOHN KING CORNER OF THE SUBJECT TRACT AND +/-BOULEVARD AND WEST QUAIL RUN ROAD 535' NORTHWEST OF THE INTERSECTION OF AND +/- 723' SOUTHEAST OF THE N. JOHN KING BOULEVARD AND EAST INTERSECTION OF N. JOHN KING QUAIL RUN ROAD

BOULEVARD AND EAST QUAIL RUN ROAD. ELEV: 530.38'

ADAM S. REEVES 98410



MASTER DRAINAGE LEGEND

—— —— —— —— CHANNEL

ON GROVE - PHASE 2 SKWALL, ROCKWALL COUNTY, TEXAS

UTILITY NOTE

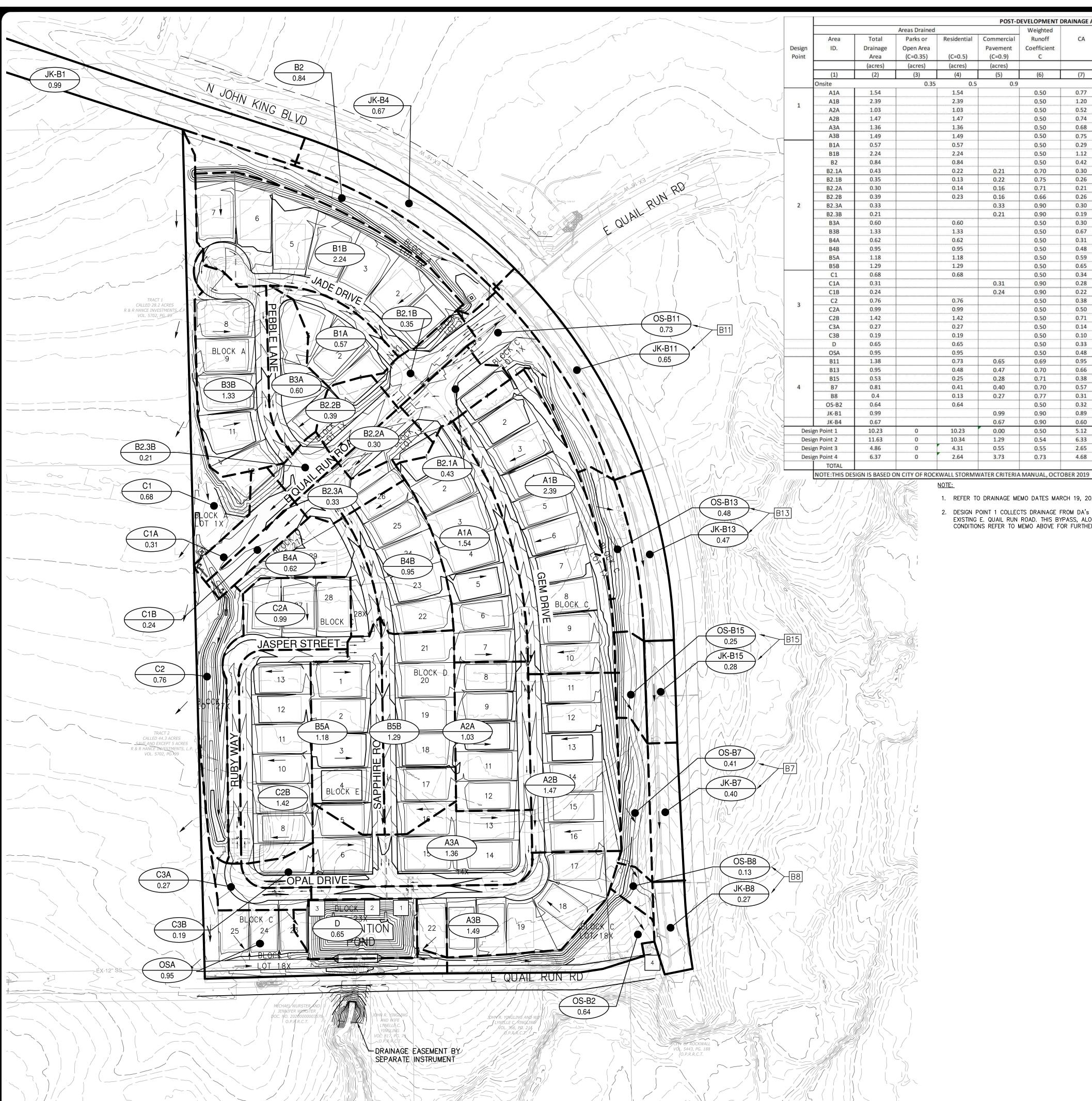
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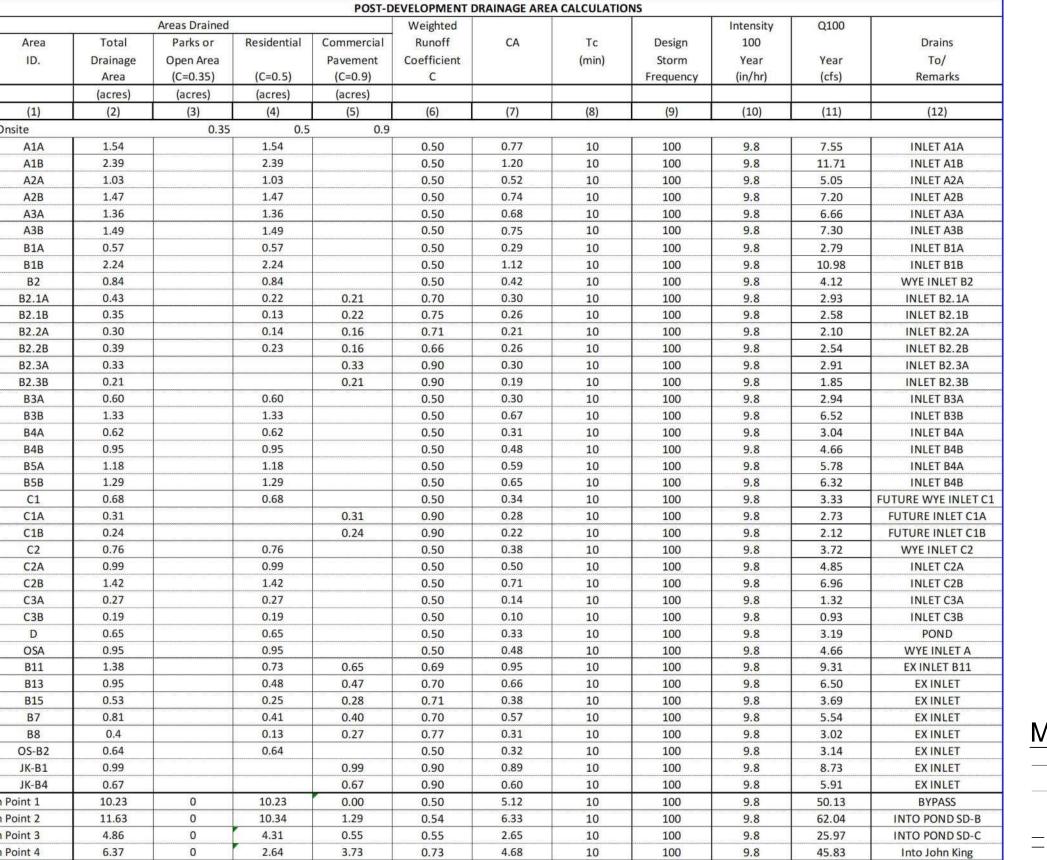
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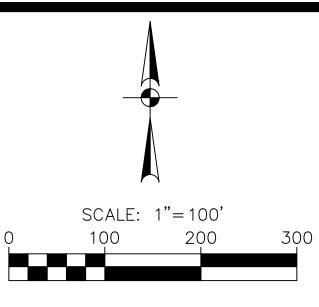
6126300 June 21 DESIGNER HECKED ASR DRAWN ML

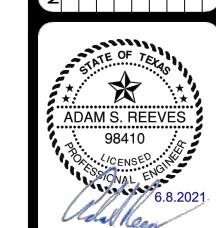






2. DESIGN POINT 1 COLLECTS DRAINAGE FROM DA'S A & BYPASSES POND, OUTFALLING DIRECTLY TO DRAINAGE AREA SOUTH OF EXISTING E. QUAIL RUN ROAD. THIS BYPASS, ALONG W/ DETAINED FLOW FROM POND RELEASE LESS THAN PREDEVELOPED CONDITIONS REFER TO MEMO ABOVE FOR FURTHER DETAIL.





MASTER DRAINAGE LEGEND

EXISTING MAJOR CONTOUR EXISTING MINOR CONTOUR DRAINAGE AREA _____ EXISTING STORM DRAIN PROPOSED FLOW DIRECTION --- DRAINAGE AREA

TIME OF CONCENTRATION DESIGN POINT

0

COMBINED DRAINAGE AREA

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	Level	Man.					Ť		Acce Disease			- Harterson -	Total	1					0.000.00	04555 ATS -0211 - ATB	CURB INLET	C. III T. C. III C					Ť				Jalata Cassalts	20					-	Inlat Decares		
Later	Locat		Chatian	Officer	Desires	Duranti		Time of	Area Runoff	1 4	D	Upstream	Total	Thereachters	Inlat Tone	Adamatanta	Lance	C	Carra Claus	Gutter Flo	200	Dendine W	of dile /Coursed	Death of	C. Har Flan	Man Allamakia Flam	Decreed Catter Contin	- Castina D	and December		Inlets Capacity		Faulustant	Leastle Dec		totat	Class.	Inlet By-pass	To	
Inlet	Alignn	ment	Station	Offset	Design	Runoff	Area	Time of	Intensity	Area	Runoff	Bypass	Gutter	Thoroughfare	Inlet Type:	E.	Long	Crown	Cross Slope		ression		Vidth/Spread		Gutter Flow	Max Allowable Flow	Depressed Gutter Sectio		eyond Depression		Conveyance	Ratio of	Equivalent	Length Red		Inlet	Flow	CXA	IO.	Describe
ID.					Frequency	Coeff	ID	Concentration	8		u	C*A	Flow	Type	On Grade/	Coefficient "n"	Slope "S"	Туре	of Pavement "Sx"	Depth	Width "W"	(Allow)	(Actual)	(Allow)	(Actual)	Based on Max. Allowable	Area Wetted P	er Area	Wetted Pe		Section Beyond		Cross	Required	Actual	Capacity	Q _{bypass}	C*A	Inlet	Remarks
						C		l'c	Carlle A	to all	(LEA)	(Column 39	Q _a		Sag	n				a (EL)	VV (51)	lallow	Tactual	Yallow	Yactual	Ponding Width	A _W P _W	A ₀	P ₀	Section	Depression	to Total Flow	Slope	Lreq'd	Lactual	Q _c	(-se)		10	
			.	50.40				(min)	(in/hr)	(ac)	(cfs)	Upstream Inlet)	(cfs)		ar.		(ft/ft)	10	(ft/ft)	(ft)	(ft)	22	22	24	25	(cfs)	(ft²) (ft)	(ft²)	(ft)	K _W	K ₀	E ₀	S _e (ft/ft)	(ft)	(ft)	(cfs)	(cfS)	20		
2	Com	Delice 6	5 - 04 02	14 501	100	0.50	^	10.0	0.00	10	7.55	12	7.5	Desidential Consesse	On Condo	0.0175	1/	18 Steelight Coorne	13	20	2.00	14.00	23	0.50	0.36	7.50	1.14 2.09	1.14	7.9	51	32	0.712	34	11.0	36	12.26	38	39	40	41
*****	Gem [5+04.03 4+77.80	14.50 L 14.50 R	100	0.50	A1A A1B	10.0	9.80	2.39	7.55		7.5	Residential Concrete	On Grade		0.017	Straight Crown		0.50	2.00	14.00	9.9	0.50	0.36	7.50 11.64	1,14 2.08 1.27 2.08	1.14	9.8	65.24	26.43	0.712	0.21	11.8 15.0	15.0	13.36	0.0	0.00		
A1B	Gem [**********	1+94.78			0.50	A1B A2A		9.80		11.71	······	11.7	Residential Concrete Residential Concrete	On Grade		0.017	Straight Crown	0.036		2.00		8.6	0.50	0.36	5.02	*** ***********************************	0.70	6.6	78.08	45.65	0.631	0.19	9.5	15.0	11.69 5.71	0.0	0.00		
A2A				14.50 L	100	0.50	AZA AZB	10.0	9.80	1.03	5.05 7.20		5.0		On Grade		0.017	Straight Crown		0.50 0.50	2.00	14.00 14.00		0.50	0.36	7.16		0.78	9.5	56.12 76.53	15.93 43.07	0.779	0.23	9.5	10.0	5.71 8.46	0.0	0.00	**********************	
A2B	Gem I		1+12.45	14.50 R				2010		4. 17		<u> </u>	7.2	Residential Concrete	On Grade			Straight Crown					11.5																	
A3A	Opal [4+69.95	14.50 L	100	0.50	A3B	10.0	9.80	1.49	7.30	0.10	7.3	Residential Concrete	Sag	0.0175	0.007	Straight Crown	0.036	0.50	2.00	14.00	11.6	0.50	0.36	7.26	1.26 2.08	1.66	9.6	76.96	43.77	0.637	0.20	8.2	10.0	9.07	0.0	0.00		
A3B	Opal I		4+58.95	14.50 R	100	0.50	A3B			1.49		0.10	8.3	Residential Concrete	Sag	0.0175	0.007	Straight Crown	0.036	0.50		14.00		0.50	0.36	8.24	1.30 2.08	1.86		81.13	50.98	0.614	0.19	8.8				0.00		
B1A	Jade [1+80.00	14.50 L	100	0.50	B1A	10.0	9.80	0.57	2.79		2.8	Residential Concrete	On Grade	**********************	0.028	Straight Crown	0.036	0.50	2.00	14.00	6.2	0.50	0.36	2.78	0.88 2.08	0.32	4.2	41.90	4.95	0.894	0.26	8.0	10.0	4.70	0.0	0.00		
B1B	Jade [14.50 R	100	0.50	B1B	10.0	9.80	2,24	10.98		11.0	Residential Concrete	On Grade		0.028	Straight Crown	0.036	0.50	2.00	14.00	10.4		0.36		1.18 2.08	1.28	8.4	68.55	30.88	0.689	0.21	16.3	15.0	9.01	1.97	0.20	B21B	
B2.1/		1000	6+55.49	32.00 L	100	0.70	B2.1A	10.0	9.80	0.43	2.93		2.9	Minor Divided Arterial	On Grade		0.009	Straight Crown	0.020	0.50	2.00	12.00	11.3	0.50	0.36	2.91	0.91 2.07	0.86	9.3	44.76	15.00	0.749	0.21	6.7	10.0	7.48	0.00	0.00		
B2.1			6+58.89	32.00 R	100	0.75	B2.1B	10.0	9.80	0.35	2.58	0.20	4.5	Minor Divided Arterial	On Grade		0.009	Straight Crown		0.50	2.00	12.00	12.0	0.50	0.36	3.38	0.94 2.07	1.00	10.0	47.13	18.29	0.720	0.20	8.2	10.0	7.28	0.00	0.00	***************************************	
B2.2			4+90.50	32.00 R	100	0.71	B2.2A	10.0	9.80	0.30	2.10		2.1	Minor Divided Arterial	On Grade		0.009	Straight Crown	0.020	0.50	2.00	12.00	10.0	0.50	0.36	2.08	0.86 2.07	0.63	8.0	40.49	9.93	0.803	0.22	5.6	10.0	8.19	0.00	0.00		
B2.2			4+94.40	32.00 L	100	0.66	B2.2B	10.0	9.80	0.39	2.54		2.5	Minor Divided Arterial	On Grade		0.009	Straight Crown	0.020	0.50	2.00	12.00	10.8	0.50	0.36	2.52	0.89 2.07	0.77	8.8	43.05	12.84	0.770	0.21	6.2	10.0	7.94	0.00	0.00		
B2.3			3+70.28	32.00 R	100	0.90	B2.3A	10.0	9.80	0.33	2.91		2.9	Minor Divided Arterial	On Grade		0.009	Straight Crown	0.020	0.50	2.00	12.00	11.3	0.50	0.36	2.89	0.91 2.07	0.86	9.3	44.67	14.88	0.750	0.21	6.7	10.0	7.49	0.00	0.00	.00111000111000111001100	
B2.3		**************	3+73.88	32.00 L	100	0.90	B2.3B	10.0	9.80	0.21	1.85		1.9	Minor Divided Arterial	On Grade	*******************	0.009	Straight Crown	0.020	0,50	2.00	12.00	9.6	0.50	0.36	1.84	0.84 2.07	0.57	7.6	39.26	8.67	0.819	0.22	5.2	10.0	8.60	0.00	0.00		
B3A	Pebble		1+77.50	14.50 R	100	0.50	B3A	10.0	9.80	0.60	2.94		2.9	Residential Concrete	On Grade		0.024	Straight Crown	0.036	0.50	2.00	14.00	6.5	0.50 0.50	0.36	2.92	0.90 2.08	0.37	4.5	43.65	5.95	0.880	0.26	7.9	10.0	5.12	0.00	0.00		
B3B	Pebble		1+77.50	14.50 L	100	0.50	B3B	10.0	9.80	1.33	6.52		6.5	Residential Concrete	On Grade		0.028	Straight Crown	0.036	0.50	2.00	14.00	8.6		0.36	6.48	1.05 2.08	0.78	6.6	56.12	15.94	0.779	0.23	12.3	15.0	10.42	0.00	0.00		
B4A	Sapphire	re Road 6	6+30.00	14.50 L	100	0.50	B4A	10.0	9.80	0.62	3.04		3.0	Residential Concrete	On Grade		0.016	Straight Crown	0.036	0.50	2.00	14.00	7.1	0.50	0.36	3.02	0.94 2.08	0.48	5.1	47.16	8.25	0.851	0.25	7.3	10.0	6.53	0.00	0.00		3
B4B	Sapphire		6+10.60	14.50 R	100	0.50	B4B	10.0	9.80	0.95	4.66		4.7	Residential Concrete	On Grade	0.0175	0.016	Straight Crown	0.036	0.50	2.00	14.00	8.4	0.50	0.36	4.63	1.03 2.08	0.73	6.4	54.78	14.62	0.789	0.23	9.0	10.0	5.93	0.00	0.00		
B5A	Sapphire	re Road 2	2+08.00	14.50 L	100	0.50	B5A	10.0	9.80	1.18	5.78		5.8	Residential Concrete	On Grade	0.0175	0.016	Straight Crown	0.036	0.50	2.00	14.00	9.1	0.50	0.36	5.75	1.08 2.08	0.90	7.1	59.44	19.45	0.753	0.22	10.1	10.0	5.64	0.15	0.01	A3B	
B5B	Sapphire	re Road 2	2+08.00	14.50 R	100	0.50	B5B	10.0	9.80	1.29	6.32		6.3	Residential Concrete	On Grade	0.0175	0.016	Straight Crown	0.036	0.50	2.00	14.00	9.4	0.50	0.36	6.28	1.10 2.08	0.98	7.4	61.42	21.70	0.739	0.22	10.6	10.0	5.48	0.84	0.09	A3B	
C1A	E Qu	uail 1	1+42.28	32.00 L	100	0.90	C1A	10.0	9.80	0.31	2.73		2.7	Minor Divided Arterial	On Grade	0.0175	0.007	Straight Crown	0.020	0.50	2.00	12.00	11.6	0.50	0.36	2.72	0.92 2.07	0.92	9.6	45.79	16.38	0.736	0.20	6.1	10.0	8.96	0.00	0.00		Future Inlet (Ultimate
C1B	E Qu	uail 1	1+42.28	32.00 R	100	0.90	C1B	10.0	9.80	0.24	2.12		2.1	Minor Divided Arterial	On Grade	0.0175	0.007	Straight Crown	0.020	0.50	2.00	12.00	10.5	0.50	0.36	2.10	0.88 2.07	0.73	8.5	42.33	11.99	0.779	0.21	5.3	10.0	9.64	0.0	0.00		Future Inlet (Ultimate
C2A	Ruby	Way 1	1+45.50	14.50 L	100	0.50	C2A	10.0	9.80	0.99	4.85	(a E-) 1000 1 1000 10 1000 10 1	4.9	Residential Concrete	Sag	0.0175	0.010	Straight Crown	0.036	0.50	2.00	14.00	9.3	0.50	0.36	4.82	1.10 2.08	0.96	7.3	60.94	21.15	0.742	0.22	7.3	10.0	6.07	0.0	0.00	.002X+0002X+0002X+4	
C2B	Ruby	Way 1	1+45.50	14.50 R	100	0.50	C2B	10.0	9.80	1.42	6.96		7.0	Residential Concrete	Sag	0.0175	0.010	Straight Crown	0.036	0.50	2.00	14.00	10.7	0.50	0.36	6.92	1.20 2.08	1.35	8.7	70.18	33.21	0.679	0.21	9.1	10.0	7.44	0.0	0.00		
C3A	Opal I	Drive 2	2+02.56	14.50 L	100	0.50	C3A	10.0	9.80	0.27	1.32		1.3	Residential Concrete	Sag	0.0175	0.007	Straight Crown	0.036	0.50	2.00	14.00	6.1	0.50	0.36	1.31	0.87 2.08	0.31	4.1	41.19	4.57	0.900	0.26	2.0	10.0	3.23	0.0	0.00		
C3B	Opal [Drive 2	2+02.56	14.50 R	100	0.50	СЗВ	10.0	9.80	0.19	0.93		0.9	Residential Concrete	Sag	0.0175	0.007	Straight Crown	0.036	0.50	2.00	14.00	5.4	0.50	0.36	0.93	0.81 2.08	0.20	3.4	36.98	2.66	0.933	0.27	1.2	10.0	2.65	0.0	0.00		
OSA					100	0.50	OSA	10.0	9.80	0.95	4.66		4.7	Channel	Drop	0.0350	0.010			0.50				0.50	0.50									4.3	20.0	21.83	0.0	0.00	/	
B2	E Qu	uail 9	9+08.24	76.85 L	100	0.50	B2	10.0	9.80	0.84	4.12		4.1	Channel	Drop	0.0350	0.010			0.50				0.50	0.50									3.8	12.0	13.10	0.0	0.00		
C1	E Qu	uail 1	1+65.76	50.50 L	100	0.50	C1	10.0	9.80	0.68	3.33		3.3	Channel	Drop	0.0350	0.010			0.50				0.50	0.50									3.1	12.0	13.10	0.0	0.00		Future Inlet (Ultimate
C2	Ruby		1+43.03	34.50 L	100	0.61	C1&C2	10.0	9.80	1.99	11.91		11.9	Channel	Drop	0.0350	0.010			0.50				0.50	0.50									10.9	25.0	27.29	0.0	0.00	A	

1												ST	ORM DRAIN	HYDRAULIC C	CALCULATIO	NS TABLE	1			1										3	1	
Collection Point Station	Pipe	Pipe Box	onduit Properties OX Conduit	Pipe Wette	ed Hudraulie		Flowline		Increme Inlet	ental Drainage Runoff		Accum (Jpstream	5torm I	Intensity F	Runoff Co	nduit Parti	ial Velocit	y Time	Friction	Friction	Н	GL			Headloss Calculation		Head	Design		HGL	Remarks
n	Length # of	5ize 5pan	Rise Type	Area Perime	ter Radius R _H	Manning's	Elevation	5lope	ID A	rea Coeff.	mental	-ulated	T _c	Frequency		Q Car	Q _C Flow	w V	in Condu	ıit Slope	Head- Loss	U/5	D/5 V	$ V_2 ^2/2g$	3	Junction	Coefficien	t Loss	HGL	of Curb B	epth elow T/C	
U/5 D/5 2 3	(ft) Barrel:	6 (in) (ft)	(ft) 8 9	(ft ²) (ft)	(ft) 12	n 13	U5 D5	(ft/ft)	,	cres) C	C*A 20	C*A 21	(min)	(years)	(in/hr)	i i	cfs) Yes/I	l l	(min) 29	(ft/ft) 30	31	32	33	34 35	5	Type 36	K _j 37	H _L 38	39	1	(ft) 41	
	<u> </u>	1					<u> </u>	<u> </u>						1		, , , , , , , , , , , , , , , , , , ,	1		•				•				•	I	1		1	
Drain Line SD-A 10+23.13 10+02.68	20.45 1	21	RCP	2.41 5.50		0.013	525.72 524.81	0.044	A1A 1.	.54 0.50	0.8	0.8	10	100	9.8	7.55 33	3.31 Yes	s 3.14	0.11	0.0006		518.62	518.61	0.00 0.1	5	Inlet at Beginning of Line	0	0.19	518.81	530.22 1	1.41	
10+02.68 9+82.54 9+82.54 6+97.13	20.14 1 285.41 1	21 24	RCP RCP	2.41 5.50 3.14 6.28	0.50	0.013 0.013	524.81 523.92 523.92 522.21	0.006	A1B 2.	.00 0.00 .39 0.50	0.0	0.8 2.0	10.11 10.22	100 100	9.8	19.19 17	3.31 Yes 7.52 No	6.11	0.78	0.0006 0.0011	0.31	518.55 518.51	518.54 518.19	0.15 0.1 0.15 0.5	8 MH	45 Bend H w/ 0 Change in Pipe Direction	0.37 0.05	0.06	518.61 518.54			
6+97.13 6+08.56 6+08.56 5+95.25 5+95.25 5+49.80	88.57 1 13.31 1 45.45 1	27	RCP RCP RCP	3.14 6.28 3.98 7.07 3.98 7.07	0.56	0.013 0.013 0.013	522.21 521.68 521.43 521.35 521.35 521.05	- 	A2B 1.	.03 0.50 .47 0.50 .00 0.00	0.5 0.7 0.0	2.5 3.2 3.2	10.99 11.19 11.22	100 100 100	9.6	30.88 23	7.52 No 3.99 No 4.97 No	7.77	0.03	0.0014 0.0013 0.0013	0.02	517.58 516.84 516.48		0.58 0.9 0.90 0.9 0.94 0.9	14	45 Wye 60 Wye 45 Bend	0.5 0.35 0.37	0.61 0.62 0.35		E2E 19	26	
5+49.80 3+36.82 3+36.82 3+08.88	212.98 1 27.94 1	27 27	RCP RCP	3.98 7.07 3.98 7.07	0.56	0.013 0.013 0.013	521.05 519.67 519.67 519.11	0.007	0.	.00 0.00 .36 0.50	0.0	3.2	11.31	100	9.6	30.81 24	4.97 No 3.58 Yes	7.75	0.46	0.0013	0.28	516.07 515.06	515.80 515.01	0.94 0.9 0.93 1.3	3	45 Bend H w/ 90 Change in Pipe Direction	0.37	0.34	516.42		8.76	
3+08.88 2+01.68 2+01.68 0+99.05	107.20 1 102.63 1	27 27	RCP RCP	3.98 7.07 3.98 7.07	0.56	0.013 0.013	519.11 516.99 516.99 514.96	0.020	A3B 1.	.49 0.50 .00 0.00	0.7 0.0	4.6 4.6	11.82 11.98	100 100	9.5	44.08 43	3.58 No 3.58 No	11.09	0.16	0.0019 0.0018	0.20	513.78 512.54	513.58 512.35	1.35 1.9 1.91 1.9	1	45 Wye H w/ 90 Change in Pipe Direction	0.5 0.55	1.24 1.04	515.01			
0+99.05 0+64.93 0+64.93 0+26.54	34.12 1 38.39 1	30 30	RCP RCP	4.91 7.85 4.91 7.85	0.63	0.013 0.013	513.59 513.11 513.11 512.57	0.014	O5A 0.	.00 0.00 .95 0.50	0.0	4.6 5.1	12.14 12.20	100 100	9.4	48.28 48	8.53 Yes 8.53 Yes	s 9.84	0.07	0.0014 0.0015	0.06	511.67 511.35	511.62 511.30	1.90 1.2 1.24 1.5	0	H w/ 90 Change in Pipe Direction Junction 0	0.55 1 0.37	0.68 0.26	512.35 511.62			AAAA AAAA AAAA
0+26.54	26.54 1	48	RCP	12.57 12.5		0.013	506.28 506.01			.00 0.00	0.0	5.1	12.27	100			13.64 Yes			0.0004		511.21	511.20	0.93 0.2		45 Bend	0.37	0.08	511.30			
0+16.74	16.68 1	24		3.14 6.28 2.41 5.50			525.15 524.13 522.57 522.46		A1A 1.	.03 0.50		0.8	10		9.8		5.83 Yes 2.77 Yes			0.0004			518.19	0.00 0.0		Inlet at Beginning of Line] 0		518.31			
vrain Line SD-A2B 0+20.51 0+00.00	20.51 1	21		2.41 5.50			522.03 521.93		A2B 1.				10				1.20 Yes			0.0004				0.00 0.1		Inlet at Beginning of Line Inlet at Beginning of Line	0		517.01			
0rain Line SD-A3A 0+00.00	15.49 1	21	•		0.44		520.31 520.17			.36 0.50			10	1		7.65 14				1				0.00 0.1		Inlet at Beginning of Line	0		515.22			
0+18.33 0+00.00	18.33 1	21	RCP	2.41 5.50	0.44	0.013	520.03 519.61	0.023	A3B 1.	.49 0.50	0.7	0.7	10	100	9.8	7.30 23	3.93 Yes	s 3.04	0.10	0.0006	0.01	513.59	513.58	0.00 0.1	4	Inlet at Beginning of Line	0	0.18	513.77	524.63 1	0.86	
Drain Line SD-B 10+37.27 10+20.52 10+20.52 10+14.39	16.75 1	21	RCP RCP	2.41 5.50 2.41 5.50		0.013 0.013	531.97 531.64 531.64 531.52			.60 0.50 .00 0.00	0.3	0.3	10 10.23	100 100		2.94 22 2.93 22	2.13 Yes 2.13 Yes	s 1.22 s 1.22	0.23	0.0002		526.04 526.02		0.00 0.0 0.02 0.0		Inlet at Beginning of Line 45 Bend	0 0.37	0.03		536.48 1	0.42	
10+20.32 10+14.39 10+14.39 9+40.37 9+40.37 6+19.80	6.13 1 74.02 1 320.57 1	24 27	RCP RCP	3.14 6.28 3.98 7.07	0.50	0.013	531.64 531.32 531.27 529.83 529.51 526.31	0.020	B3B 1.	.00 0.00 .33 0.50 .66 0.59	0.0 0.7 3.3	0.3 1.0 4.3	10.23 10.31 10.72	100	9.7	9.41 31	2.13 Yes 1.59 Yes 0.97 No	s 2.99	0.41	0.0002 0.0005 0.0018	0.04	525.89 524.91	525.85	0.02 0.0 0.02 0.1 0.14 1.7	.4	45 Wye Hw/90 Change in Pipe Direction	0.57		526.02	535.30	9.28	
6+19.80 6+06.55 6+06.55 1+98.97	13.25 1 407.58 1	27	RCP RCP	3.98 7.07 3.98 7.07	0.56	0.013 0.013	526.31 526.17 526.17 520.06	0.010	B4A 0.	.62 0.50 .95 0.50		4.6	11.23 11.25	100 100 100	9.6	44.34 30	0.97 No 7.93 No	11.15	0.02	0.0019 0.0021	0.02	523.28 523.13		1.71 1.9 1.93 2.3	3	45 Wye H w/ 0 Change in Pipe Direction	0.5 0.05		524.35	532.00	8.75	
1+98.97 1+92.84 1+92.84 0+53.38	6.13 1 139.46 1	36 42	RCP RCP	7.07 9.42 9.62 11.0	0.75	0.013	519.24 519.15 515.78 515.22	0.015	B5B 1.	.29 0.50 .18 0.50	0.6 0.6	5.7 6.3	11.81 11.82	100	9.5	54.56 83	1.69 Yes 3.63 Yes	s 7.72	0.01	0.0011 0.0008	0.01	522.55 522.51	522.54	2.35 0.9 0.93 0.6	3	45 Wye H w/ 0 Change in Pipe Direction	0.5 0.05	-0.25 0.03		525.40	3.10	A A A A A A A A A A A A A A A A A A A
Orain Line SD-B1 3+88.17 3+71.43	16.74 1	21	RCP	2.41 5.50			734.57 734.33			.57 0.50	0.3	0.3	10	100	9.8		8.95 Yes	***************************************		0.0002		527.50		0.00 0.0		Inlet at Beginning of Line	0	0.03		539.07 1	1.54	
3+71.43 3+65.30 3+65.30 2+90.06	6.13 1 75.24 1	21 21	RCP RCP	2.41 5.50 2.41 5.50	0.44	0.013		0.014	B1B 2.	.00 0.00 .24 0.50	0.0	0.3	10.24	100	9.7	13.69 18	8.95 Yes 8.95 Yes	s 5.69	0.22	0.0002	0.08	527.49 527.00	526.91	0.02 0.0	0	45 Bend 45 Wye	0.37	0.01	527.49		4.00	
2+90.06 2+04.28 2+04.28 1+93.29 1+93.29 0+35.92	85.78 1 10.99 1 157.37 1	24	RCP RCP	2.41 5.50 3.14 6.28 3.14 6.28	0.50		733.17 732.18 732.18 732.05 732.05 730.24	0.012	B2.1A 0.	.84 0.50 .43 0.70 .35 0.75		1.8 2.1 2.4	10.55 10.74 10.77	100	9.7	20.55 24	6.99 No 4.26 Yes 4.26 Yes	s 6.54	0.03	0.0015 0.0012 0.0013	0.01	526.45 526.08 526.03	526.07	0.50 0.8 0.84 0.6 0.66 0.8	6	H w/ 90 Change in Pipe Direction 45 Wye H w/ 0 Change in Pipe Direction	0.55 0.5 0.05	0.46		538.71 1 537.15 1	1.80	
0+35.92	4.09 1 31.83 1	24	RCP RCP RCP RCP RCP	3.14 6.28 3.14 6.28	0.50		730.24 730.20 730.20 729.83	0.012	B2.2A 0.	.30 0.71 .39 0.66	0.2		11.13	100 100 100 100 100	9.6	25.00 24	4.26 No 4.26 No	7.96	0.01	0.0013 0.0014 0.0016	0.01	525.25	525.25	0.84 0.9 0.98 1.1	8	45 Wye 60 Wye	0.5	0.04 0.56 0.84		337.13		
Orain Line SD-B1B 0+00.00	20.51 1	21		2.41 5.50	0.44	0.013	534.76 534.24	0.025	B1B 2.	.24 0.50	1.1	1.1	10	100	9.8	9.01 25	5.20 Yes	s 3.75	0.09	0.0007	0.02	526.93	526.91	0.00 0.2	2	Inlet at Beginning of Line	0	0.27	527.20	538.85 1	1.65	
Orain Line SD-B2 2+16.79 0+87.41	129.38 1	21	RCP RCP	2.41 5.50 2.41 5.50	0.44	0.013	534.36 533.65 533.65 533.17	0.006	B2 0.	.84 0.50 .00 0.00	0.4	0.4	10 11.26	100 100	9.8	4.12 13 4.03 13	1.75 Yes	s 1.71	1.26	0.0003	0.04	526.98	526.94	0.00 0.0 0.05 0.0	15	Inlet at Beginning of Line	0 0.37	0.06	527.04 526.96	539.80 1	2.76	
0+87.41 0+00.00 Drain Line SD-B2.1A 0+26.17 0+00.00	87.41 1 26.17 1	21		2.41 5.50 2.41 5.50			533.65 533.17						10.26				1.75 Yes	1						0.00 0.0		45 Bend Inlet at Beginning of Line	0.37		526.36			
Orain Line SD-B2.1B 0+52.49 0+00.00	52.49	21			0.44		533.07 532.05						10	100			<u>.</u>							0.00 0.0		Inlet at Beginning of Line	0		526.16			
0+26.15 0+00.00	26.15 1	21			0.44	1	531.33 530.24		<u>'</u>	<u>'</u>		0.2	10			2.10 32	<u> </u>							0.00 0.0		Inlet at Beginning of Line	0		525.84		•	
Orain Line SD-B2.2B 0+53.63 0+00.00	53.63 1	21	RCP	2.41 5.50	0.44	0.013	531.38 530.45	0.017	B2.2B 0.	.39 0.66	0.3	0.3	10	100	9.8	2.54 20	0.84 Yes	s 1.06	0.85	0.0002	0.01	525.26	525.25	0.00 0.0	2	Inlet at Beginning of Line	0	0.02	525.28	535.86 1	0.58	
0+73.46 0+47.34	26.12 1	21	RCP	2.41 5.50	0.44	0.013	530.48 530.25	0.009	B2.3A 0.	.33 0.90	0.3	0.3	10 10.36	100	9.8	2.91 14	4.95 Yes 4.95 Yes	s 1.21	0.36	0.0002		524.43	524.43	0.00 0.0	2	Inlet at Beginning of Line	0	0.03	524.46	534.99 1	0.53	
0+47.34 0+35.94 0+35.94 0+00.00	11.40 1 35.94 1	21 21	RCP RCP RCP	2.41 5.50 2.41 5.50			530.25 530.15 530.15 529.83	0.009		.00 0.00 .21 0.90	0.0	0.3	10.36 10.52	100 100 100			4.95 Yes 4.95 Yes			0.0002 0.0004		524.42 524.37	524.42 524.35	0.02 0.0 0.02 0.0		45 Bend 60 Wye	0.37 0.35	0.01 0.05	524.43 524.42			
Orain Line SD-B2.3B 0+52.57 0+00.00 Orain Line SD-B3B	52.57 1	21	RCP	2.41 5.50	0.44	0.013	530.51 530.15	0.007	B2.3B 0.	.21 0.90	0.2	0.2	10	100	9.8	1.85 13	3.16 Yes	s 0.77	1.14	0.0002	0.01	526.96	526.96	0.00 0.0	1	Inlet at Beginning of Line	0	0.01	526.98	535.01	8.03	
0+20.51 0+00.00 Orain Line SD-B3B	20.51 1	21	····	<u> </u>	0.44	1	531.88 531.51			······································		·	*	100			******							0.00 0.1		Inlet at Beginning of Line			526.17		•	
0+20.57	20.57 1	21	RCP	2.41 5.50	0.44	0.013	527.96 526.88	0.053	B4A 0.	.62 0.50	0.3	0.3	10	100	9.8	3.04 36	6.37 Yes	s 1.26	0.27	0.0002	0.01	524.36	524.35	0.00 0.0	2	Inlet at Beginning of Line	0	0.03	524.39	532.46	8.07	
0+16.80 0+00.00 Drain Line SD-BSA	16.80 1	21		2.41 5.50		·	527.64 526.75					· · · · · · · · · · · · · · · · · · ·	10				6.55 Yes							0.00 0.0		Inlet at Beginning of Line	0		523.33			
0+19.64 0+00.00 Drain Line SD-BSB	19.64 1	21			0.44		521.03 520.56						10			5.61 24								0.00 0.0		Inlet at Beginning of Line	0		522.65			
0+27.93 0+00.00 Storm Drain Line SD-C1 (Ultimate Co 1+52.42 1+18.92	27.93 1 ndition) 1	21	RCP	2.41 5.50		0.013	521.05 520.40 529.11 529.01			.68 0.50	0.6	0.6	10	100		3.33 8	4.13 Yes			0.0004		522.31		0.00 0.0		Inlet at Beginning of Line Inlet at Beginning of Line		0.00	530.62			ure 5trom Line - Ultimate C
1+32.42 1+16.92 1+18.92 0+54.92 0+54.92 0+34.42	64.00 1 20.50 1	21 21	RCP RCP	2.41 5.50	0.44	0.013	529.01 528.82	0.003	C1A 0.	.31 0.90 .24 0.90	0.3	0.6	10.40 10.40	100	9.7 9.7	6.02 8	3.68 Yes 3.68 Yes	s 2.50	0.43	0.0003	0.03	530.59 530.54 530.50	530.50	0.00 0.0 0.00 0.1 0.03 0.1	.0 MH	H w/ 45 Change in Pipe Direction H w/ 0 Change in Pipe Direction	0.42	0.04 0.04 0.01	530.58	533.39	2.81 Futu	ure 5trom Line - Oltimate C ure 5trom Line - Ultimate C ure 5trom Line - Ultimate C
0+34.42	25.00 1	21	RCP	2.41 5.50 2.41 5.50	0.44		528.76 528.65			.00 0.00	0.0	0.8	10.50	100 100	9.7	8.11 10	0.51 Yes		0.10	0.0007		530.42		0.18 0.1		45 Bend	0.37	0.07	530.48		Futu	ure 5trom Line - Ultimate C
3+99.70 3+83.02 3+83.02 3+76.60	16.68 1 6.42 1	24 24	RCP RCP	3.14 6.28	0.50 0.50	0.013	520.68 520.48		0.	.99 0.50 .00 0.00			10 10.18	100	9.8	4.85 40	0.34 Yes	s 1.54	0.07	0.0003 0.0003	0.00	523.37 523.35	523.35	0.00 0.0 0.04 0.0	4	Inlet at Beginning of Line 45 Bend	0 0.37	0.05 0.01	523.37	525.77	2.35	
3+76.60 3+56.98 3+56.98 3+36.60	19.62 1 20.38 1	24 27 27	RCP RCP	3.14 6.28 3.98 7.07	0.56	0.013	519.86 519.69		C1&C2 1.	.42 0.50 .99 0.61	0.7 1.2	2.4	10.25		9.74	23.58 27	0.34 Yes	s 5.93	0.06	0.0007	0.02	523.11	523.09	0.04 0.2 0.22 0.5	5 MH	45 Wye H w/ 0 Change in Pipe Direction	0.5	0.20	523.14	525.94	2.80	
3+36.60 2+82.30 2+82.30 2+35.80 2+35.80 2+19.06	54.30 1 46.50 1 16.74 1	36	RCP RCP RCP	3.98 7.07 7.07 9.42	0.75	0.013	519.69 519.26 519.26 518.89 518.89 518.75	0.008	0.	.00 0.00 .00 0.00	0.0	2.4	10.39 10.55 10.78	100	9.71	23.50 59	7.70 Yes 9.66 Yes 9.66 Yes	s 3.32	0.23	0.0010 0.0005 0.0005	0.02	522.89 522.76 522.64	522.74	0.55 0.5 0.55 0.1	.7 MH	45 Bend H w/ 45 Change in Pipe Direction 45 Wye	0.37 0.42	0.20 0.07	522.83	525.48	2.65	
2+35.80 2+19.06 2+19.06 1+65.03 1+65.03 1+25.38	16.74 1 54.03 1 39.65 1	36 36 36	RCP RCP RCP	7.07 9.42 7.07 9.42 7.07 9.42	0.75	0.013	518.75 518.32	0.008 0.008 0.007	C3B 0.	.27 0.50 .19 0.50 .00 0.00		<u> </u>	10.78 10.86 11.11	100	9.66	25.59 59	9.66 Yes 9.66 Yes 5.80 Yes	s 3.62	0.25	0.0005 0.0005 0.0005	0.03		522.59	0.17 0.1 0.19 0.2 0.20 0.2	0 MH	45 Wye H w/ 0 Change in Pipe Direction I w/ 45 Change in Pipe Direction	0.5 0.05 0.42	0.10 0.01 0.08	522.63	525.54 525.70		
1+25.38 1+00.00 rain Line SD-C2	25.38 1	36	RCP	7.07 9.42				0.007		.00 0.00	0.0	2.7	11.29				5.80 Yes			0.0005				0.20 0.2		45 Bend	0.37	0.08				
0+57.85 0+00.00 ain Line SD-C2B	57.85 1	30		4.91 7.85		0.013	520.09 519.85	0.004	C1&C2 1.	.99 0.61	1.2	1.2	10	100	9.8	11.91 26	6.58 Yes	s 2.43	0.40	0.0004	0.02	523.16	523.14	0.00 0.0	9	Inlet at Beginning of Line	0	0.00	523.16	525.77	2.61	Areas C1A, C1B & C2
0+20.58 0+00.00 in Line SD-C3A	20.58 1	21		2.41 5.50			521.35 520.71	1		.42 0.50	0.7	0.7	10	100	9.8	6.96 27	7.99 Yes	s 2.89	0.12	0.0006	0.01	523.36	523.35	0.00 0.1	3	Inlet at Beginning of Line	0	0.00	523.36	525.77	2.41	
0+16.74 0+00.00	16.74 1	1 1	Den	2.41 5.50	0.44	0.013	520.94 520.13		C3A 0.	.27 0.50		0.1		100	9.8	1.32 34	4.86 Yes	s 0.55	0.51	0.0001	0.00			0.00 0.0		Inlet at Beginning of Line			522.74			

RCP 2.41 5.50 0.44 0.013 519.46 519.29 0.008 C3B 0.19 0.50 0.1 0.1 10 100 9.8 0.93 14.35 Yes 0.39 0.89 0.001 0.00 522.63 522.63 0.00 0.00 Inlet at Beginning of Line 0 0.00 522.63 525.44 2.81

REFER TO DRAINAGE STUDY REPORT FOR FLOWS

REFER TO DRAINAGE STUDY REPORT FOR FLOWS

REFER TO DRAINAGE STUDY REPORT FOR FLOWS

 88.00
 169.96
 Yes
 7.00
 6.33
 0.0008
 0.04
 510.34
 510.30
 0.00
 0.76
 Inlet at Beginning of Line
 0
 0.95
 511.29
 522.40
 11.11

 88.00
 169.96
 Yes
 7.00
 8.21
 0.0008
 0.05
 510.11
 510.07
 0.76
 0.76
 30 Bend
 0.25
 0.19
 510.30
 510.30

 88.00
 143.64
 Yes
 7.00
 3.69
 0.0008
 0.02
 510.03
 510.01
 0.76
 0.76
 MH w/ 0 Change in Pipe Direction
 0.05
 0.04
 510.07
 518.11
 8.04

0+20.58 0+00 Storm Drain Line SD-Pond Outfall

2+27.65

1+83.29

0+00.00

1+83.29

1+25.81

1+00.00

44.36 57.48 25.81

 RCP
 12.57
 12.57
 1.00
 0.013
 507.69
 507.07
 0.014

 RCP
 12.57
 12.57
 1.00
 0.013
 507.07
 506.27
 0.014

 RCP
 12.57
 12.57
 1.00
 0.013
 506.27
 506.01
 0.010

PAPE-DAWSON ENGINEERS

GIDEON GROVE - PHASE 2
OF ROCKWALL, ROCKWALL COUNTY, TEXAS

UTILITY NOTE

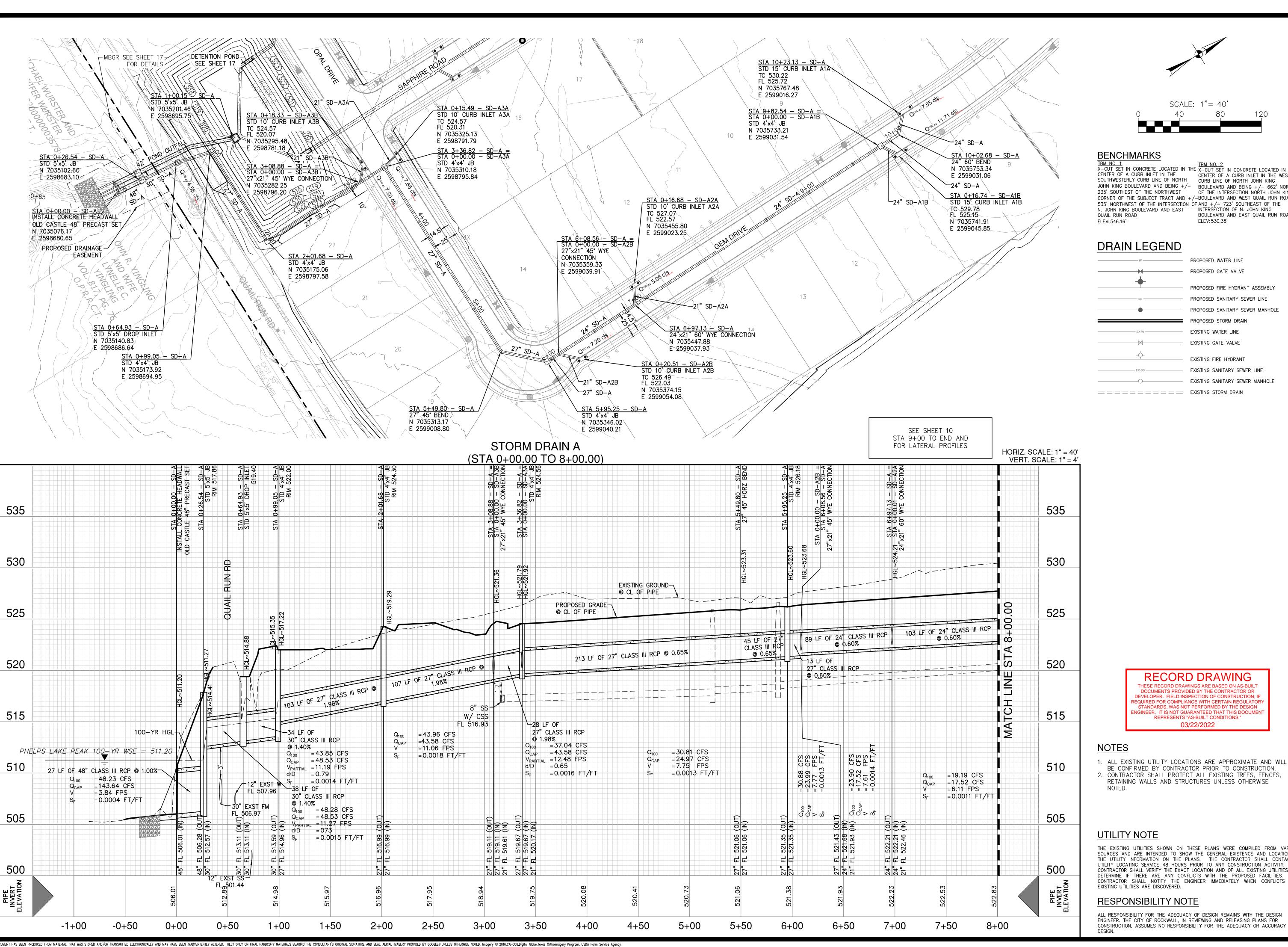
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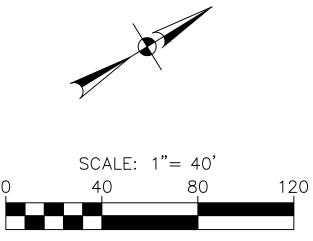
THESE RECORD DRAWINGS ARE BASED ON AS-BUILT DOCUMENTS PROVIDED BY THE CONTRACTOR OR DEVELOPER. FIELD INSPECTION OF CONSTRUCTION, IF REQUIRED FOR COMPLIANCE WITH CERTAIN REGULATORY STANDARDS, WAS NOT PERFORMED BY THE DESIGN ENGINEER. IT IS NOT GUARANTEED THAT THIS DOCUMENT REPRESENTS "AS-BUILT CONDITIONS." 03/22/2022

RESPONSIBILITY NOTE

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PLAT NO.	#	
JOB NO	6126300	
DATE	June 21	
DESIGNER	ML	
CHECKED	ASR DRAWN	ML
_		



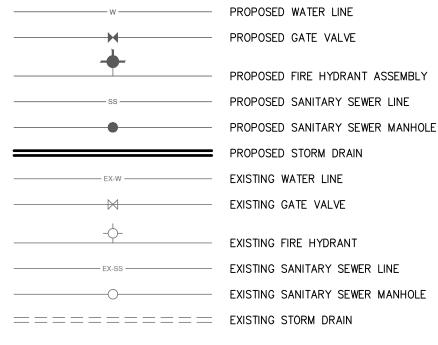


TBM NO. 1 TBM NO. 2

X-CUT SET IN CONCRETE LOCATED IN THE X-CUT SET IN CONCRETE LOCATED IN THE CENTER OF A CURB INLET IN THE SOUTHWESTERLY CURB LINE OF NORTH JOHN KING BOULEVARD AND BEING +/- BOULEVARD AND BEING +/- 662' NORTH 235' SOUTHEST OF THE NORTHWEST CORNER OF THE SUBJECT TRACT AND +/-BOULEVARD AND WEST QUAIL RUN ROAD 535' NORTHWEST OF THE INTERSECTION OF AND +/- 723' SOUTHEAST OF THE N. JOHN KING BOULEVARD AND EAST

CENTER OF A CURB INLET IN THE WEST CURB LINE OF NORTH JOHN KING OF THE INTERSECTION NORTH JOHN KING INTERSECTION OF N. JOHN KING

BOULEVARD AND EAST QUAIL RUN ROAD. ELEV: 530.38'



PAPE-DAV ENGINEEI

0

ADAM S. REEVES

98410

STORM DRAIN A PLAN AND PROFIL

GIDEON GROVE - PHASE OF ROCKWALL, ROCKWALL COUNTY

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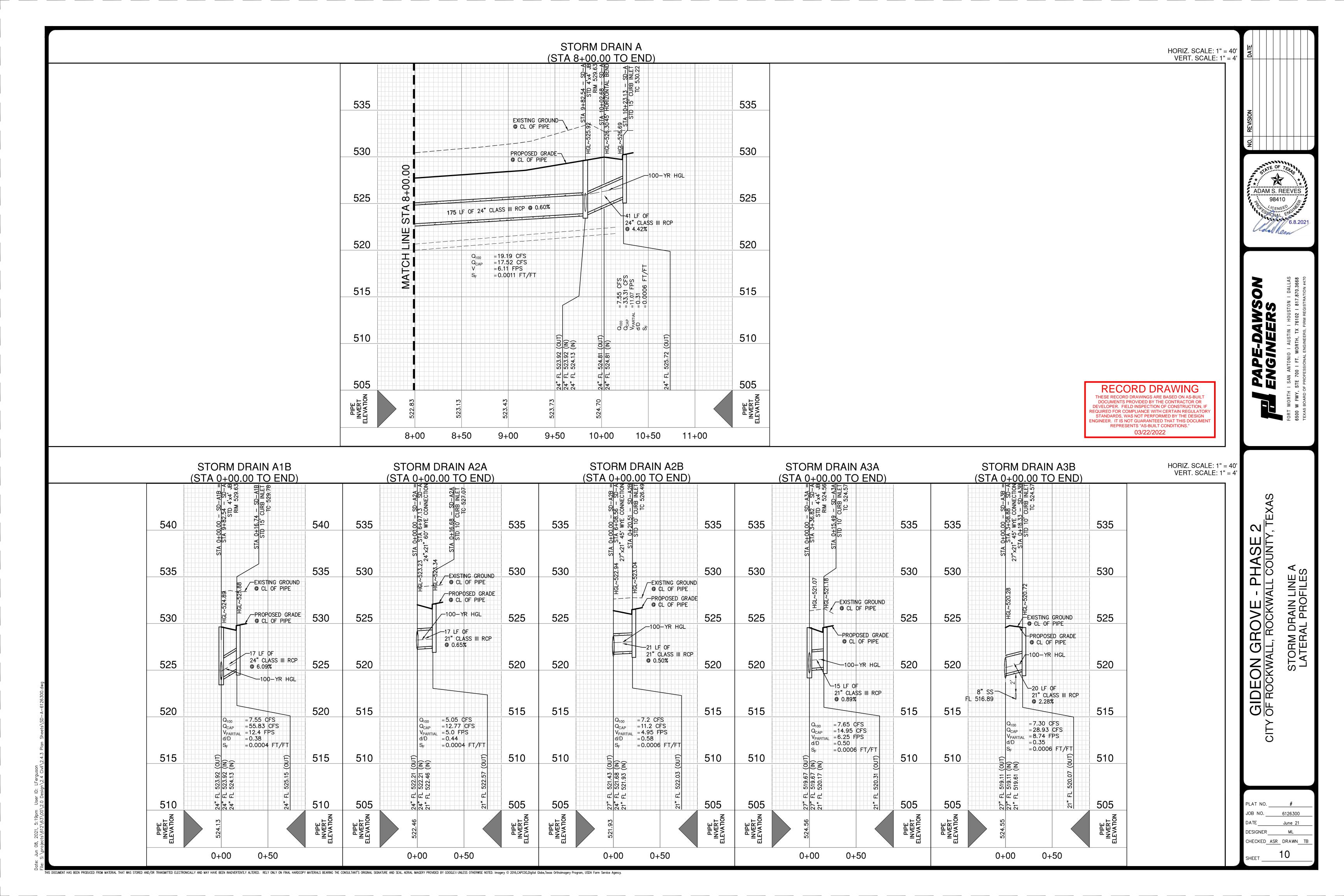
REPRESENTS "AS-BUILT CONDITIONS."

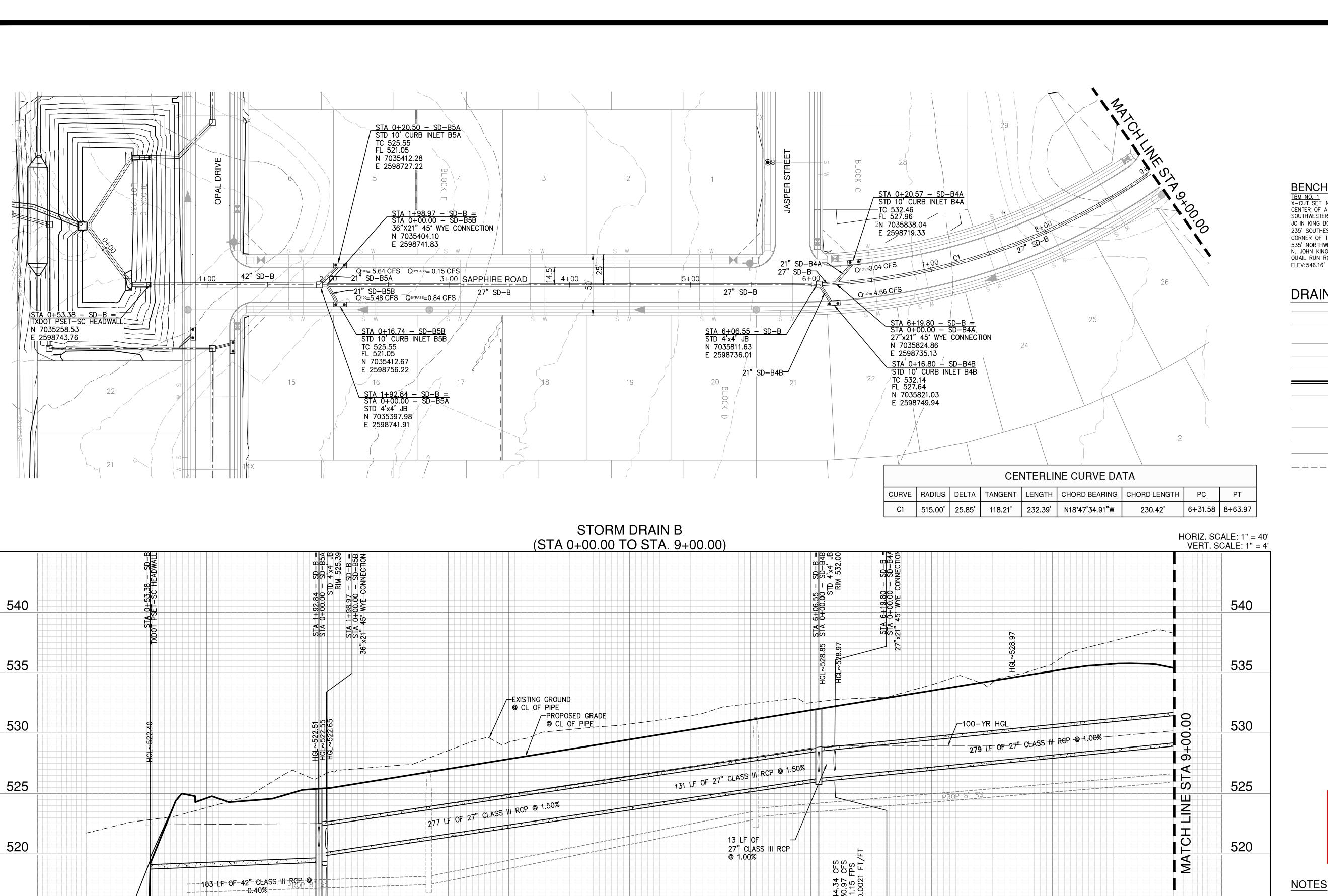
03/22/2022

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	#	
	6126300	
	June 21	
	ML	
ASR	DRAWN_	TB
	ASR	6126300 June 21





= 37.93 CFS

=12.30 FPS

=0.0021 FT/FT

5+00

5+50

6+00

=54.56 CFS

=81.69 CFS

=12.35 FPS

=0.0011 FT/FT

3+00

3+50

= 0.59

d/D

515. 520. 519. 520.

2+00

W/ CSS

0+50

FL 513.20

=60.15 CFS

=63.63 CFS

= 7.52 FPS

=0.0008 FT/FT

1+50

= 0.77

66 LF PILOT CHANNEL

W/ 4" THICK CONCRETE

SÉE DETAIL ON SHEET 18

44.00.0

Q Q > \hat{\alpha}

27" FL 526.38 (OUT 27" FL 526.38 (IN) 21" FL 526.88 (IN)

7+00

7+50

6+50

= 30.97 CFS

=10.50 FPS

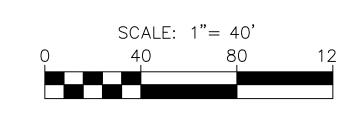
=0.0018 FT/FT

8+00

8+50

9+00



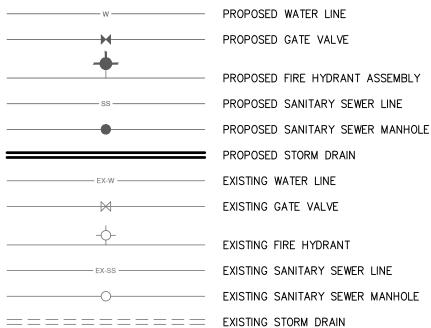


BENCHMARKS

TBM NO. 1
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N. JOHN KING BOULEVARD AND EAST INTERSECTION OF N. JOHN KING
QUAIL RUN ROAD BOULEVARD AND EAST QUAIL RUN ROAD.

CENTER OF A CURB INLET IN THE WEST CURB LINE OF NORTH JOHN KING ELEV: 530.38'

DRAIN LEGEND



RECORD DRAWING

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NOTES

515

510

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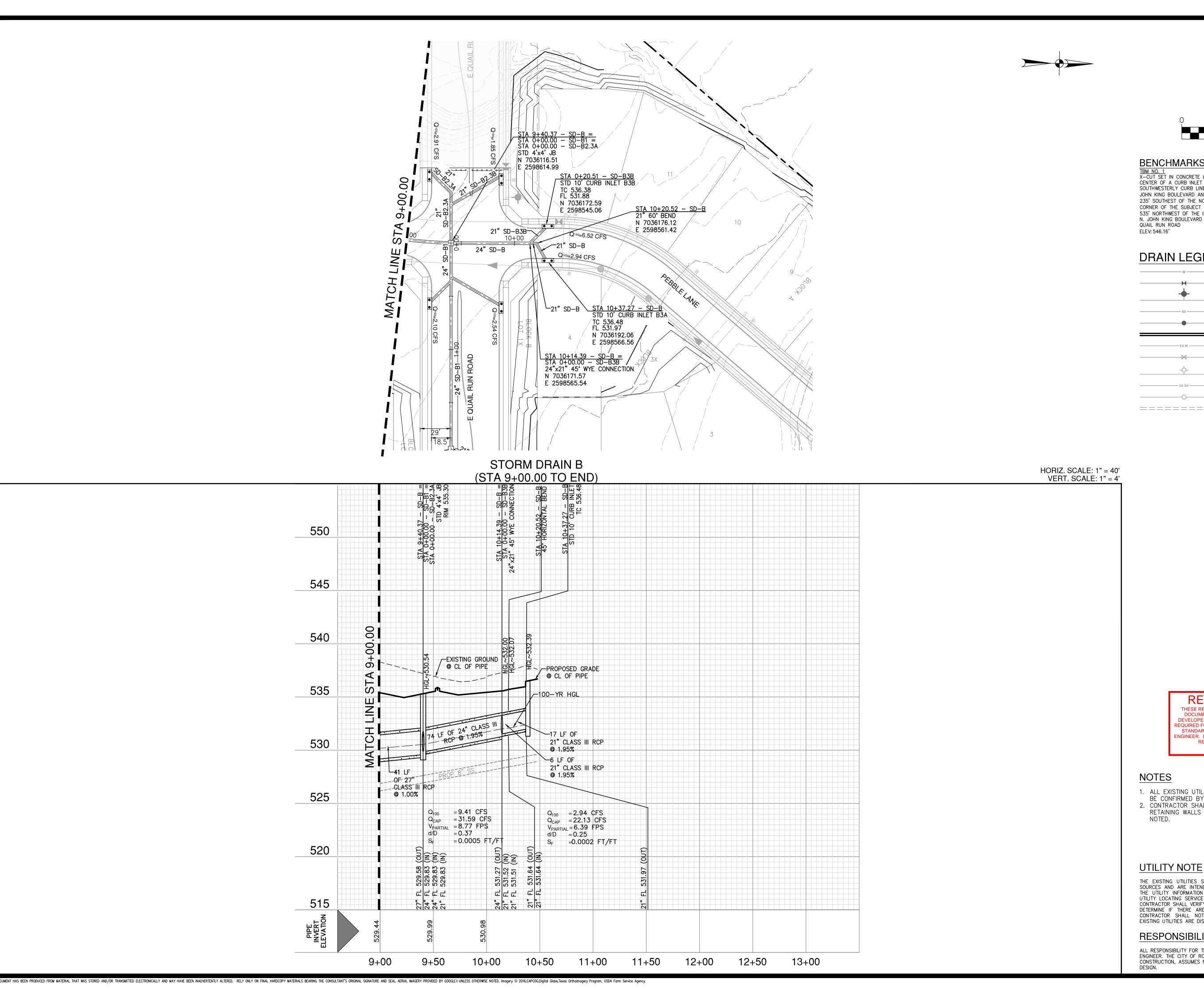
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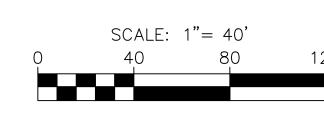
June 21 ESIGNER HECKED<u>ASR</u> DRAWN<u>TB</u>

ADAM S. REEVES 98410

8

STORM DRAIN LINE PLAN AND PROFILE 1



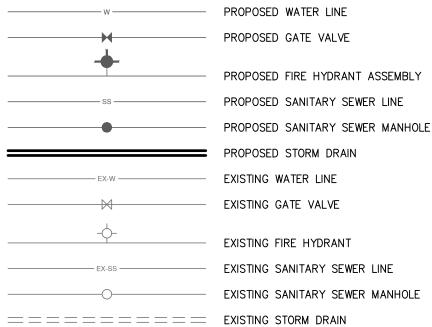


TBM NO. 1
X-CUT SET IN CONCRETE LOCATED IN THE X-CUT SET IN CONCRETE LOCATED IN THE CENTER OF A CURB INLET IN THE CENTER OF A CURB INLET IN THE WEST SOUTHWESTERLY CURB LINE OF NORTH CURB LINE OF NORTH JOHN KING JOHN KING BOULEVARD AND BEING +/- BOULEVARD AND BEING +/- 662' NORTH 235' SOUTHEST OF THE NORTHWEST CORNER OF THE SUBJECT TRACT AND +/-BOULEVARD AND WEST QUAIL RUN ROAD 535' NORTHWEST OF THE INTERSECTION OF AND +/- 723' SOUTHEAST OF THE N. JOHN KING BOULEVARD AND EAST INTERSECTION OF N. JOHN KING

OF THE INTERSECTION NORTH JOHN KING ELEV: 530.38'

INTERSECTION OF N. JOHN KING BOULEVARD AND EAST QUAIL RUN ROAD.

DRAIN LEGEND



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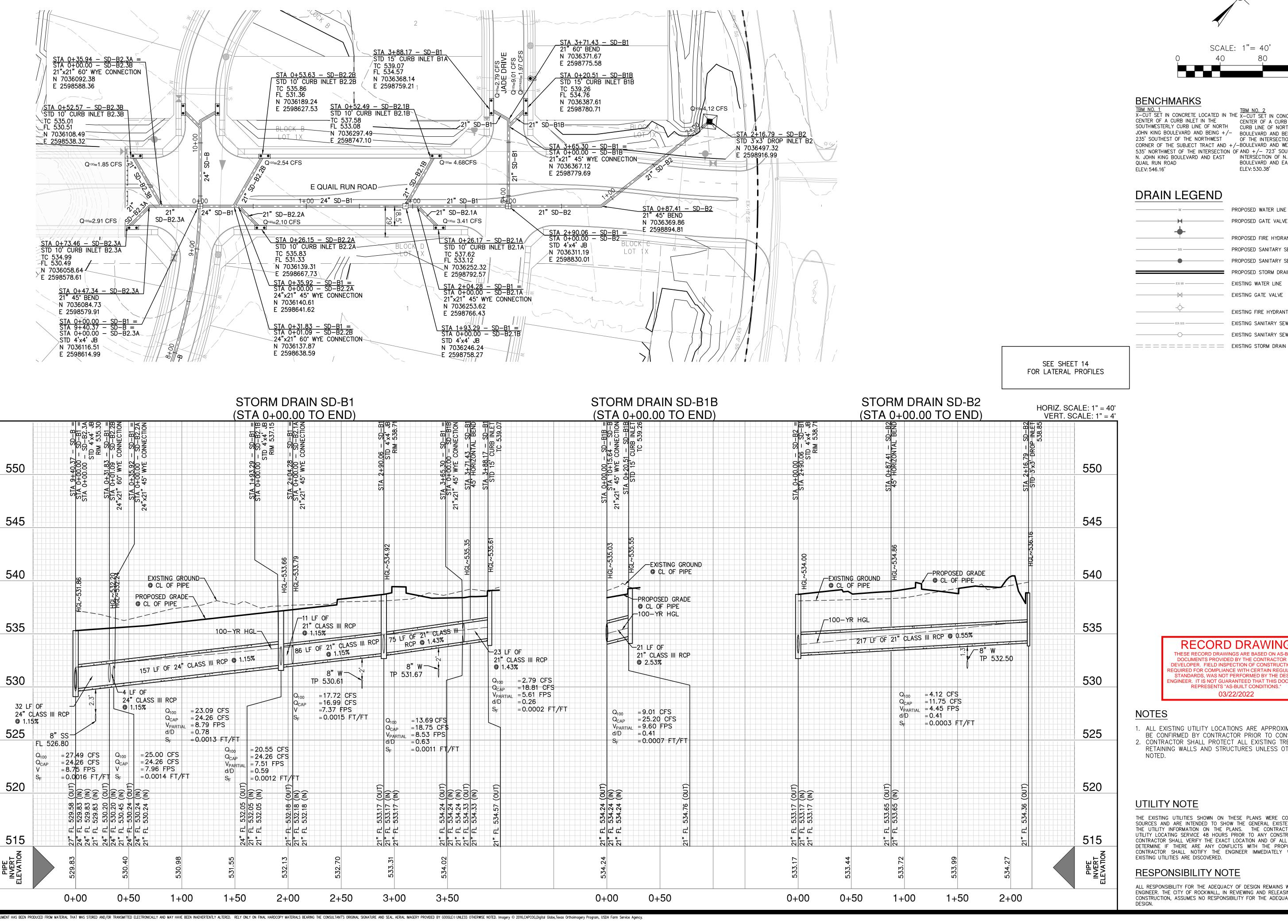
RESPONSIBILITY NOTE

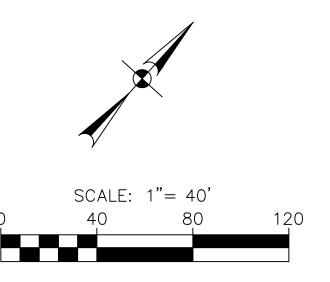
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June 21 DESIGNER CHECKED ASR DRAWN TB 12

ADAM S. REEVES 98410

STORM DRAIN LINE B PLAN AND PROFILE 2 OF

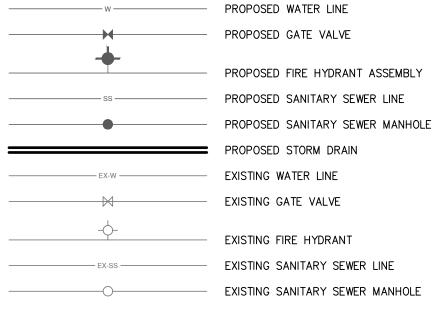




TBM NO. 1 X-CUT SET IN CONCRETE LOCATED IN THE X-CUT SET IN CONCRETE LOCATED IN THE CENTER OF A CURB INLET IN THE SOUTHWESTERLY CURB LINE OF NORTH JOHN KING BOULEVARD AND BEING +/- BOULEVARD AND BEING +/- 662' NORTH 235' SOUTHEST OF THE NORTHWEST CORNER OF THE SUBJECT TRACT AND +/-BOULEVARD AND WEST QUAIL RUN ROAD 535' NORTHWEST OF THE INTERSECTION OF AND +/- 723' SOUTHEAST OF THE N. JOHN KING BOULEVARD AND EAST ELEV: 530.38'

CENTER OF A CURB INLET IN THE WEST CURB LINE OF NORTH JOHN KING OF THE INTERSECTION NORTH JOHN KING INTERSECTION OF N. JOHN KING
BOULEVARD AND EAST QUAIL RUN ROAD.

DRAIN LEGEND



RECORD DRAWING

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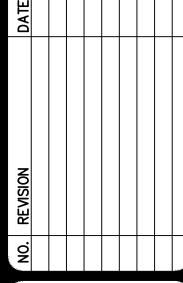
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6126300 June 21 ESIGNER HECKED<u>ASR</u> DRAWN<u>TB</u> 13



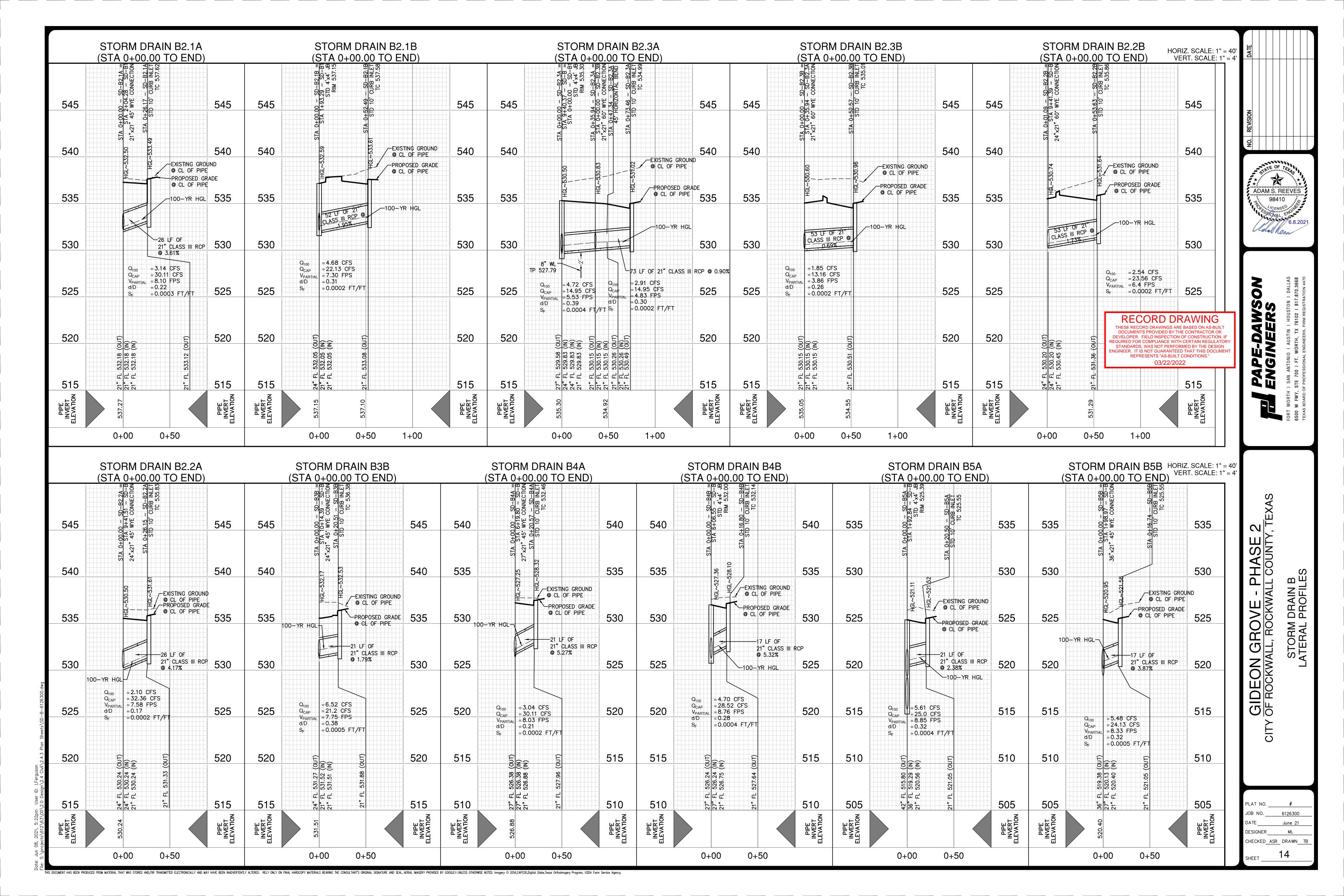
ADAM S. REEVES 98410

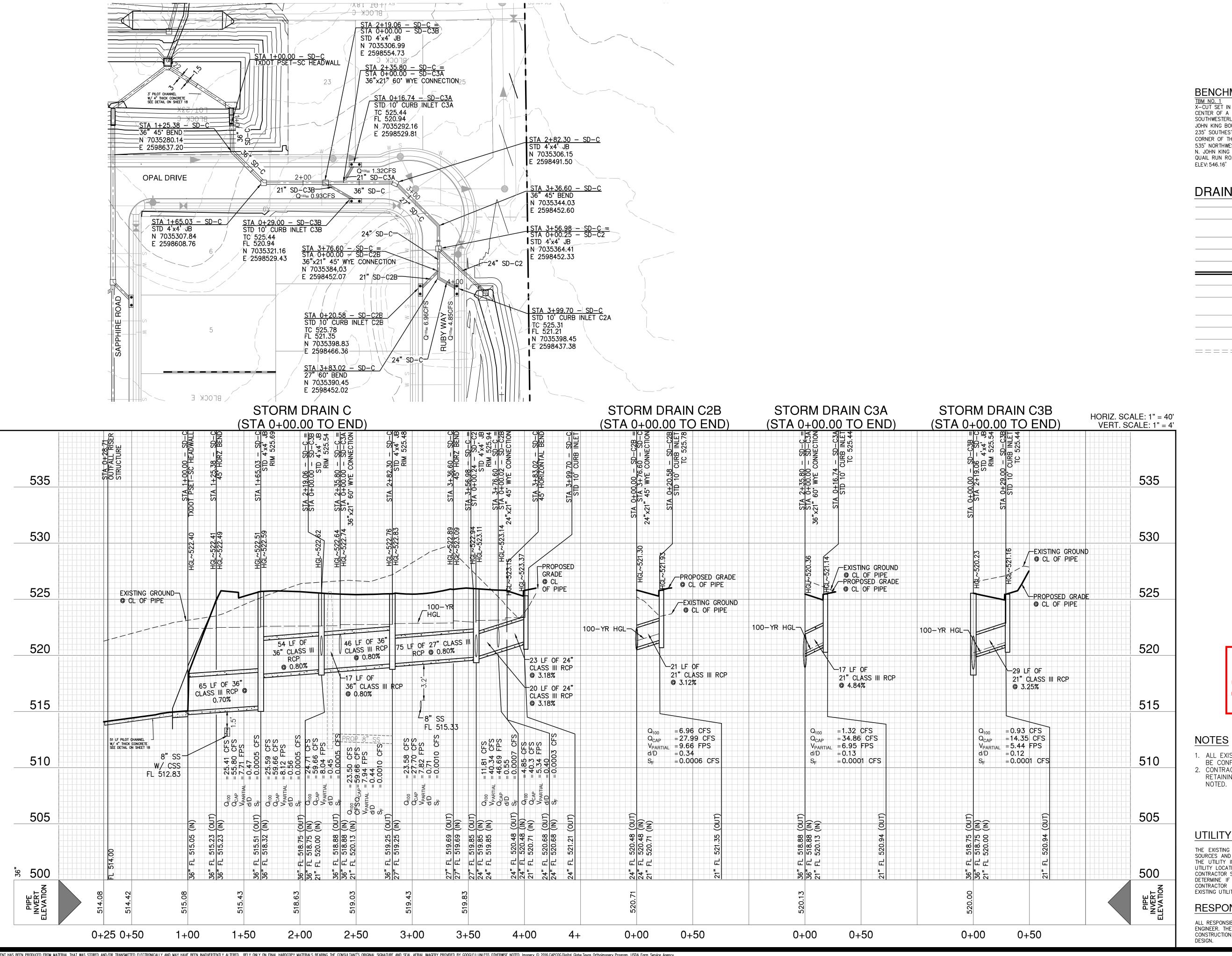
80 PAPE-DAWS ENGINEERS

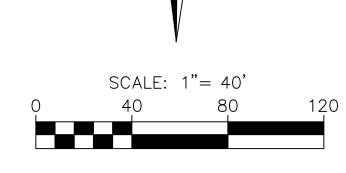
STORM DRAIN LINE AN AND PROFILE 2

ON GROVE - PHASE KWALL, ROCKWALL COUNTY

GIDE(OF ROCK







TBM NO. 1 TBM NO. 2

X-CUT SET IN CONCRETE LOCATED IN THE X-CUT SET IN CONCRETE LOCATED IN THE CENTER OF A CURB INLET IN THE SOUTHWESTERLY CURB LINE OF NORTH JOHN KING BOULEVARD AND BEING +/- BOULEVARD AND BEING +/- 662' NORTH 235' SOUTHEST OF THE NORTHWEST CORNER OF THE SUBJECT TRACT AND +/-BOULEVARD AND WEST QUAIL RUN ROAD 535' NORTHWEST OF THE INTERSECTION OF AND +/- 723' SOUTHEAST OF THE N. JOHN KING BOULEVARD AND EAST QUAIL RUN ROAD

CENTER OF A CURB INLET IN THE WEST CURB LINE OF NORTH JOHN KING OF THE INTERSECTION NORTH JOHN KING INTERSECTION OF N. JOHN KING BOULEVARD AND EAST QUAIL RUN ROAD.

ELEV: 530.38'

DRAIN LEGEND

	PROPOSED WATER LINE
	PROPOSED GATE VALVE
	PROPOSED FIRE HYDRANT ASSEMBLY
SS	PROPOSED SANITARY SEWER LINE
•	PROPOSED SANITARY SEWER MANHOLE
	PROPOSED STORM DRAIN
EX-W	EXISTING WATER LINE
——————————————————————————————————————	EXISTING GATE VALVE
<u> </u>	EXISTING FIRE HYDRANT
EX-SS	EXISTING SANITARY SEWER LINE
<u> </u>	EXISTING SANITARY SEWER MANHOLE
========	EXISTING STORM DRAIN

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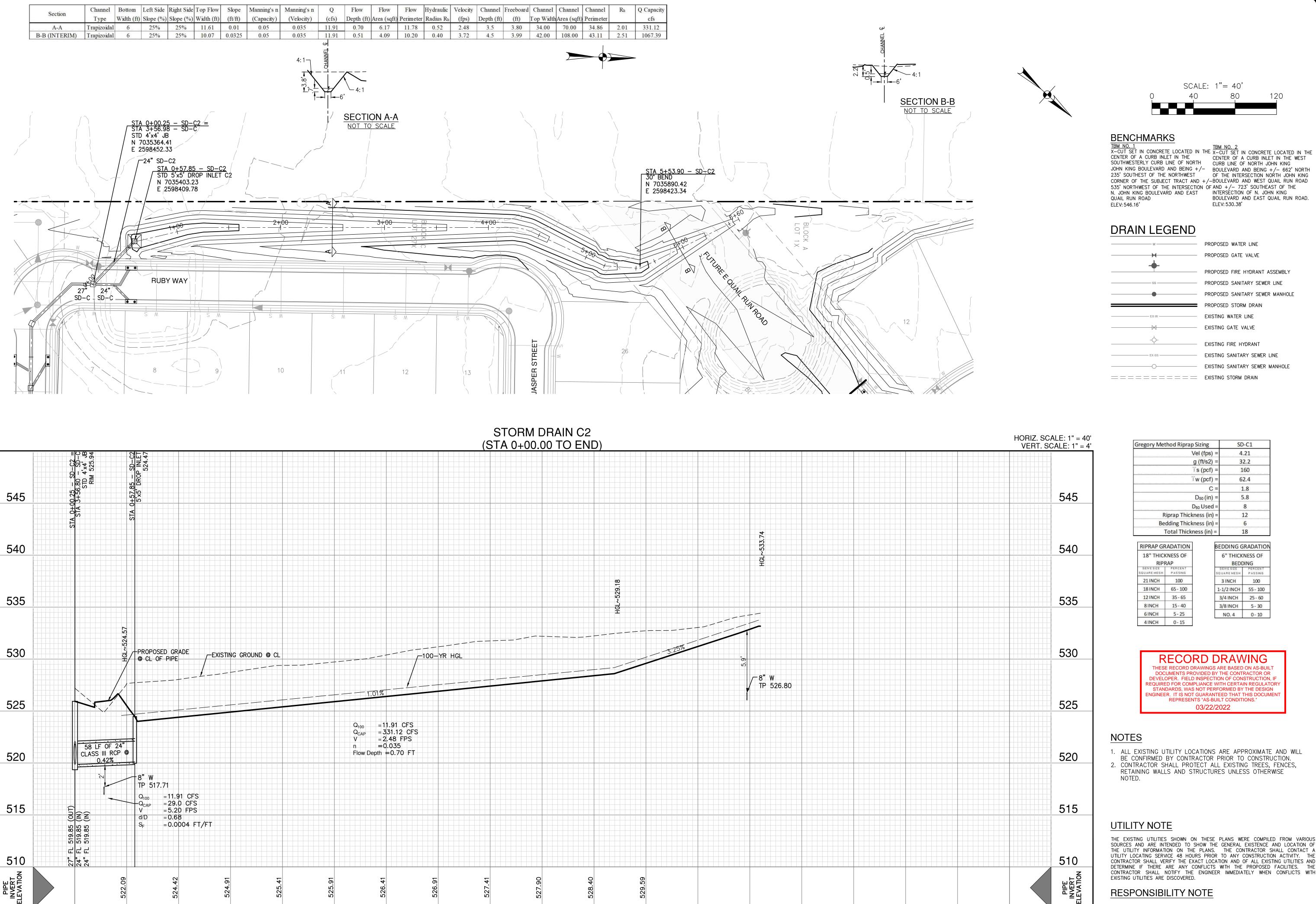
6126300 June 21 ESIGNER HECKED<u>ASR</u> DRAWN<u>TB</u> 15

ADAM S. REEVES 98410

PAPE-DAWS ENGINEERS

0

GIDEON GROVE - PHASE 2
OF ROCKWALL, ROCKWALL COUNTY, TEXAS ORM DRAIN LINE PLAN & PROFILE



5+50

6+00

6+50

7+00

7+50

8+00

8+50

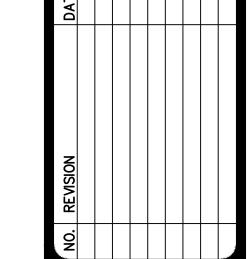
9+00

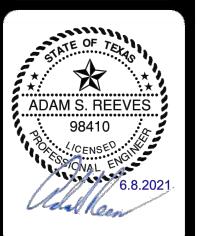
0+50

1+50

3+00

3+50





PAPE-DAWSON ENGINEERS

DRAIN LEGEND

PROPOSED WATER LINE PROPOSED GATE VALVE PROPOSED FIRE HYDRANT ASSEMBLY PROPOSED SANITARY SEWER LINE PROPOSED SANITARY SEWER MANHOLE PROPOSED STORM DRAIN EXISTING WATER LINE EXISTING GATE VALVE EXISTING FIRE HYDRANT EXISTING SANITARY SEWER LINE EXISTING SANITARY SEWER MANHOLE

ELEV: 530.38'

Gregory Method Riprap Sizing	SD-C1
Vel (fps) =	4.21
g (ft/s2) =	32.2
îs (pcf) =	160
Ƴw (pcf) =	62.4
C =	1.8
D ₅₀ (in) =	5.8
D ₅₀ Used =	8
Riprap Thickness (in) =	12
Bedding Thickness (in) =	6
Total Thickness (in) =	18

- 0.5	otal linemiese	, (,	
RIPRAP GR	ADATION	BEDDING G	RADAT
18" THICK	MATERIAL STATE OF THE STATE OF	6" THICK BEDD	W. T. Washington
SEIVE SIZE SQUARE MESH	PERCENT PASSING	SEIVE SIZE SQUARE MESH	PERCEN PASSIN
21 INCH	100	3 INCH	100
18 INCH	65 - 100	1-1/2 INCH	55 - 10
12 INCH	35 - 65	3/4 INCH	25 - 60
8 INCH	15 - 40	3/8 INCH	5 - 30
6 INCH	5 - 25	NO. 4	0 - 10
4 INCH	0 - 15		

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- 2. CONTRACTOR SHALL PROTECT ALL EXISTING TREES, FENCES, RETAINING WALLS AND STRUCTURES UNLESS OTHERWISE NOTED.

UTILITY NOTE

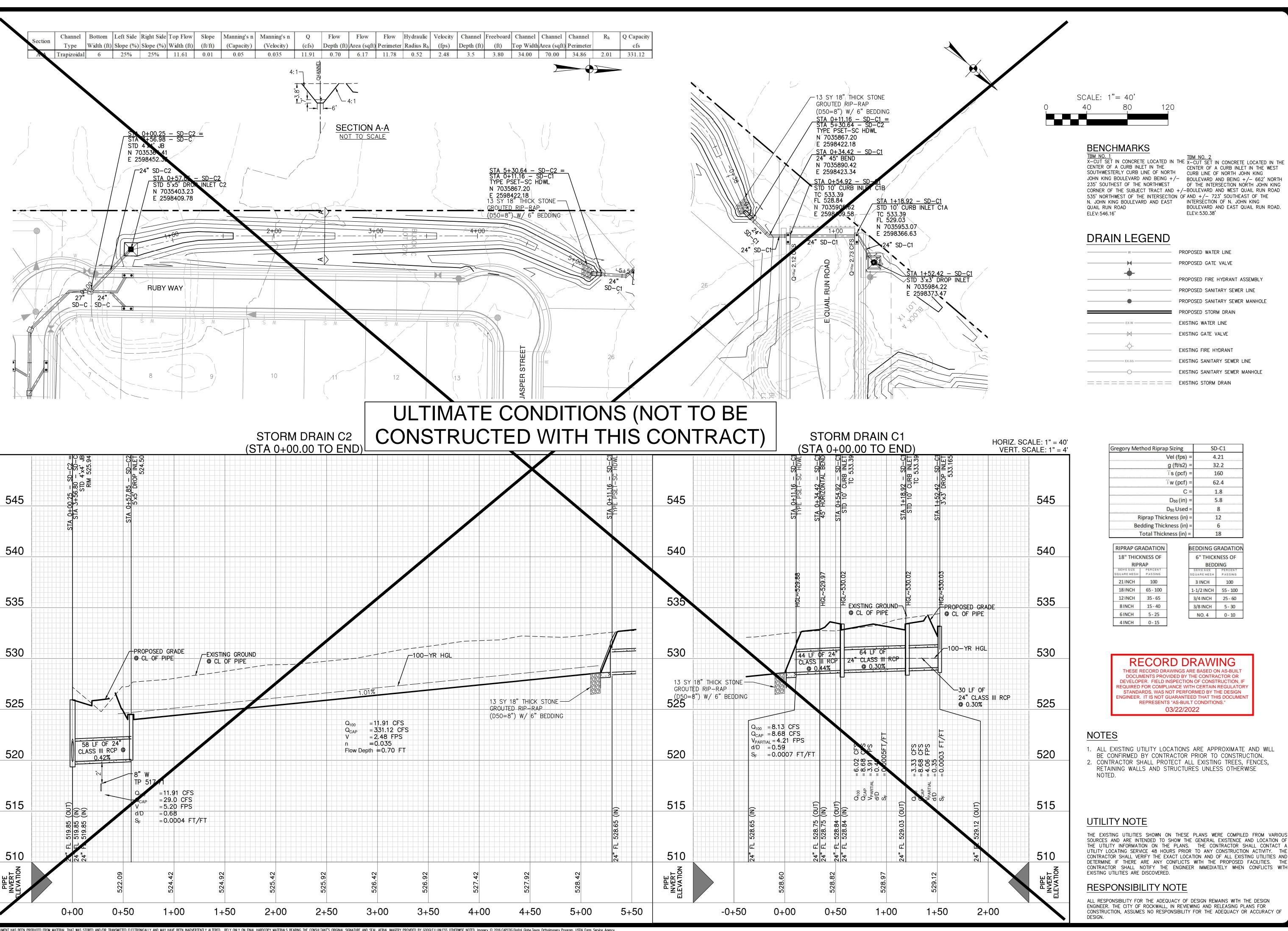
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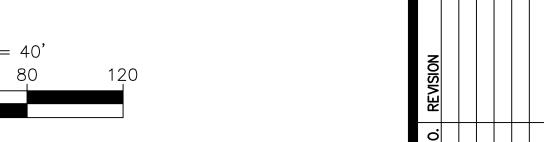
RESPONSIBILITY NOTE

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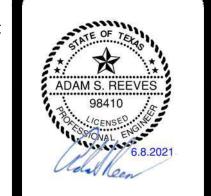
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PLAT NO.		#	
JOB NO		6126300	
DATE		June 21	
DESIGNER		ML	
CHECKED_	ASR	DRAWN_	TB
		10	

ORM DRAIN LINE PLAN & PROFILE





OF THE INTERSECTION NORTH JOHN KING



	PROPOSED WATER LINE
	PROPOSED GATE VALVE
	PROPOSED FIRE HYDRANT ASSEMBLY
ss	PROPOSED SANITARY SEWER LINE
•	PROPOSED SANITARY SEWER MANHOLE
	PROPOSED STORM DRAIN
EX-W	EXISTING WATER LINE
$\overline{}$	EXISTING GATE VALVE
<u> </u>	EXISTING FIRE HYDRANT
EX-SS	EXISTING SANITARY SEWER LINE
<u> </u>	EXISTING SANITARY SEWER MANHOLE
	FXISTING STORM DRAIN

Gregory Method Riprap Sizing	SD-C1
Vel (fps) =	4.21
g (ft/s2) =	32.2
îs (pcf) =	160
Ƴw (pcf) =	62.4
C =	1.8
D ₅₀ (in) =	5.8
D ₅₀ Used =	8
Riprap Thickness (in) =	12
Bedding Thickness (in) =	6
Total Thickness (in) =	18

RIPRAP GR	ADATION	BEDDING G	RADATI
18" THICK	525C-054	6" THICK BEDD	NA THE STATE OF TH
SEIVE SIZE SQUARE MESH	PERCENT PASSING	SEIVE SIZE SQUAREMESH	PERCEN PASSINI
21 INCH	100	3 INCH	100
18 INCH	65 - 100	1-1/2 INCH	55 - 10
12 INCH	35 - 65	3/4 INCH	25 - 60
8 INCH	15 - 40	3/8 INCH	5 - 30
6 INCH	5 - 25	NO. 4	0 - 10
4 INCH	0 - 15		

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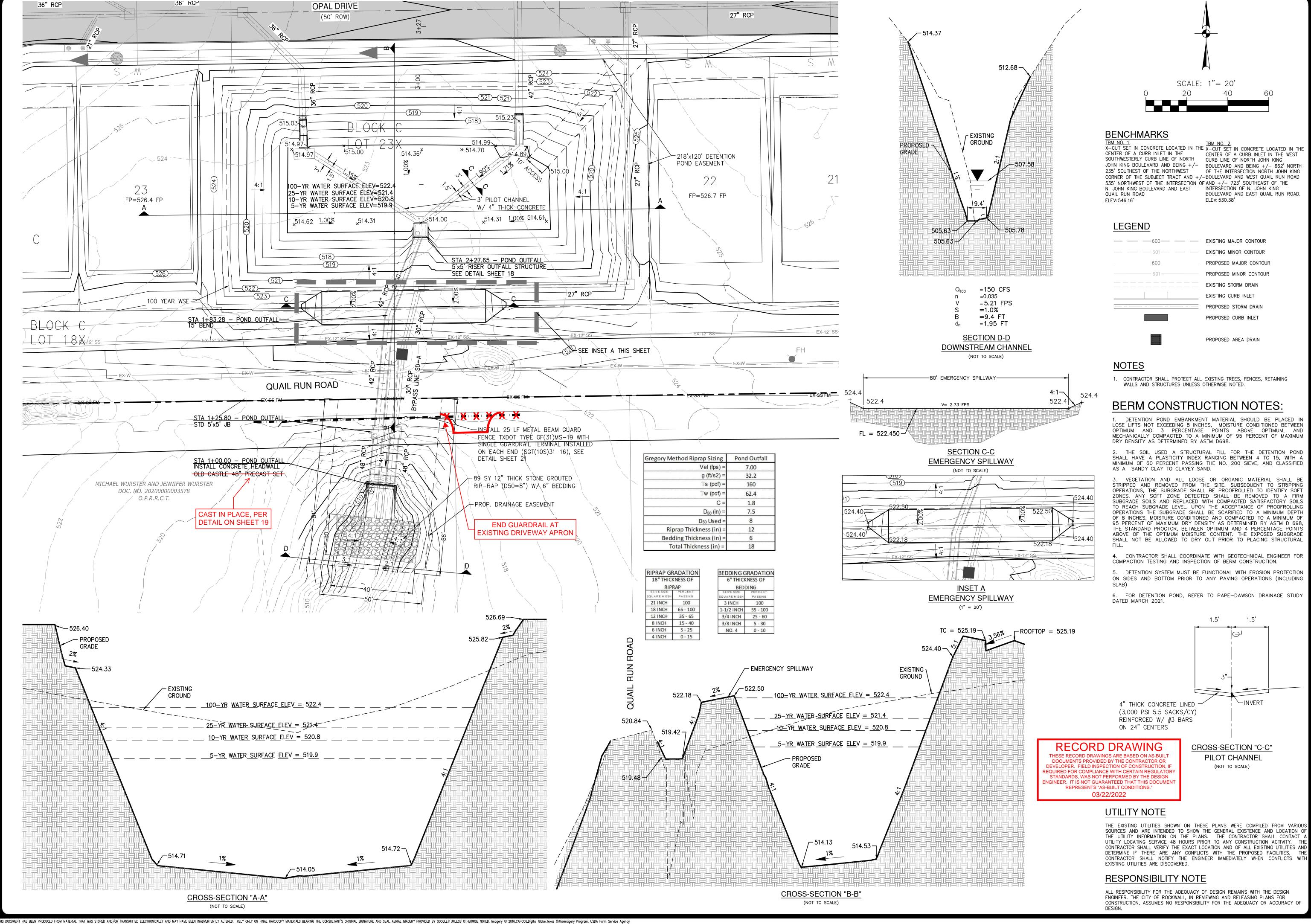
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PLAT NO.	#
JOB NO	6126300
DATE	June 2021
DESIGNER	ML
CHECKED_	ASR DRAWN TB



NO. REVISION DATE

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6.8.2021

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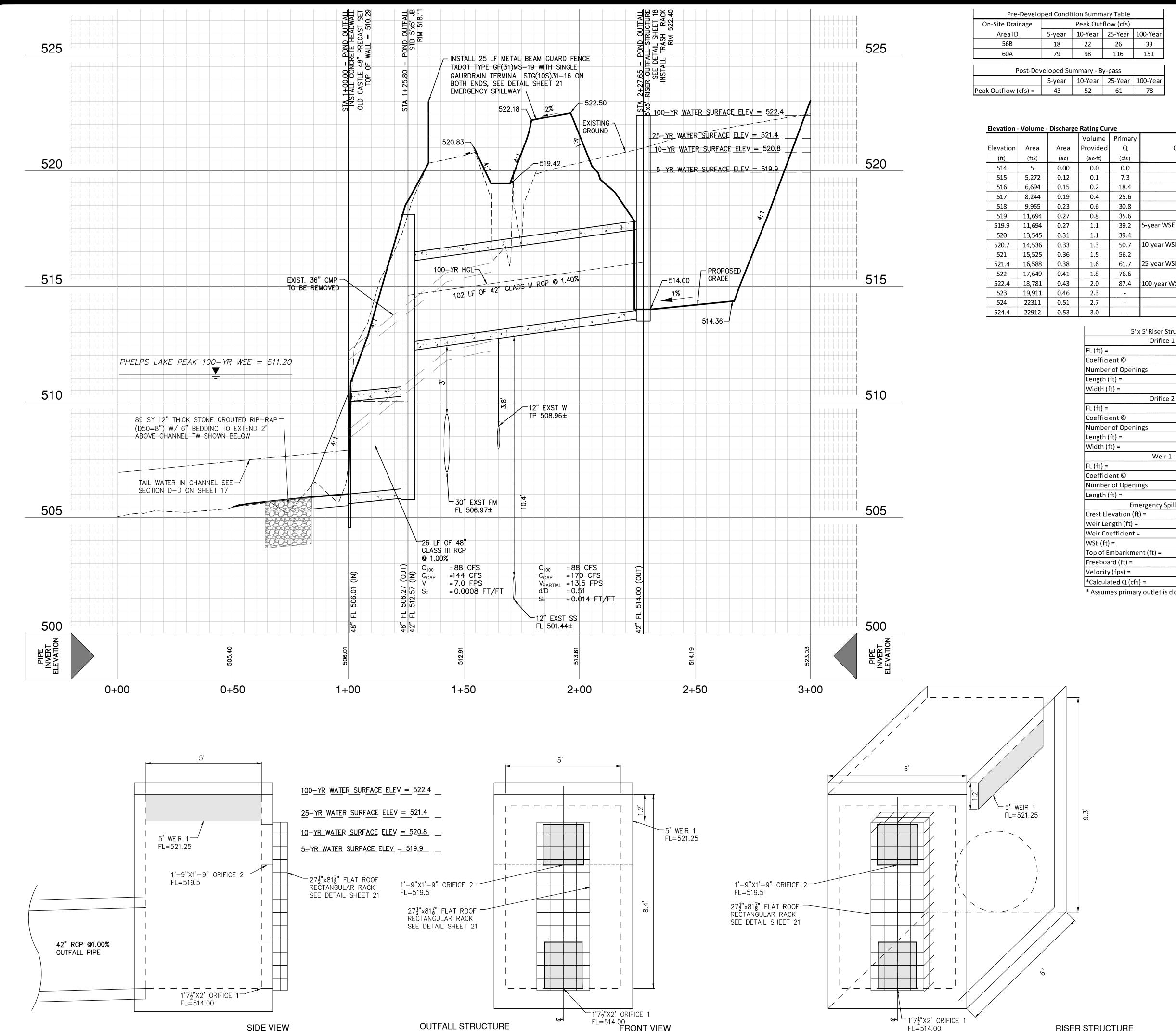
PLAT NO. #

JOB NO. 6126300

DATE June 21

DESIGNER OEK

CHECKED ASR DRAWN OEK



SIDE VIEW

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Outflow (cfs) =	79	91	111	150
Det	ention Sur	nmary Tab	le	
	5-year	10-Year	25-Year	100-Yea
Inflow (cfs) =	73	88	103	131
Outflow (cfs) =	39	52	62	87
Storage (ac-ft) =	1.1	1.3	1.6	2.0
k Elevation (ft) =	519.9	520.7	521.4	522.4
		Detention Sur 5-year Inflow (cfs) = 73 Outflow (cfs) = 39 Storage (ac-ft) = 1.1	Detention Summary Tab 5-year 10-Year Inflow (cfs) = 73 88 Outflow (cfs) = 39 52 Storage (ac-ft) = 1.1 1.3	Detention Summary Table 5-year 10-Year 25-Year

Post-Developed Summary - Pond & By-pass Combined

5-year | 10-Year | 25-Year | 100-Year

			Volume	Primary	
Elevation	Area	Area	Provided	Q	Comments
(ft)	(ft2)	(ac)	(a c-ft)	(cfs)	
514	5	0.00	0.0	0.0	
515	5,272	0.12	0.1	7.3	
516	6,694	0.15	0.2	18.4	
517	8,244	0.19	0.4	25.6	
518	9,955	0.23	0.6	30.8	
519	11,694	0.27	0.8	35.6	
519.9	11,694	0.27	1.1	39.2	5-year WSE
520	13,545	0.31	1.1	39.4	
520.7	14,536	0.33	1.3	50.7	10-year WSE
521	15,525	0.36	1.5	56.2	
521.4	16,588	0.38	1.6	61.7	25-year WSE
522	17,649	0.41	1.8	76.6	
522.4	18,781	0.43	2.0	87.4	100-year WSE
523	19,911	0.46	2.3	-	
524	22311	0.51	2.7	-	
524.4	22912	0.53	3.0		

5' x 5' Riser Structure	
Orifice 1	
FL (ft) =	514
Coefficient ©	0.67
Number of Openings	1
Length (ft) =	2
Width (ft) =	1.625
Orifice 2	
FL (ft) =	519.5
Coefficient ©	0.67
Number of Openings	1
Length (ft) =	1.75
Width (ft) =	1.75
Weir 1	
FL (ft) =	521.25
Coefficient ©	2.65
Number of Openings	1
Length (ft) =	5
Emergency Spillway 1	
Crest Elevation (ft) =	522.4
Weir Length (ft) =	80
Weir Coefficient =	2.7
WSE (ft) =	522.5
Top of Embankment (ft) =	524.4
Freeboard (ft) =	1.9
	T T

* Assumes primary outlet is clogged

RISER STRUCTURE

FL=514.00

BENCHMARKS

TBM NO. 1 X-CUT SET IN CONCRETE LOCATED IN THE X-CUT SET IN CONCRETE LOCATED IN THE CENTER OF A CURB INLET IN THE WEST CENTER OF A CURB INLET IN THE SOUTHWESTERLY CURB LINE OF NORTH CURB LINE OF NORTH JOHN KING JOHN KING BOULEVARD AND BEING +/- BOULEVARD AND BEING +/- 662' NORTH 235' SOUTHEST OF THE NORTHWEST OF THE INTERSECTION NORTH JOHN KING CORNER OF THE SUBJECT TRACT AND +/-BOULEVARD AND WEST QUAIL RUN ROAD 535' NORTHWEST OF THE INTERSECTION OF AND +/- 723' SOUTHEAST OF THE N. JOHN KING BOULEVARD AND EAST INTERSECTION OF N. JOHN KING BOULEVARD AND EAST QUAIL RUN ROAD. QUAIL RUN ROAD ELEV: 546.16' ELEV: 530.38'

LEGEND

 	EXISTING MAJOR CONTOUR
- — 601 — —	EXISTING MINOR CONTOUR
600	PROPOSED MAJOR CONTOUR
601 —	PROPOSED MINOR CONTOUR
	EXISTING STORM DRAIN
	EXISTING CURB INLET
	PROPOSED STORM DRAIN
	PROPOSED CURB INLET

NOTES

1. CONTRACTOR SHALL PROTECT ALL EXISTING TREES, FENCES, RETAINING WALLS AND STRUCTURES UNLESS OTHERWISE NOTED.

PROPOSED AREA DRAIN

BERM CONSTRUCTION NOTES:

1. DETENTION POND EMBANKMENT MATERIAL SHOULD BE PLACED IN LOSE LIFTS NOT EXCEEDING 8 INCHES, MOISTURE CONDITIONED BETWEEN OPTIMUM AND 4 PERCENTAGE POINTS ABOVE OPTIMUM, AND MECHANICALLY COMPACTED TO A MINIMUM OF 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D698.

2. THE SOIL USED A STRUCTURAL FILL FOR THE DETENTION POND SHALL HAVE A PLASTICITY INDEX RANGING BETWEEN 25 TO 50, WITH A MINIMUM OF 60 PERCENT PASSING THE NO. 200 SIEVE, AND CLASSIFIED AS A FAT CLAY (CH).

VEGETATION AND ALL LOOSE OR ORGANIC MATERIAL SHALL BE STRIPPED AND REMOVED FROM THE SITE. SUBSEQUENT TO STRIPPING OPERATIONS, THE SUBGRADE SHALL BE PROOFROLLED TO IDENTIFY SOFT ZONES. ANY SOFT ZONE DETECTED SHALL BE REMOVED TO A FIRM SUBGRADE SOILS AND REPLACED WITH COMPACTED SATISFACTORY SOILS TO REACH SUBGRADE LEVEL. UPON THE ACCEPTANCE OF PROOFROLLING OPERATIONS THE SUBGRADE SHALL BE SCARIFIED TO A MINIMUM DEPTH OF 8 INCHES, MOISTURE CONDITIONED AND COMPACTED TO A MINIMUM OF 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D 698, THE STANDARD PROCTOR, BETWEEN OPTIMUM AND 4 PERCENTAGE POINTS ABOVE OF THE OPTIMUM MOISTURE CONTENT. THE EXPOSED SUBGRADE SHALL NOT BE ALLOWED TO DRY OUT PRIOR TO PLACING STRUCTURAL

4. CONTRACTOR SHALL COORDINATE WITH GEOTECHNICAL ENGINEER FOR COMPACTION TESTING AND INSPECTION OF BERM CONSTRUCTION.

5. DETENTION SYSTEM MUST BE FUNCTIONAL WITH EROSION PROTECTION ON SIDES AND BOTTOM PRIOR TO ANY PAVING OPERATIONS (INCLUDING

6. FOR DETENTION POND, REFER TO PAPE-DAWSON DRAINAGE STUDY DATED MARCH 2021.

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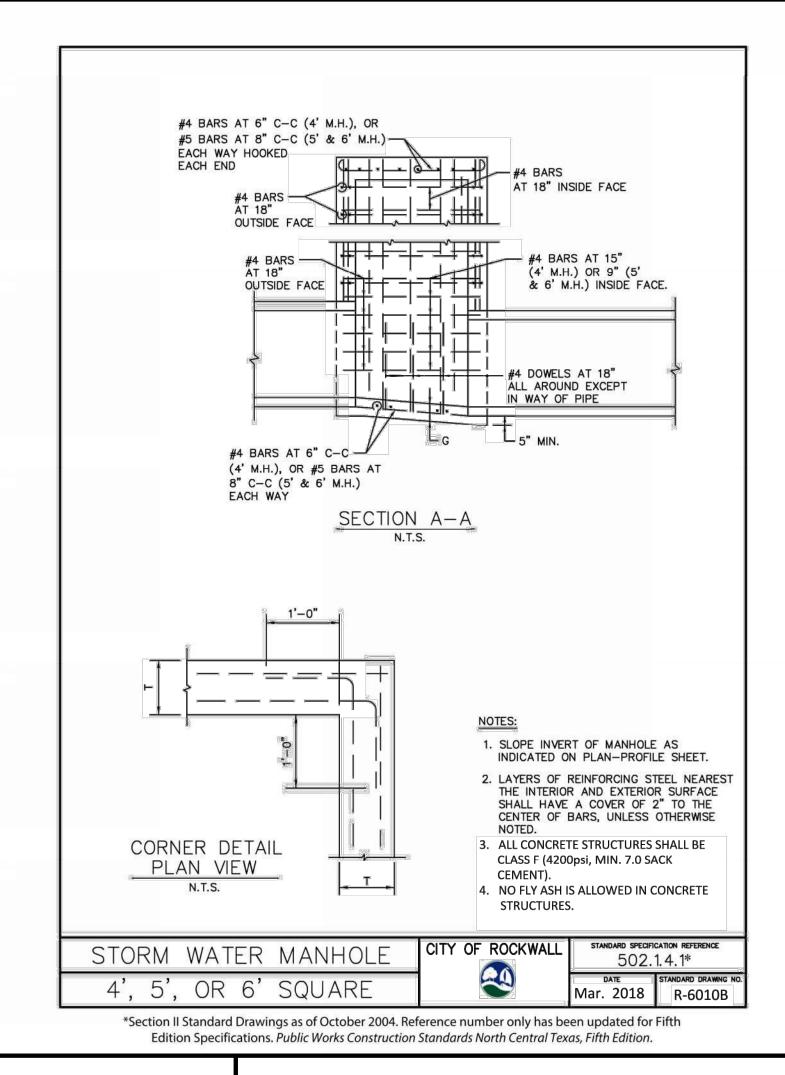
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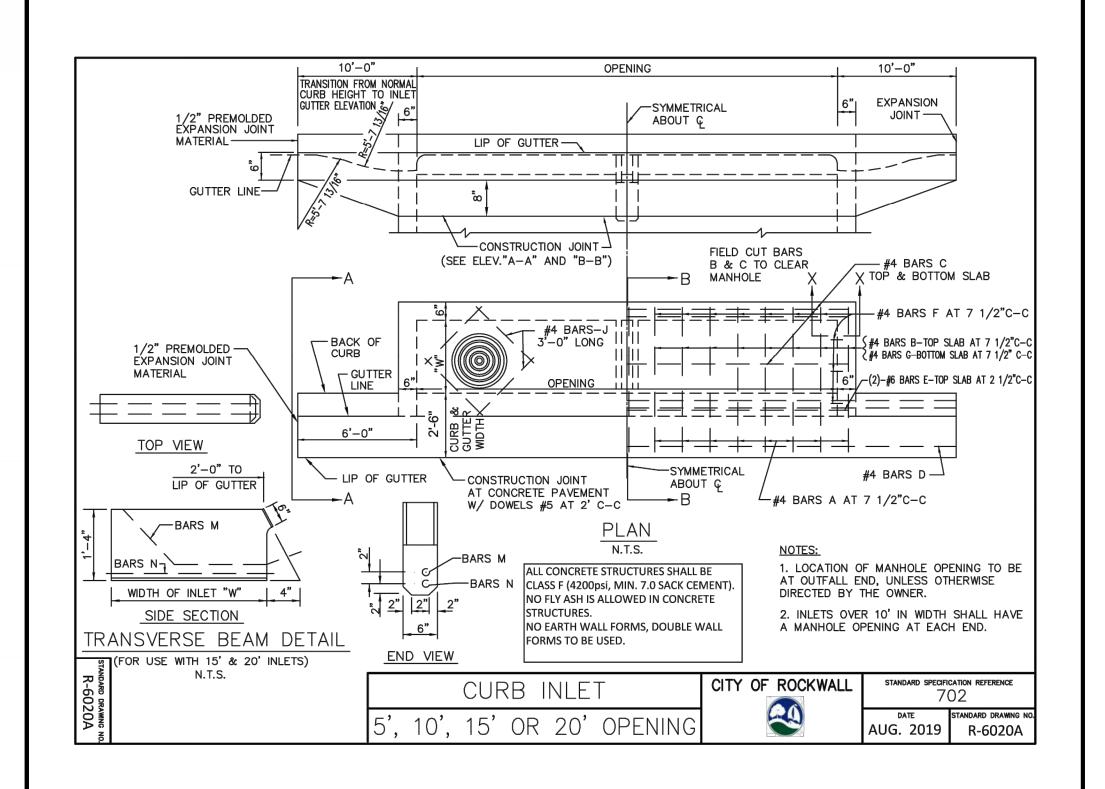
PAPE-DAWS ENGINEERS

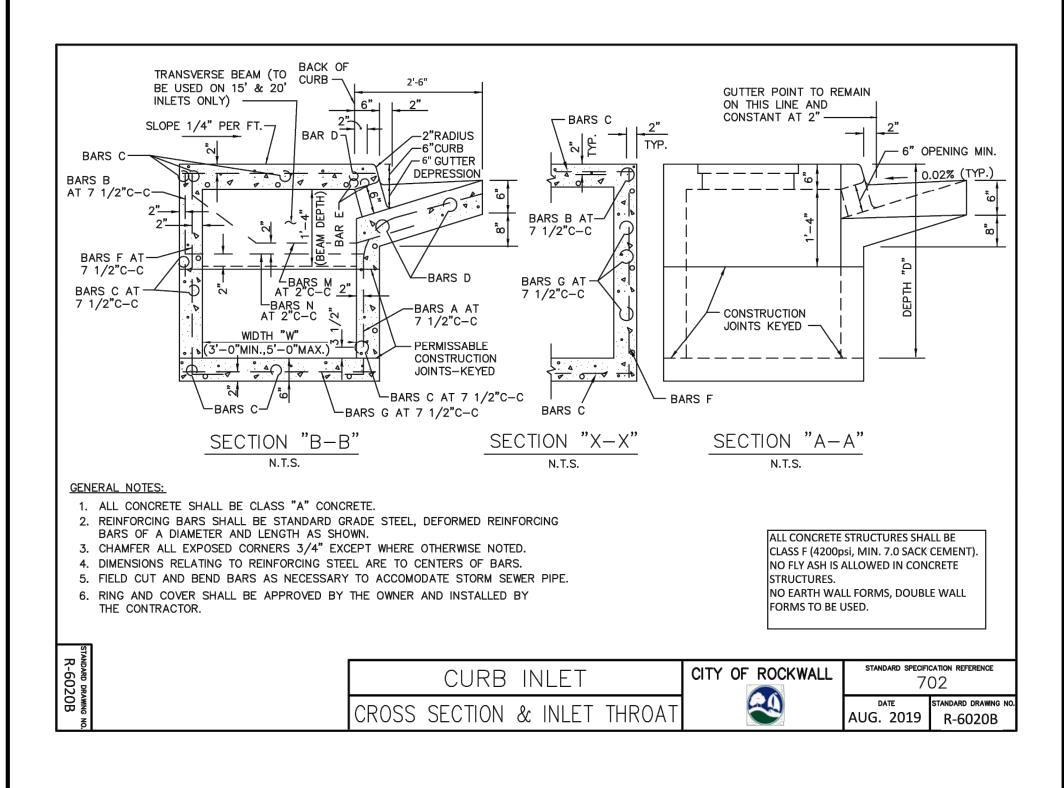
6126300 June 21 ESIGNER OEK HECKED<u>A</u>SR DRAWN OEK 18

*Section II Standard Drawings as of October 2004. Reference number only has been updated for Fifth

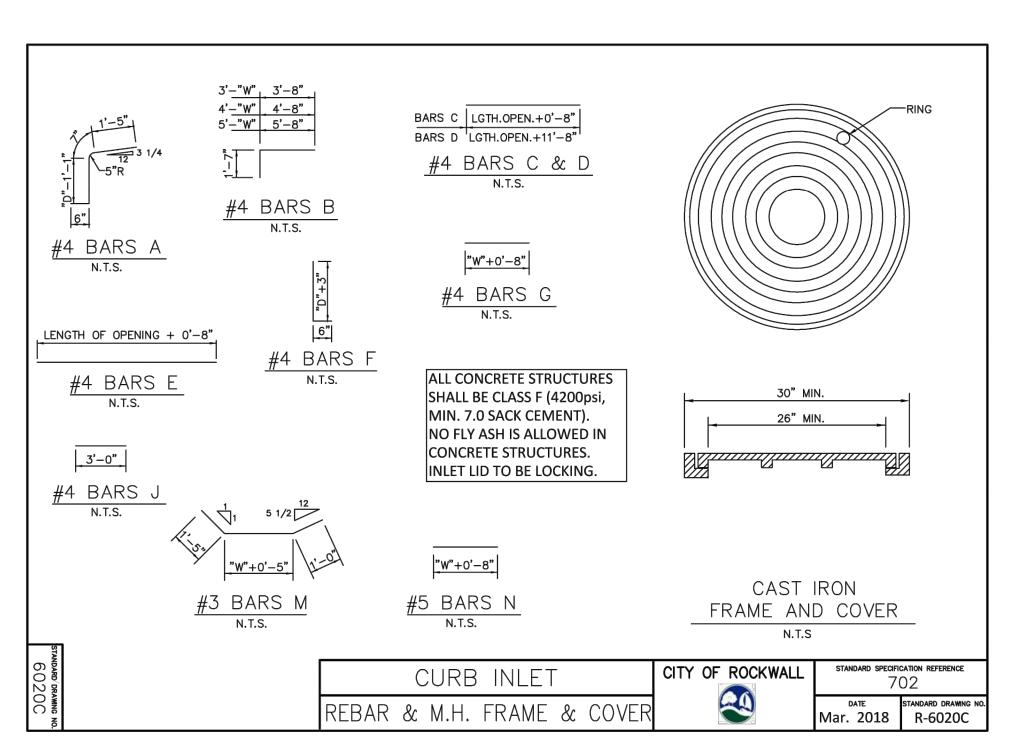
Edition Specifications. Public Works Construction Standards North Central Texas, Fifth Edition.

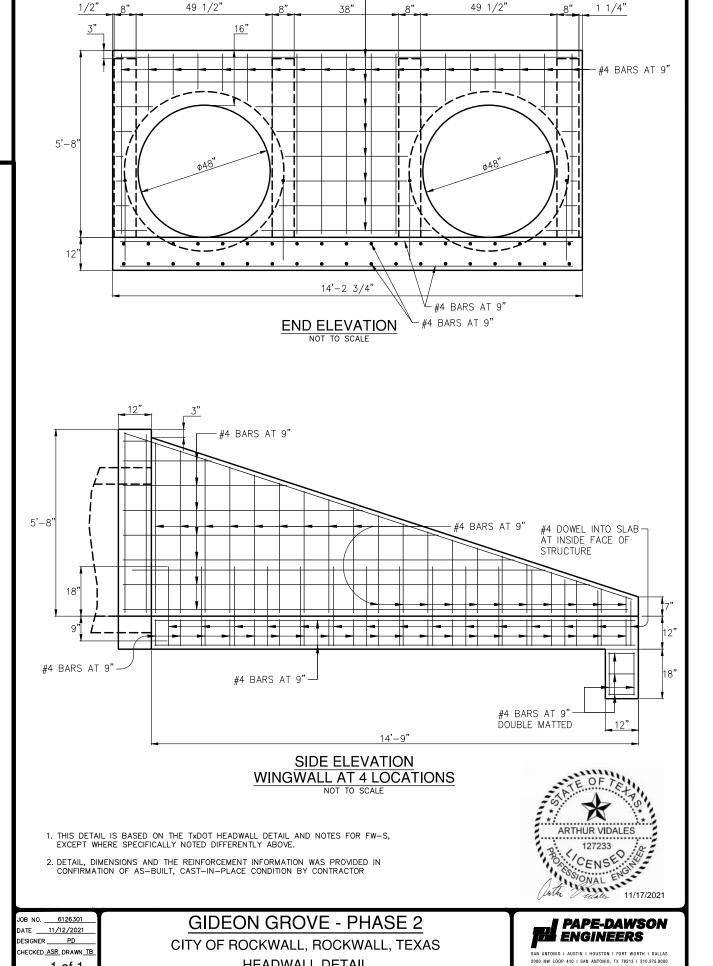






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HEADWALL DETAIL

-#4 BARS AT 9"

RECORD DRAWING THESE RECORD DRAWINGS ARE BASED ON AS-BUILT DOCUMENTS PROVIDED BY THE CONTRACTOR OR DEVELOPER. FIELD INSPECTION OF CONSTRUCTION, IF EQUIRED FOR COMPLIANCE WITH CERTAIN REGULATOR

6126300 June 21 CoR DESIGNER CHECKED COR DRAWN COR

LAT NO.

ADAM S. REEVES

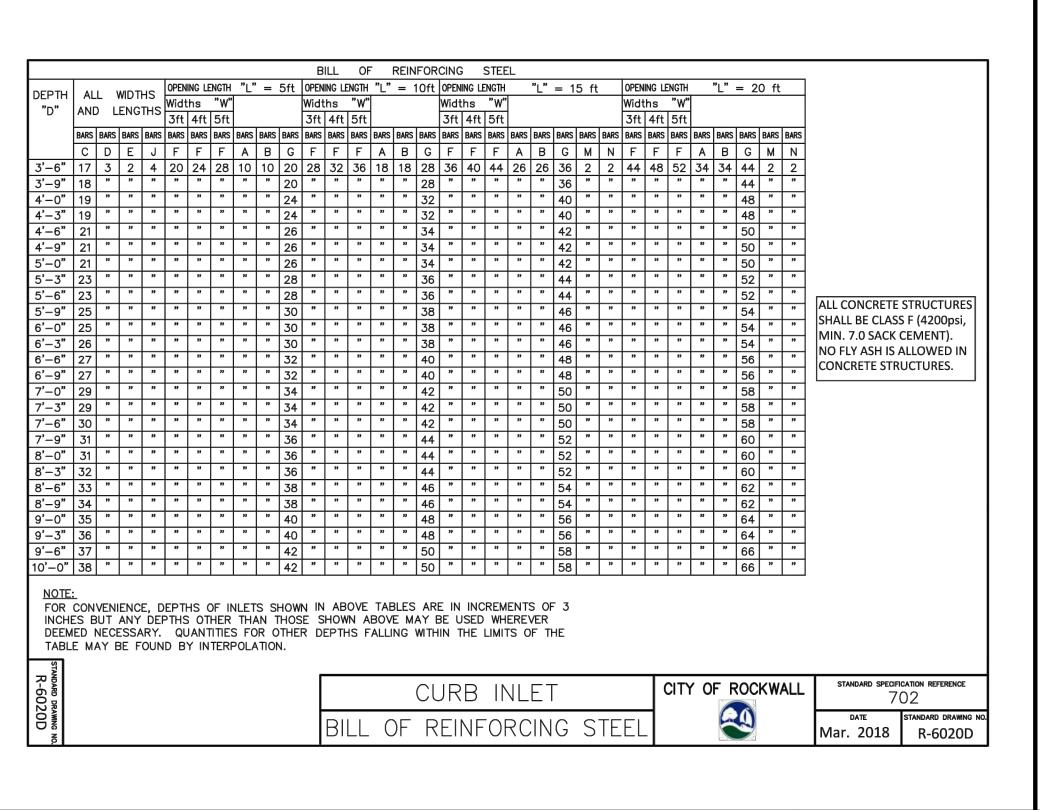
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PAPE-DAWS ENGINEERS

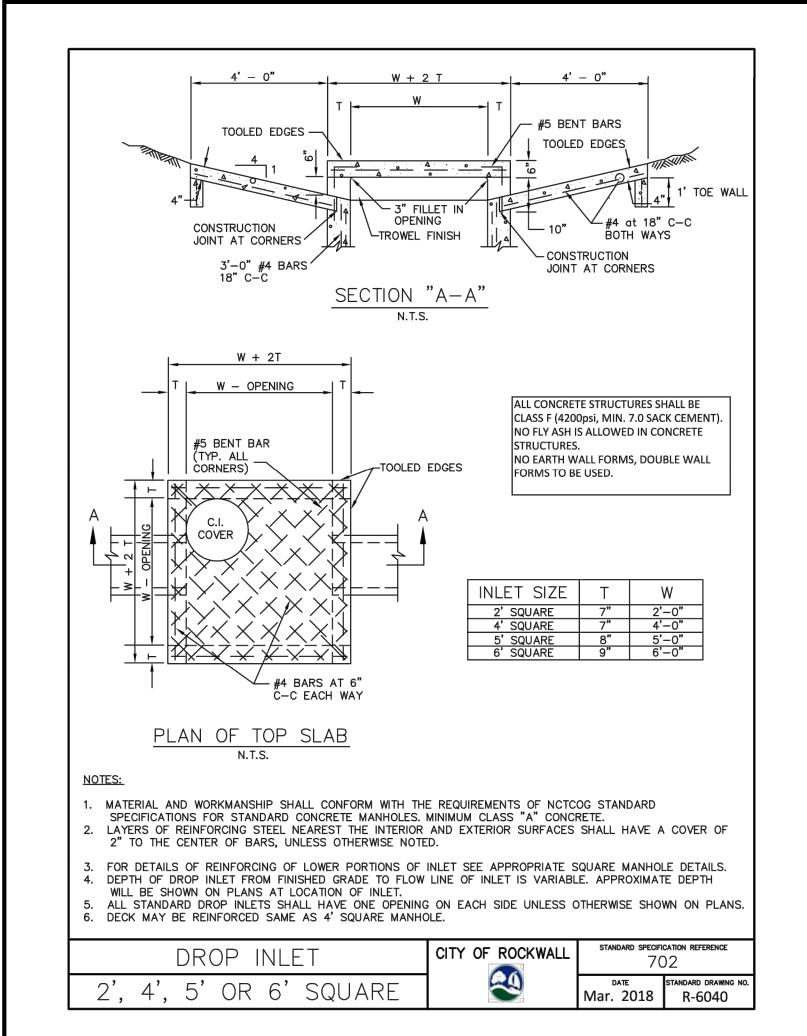
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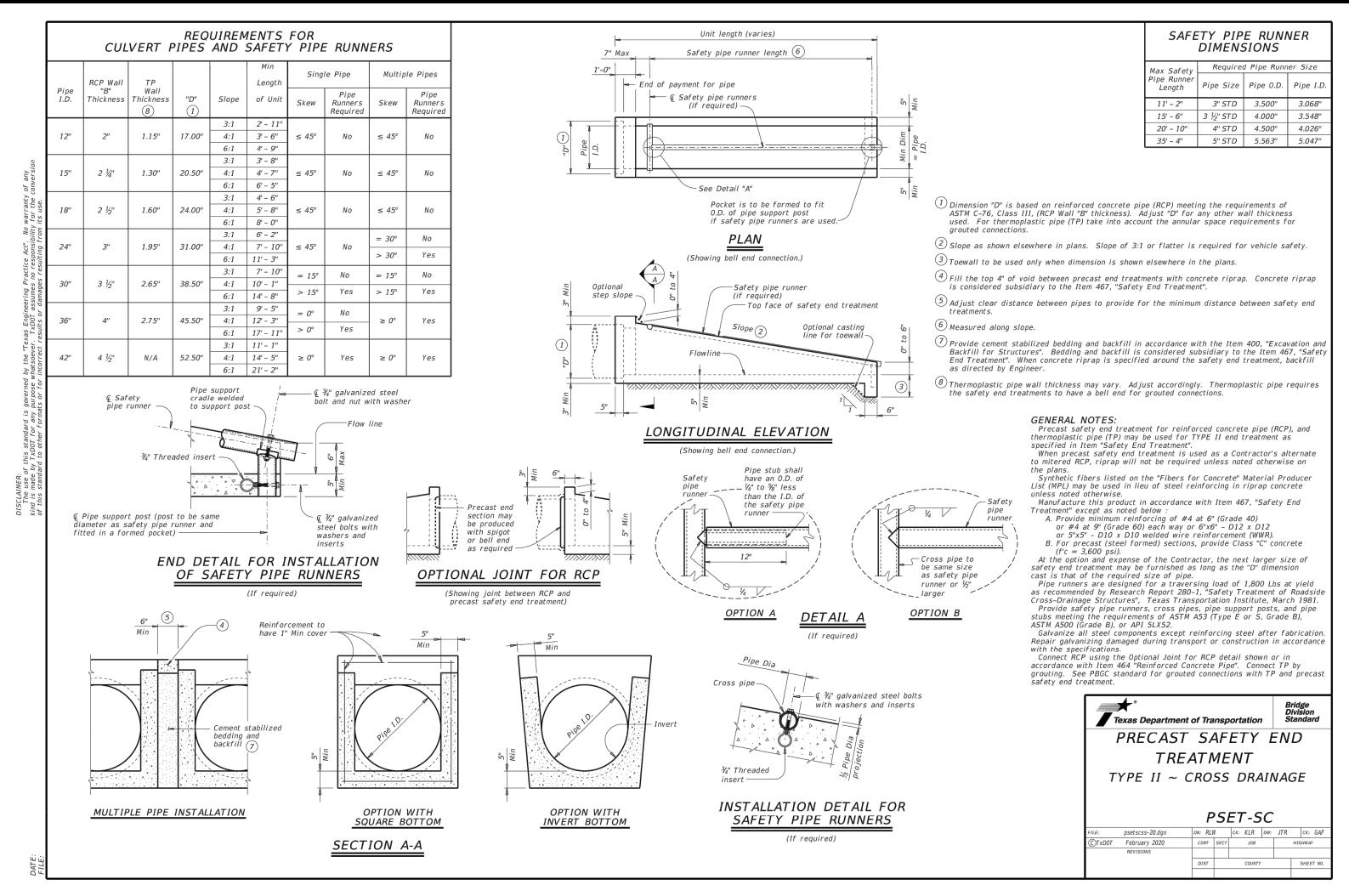
03/22/2022



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DEPTH			i'-0" (OPENIN	T				5'-0" (0'-0" 			
"D"					WIDTH																			
		l			CONC					ı	1	I				l					l .		CONC	
7' 0"	C.Y.	LBS.	C.Y.	LBS.	C.Y.	LBS.	C.Y.	LBS.	C.Y.	LBS.	C.Y.	LBS.	C.Y.	LBS.	C.Y.	LBS.	C.Y.	LBS.	C.Y.	LBS.	C.Y.	LBS.	C.Y.	LBS.
3'-6" 3'-9"	2.62	306 309	2.95 3.04	332 341	3.28 3.39	373 373	4.12 4.25	479 494	4.64 4.78	521 536	5.20	564 579	5.69 5.87	667 687	6.40 6.58	721 741	7.10 7.30	775 796	7.20 7.42	846 874	8.11 8.34	909	9.03	976 1010
3 – 9 4'–0"	2.78	328	3.14	364	3.49	399	4.38	518	4.76	565	5.49	610	6.05	718	6.77	776	7.49	835	7.64	909	8.58	976	9.27	1046
4'-3"	2.87	334	3.23	370	3.59	406	4.51	526	5.06	573	5.64	619	6.22	729	6.95	787	7.69	847	7.87	922	8.81	990	9.75	1061
''-6 "	2.95	356	3.32	394	3.69	431	4.64	558	5.20	607	5.79	656	6.40	770	7.14	830	7.88	891	8.09	973	_	1043	9.99	1115
4'-9"	3.03	361	3.41	410	3.79	438	4.77	566	5.34	616	5.94	665	6.57	780	7.32	841	8.07	903	8.31	986	9.27	1056	10.23	1129
5'-0"	3.12	367	3.51	416	3.90	445	4.90	574	5.47	624	6.09	674	6.75	791	7.51	853	8.27	915	8.53	999	9.50	1070	10.47	1144
5'-3"	3.20	383	3.60	424	4.00	465	5.03	600	5.61	652	6.23	704	6.93	827	7.69	890	8.46	955	8.76	1044	9.73	1118	10.71	1194
5'-6"	3.28	389	3.69	430	4.10	472	5.16	608	5.75	661	6.38	713	7.11	837	7.88	901	8.66	967	8.98	1057		1131	10.95	1208
5'-9"	3.37	405	3.78	451	4.20	495	5.29	635	5.89	690	6.53	744	7.28	874	8.07	940	8.85	1007	9.20			1178	11.19	1258
6'-0"	3.45	415	3.88	460	4.30	504	5.42	646	6.03	702	6.68	757	7.45	888	8.25	954			9.42		10.43		11.43	1276
6'-3"	3.53	425	3.97	470	4.41	515	5.55	661	6.17	718	6.83	773	7.63	908	8.44	975			9.64				11.67	1305
6'-6" 6'-9"	3.62	437 441	4.06 4.15	486	4.51 4.61	532 537	5.68 5.81	681 688	6.31	739 747	6.97 7.12	797	7.81 7.98	935 945	8.62	1005	9.43	1057 1066	9.87 10.09		10.89 11.12	1258	11.92 12.15	1340 1355
0 -9 7'-0"	3.78	460	4.15	490 510	4.71	560	5.94	716	6.59	777	7.12	806	8.16	945	8.81 8.99	1013	9.82	1126	10.09		11.35	1319	12.13	1404
7'-3"	3.86	465	4.34	516	4.81	567	6.07	724	6.72	785	7.42	846	8.33	992	9.18	1065	10.02		10.53		11.59	1333	12.64	1418
7'-6"	3.95	477	4.43	529	4.91	570	6.20	742	6.86	804	7.57	866	8.51	1016	9.36	1089	10.21		10.75	1290		1365	12.88	1451
7'-9"	4.03	491	4.53	544	5.02	597	6.33	762	7.00	826	7.71	890	8.67	1040	9.55	1116	10.41		10.98				13.12	1498
8'-0"	4.12	496	4.62	550	5.12	604	6.46	770	7.14	834	7.86	899	8.86	1051	9.73	1129	10.60	1205	11.20	1325	12.28	1412	13.36	1510
8'-3"	4.20	504	4.71	559	5.22	613	6.59	784	7.28	849	8.01	915	9.04	1069	9.92	1149	10.80	1228	11.42	1353	12.51	1440	13.60	1529
8'-6"	4.28	519	4.80	576	5.32	632	6.71	804	7.42	871	8.16	938	9.21	1107	10.10	1176	10.99	1257	11.64	1385	12.74	1474	13.84	1565
8'-9"	4.37	528	4.90	586	5.42	643	6.84	819	7.56	886	8.31	954	9.39				11.18		11.87	1410		1500	14.08	1592
9'-0"	4.45	545	4.99	605	5.53	664	6.97	842	7.70	912	8.46	982	9.56		10.47					1447		1539	14.32	1631
9'-3" 9'-6"	4.53	554 568	5.08 5.17	614	5.63 5.73	674	7.10	858 878	7.84	929 950	8.60	1022	9.74		10.66 10.84		11.57	1365	12.31	1505		1563 1600	14.56	1660
9 -0"			5.36	630	5.73	692 708	7.49	900	7.97 8.11	974			10.27										15.29	1696
INCHE:	S BUT	IIENCE, ANY E ESSAR	DEPTH EPTHS Y. QU	IS OF OTHE	INLETS R THAN ES FOR RPOLAT	N THOS	SE SHO	WN A	BOVE N	ES ARE MAY BE WITHI	E IN IN	CREMEI WHER LIMITS	NTS OF EVER OF TH	- 3 HE		ALL (CONCR Opsi, W	ETE STI IN. 7.0 IS ALL	RUCTU SACK (RES SH CEMEN IN COI	ALL BE IT). NCRETE	CLASS STRU	F CTURES	ON REFERENCE
WING NO.							(SUN	ИΜ	AR`	Y C)F (QU/	$\sqrt{N^2}$		ES		6			Ma	DATE r. 201		NDARD DRAWING R-6020E



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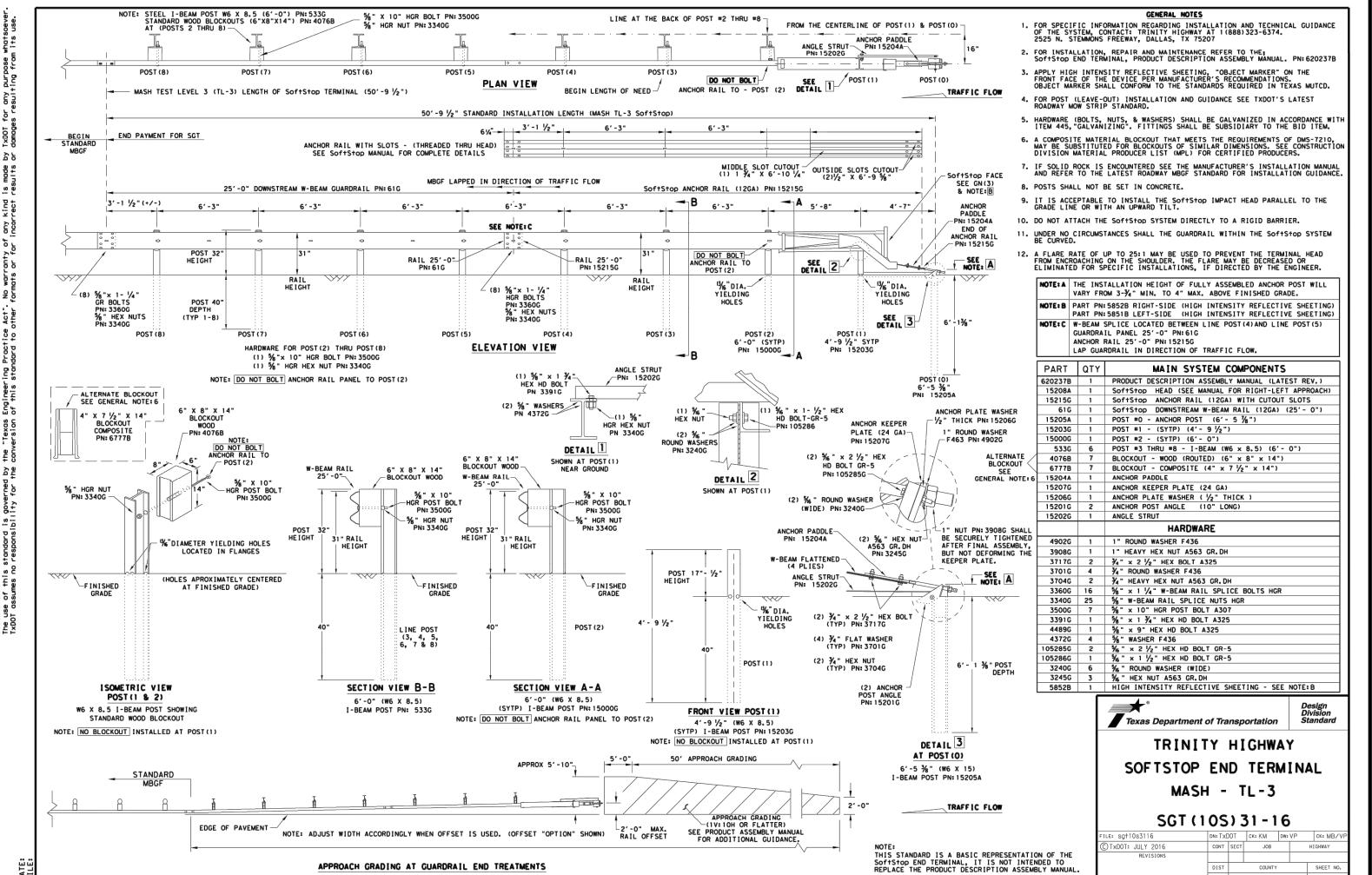
ON GROVE - PHASE KWALL, ROCKWALL COUNTY 03/22/2022

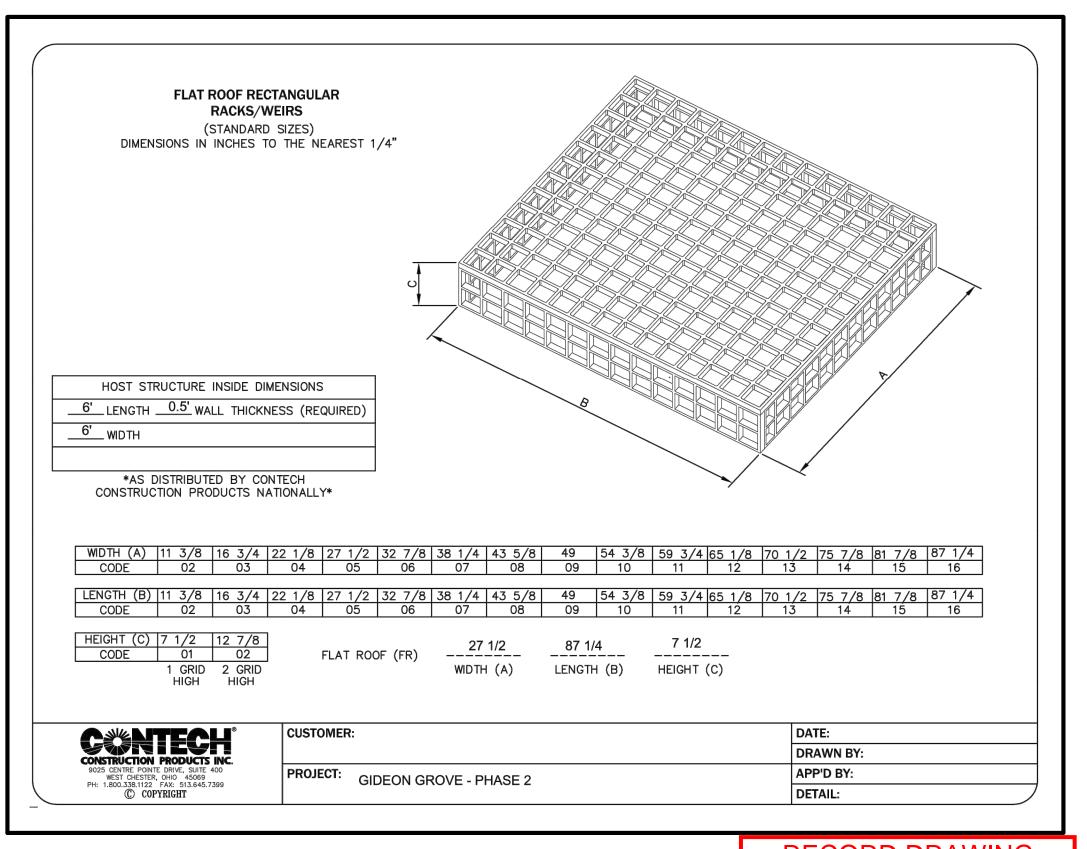
PLAT NO.		#	
JOB NO		6126300	
DATE		June 21	
DESIGNER		CoR	
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ADAM S. REEVES

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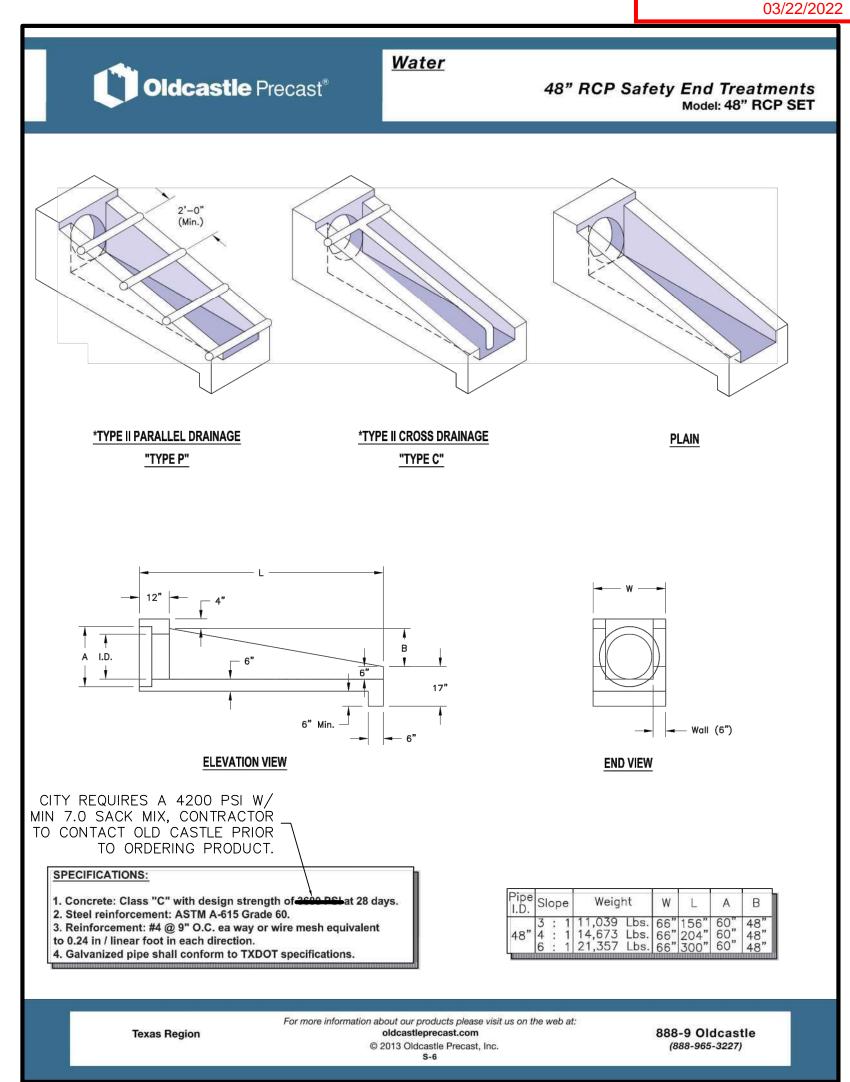
PAPE-DAWSON ENGINEERS





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ADAM S. REEVES

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GIDEON GROVE - PHASE 2
CITY OF ROCKWALL, ROCKWALL COUNTY, TEXA
STORM DRAINAGE DETAILS

PLAT NO. #

JOB NO. 6126300

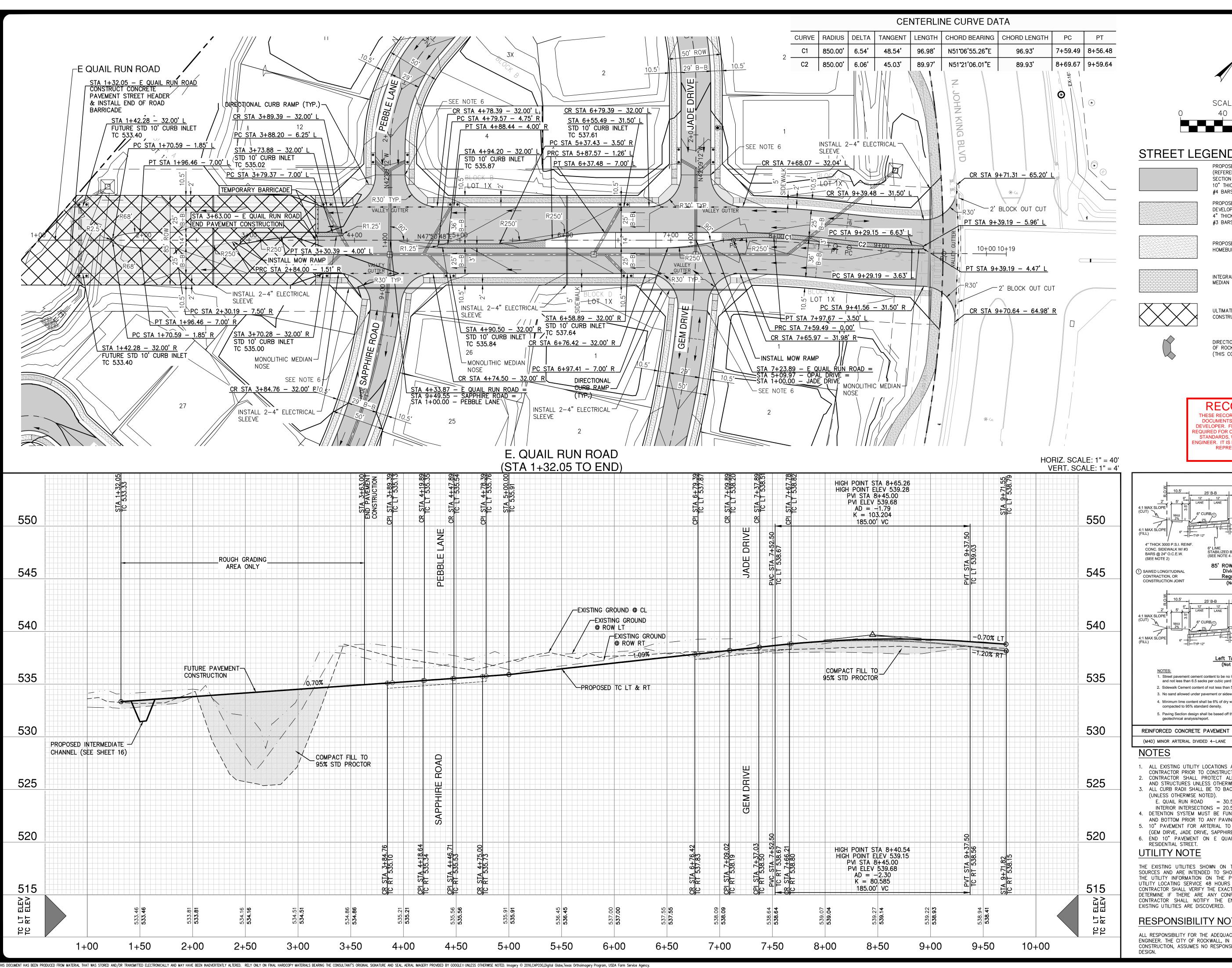
DATE June 21

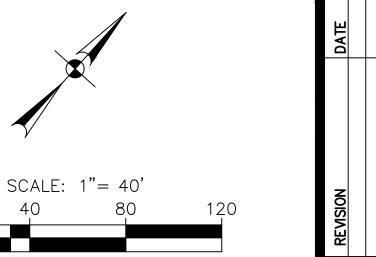
DESIGNER ---
CHECKED ---- DRAWN ----

APPROACH GRADING AT GUARDRAIL END TREATMENTS

APPROACH GRADING AT GUARDRAIL END TREATMENTS

SOTTSTOP END TERMINAL, 11 15 NOT INTENDED TO REPLACE THE PRODUCT DESCRIPTION ASSEMBLY MANUAL.





PROPOSED PAVEMENT (REFERENCE TYPICAL STREET SECTION FOR SPECIFICATION) 10" THICK 3600 PSI (6.5 SACK MIX) #4 BARS @ 18" O.C.E.W PROPOSED CONCRETE SIDEWALK BY

DEVELOPER (THIS CONTRACT) 4" THICK 3000 PSI (5.5 SACK MIX) #3 BARS @ 24" O.C.E.W.

PROPOSED CONCRETE SIDEWALK BY HOMEBUILDER (NOT IN THIS CONTRACT)

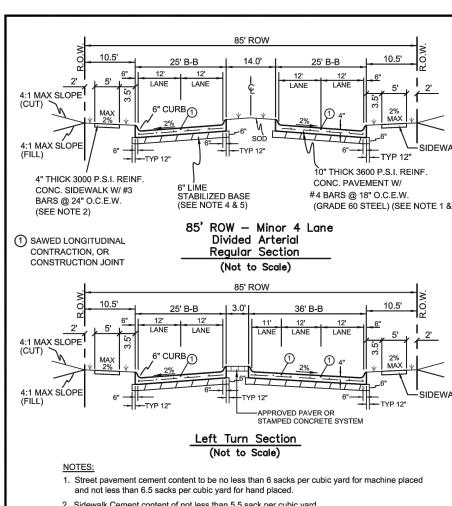


DIRECTIONAL CURB RAMP PER CITY OF ROCKWALL DETAIL R-2125A (THIS CONTRACT)

INTEGRAL STAMPED AND STAINED MEDIAN PAVING PER DETAIL R-2130

CONSTRUCTED WITH THIS CONTRACT)

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2. Sidewalk Cement content of not less than 5.5 sack per cubic yard. No sand allowed under pavement or sidewalks

4. Minimum lime content shall be 6% of dry weight of material (at least 27 lbs/SY) compacted to 95% standard density. 5. Paving Section design shall be based off the 30 year projected traffic volumes and geotechnical analysis/report

(M4D) MINOR ARTERIAL DIVIDED 4-LANE

1. ALL EXISTING UTILITY LOCATIONS ARE APPROXIMATE AND WILL BE CONFIRMED CONTRACTOR PRIOR TO CONSTRUCTION. CONTRACTOR SHALL PROTECT ALL EXISTING TREES, FENCES, RETAINING WAL

CITY OF ROCKWALL

DATE DRAWING NO.
AUG. '15 R-2020

- AND STRUCTURES UNLESS OTHERWISE NOTED.
- ALL CURB RADII SHALL BE TO BACK OF CURB AS FOLLOWS:
- (UNLESS OTHERWISE NOTED). E. QUAIL RUN ROAD = 30.5
- INTERIOR INTERSECTIONS = 20.5' 4. DETENTION SYSTEM MUST BE FUNCTIONAL, WITH EROSION PROTECTION ON SIDE AND BOTTOM PRIOR TO ANY PAVING OPERATIONS (INCLUDING SLAB).
- 5. 10" PAVEMENT FOR ARTERIAL TO GO TO CURB RETURN OF EACH SIDE STREET (GEM DIRVE, JADE DRIVE, SAPPHIRE ROAD AND PEBBLE LANE.
- 6. END 10" PAVEMENT ON E QUAIL RUN ROAD AND BEGIN 6" PAVEMENT OF RESIDENTIAL STREET.

UTILITY NOTE

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RESPONSIBILITY NOTE

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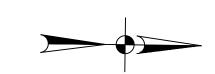
6126300 June 21 ESIGNER HECKED ASR DRAWN TB 22

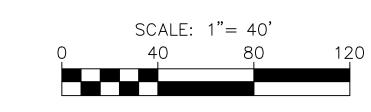
ADAM S. REEVES 98410

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E QUAIL RUN ROAD PLAN AND PROFILE

CURVE RAI	RADIUS	DELTA	TANGENT	LENGTH	CHORD BEARING	CHORD LENGTH	PC	PT
C1 50	50.00'	90.00'	50.00'	78.54'	N44°14'29.86"E	70.71'	4+80.00	5+58.





PROPOSED PAVEMENT
(REFERENCE TYPICAL STREET
SECTION FOR SPECIFICATION) 6" THICK 3600 PSI (6.5 SACK MIX) #3 BARS @ 24" O.C.E.W

> PROPOSED CONCRETE SIDEWALK BY DEVELOPER (THIS CONTRACT) 4" THICK 3000 PSI (5.5 SACK MIX) #3 BARS @ 24" O.C.E.W.

PROPOSED CONCRETE SIDEWALK BY HOMEBUILDER (NOT IN THIS CONTRACT)

DIRECTIONAL CURB RAMP PER CITY

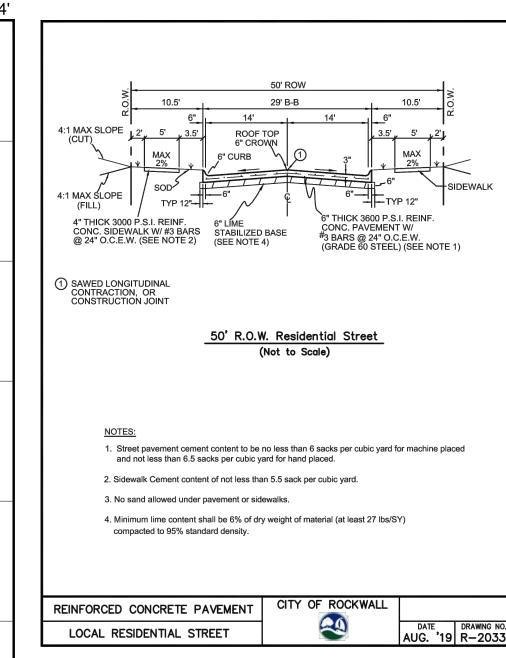
OF ROCKWALL DETAIL R-2125A (THIS CONTRACT)

6" CROWN PER CITY STDS

SECTION A-A NOT TO SCALE

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NOTES

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RUBY WAY PLAN AND PROFILE

June 21

HECKED<u>ASR</u> DRAWN<u>TB</u>

23

DESIGNER

ADAM S. REEVES

98410

PAPE-DAWSON ENGINEERS

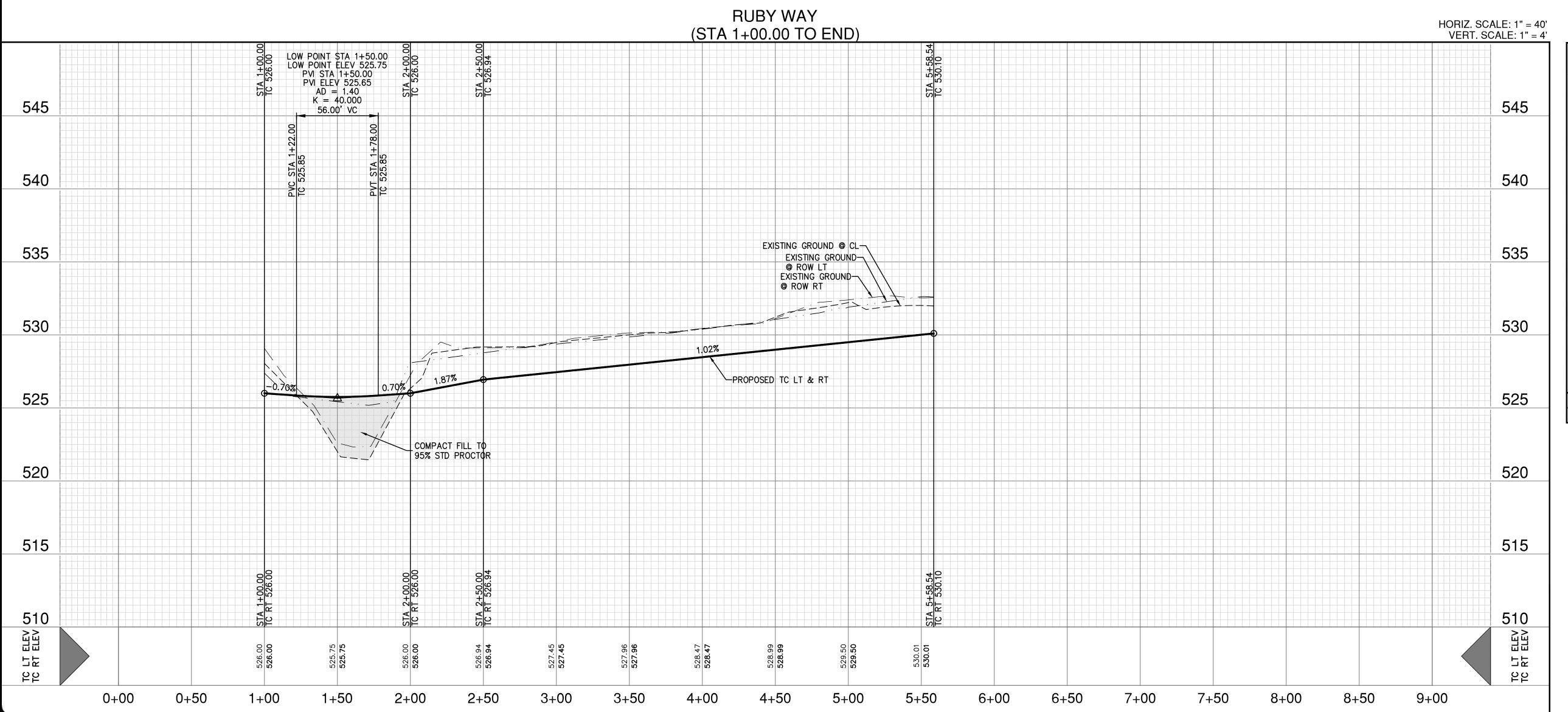
	R20' 1+00	2+00	RUBY WAY N0°45'30"W	3+00	50' ROW	4+00		50'	+58.54 - RUBY WAY = -00.00 - JASPER STREET	
24		STA 1+45.50 - 14 STD 10' CURB INLI TC 525.75 STA 1+00.00 - RUBY WAY = STA 1+00.00 - OPAL DRIVE CR STA 1+00.00 - 14.50' R 8	4.50' R ET 2'	10	110.5	CR STA 4+80.00 - 14 CR STA 5+	4.50' R P 58.54 - 14.50' R	TC=	26 +09.24 (14.50' L) 530.23	
.3	Language Part Control of the Control						5' SIDEWALK	JASPER S	27	

CR STA 1+13.69 - 14.50' L STA 1+45.50 - 14.50' L

TC 525.75

STD 10' CURB INLET_

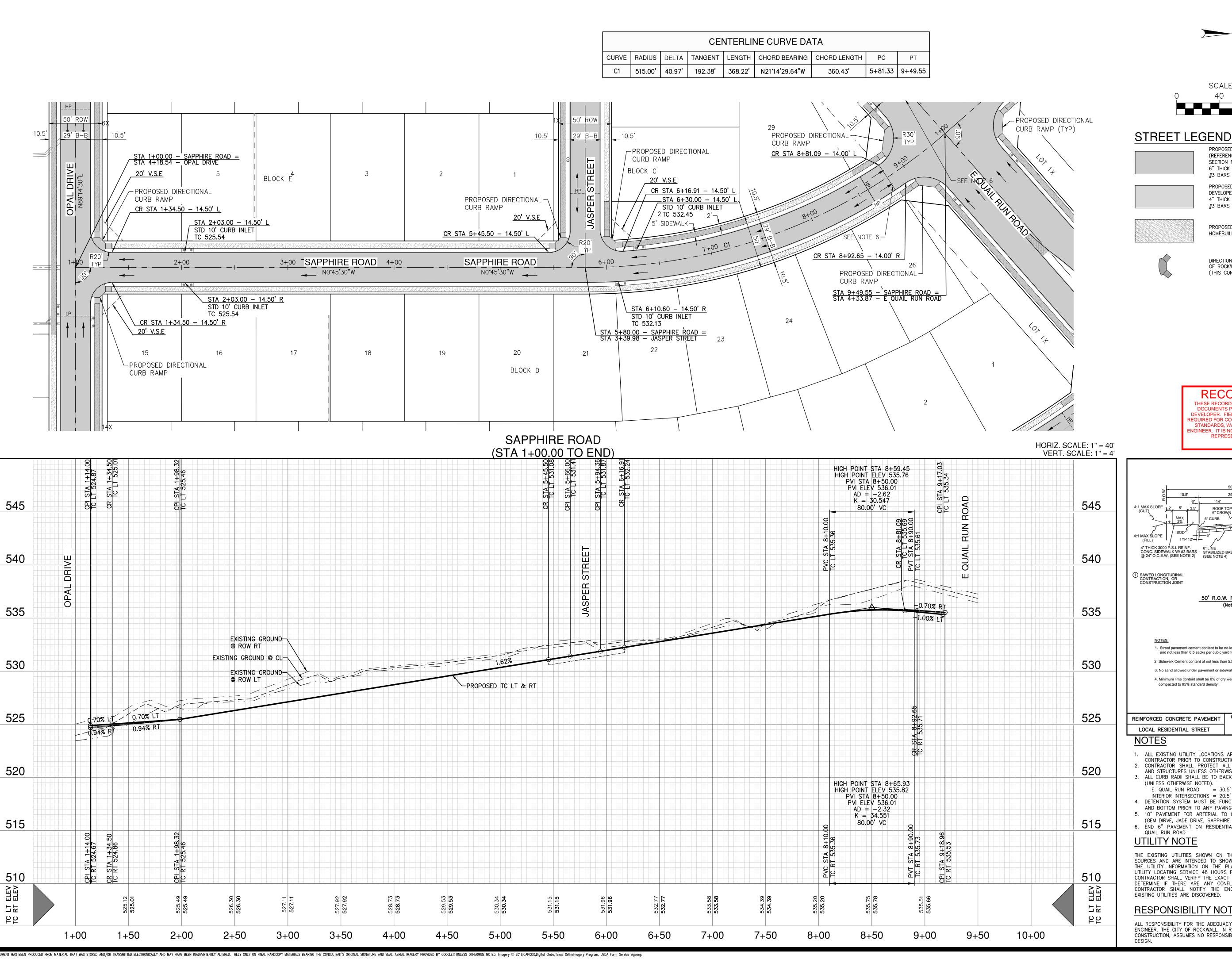
<u>CR STA 1+04.00 - 17.00' </u>



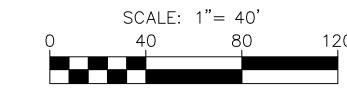
STA 4+79.61 - 16.56' L

TC 529.34

CR STA 4+70.76 - 14.50'







PROPOSED PAVEMENT (REFERENCE TYPICAL STREET SECTION FOR SPECIFICATION) 6" THICK 3600 PSI (6.5 SACK MIX) #3 BARS @ 24" O.C.E.W

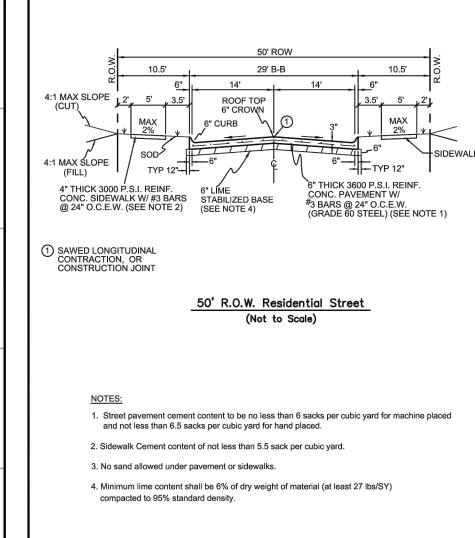
PROPOSED CONCRETE SIDEWALK BY DEVELOPER (THIS CONTRACT) 4" THICK 3000 PSI (5.5 SACK MIX) #3 BARS @ 24" O.C.E.W.

PROPOSED CONCRETE SIDEWALK BY HOMEBUILDER (NOT IN THIS CONTRACT)

> DIRECTIONAL CURB RAMP PER CITY OF ROCKWALL DETAIL R-2125A (THIS CONTRACT)

RECORD DRAWING

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CITY OF ROCKWALL

AUG. '19 R-2033

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- (GEM DIRVE, JADE DRIVE, SAPPHIRE ROAD AND PEBBLE LANE. 6. END 6" PAVEMENT ON RESIDENTIAL STREET AND BEGIN 10" PAVEMENT ON

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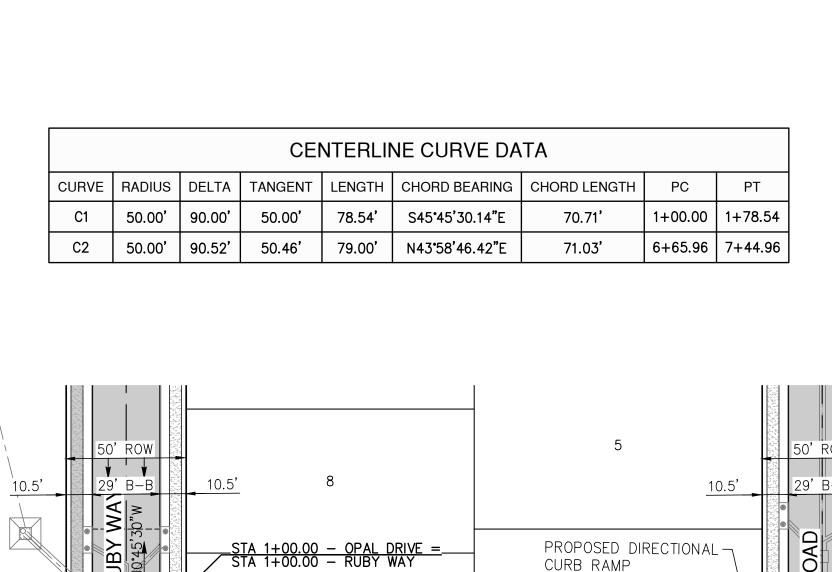
6126300 June 21 ESIGNER HECKED<u>ASR</u> DRAWN<u>TB</u>

24

ADAM S. REEVES 98410

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SAPPHIRE ROAD PLAN & PROFILE



CR STA 1+00.00 - 14.50' L

STA 1+55.81 - 26.92' R

STA 1+13.01 - 25.81' R TC 526.14

<u>CR STA 1+</u>78.54 – 14.50' L

TC 525.42

STA 2+02.56 - 14.50' L STD 10' CURB INLET

CR STA 1+83.01 - 14.50' R STD 10' CURB INLET CR STA 1+75.93 - 16.23' R TC 525.42

<u>CR STA 3+84.04 - 14.50' l</u>

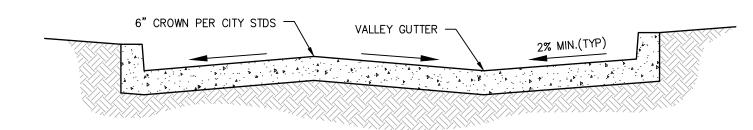
5' SIDEWALK 7 2'7

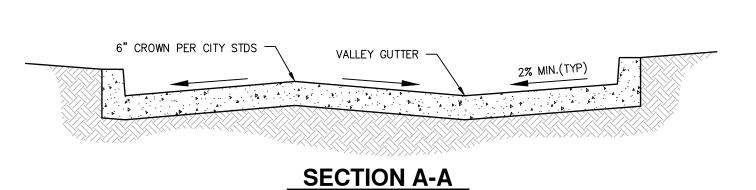
STA 2+02.56 - 14.50' R

OPAL DRIVE

BLOCK C

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NOT TO SCALE

CR STA 7+44.96 - 14.50' L

CR STA 6+65.96 - 14.50' L

5' SIDEWALK 7 2'-

6+00

<u>CR STA 6+56.79 - 14.50' R</u>

STA 6+65.71 - 16.60' R TC 525.91

-PROPOSED DIRECTIONAL

____TC 524.55

TC 524.55

5+,00

STA 4+58.95 - 14.50' R STD 10' CURB INLET

CR STA 4+53.04 - 14.50' L

STA 4+69.95 - 14.50' L $\frac{1}{5}$ STD 10' CURB INLET

CURB RAMP

PPHIRE 1

└∫5' SIDEWALK

LANDSCAPE BUFFER:

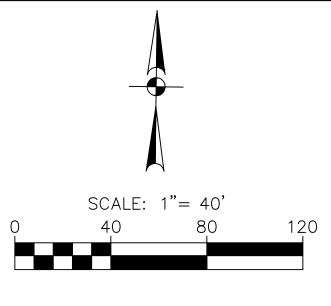
10' TRAIL ALONG JOHN KING

BOULEVARD TO BE CONSTRUCTED WITH THIS PROJECT SEE LANDSCAPE PLANS

PEDESTRIAN ACCESS EASEMENT

STA 1+00.25 - 16.60' R TC 526.46

STA 6+94.73 - 29.89' R



STREET LEGEND

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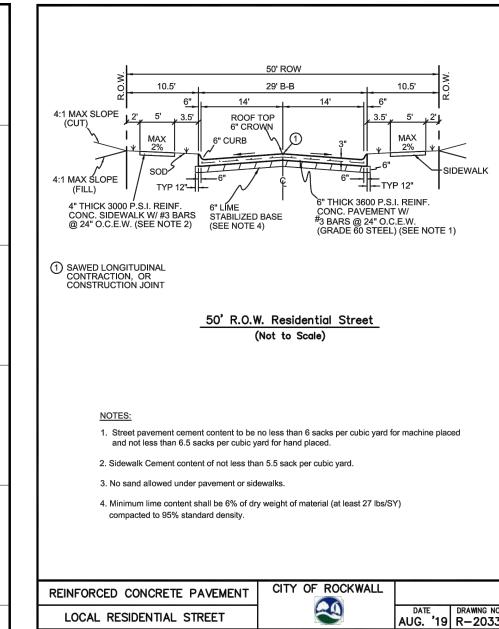
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> DIRECTIONAL CURB RAMP PER CITY OF ROCKWALL DETAIL R-2125A

(THIS CONTRACT)

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ADAM S. REEVES 98410 SO

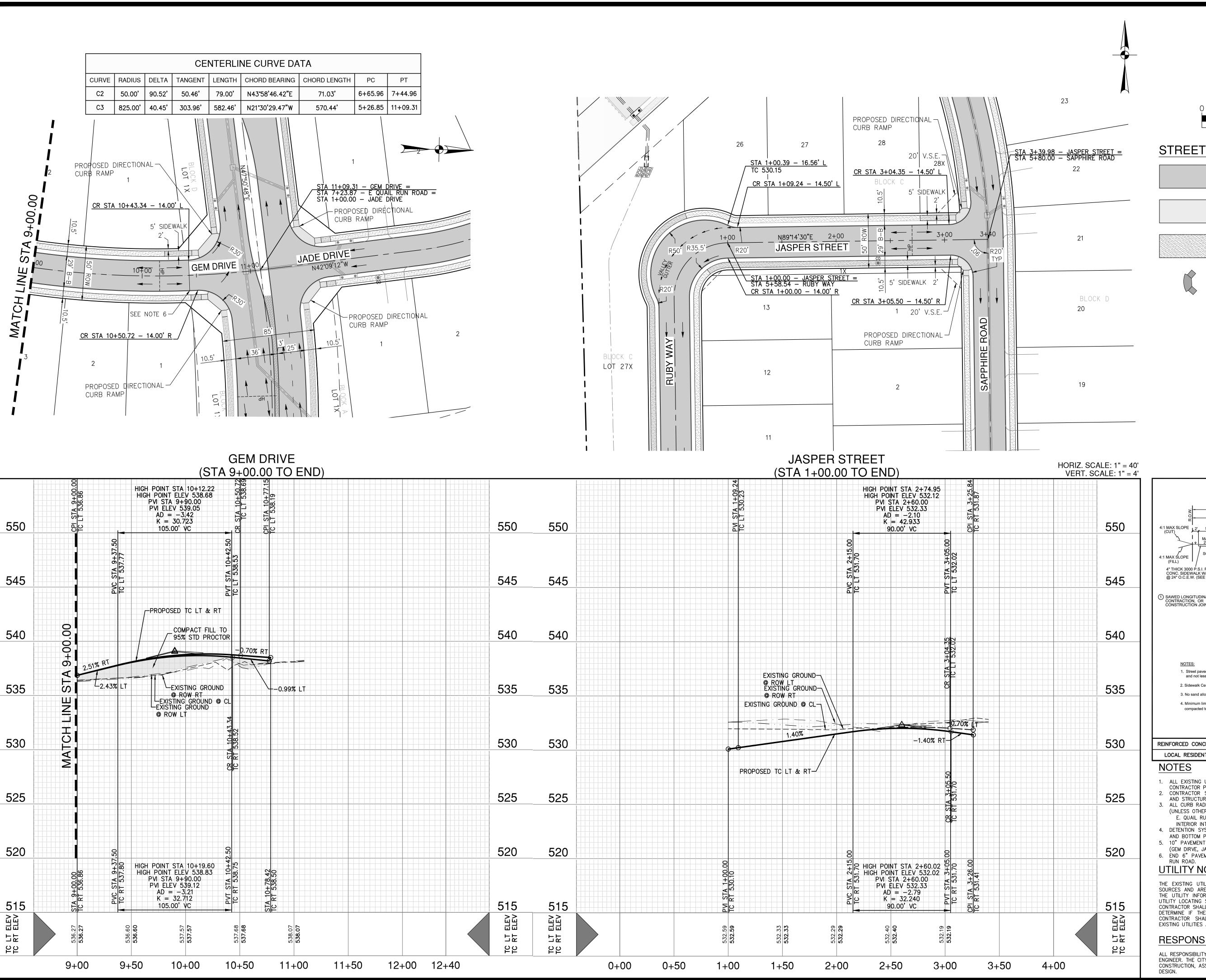
PAPE-DAWS

OPAL DRIVE PLAN AND PROFIL

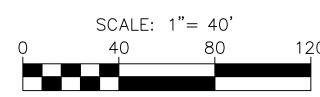
6126300 June 21 DESIGNER HECKED<u>ASR</u> DRAWN<u>TB</u>

25

		TC 525.87 STA 1+32.33 - 30.71' R TC 526.11			OT 23X	JOSIDEWALK	STA 4+18.54 - OPAL DRIVE STA 1+00.00 - SAPPHIRE F	Z = ROAD			TC 526	.38					DOCUMEN DEVELOPER REQUIRED FO STANDARD ENGINEER. IT REF
								DRIVE .00 TO END								HORIZ. SCALE: 1" = 40' VERT. SCALE: 1" = 4'	REF
540	STA 1+00.00 TC LT 526.00	LOW POINT STA 1+98 LOW POINT ELEV 525 PVI STA 1+98.00 PVI ELEV 525.32 AD = 1.40 K = 40.000 56.00' VC	B.00 HIGH POINT STA 2 5.41 HIGH POINT ELEV PVI STA 2+69 PVI ELEV 525 AD = -1.4 K = 30.00 42.00' VC	2+69.43 / 525.74 99.43 5.82 40		CPI STA 4+04.54 CPI STA 4+32.54 CR STA 4+53.04 CR STA 4+53.04 TC LT 524.56	(OTA ITOU				STA 7+44.96 TC LT 526.41					540	4:1 MAX SLOPE 2' 5' 3.5' RO
535		LT STA		PVT STA 2+90.43 TC LT 525.67	3+84.04 LT 525.01	LOW POINT S LOW POINT S LOW POINT S PVI STA PVI ELEV AD = K = 4 56.00	0.000 7 VC									535	4:1 MAX SLOPE SOD TYP 12"-6" CURE SOD TYP 12"-6" STABIL STABIL (SEE NOTE 2) 1 SAWED LONGITUDINAL CONTRACTION, OR CONSTRUCTION JOINT
530		/_EXISTING GROUN			10 TA 3+	HRE R(STA 4+92 LT 524.65									530	_50' R.(
525		-0.70%	0.70%		-0.70%	COMPACT FILL TO 95% STD PROCTOR		0,	70%	PROPOSED TC LT	T & RT					525	NOTES: 1. Street pavement cement content to and not less than 6.5 sacks per cu 2. Sidewalk Cement content of not les 3. No sand allowed under pavement cu 4. Minimum lime content shall be 6% compacted to 95% standard density
2.4.3 Plan Sheets\STPP-C-6126300.dwg		LOW POINT STA 1+98 LOW POINT ELEV 529 PVI STA 1+98.00 PVI ELEV 525.32 AD = 1.40 K = 40.000 K = 40.000	HIGH POINT STA: 3.00 HIGH POINT ELEV 5.41 PVI STA 2+69 PVI ELEV 525 AD = -1.4 K = 30.00	2+69.43 / 525.74 9.43 5.82 40			1.40									515	LOCAL RESIDENTIAL STREET NOTES 1. ALL EXISTING UTILITY LOCATION CONTRACTOR PRIOR TO CONSTI 2. CONTRACTOR SHALL PROTECT AND STRUCTURES UNLESS OTH 3. ALL CURB RADII SHALL BE TO (UNLESS OTHERWISE NOTED). E. QUAIL RUN ROAD = INTERIOR INTERSECTIONS = 4. DETENTION SYSTEM MUST BE AND BOTTOM PRIOR TO ANY P 5. 10" PAVEMENT FOR ARTERIAL (GEM DIRVE, JADE DRIVE, SAPE
jects/612\63\00\2.0 Design\2.4 Civil\2.4 TC LT ELEV G TC RT ELEV G O	526.00 STA 1+00.00 526.00 TC RT 526.00	STA 1+7 RT 525.51	525.68 PVC STA 2+48.4 525.68 TC RT 525.51 525.68 A TC RT 525.67 A TC RT 525.67	525.60 525.60 525.60	525.25 525.25 525.25	4.90 PVC STA 4+36 4.57 4.57 4.57	524.70 TC RT 524.65 5000 0000 0000 0000 0000 0000 0000 0	525.05 525.05 525.05 525.40 525.40	525.75	526.10 526.10	STA 7+44.96 TC RT 526.41					TC LT ELEV CO 20	UTILITY NOTE THE EXISTING UTILITIES SHOWN OF SOURCES AND ARE INTENDED TO THE UTILITY INFORMATION ON THE UTILITY LOCATING SERVICE 48 HOLE CONTRACTOR SHALL VERIFY THE EXAMINE IF THERE ARE ANY OF CONTRACTOR SHALL NOTIFY THE EXISTING UTILITIES ARE DISCOVERED RESPONSIBILITY N
ile: S: \pro	1+00	1+50 2+00	2+50	3+00	3+50 4+0	00 4+50	5+00 5-	+50 6+00	6+50	7+00	7+50	8+00	8+50	9+00	9+50	10+00	ALL RESPONSIBILITY FOR THE ADEC ENGINEER. THE CITY OF ROCKWALL, CONSTRUCTION, ASSUMES NO RESP DESIGN.



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STREET LEGEND

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PROPOSED CONCRETE SIDEWALK BY DEVELOPER (THIS CONTRACT) 4" THICK 3000 PSI (5.5 SACK MIX) #3 BARS @ 24" O.C.E.W.

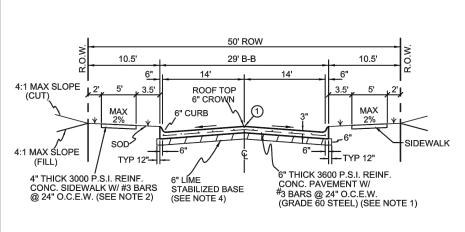
PROPOSED CONCRETE SIDEWALK BY HOMEBUILDER (NOT IN THIS CONTRACT)



DIRECTIONAL CURB RAMP PER CITY OF ROCKWALL DETAIL R-2125A (THIS CONTRACT)

RECORD DRAWING

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50' R.O.W. Residential Street

1. Street pavement cement content to be no less than 6 sacks per cubic yard for machine placed and not less than 6.5 sacks per cubic yard for hand placed. 2. Sidewalk Cement content of not less than 5.5 sack per cubic yard.

3. No sand allowed under pavement or sidewalks. 4. Minimum lime content shall be 6% of dry weight of material (at least 27 lbs/SY) compacted to 95% standard density.

CITY OF ROCKWALL REINFORCED CONCRETE PAVEMENT AUG. '19 R-2033 LOCAL RESIDENTIAL STREET

- ALL EXISTING UTILITY LOCATIONS ARE APPROXIMATE AND WILL BE CONFIRMED F CONTRACTOR PRIOR TO CONSTRUCTION. CONTRACTOR SHALL PROTECT ALL EXISTING TREES, FENCES, RETAINING WALI
- AND STRUCTURES UNLESS OTHERWISE NOTED. ALL CURB RADII SHALL BE TO BACK OF CURB AS FOLLOWS:
- (UNLESS OTHERWISE NOTED). E. QUAIL RUN ROAD = 30.5
- INTERIOR INTERSECTIONS = 20.5' 4. DETENTION SYSTEM MUST BE FUNCTIONAL, WITH EROSION PROTECTION ON SIDES AND BOTTOM PRIOR TO ANY PAVING OPERATIONS (INCLUDING SLAB).
- 10" PAVEMENT FOR ARTERIAL TO GO TO CURB RETURN OF EACH SIDE STREET (GEM DIRVE, JADE DRIVE, SAPPHIRE ROAD AND PEBBLE LANE. 6. END 6" PAVEMENT ON RESIDENTIAL STREET BEGIN 10" PAVEMENT ON E QUAIL

UTILITY NOTE

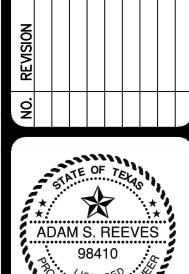
THE EXISTING UTILITIES SHOWN ON THESE PLANS WERE COMPILED FROM VARIOUS SOURCES AND ARE INTENDED TO SHOW THE GENERAL EXISTENCE AND LOCATION O THE UTILITY INFORMATION ON THE PLANS. THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICE 48 HOURS PRIOR TO ANY CONSTRUCTION ACTIVITY. CONTRACTOR SHALL VERIFY THE EXACT LOCATION AND OF ALL EXISTING UTILITIES AND DETERMINE IF THERE ARE ANY CONFLICTS WITH THE PROPOSED FACILITIES. TH CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY WHEN CONFLICTS WITH EXISTING UTILITIES ARE DISCOVERED.

RESPONSIBILITY NOTE

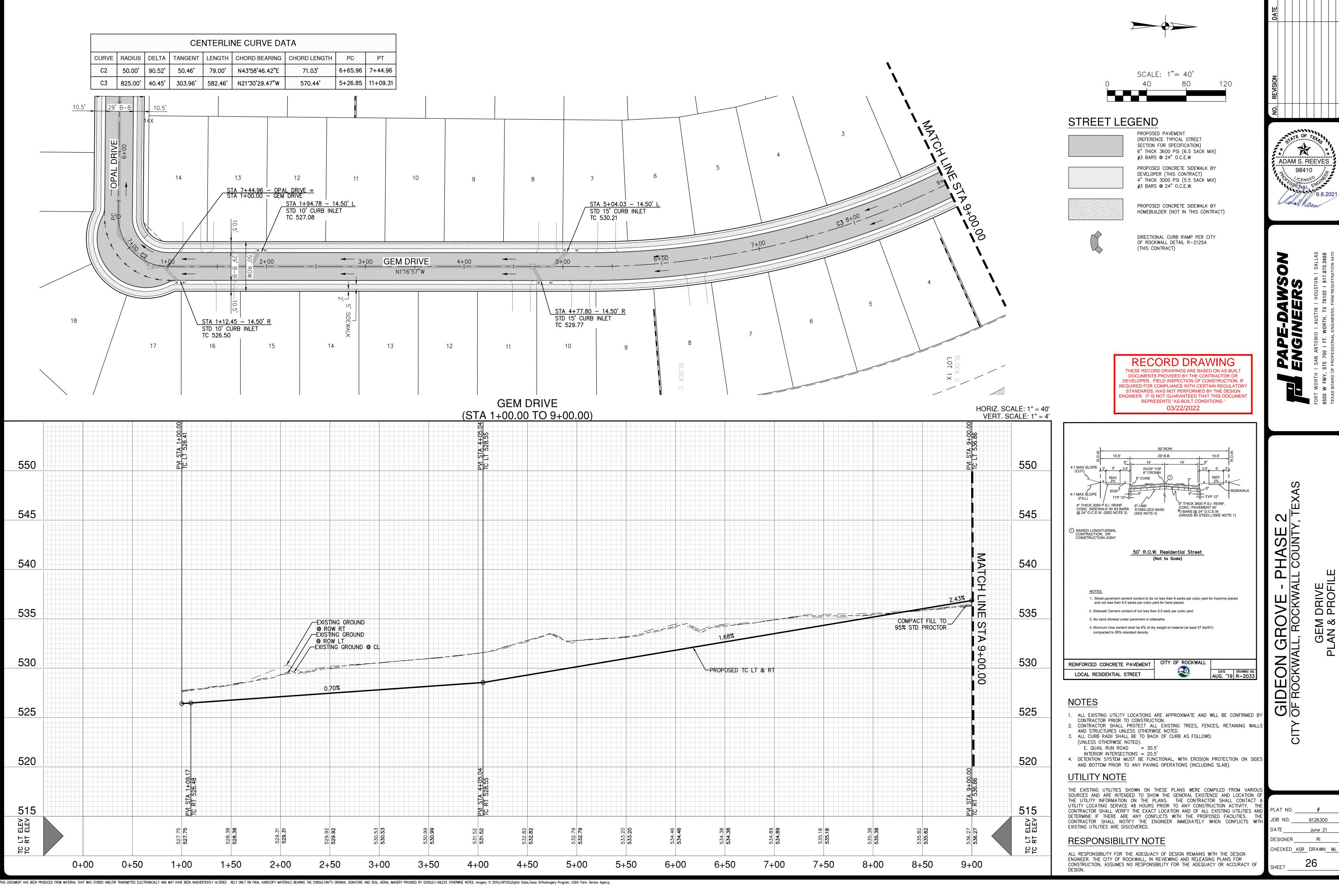
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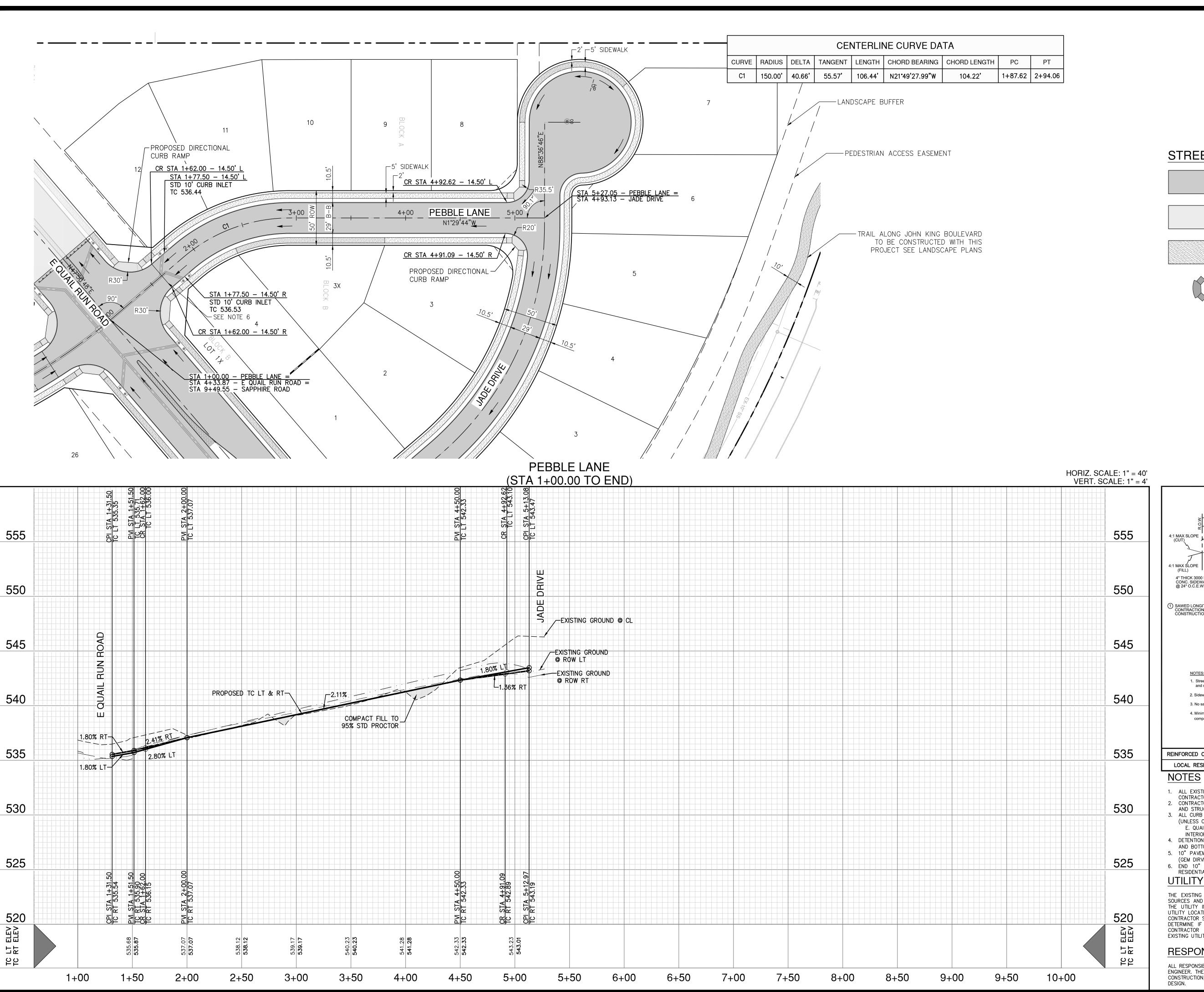
6126300 June 21 DESIGNER HECKED<u>ASR</u> DRAWN<u>ML</u>

S JASPER ST AND PROFILE

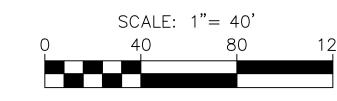


0 PAPE-DAWS(ENGINEERS









PROPOSED PAVEMENT (REFERENCE TYPICAL STREET SECTION FOR SPECIFICATION) 6" THICK 3600 PSI (6.5 SACK MIX) #3 BARS @ 24" O.C.E.W

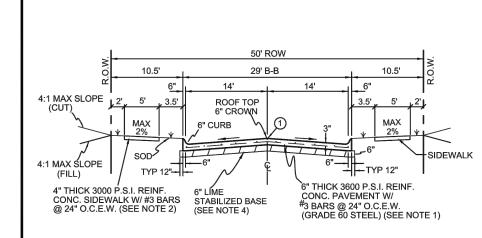
PROPOSED CONCRETE SIDEWALK BY DEVELOPER (THIS CONTRACT) 4" THICK 3000 PSI (5.5 SACK MIX) #3 BARS @ 24" O.C.E.W.

PROPOSED CONCRETE SIDEWALK BY HOMEBUILDER (NOT IN THIS CONTRACT)

DIRECTIONAL CURB RAMP PER CITY OF ROCKWALL DETAIL R-2125A (THIS CONTRACT)

RECORD DRAWING

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50' R.O.W. Residential Street

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- 2. Sidewalk Cement content of not less than 5.5 sack per cubic yard. 3. No sand allowed under pavement or sidewalks.
- 4. Minimum lime content shall be 6% of dry weight of material (at least 27 lbs/SY)

REINFORCED CONCRETE PAVEMENT	CITY OF ROCKWALL		
LOCAL RESIDENTIAL STREET	50	DATE AUG. '19	R-20

- 1. ALL EXISTING UTILITY LOCATIONS ARE APPROXIMATE AND WILL BE CONFIRMED CONTRACTOR PRIOR TO CONSTRUCTION.
- CONTRACTOR SHALL PROTECT ALL EXISTING TREES, FENCES, RETAINING WALI AND STRUCTURES UNLESS OTHERWISE NOTED.
- 3. ALL CURB RADII SHALL BE TO BACK OF CURB AS FOLLOWS: (UNLESS OTHERWISE NOTED).
- E. QUAIL RUN ROAD = 30.5' INTERIOR INTERSECTIONS = 20.5'
- 4. DETENTION SYSTEM MUST BE FUNCTIONAL, WITH EROSION PROTECTION ON SIDES AND BOTTOM PRIOR TO ANY PAVING OPERATIONS (INCLUDING SLAB).
- 5. 10" PAVEMENT FOR ARTERIAL TO GO TO CURB RETURN OF EACH SIDE STREET (GEM DIRVE, JADE DRIVE, SAPPHIRE ROAD AND PEBBLE LANE. 6. END 10" PAVEMENT ON E QUAIL RUN ROAD AND BEGIN 6" PAVEMENT ON

RESIDENTIAL STREET. **UTILITY NOTE**

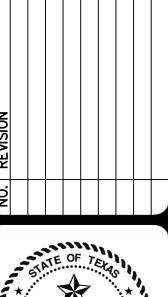
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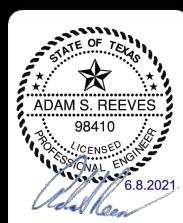
RESPONSIBILITY NOTE

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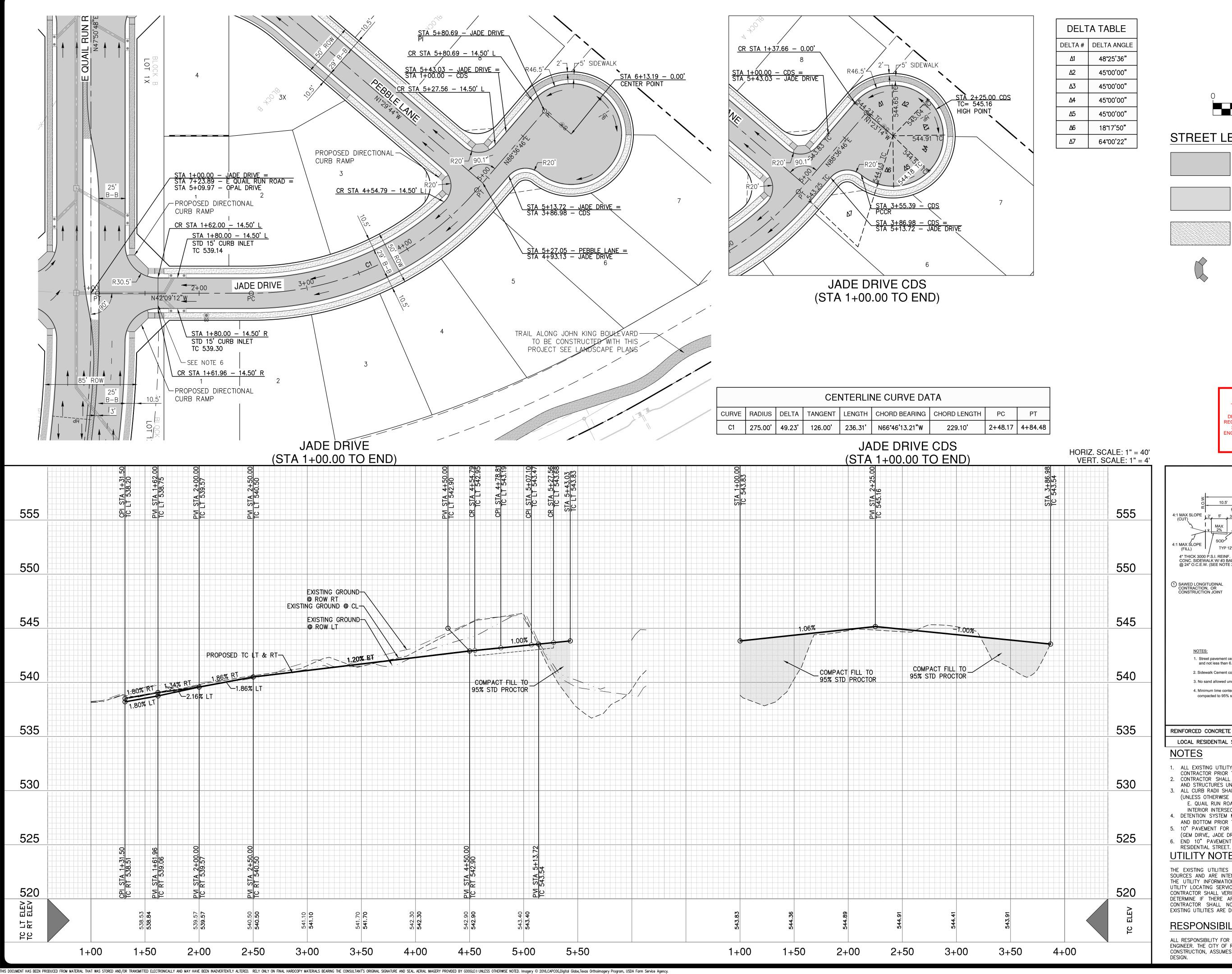
June 21 DESIGNER HECKED<u>ASR</u> DRAWN<u>TB</u> 28

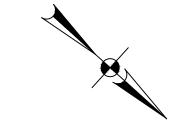
PEBBLE LANE PLAN AND PROFILE

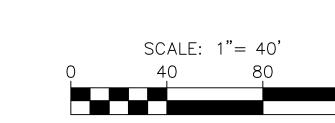


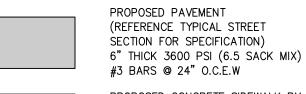


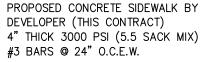
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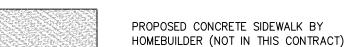










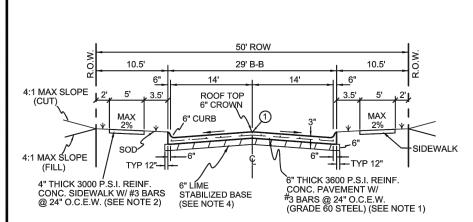




DIRECTIONAL CURB RAMP PER CITY OF ROCKWALL DETAIL R-2125A (THIS CONTRACT)

RECORD DRAWING

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50' R.O.W. Residential Street (Not to Scale)

- 1. Street pavement cement content to be no less than 6 sacks per cubic yard for machine placed and not less than 6.5 sacks per cubic yard for hand placed.
- 2. Sidewalk Cement content of not less than 5.5 sack per cubic yard.
- 3. No sand allowed under pavement or sidewalks.
- 4. Minimum lime content shall be 6% of dry weight of material (at least 27 lbs/SY) compacted to 95% standard density.

LOCAL RESIDENTIAL STREET	29	AUG. '19	DRAWING N
REINFORCED CONCRETE PAVEMENT	CITY OF ROCKWALL		

- ALL EXISTING UTILITY LOCATIONS ARE APPROXIMATE AND WILL BE CONFIRMED CONTRACTOR PRIOR TO CONSTRUCTION.
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- 5. 10" PAVEMENT FOR ARTERIAL TO GO TO CURB RETURN OF EACH SIDE STREET
- (GEM DIRVE, JADE DRIVE, SAPPHIRE ROAD AND PEBBLE LANE. 6. END 10" PAVEMENT ON E QUAIL RUN ROAD AND BEGIN 6" PAVEMENT ON

UTILITY NOTE

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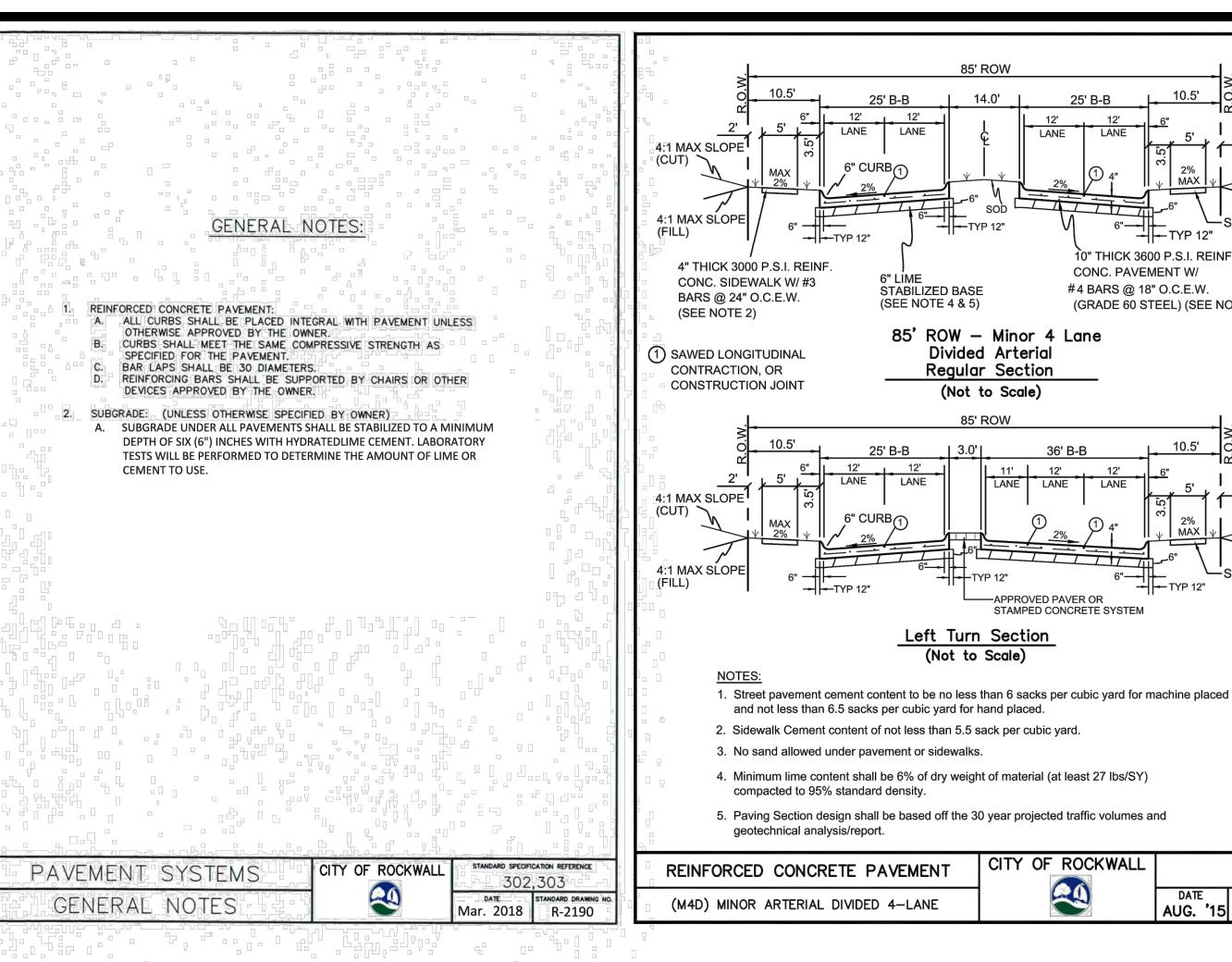
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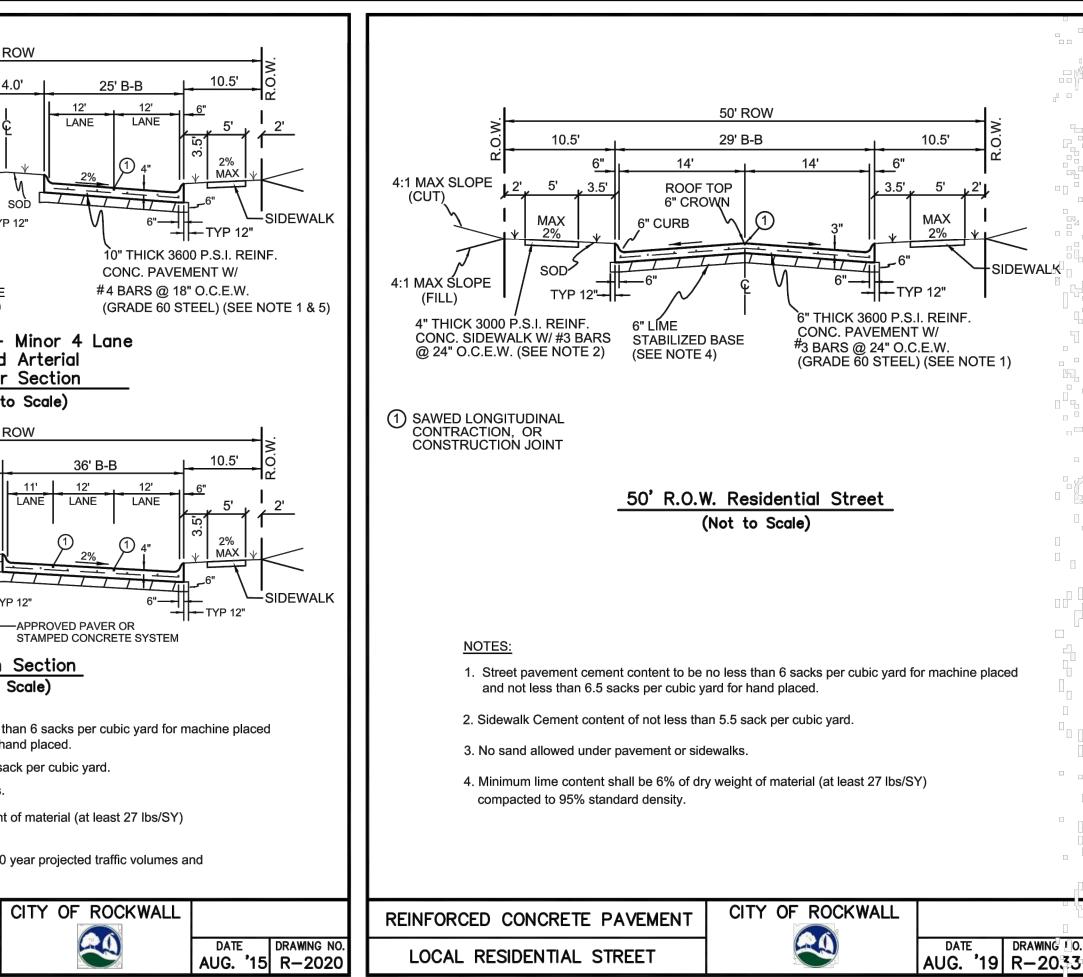
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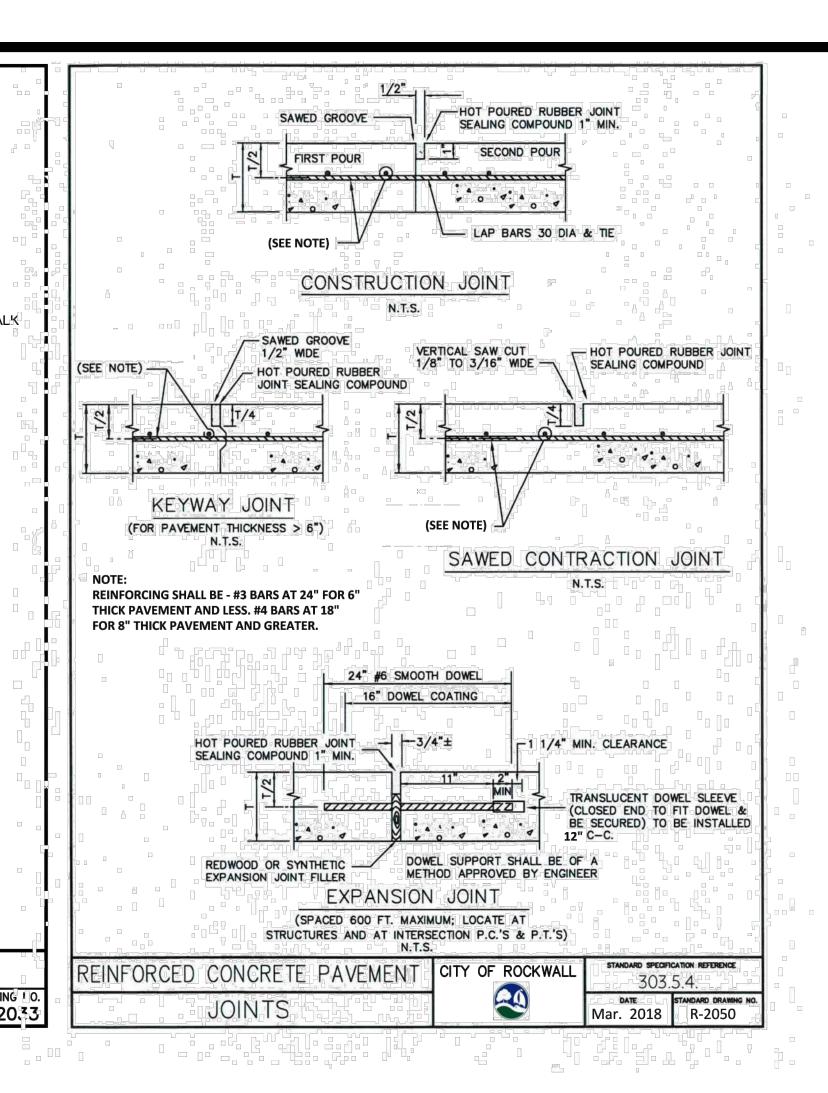
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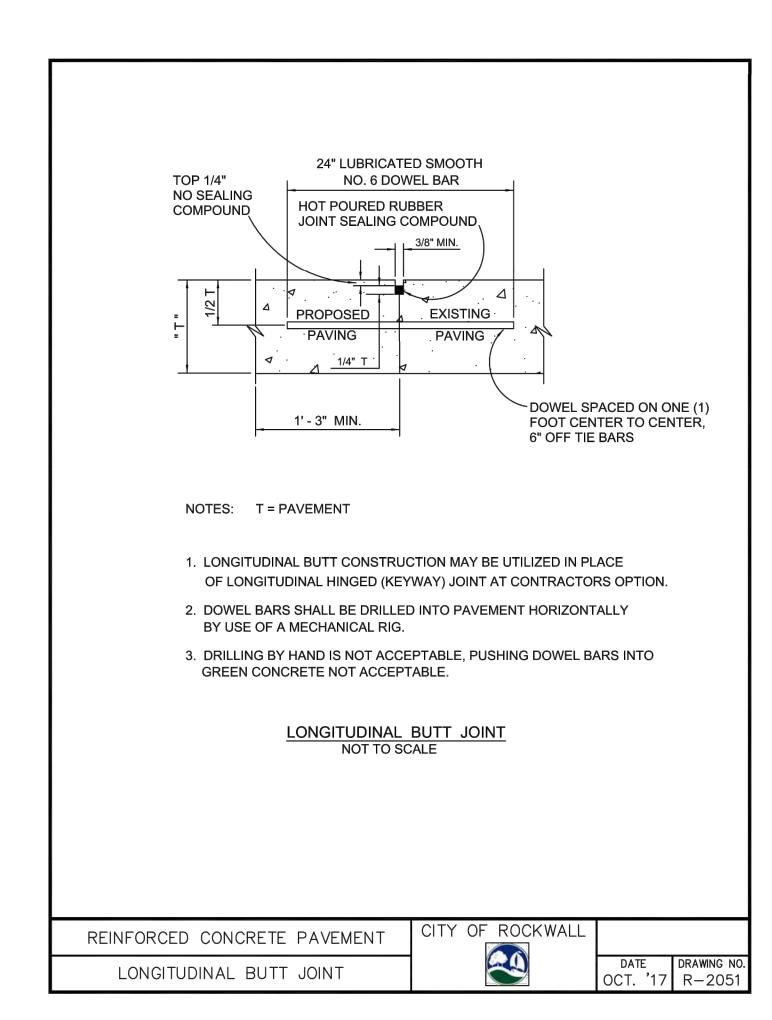
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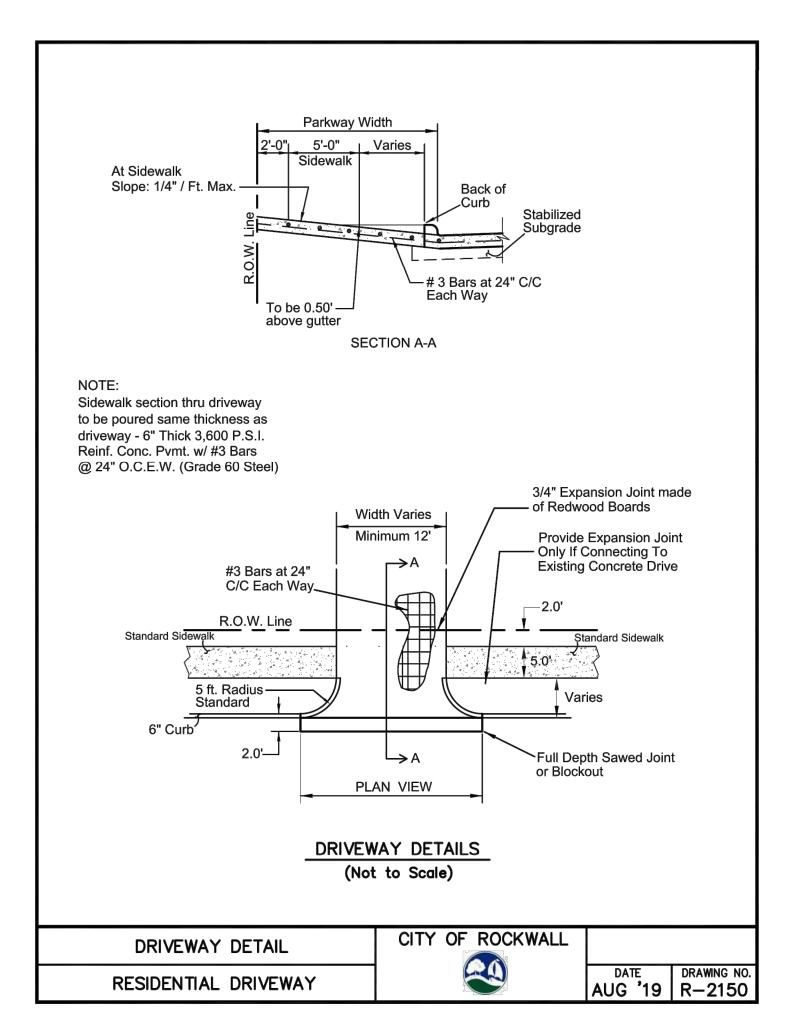


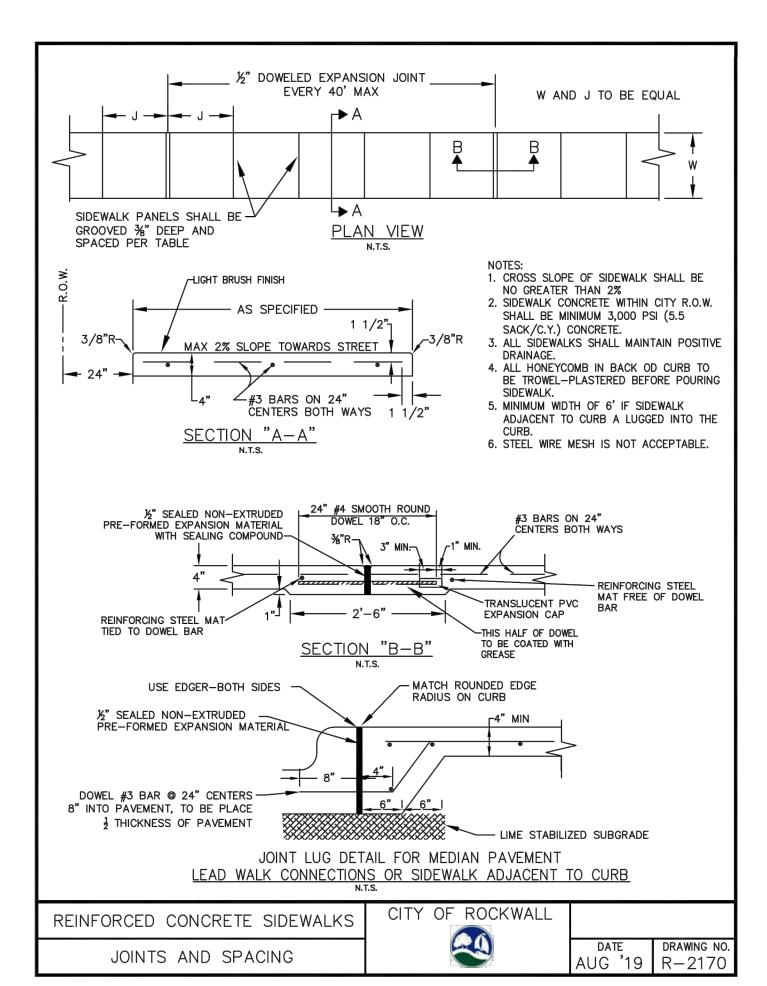
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RECORD DRAWING
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03/22/2022

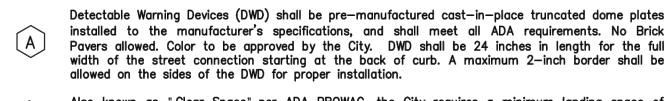
GIDEON GROVE - PHAS CITY OF ROCKWALL, ROCKWALL COUR

ADAM S. REEVES

98410

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PLAT NO.	#
JOB NO	6126300
DATE	June 21
DESIGNER	
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- Also known as "Clear Space" per ADA PROWAG, the City requires a minimum landing space of 5—foot by 5—foot at the bottom of every ramp. This landing space shall have a cross slope in both directions that does not exceed 2.0%. and shall be wholly outside the parallel vehicular travel path.
- The ramp component of the directional curb ramp shall have a continuous longitudinal slope more than 5% and less than 8.3%. The ramp shall also have a cross slope of no more than 2.0%. Length of ramp can vary, but shall not exceed 15 feet to achieve desired elevation change.
- Also known as "Turning Space" per ADA PROWAG, a minimum landing space of 5-foot by 5-foot shall be at the top of every ramp. This landing (turning) space shall have a cross slope in both directions that does not exceed 2.0%. Landing must match width of sidewalk and length shall be the same distance ("Squared" Landing).
- All curb ramps shall have grade breaks at the top and bottom that are perpendicular to the direction of the ramp run. Where the ends of the bottom grade break are less than or equal to 5 feet, the DWD shall be placed within the ramp at the bottom grade break. Where either end of the bottom grade break is greater than 5 feet, the DWD shall be placed behind the back of the curb.
- Paving contractor shall leave block out with a keyway joint installed, minimum of 18 inches measured from back of curb. Block out shall be poured monolithically with Curb Ramp. Concrete shall tie to street paving with a keyway joint per NCTCOG detail 2050. No curb shall be constructed where a DWD is provided. The curb on either side shall have a typical 5 foot taper to transition from the standard 6—inch curb height to be flush with ramp.
- All work associated with accessible routes shall be installed flush with all features to minimize vertical surface discontinuities. Each segment along accessible route shall be flush with no more (zero tolerance) than a 1/4-inch grade separation (elevation difference), or 1/2-inch grade separation if beveled (bevel slope shall not be steeper than 50%).
- A sidewalk header shall be constructed at ends of all work performed.
- Street crossings shall adhere to same guidelines as other accessible routes within public right—of—way, and shall be for the full width of the in—line accessible route. Cross slope shall not exceed 2%*. New street construction shall incorporate all ADA design requirements. It shall be the responsibility of the Design Professional and Contractor to ensure all street crossings meet the requirements of PROWAG. Street alterations on existing streets to bring to compliance shall be at the City Engineer's discretion.
- J All curbs constructed as part of an ADA Ramp shall match City curb standards.
 - * See PROWAG special design considerations when street crossing has no stop or yield condition.

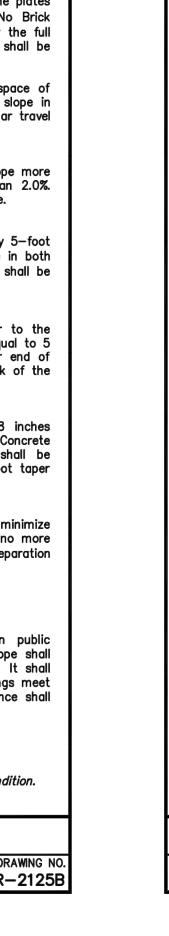
DIRECTIONAL CURB RAMP

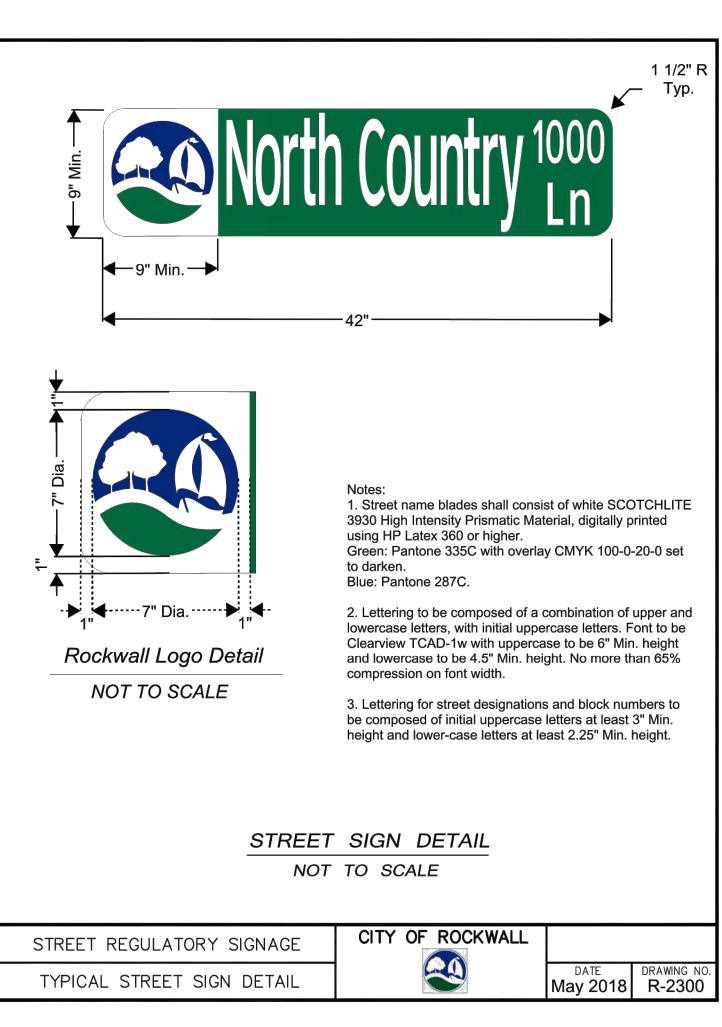
CITY OF ROCKWALL

DATE DRAWING NO. MAR. '17 R-2125B

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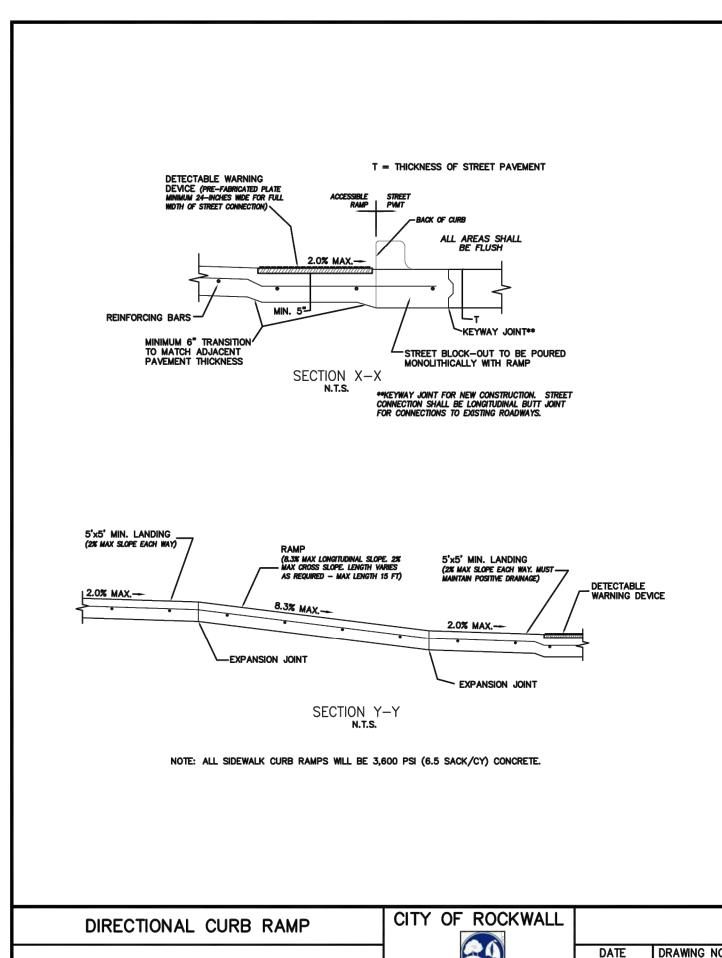
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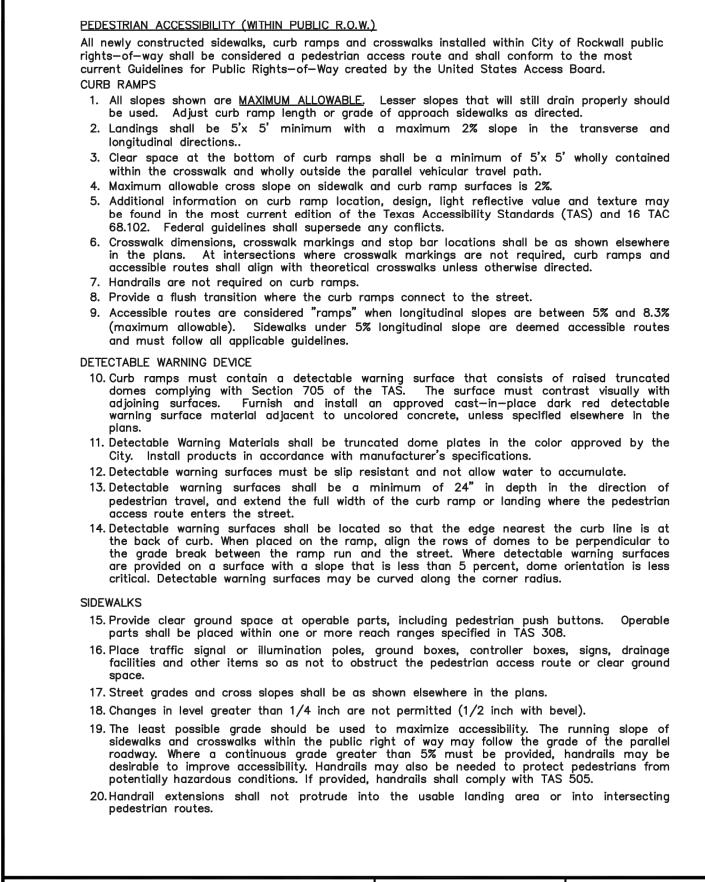
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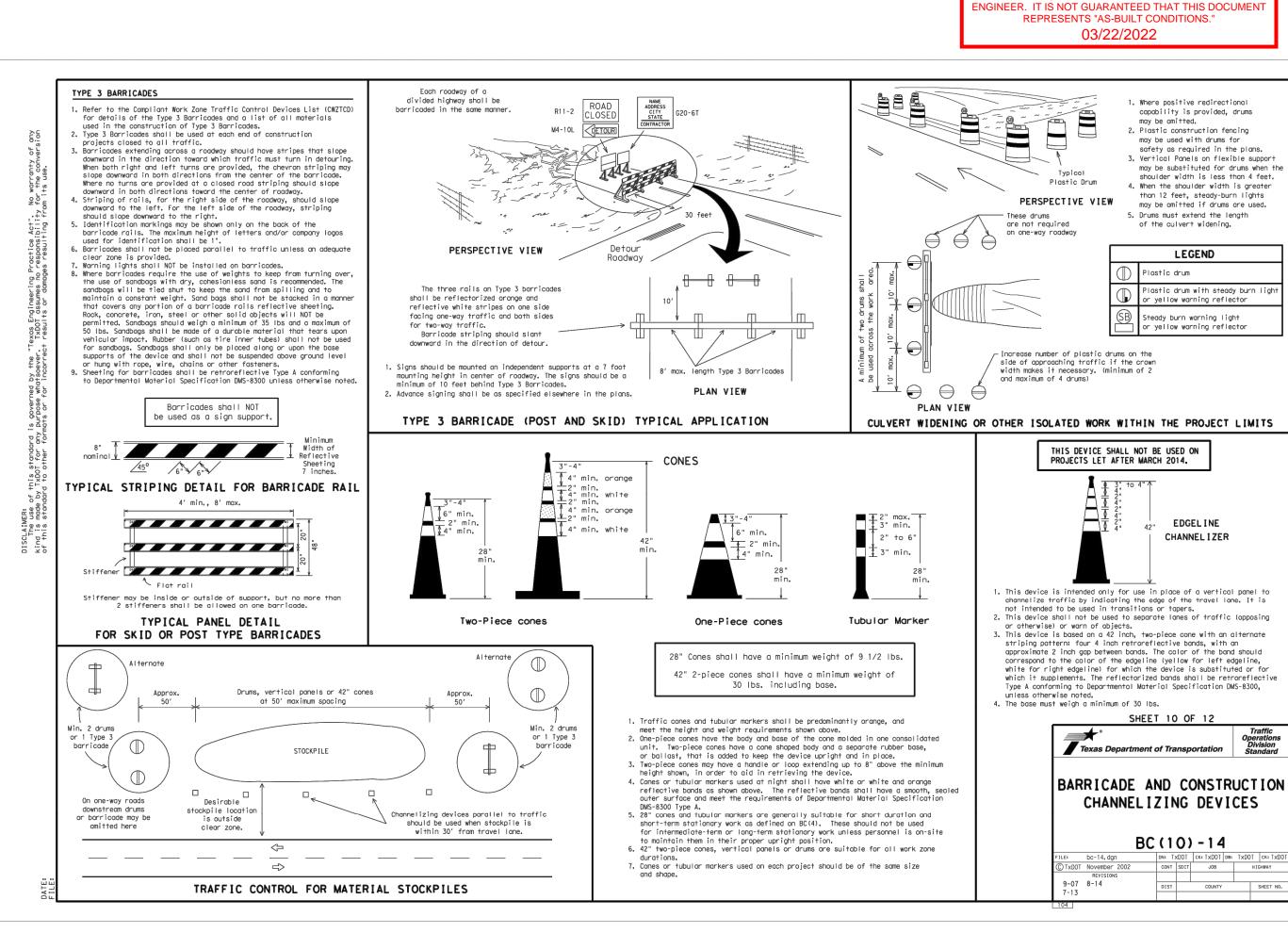
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03/22/2022







DIRECTIONAL CURB RAMP

CITY OF ROCKWALL

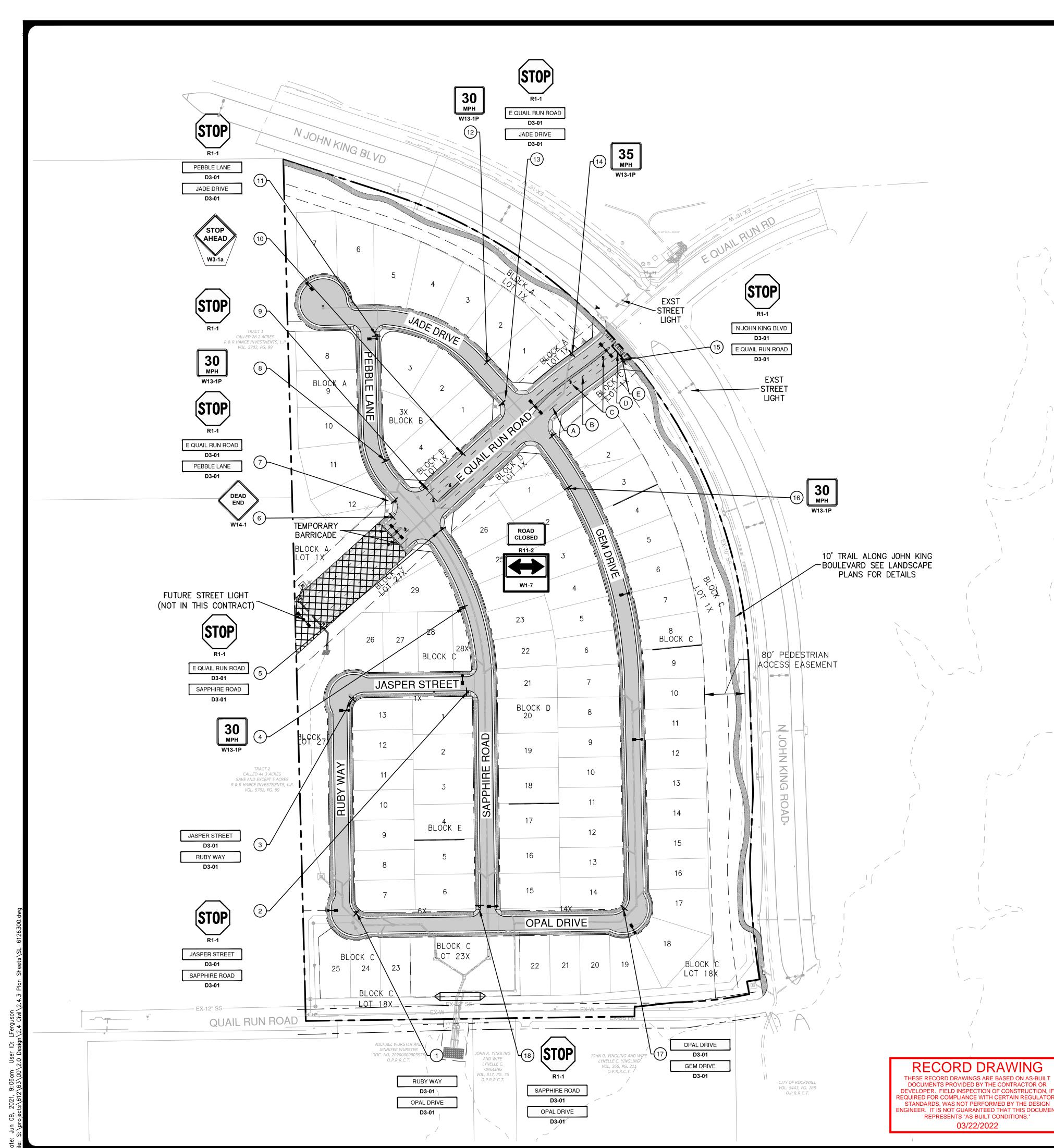
DATE | DRAWING NO. MAR. '17 | R-2125C

DIRECTIONAL CURB RAMP

CITY OF ROCKWALL

DATE | DRAWING NO. MAR. '17 | R-2125C

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CITY OF ROCKWALL STANDARD NOTES

2.16.2 STREET NAME BLADES

A. STREET NAME SIGN BLADES SHALL BE DOUBLE—SIDED WITH ROUNDED CORNERS.

B. STREET NAME BLADES SHALL BE NINE—INCH (9") TALL FLAT ALUMINUM. THE BLADES SHALL BE 0.080 INCHES

THICK AND BE A MINIMUM OF 36" LONG.

C. THE LETTERING FOR THE STREET SIGNS SHALL BE 3M 3930 HIGH INTENSITY PRISMATIC MATERIAL SHEETING

FOR STREET, REGULATORY AND WARNING SIGNS AND SHALL BE HIGH INTENSITY DIAMOND GRADE TYPE III PRISMATIC. THE STREET SIGN BACKGROUND SHALL BE GREEN AND THE LEGEND SHALL BE WHITE.

D. THE STREET SIGN BLADE MUST INCORPORATE THE CURRENT CITY OF ROCKWALL LOGO. THE LOGO SHALL CONSIST OF WHITE SCOTCHLITE SERIES 3930 HIGH INTENSITY PRISMATIC MATERIAL. (PRODUCT CODE 3930)

E. BLOCK NUMBERS ARE REQUIRED ON ALL STREET NAME BLADES AND SHALL BE LOCATED ON THE TOP RIGHT

CORNER OF THE STREET BLADE.

F. THE LETTERING FOR THE STREET BLADES SHALL BE COMPOSED OF A COMBINATION OF LOWER-CASE LETTERS WITH INITIAL UPPER-CASE LETTERS. THE CLEARVIEW TCAD-1W FONT SHALL BE USED. THE LETTERING SHALL BE COMPOSED OF INITIAL UPPER-CASE LETTERS OF AT LEAST 6 INCHES IN HEIGHT AND LOWER CASE LETTERS OF AT LEAST 4.5 INCHES IN HEIGHT. FOR SUPPLEMENTARY LETTERING TO INDICATE THE TYPE OF STREET (SUCH AS STREET, AVENUE OR ROAD) SHALL BE COMPOSED OF INITIAL UPPER-CASE LETTERS AT LEAST 3-INCHES IN HEIGHT AND LOWER-CASE LETTERS AT LEAST 2.25 INCHES IN HEIGHT. ABBREVIATIONS MAY BE USED (FOR EXAMPLE ST., AVE., OR RD) EXCEPT THE STREET NAME ITSELF. THE SUPPLEMENTARY LETTERING SHALL BE LOCATED AT THE LOWER RIGHT CORNER OF THE STREET BLADE, UNDER THE BLOCK NUMBER.

G. THE STREET BLADE SIGN SHALL CONSIST OF GREEN SCOTCHLITE 3930 HIGH INTENSITY PRISMATIC MATERIAL BACKGROUND (PRODUCT CODE 3937) AND WHITE SCOTCHLITE 3930 HIGH INTENSITY PRISMATIC MATERIAL FOR THE LETTERING (PRODUCT CODE — 3930). THE BACKGROUND SHEETING SHALL BE WHITE 3M 390 HIGH INTENSITY PRISMATIC MATERIAL. THE BACKGROUND MATERIAL SHALL BE APPLIED TO THE FULL WIDTH AND HEIGHT OF THE SIGN BLANK LEAVING NO METAL EXPOSED. THE BACKGROUND MATERIAL SHALL BE ONE CONTINUOUS PIECE OF MATERIAL. PATCHING OF BACKGROUND MATERIAL IS NOT ALLOWED AND ANY SIGN WITH PATCHING MATERIAL OF ANY TYPE WILL BE REJECTED BY THE CITY.

a. ALTERNATIVE OPTION: AS AN ALTERNATIVE, THE FOREGROUND COLOR MAY BE GREEN TRANSPARENT SCOTCHLITE ELECTROCUT1177 FILM (E.C. FILM). LETTERING SHALL BE CUT OUT AND REMOVED PRODUCING A SINGLE CONTINUOUS PIECE OF GREEN TRANSPARENT FILM MATERIAL.

2.16.3 STANDARD STREET SIGN POLE AND FIXTURES

A. STANDARD STREET SIGN POST — SHALL BE 12' LONG — MINIMUM (2-3/8") GALVANIZED STEEL ROUND POST WITH A MINIMUM OF 60 MIL WALL THICKNESS.

B. STANDARD POST INSTALLATION DEPTH — SIGN POST SHALL BE INSTALLED INTO SOLID GROUND TO A MINIMUM DEPTH OF 24—INCHES AND ANCHORED WITH A MINIMUM OF 60LBS OF CONCRETE.

C. STANDARD POST BRACKET — SHALL BE (18") CAST ALUMINUM ROUND POST BRACKET STREET SIGN MOUNT FOR BOTTOM STREET BLADE.

D. STANDARD TOP CROSSING BRACKET — SHALL BE (12") CAST ALUMINUM TOP CROSSING STREET SIGN BRACKET

MOUNT FOR TOP STREET BLADE.

E. STANDARD MOUNTING BRACKET ASSEMBLIES — SHALL BE (2-2/8") DIAMETER ALUMINUM ROUND POST

2.16.4 DECORATIVE SIGN POLES AND FIXTURES

INTERLOCKING BRACKET X 2 PER POLE.

THE CITY OF ROCKWALL WILL ALLOW THE INSTALLATION OF DECORATIVE SIGNS AND POSTS OR OTHER NON-STANDARD ITEMS BY DEVELOPERS/HOMEOWNERS ASSOCIATIONS ON A CASE-BY-CASE BASIS PROVIDED THAT THEIR INSTALLATION DOES NOT RESULT IN AN ADVERSE IMPACT TO THE PUBLIC SAFETY AND THAT THERE IS NO COST TO THE CITY FOR INSTALLATION OR MAINTENANCE. RESIDENTIAL DEVELOPER REQUESTING SUCH INSTALLATIONS WILL BE REQUIRED TO GIVE THE RECORDED DOCUMENTATION OF AN INCORPORATED HOMEOWNER'S ASSOCIATION (HOA) TO THE CITY. THE CITY OF ROCKWALL MAINTAINS ONLY STANDARD STREET AND REGULATORY SIGNS/POST INSTALLED ON PUBLIC STREETS WITHIN ITS DESIGNATED RIGHT-OF-WAYS. THE CITY OF ROCKWALL DOES NOT MAINTAIN DECORATIVE SIGN POLES AND FIXTURES INSTALLED BY DEVELOPERS OR HOA.

IF THE DEVELOPER ELECTS TO INSTALL NON-STANDARD DECORATIVE SIGNS, SIGN POLES AND FIXTURES, THE DESIGNATED HOA MUST ENTER INTO A MAINTENANCE AGREEMENT WITH THE CITY COVERING THE HOLD HARMLESS PROVISIONS. THESE PROVISIONS SHALL BE NOTED ON THE APPROVED FINAL PLAT FOR THE SUBDIVISION. THE PLATTED MAINTENANCE PROVISIONS WILL SERVE AS THE AGREEMENT AND APPLIES TO ALL NON-STANDARD DECORATIVE SIGNS, POLES/POST, HARDWARE, OR ANY OTHER ATTACHMENTS. THE CITY OF ROCKWALL HAS NO MAINTENANCE OR OTHER RESPONSIBILITY TO THESE ITEMS. THE OWNERSHIP AND MAINTENANCE OF ALL SUCH SIGNS, POLES AND FIXTURES BECOME THE MAINTENANCE RESPONSIBILITY OF THE DESIGNATED HOA.

DECORATIVE SIGN POLE/FIXTURE SUBMITTALS:

A DETAIL OF THE DECORATIVE SIGN POLES, POLE FIXTURES AND BASE MOUNTING SHALL BE INCLUDED WITH THE SUBMITTAL OF THE CIVIL ENGINEERING CONSTRUCTION PLANS. THE SUBMITTAL SHALL ALSO INCLUDE A STREET/SITE PLAN INDICATING THE LOCATION AND IDENTIFICATION OF ALL PROPOSED SIGNAGE AND POST TO SCALE.

HOA MAINTENANCE - RESPONSIBILITIES AND PROVISIONS:

- A. THE HOA IS RESPONSIBLE FOR MAINTAINING ALL NON-STANDARD DECORATIVE SIGNS, POLES/POST, HARDWARE, ATTACHMENTS OR OTHER APPROVED NON-STANDARD ITEMS UNDER THIS AGREEMENT. THE CITY OF ROCKWALL HAS NO MAINTENANCE OR OTHER RESPONSIBILITY TO THESE ITEMS. THE CITY OF ROCKWALL AND THE HOA AGREE THE ASSOCIATION WILL BEAR ANY AND ALL MAINTENANCE COST RELATED TO THE SAID IMPROVEMENTS. THE CITY HAS THE STATUTORY AUTHORITY TO INSTALL AND MAINTAIN TRAFFIC CONTROL DEVICES FOR VEHICULAR TRAFFIC ON PUBLIC STREETS/ROADS WITHIN THE CITY LIMITS OF THE CITY OF ROCKWALL, TEXAS. THIS AGREEMENT IN NO WAY CONSTITUTES A CHANGE IN THAT AUTHORITY AND DOES NOT CONSTITUTE ANY DELEGATION OF THIS AUTHORITY TO THE ASSOCIATION.
- B. THE CITY OF ROCKWALL RESERVES THE RIGHT TO INSTALL TEMPORARY REPLACEMENT SIGNS USING STANDARD SIGN POST MOUNTING OR ALTERNATE TEMPORARY MOUNTING WHEN DECORATIVE SIGN POSTS AND SIGNS ARE DAMAGED. ROUTINE MAINTENANCE/REPLACEMENT OF DAMAGED SIGNS, POSTS AND ANY SIGN MOUNTING BACKBOARD/TRIM/HARDWARE OR OTHER FIXTURES IS THE SOLE RESPONSIBILITY OF THE HOA AND MUST BE REPAIRED WITHIN 4 WEEKS OF REPORTING TO THE HOA.
- C. THE CITY OF ROCKWALL WILL NOT HANDLE, STORE OR BE RESPONSIBLE FOR ANY DECORATIVE NON-STANDARD SIGN, POST OR ASSOCIATED FIXTURES INSTALLED UNDER THIS AGREEMENT.
- D. ALL SIGNS (REGULATORY AND WARNING) SHALL BE IN CONFORMANCE WITH THE "TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES" (TEXAS MUTCD) AND THE "STANDARD HIGHWAY SIGN DESIGNS FOR TEXAS."
- E. SIGN POSTS MUST BE OF SUFFICIENT HEIGHT TO MOUNT THE SIGN IN CONFORMANCE WITH THE CURRENT (TEXAS MUTCD) REQUIREMENTS. MOST TYPICAL INSTALLATIONS REQUIRE A VERTICAL CLEARANCE OF 7 FEET FROM THE BOTTOM EDGE OF THE SIGN TO THE GROUND SURFACE. OVERHEAD SIGNS MUST CONFORM TO ALL REQUIRED STANDARDS.
- F. SIGNS/POSTS MUST BE INSTALLED IN LOCATIONS AS PROVIDED IN THE APPROVED ENGINEERING/CONSTRUCTION PLANS OR AS OTHERWISE APPROVED BY THE CITY OF ROCKWALL. ON OCCASION, IT MAY BE NECESSARY TO RE-LOCATE SIGNAGE/POLES BASED ENGINEERING JUDGMENT, STUDY OR WHEN OTHERWISE DEEMED NECESSARY BY THE CITY.

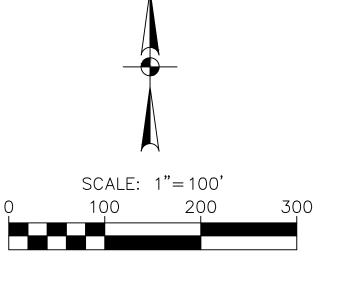
G. THE CITY OF ROCKWALL RESERVES THE RIGHT TO APPROVE OR DISAPPROVE ANY SIGN/POLE DESIGN AND/OR

LOCATION. THE CITY OF ROCKWALL MUST APPROVE THE COLOR OF SIGNPOSTS AND ANY REQUESTED SIGN

PAVEMENT MARKING SUMMARY					
LOCATION	MARKING TYPE				
A	BROKEN WHITE LANE LINE				
В	WIDE WHITE LINE				
С	STANDARD WHITE PAVEMENT ARROW				
D	24" WHITE STOP LINE				
E	STANDARD WHITE CROSSWALK MARKINGS				

PAVEMENT MARKING NOTE

1. REFERENCE SHEET 33 PAVEMENT MARKING DETAILS FOR ADDITIONAL INFORMATION REGARDING PAVEMENT MARKINGS



LEGEND

PROPOSED STREET LIGHT

PROPOSED STREET LIGHT

(DOUBLE HEADED)

PROPOSED SIGN

ULTIMATE BUILD OUT (NOT TO BE CONSTRUCTED WITH THIS CONTRACT)

ROPOSED EXISTING

EX-W WATER LINE

GATE VALVE

GATE VALVE

FIRE HYDRANT ASSEMBLY

EX-SS SANITARY SEWER LINE

SANITARY SEWER MANHOLE

SIGN SUMMARY

LOCATION	SIGN TYPE	SIGN TEXT
1	D3-1	RUBY WAY
	D3-1	OPAL DRIVE
2	R1-1	STOP
<u> </u>	D3-1	JASPER STREET
	D3-1	SAPPHIRE ROAD
3	D3-1	JASPER STREET
	D3-1	RUBY WAY
4	W13-1P	30 MPH
	1110	00 WITTI
5	R1-1	STOP
	D3-1	E QUAIL RUN ROAD
	D3-1	SAPPHIRE ROAD
6	W14-1	DEAD END
υ	VV 4 	L DEAD EIND
7	R1-1	STOP
	D3-1	E QUAIL RUN ROAD
	D3-1	PEBBLE LANE
	W47 4D	70 MDH
8	W13-1P	30 MPH
9	R1-1	STOP
10	W3-1a	STOP AHEAD
11	D1 1	CTOD
11	R1-1 D3-1	STOP PEBBLE LANE
	D3-1	JADE DRIVE
	DO 1	ONDE BINIVE
12	W13-1P	30 MPH
13	R1-1	STOP
	D3-1	E QUAIL RUN ROAD
	D3-1	JADE DRIVE
14	W13-1P	30 MPH
· · ·		
15	R1-1	STOP
	D3-1	N JOHN KING BLVD
	D3-1	E QUAIL RUN ROAD
16	W13-1P	30 MPH
10	1110 11	JO IVII 11
17	D3-1	OPAL DRIVE
	D3-1	GEM DRIVE
4.0	D4 4	0700
18	R1-1	STOP
	D3-1	SAPPHIRE ROAD
	D3-1	OPAL DRIVE

UTILITY NOTE

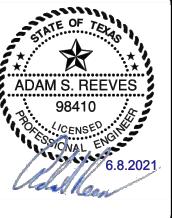
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NO. REVISION

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FORT WORTH I SAN ANTONIO I AUSTIN I HOUSTO 6500 W FWY, STE 700 I FT. WORTH, TX 76102 I 8

GIDEON GROVE - PHASE Z ITY OF ROCKWALL, ROCKWALL COUNTY, TE

PLAT NO. #

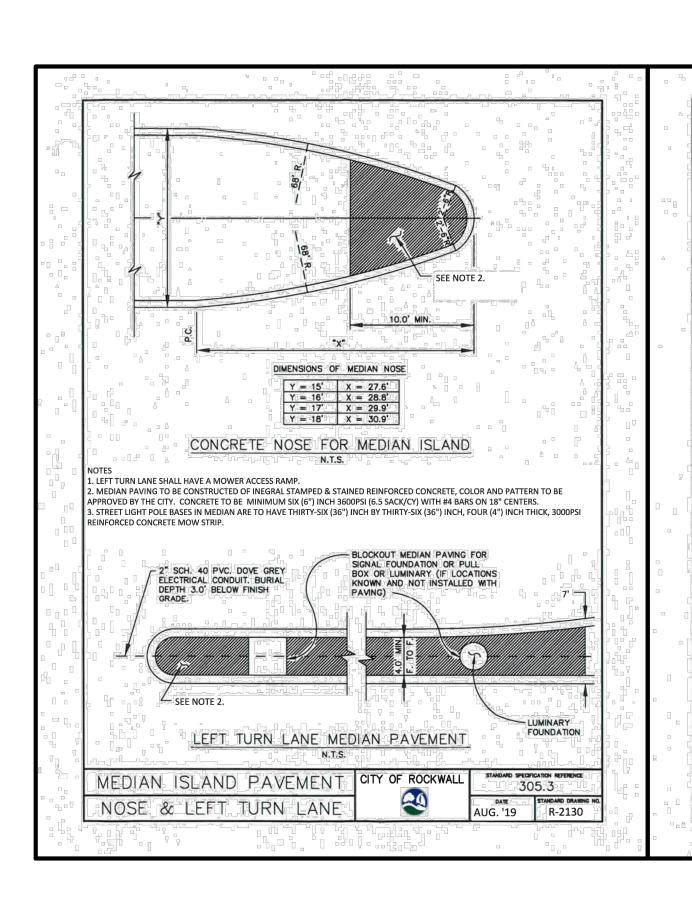
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DATE June 21

DESIGNER ML

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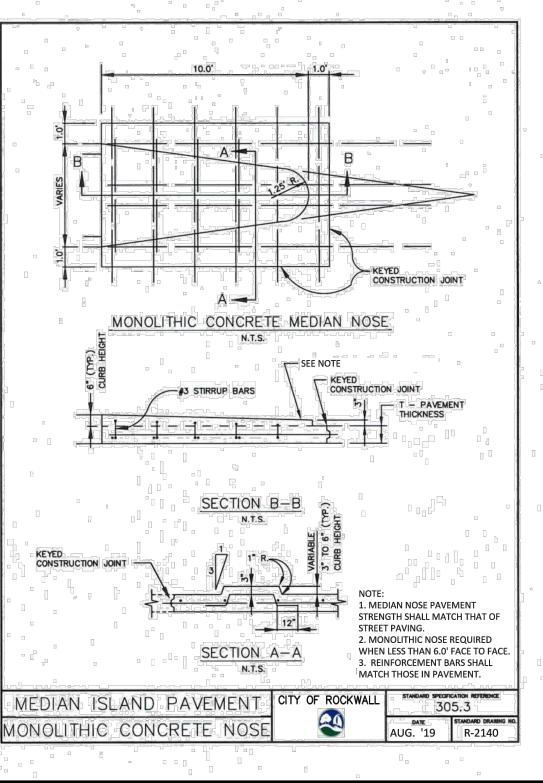
CITY OF ROCKWALL

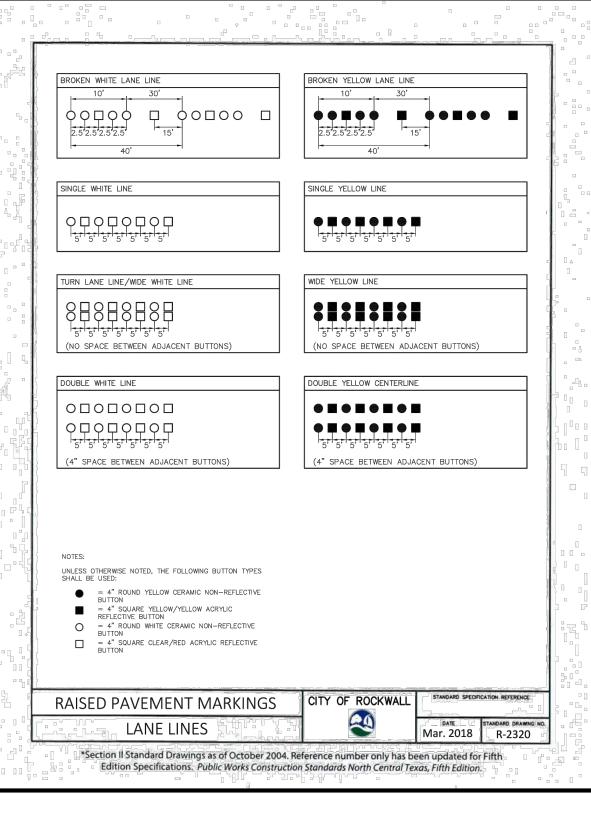
*Section II Standard Drawings as of October 2004. Reference number only has been updated for Fifth

Edition Specifications. Public Works Construction Standards North Central Texas, Fifth Edition.

TYPICAL THOROUGHFARE

LAYOUTS



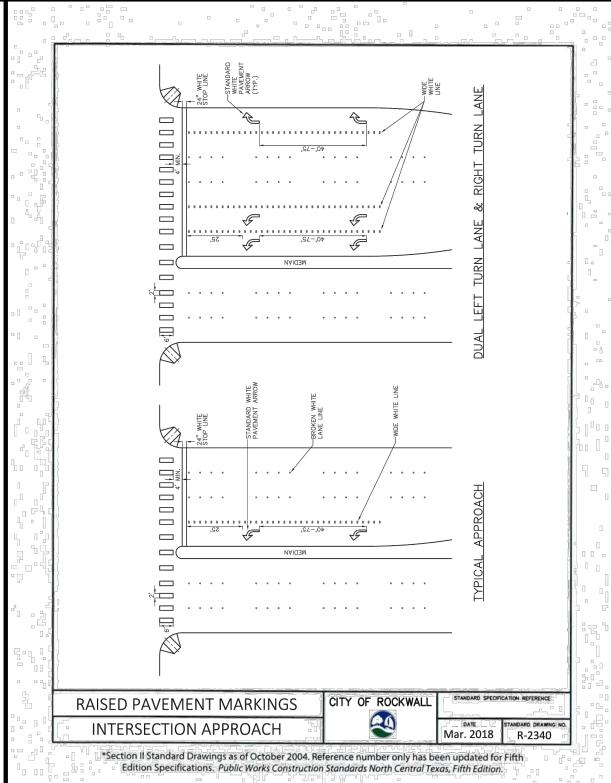


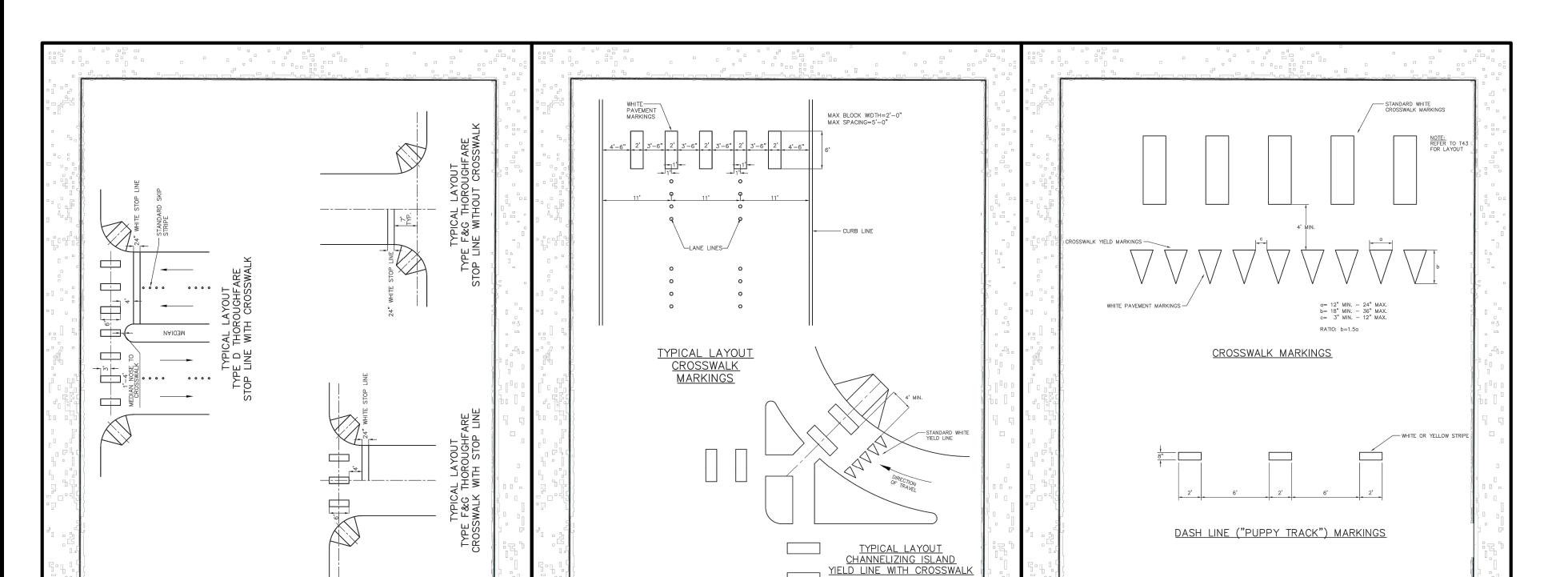
CITY OF ROCKWALL

*Section II Standard Drawings as of October 2004. Reference number only has been updated for Fifth Edition Specifications. Public Works Construction Standards North Central Texas, Fifth Edition.

TYPICAL CROSSWALK AND

DASHED MARKINGS





CITY OF ROCKWALL

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Edition Specifications. Public Works Construction Standards North Central Texas, Fifth Edition.

Mar. 2018 R-2370

TYPICAL CROSSWALK

LAYOUTS

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THESE RECORD DRAWINGS ARE BASED ON AS-BUILT DOCUMENTS PROVIDED BY THE CONTRACTOR OR REPRESENTS "AS-BUILT CONDITIONS." 03/22/2022

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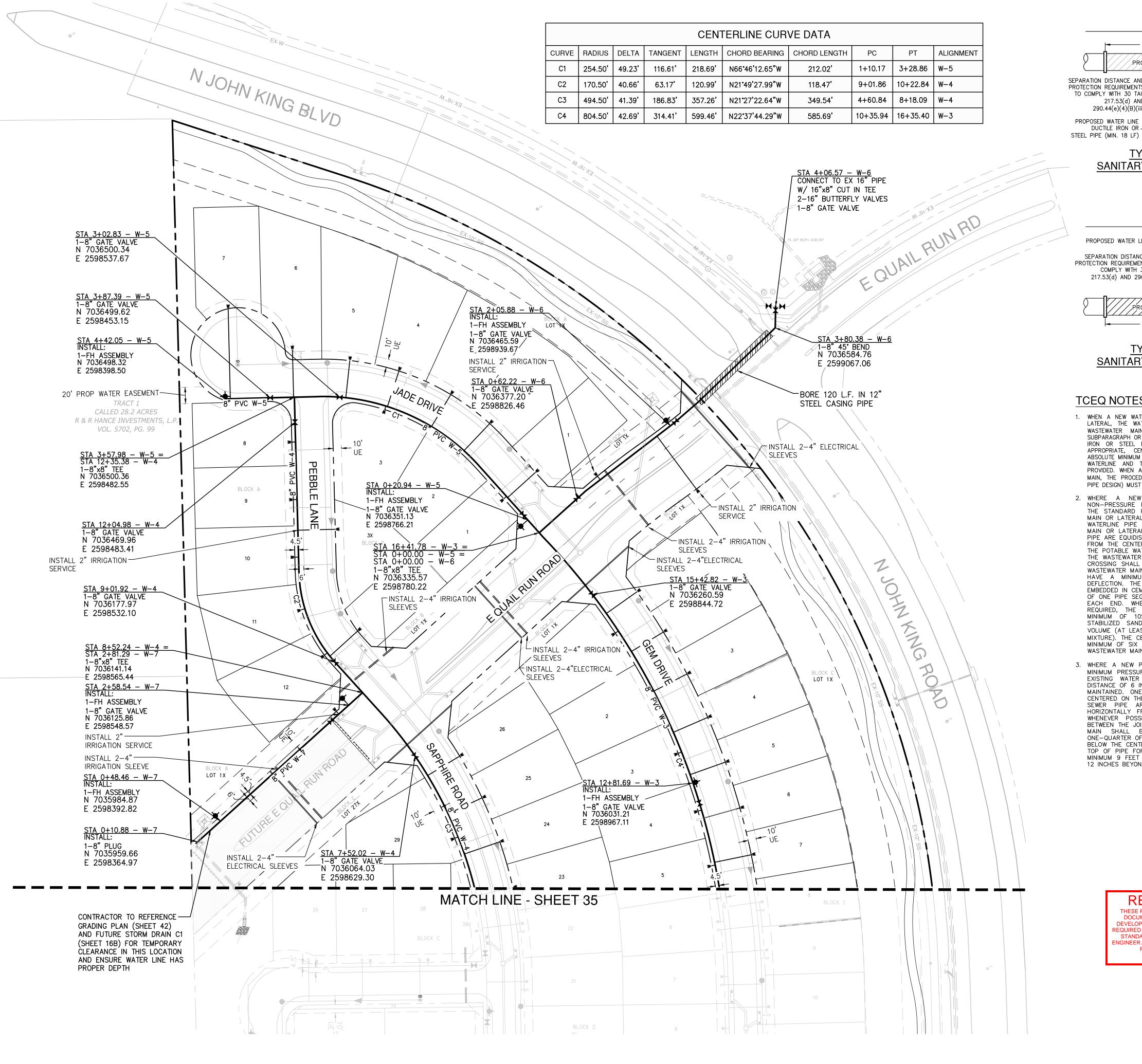
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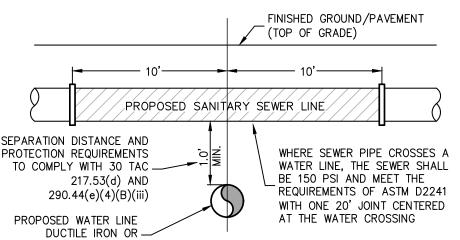
PAPE-DAWSON ENGINEERS

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DEVELOPER. FIELD INSPECTION OF CONSTRUCTION, IF REQUIRED FOR COMPLIANCE WITH CERTAIN REGULATORY STANDARDS, WAS NOT PERFORMED BY THE DESIGN ENGINEER. IT IS NOT GUARANTEED THAT THIS DOCUMENT

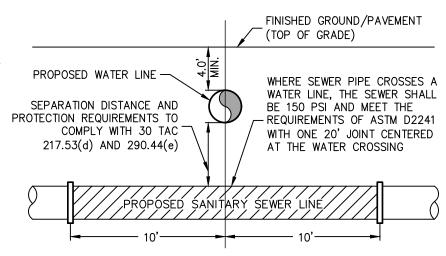


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TYPICAL WATER BELOW SANITARY SEWER CROSSING DETAIL

NOT-TO-SCALE



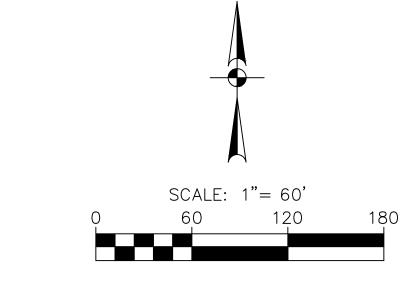
TYPICAL WATER ABOVE SANITARY SEWER CROSSING DETAIL

NOT-TO-SCALE

TCEQ NOTES

- 1. WHEN A NEW WATERLINE CROSSES UNDER A WASTEWATER MAIN OF LATERAL, THE WATERLINE SHALL BE ENCASED AS DESCRIBED FOR WASTEWATER MAINS OR LATERALS IN CLAUSE (ii) OF THIS SUBPARAGRAPH OR CONSTRUCTED OF A MINIMUM OF 18 LF OF DUCTILE IRON OR STEEL PIPE WITH MECHANICAL OR WELDED JOINTS AS APPROPRIATE, CENTERED AT THE WASTEWATER CROSSING. AN ABSOLUTE MINIMUM SEPARATION DISTANCE OF ONE FOOT BETWEEN THE WATERLINE AND THE WASTEWATER MAIN OR LATERAL SHALL BE PROVIDED. WHEN A NEW WATERLINE CROSSES UNDER A WASTEWATER MAIN, THE PROCEDURES IN \$217.53(d) OF THIS TITLE (RELATING TO PIPE DESIGN) MUST BE FOLLOWED.
- 2. WHERE A NEW POTABLE WATERLINE CROSSES A NEW, NON-PRESSURE RATED WASTEWATER MAIN OR LATERAL AND THE STANDARD PIPE SEGMENT LENGTH OF THE WASTEWATER MAIN OR LATERAL IS AT LEAST 18 FEET, ONE SEGMENT OF THE WATERLINE PIPE SHALL BE CENTERED OVER THE WASTEWATER MAIN OR LATERAL SUCH THAT THE JOINTS OF THE WATERLINE PIPE ARE EQUIDISTANT AND AT LEAST NINE FEET HORIZONTALLY FROM THE CENTERLINE OF THE WASTEWATER MAIN OR LATERAL THE POTABLE WATERLINE SHALL BE AT LEAST TWO FEET ABOVE THE WASTEWATER MAIN OR LATERAL. WHENEVER POSSIBLE, THE CROSSING SHALL BE CENTERED BETWEEN THE JOINTS OF THE WASTEWATER MAIN OR LATERAL. THE WASTEWATER PIPE SHALL HAVE A MINIMUM PIPE STIFFNESS OF 115 PSI AT 5.0% DEFLECTION. THE WASTEWATER MAIN OR LATERAL SHALL BE EMBEDDED IN CEMENT STABILIZED SAND FOR THE TOTAL LENGTH OF ONE PIPE SEGMENT PLUS 12 INCHES BEYOND THE JOINT ON EACH END. WHERE CEMENT STABILIZED SAND BEDDING IS REQUIRED, THE CEMENT STABILIZED SAND SHALL HAVE A MINIMUM OF 10% CEMENT PER CUBIC YARD OF CEMENT STABILIZED SAND MIXTURE, BASED ON LOOSE DRY WEIGHT VOLUME (AT LEAST 2.5 BAGS OF CEMENT PER CUBIC YARD OF MIXTURE). THE CEMENT STABILIZED SAND BEDDING SHALL BE A MINIMUM OF SIX INCHES ABOVE AND FOUR INCHES BELOW THE WASTEWATER MAIN OR LATERAL.
- 3. WHERE A NEW PVC WASTEWATER MAIN OR LATERAL (WITH A MINIMUM PRESSURE RATING OF 150 PSI) CROSSES UNDER AN EXISTING WATER LINE. AN ABSOLUTE MINIMUM SEPARATION DISTANCE OF 6 INCHES BETWEEN OUTSIDE DIAMETERS SHALL BE MAINTAINED. ONE SEGMENT OF THE SEWER PIPE SHALL CENTERED ON THE WATER LINE SUCH THAT THE JOINTS OF THE SEWER PIPE ARE EQUIDISTANT AND AT LEAST 9 FEET HORIZONTALLY FROM THE CENTERLINE OF THE WATER LINE.
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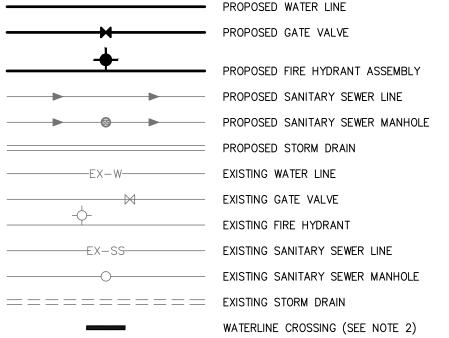


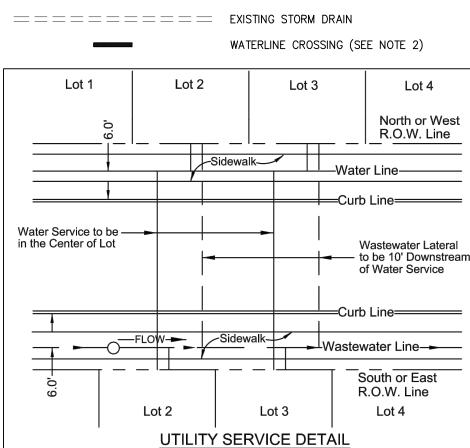
BENCHMARKS

TBM NO. 1 X-CUT SET IN CONCRETE LOCATED IN THE X-CUT SET IN CONCRETE LOCATED IN THE CENTER OF A CURB INLET IN THE WEST CENTER OF A CURB INLET IN THE SOUTHWESTERLY CURB LINE OF NORTH CURB LINE OF NORTH JOHN KING JOHN KING BOULEVARD AND BEING +/- BOULEVARD AND BEING +/- 662' NORTH 235' SOUTHEST OF THE NORTHWEST CORNER OF THE SUBJECT TRACT AND +/-BOULEVARD AND WEST QUAIL RUN ROAD 535' NORTHWEST OF THE INTERSECTION OF AND +/- 723' SOUTHEAST OF THE N. JOHN KING BOULEVARD AND EAST QUAIL RUN ROAD ELEV: 546.16'

OF THE INTERSECTION NORTH JOHN KING INTERSÉCTION OF N. JOHN KING BOULEVARD AND EAST QUAIL RUN ROAD. ELEV: 530.38'

WATER LEGEND





- 1. ALL EXISTING UTILITY LOCATIONS ARE APPROXIMATE AND WILL BE CONFIRMED (CONTRACTOR PRIOR TO CONSTRUCTION.
- WHERE A NEW POTABLE WATERLINE CROSSES A NEW, NON-PRESSURE RATED WASTEWATER MAIN OR LATERAL AND THE STANDARD PIPE SEGMENT LENGTH OF THE WASTEWATER MAIN OR LATERAL IS AT LEAST 18 FEET, ONE SEGMENT OF THE WATERLINE PIPE SHALL BE CENTERED OVER THE WASTEWATER MAIN OR LATERAL SUCH THAT THE JOINTS OF THE WATERLINE PIPE ARE EQUIDISTANT AND AT LEAST NINE FEET HORIZONTALLY FROM THE CENTERLINE OF THE WASTEWATER MAIN OR LATERAL. THE POTABLE WATERLINE SHALL BE AT LEAST TWO FEET ABOVE THE WASTEWATER MAIN OR LATERAL. WHENEVER POSSIBLE, THE CROSSINC SHALL BE CENTERED BETWEEN THE JOINTS OF THE WASTEWATER MAIN OF LATERAL. THE WASTEWATER PIPE SHALL HAVE A MINIMUM PIPE STIFFNESS OF 115 PSI AT 5.0% DEFLECTION. THE WASTEWATER MAIN OR LATERAL SHALL BE EMBEDDED IN CEMENT STABILIZED SAND FOR THE TOTAL LENGTH OF ONE PIPE SEGMENT PLUS 12 INCHES BEYOND THE JOINT ON EACH END. WHERE CEMENT STABILIZED SAND BEDDING IS REQUIRED, THE CEMENT STABILIZED SAND SHALL HAVE A MINIMUM OF 105 CEMENT PER CUBIC YARD OF CEMENT STABILIZED SAND MIXTURE, BASED ON LOOSE DRY WEIGHT VOLUME (AT LEAST 2.5 BAGS OF CEMENT PER CUBIC YARD OF MIXTURE). THE CEMENT STABILIZED SAND BEDDING SHALI BE A MINIMUM OF SIX INCHES ABOVE AND FOUR INCHES BELOW THE WASTEWATER MAIN OR LATERAL.
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- 4. FIRE HYDRANT ASSEMBLY TO INCLUDE 1-8"x6" TEE, 1-6" 90° HORIZONTAL BEND, 1-6" VALVE, AND 1-FIRE HYDRANT.

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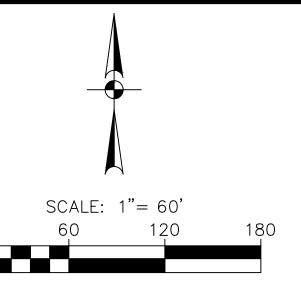
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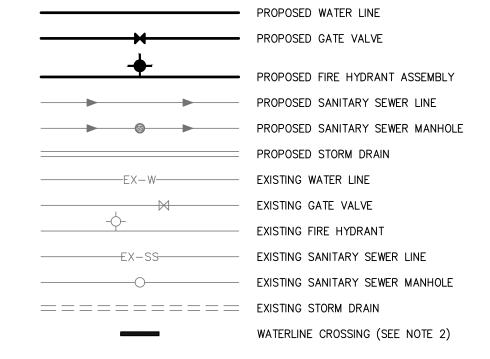
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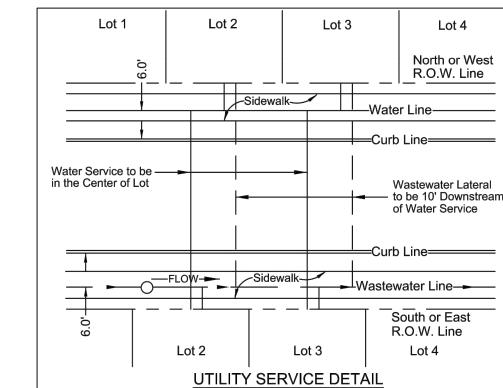


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PAPE-DAWS ENGINEERS

6126300 June 21 ESIGNER HECKED DRAWN

35

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SANITARY SEWER CROSSING DETAIL

03/22/2022

TCEQ NOTES

WHEN A NEW WATERLINE CROSSES UNDER A WASTEWATER MAIN OR LATERAL, THE WATERLINE SHALL BE ENCASED AS DESCRIBED FOR WASTEWATER MAINS OR LATERALS IN CLAUSE (ii) OF THIS SUBPARAGRAPH OR CONSTRUCTED OF A MINIMUM OF 18 LF OF DUCTILE IRON OR STEEL PIPE WITH MECHANICAL OR WELDED JOINTS AS APPROPRIATE, CENTERED AT THE WASTEWATER CROSSING. AN ABSOLUTE MINIMUM SEPARATION DISTANCE OF ONE FOOT BETWEEN THE WATERLINE AND THE WASTEWATER MAIN OR LATERAL SHALL BE PROVIDED. WHEN A NEW WATERLINE CROSSES UNDER A WASTEWATER MAIN, THE PROCEDURES IN §217.53(d) OF THIS TITLE (RELATING TO PIPE DESIGN) MUST BE FOLLOWED.

TYPICAL WATER ABOVE

NOT-TO-SCALE

FINISHED GROUND/PAVEMENT

WHERE SEWER PIPE CROSSES A

WATER LINE, THE SEWER SHALL

REQUIREMENTS OF ASTM D2241

WITH ONE 20' JOINT CENTERED

BE 150 PSI AND MEET THE

AT THE WATER CROSSING

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(TOP OF GRADE)

(TOP OF GRADE)

PRÓPOSED SANITARY SEWER LINE,

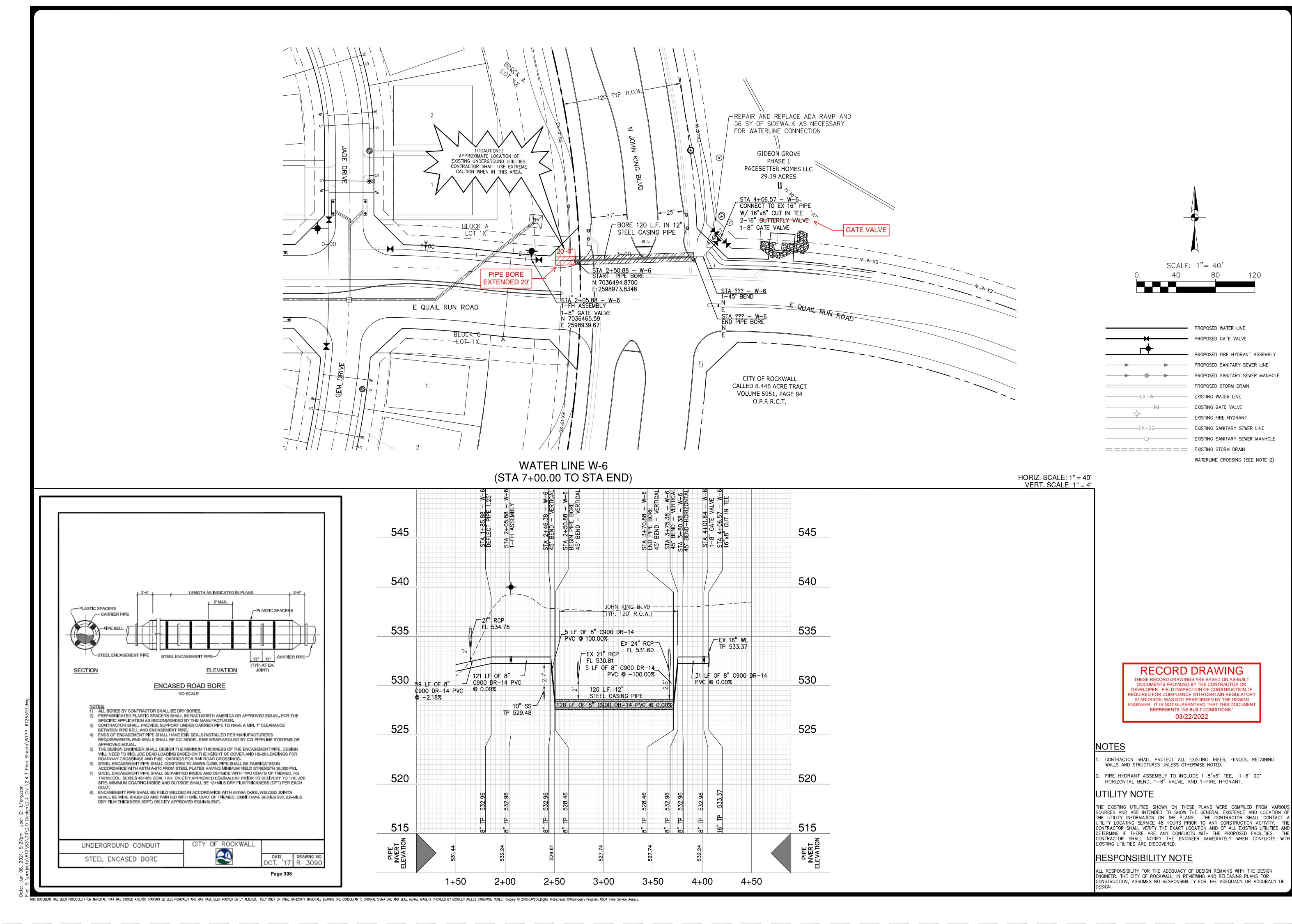
TYPICAL WATER BELOW

217.53(d) AND

COMPLY WITH 30 TAC

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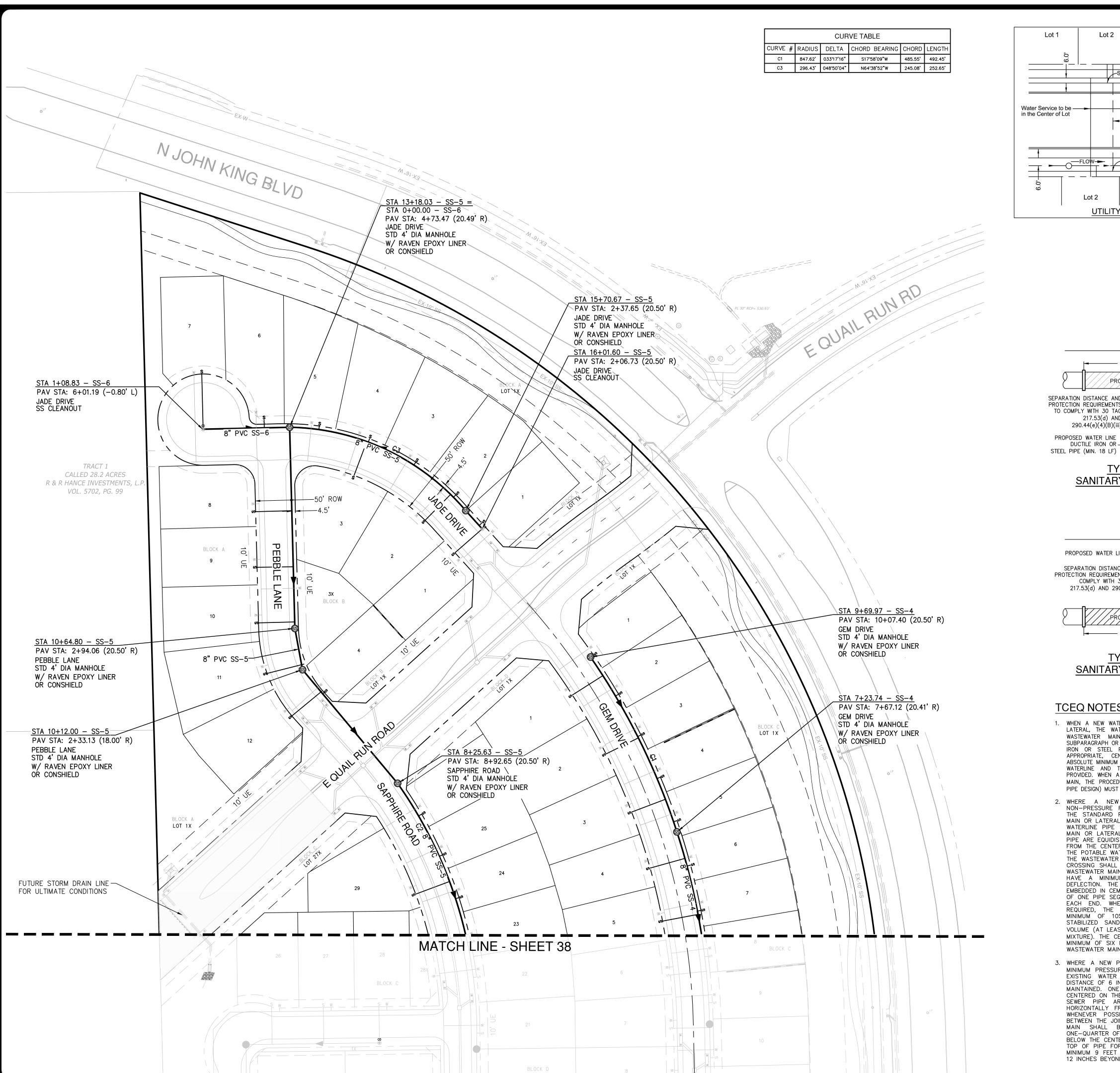
0 PAPE-DAWS(ENGINEERS

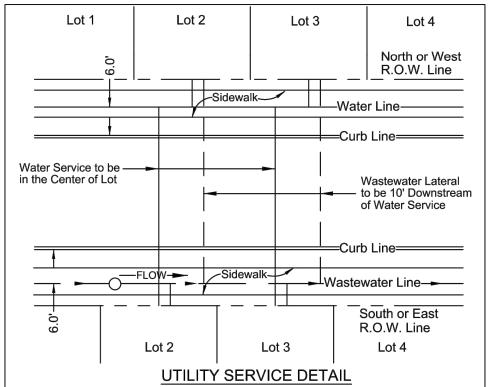
GIDEON GROVE - PHASE 2
OF ROCKWALL, ROCKWALL COUNTY, TEXA

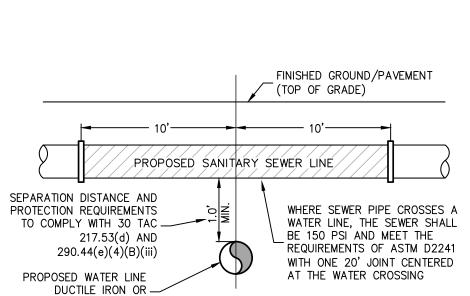
WATER LINE W-6 PLAN AND PROFIL

6126300 June 21 DESIGNER HECKED____ DRAWN__

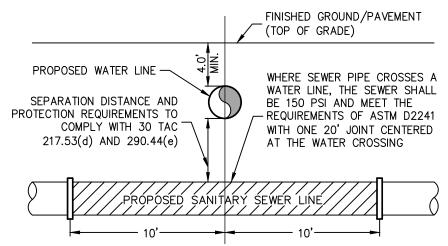
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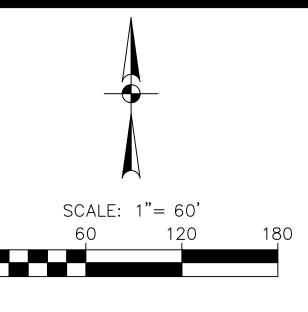
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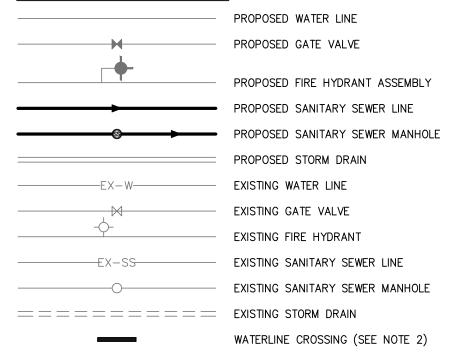


BENCHMARKS

 $\frac{\text{TBM NO. 1}}{\text{X-CUT SET IN CONCRETE LOCATED IN THE}} \times \frac{\text{TBM NO. 2}}{\text{X-CUT SET IN CONCRETE LOCATED IN THE}}$ CENTER OF A CURB INLET IN THE CENTER OF A CURB INLET IN THE WEST SOUTHWESTERLY CURB LINE OF NORTH CURB LINE OF NORTH JOHN KING JOHN KING BOULEVARD AND BEING +/- BOULEVARD AND BEING +/- 662' NORTH 235' SOUTHEST OF THE NORTHWEST CORNER OF THE SUBJECT TRACT AND +/-BOULEVARD AND WEST QUAIL RUN ROAD 535' NORTHWEST OF THE INTERSECTION OF AND ± -723 ' SOUTHEAST OF THE N. JOHN KING BOULEVARD AND EAST INTERSÉCTION OF N. JOHN KING QUAIL RUN ROAD ELEV: 546.16'

OF THE INTERSECTION NORTH JOHN KING BOULEVARD AND EAST QUAIL RUN ROAD. ELEV: 530.38'

SEWER LEGEND



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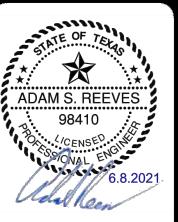
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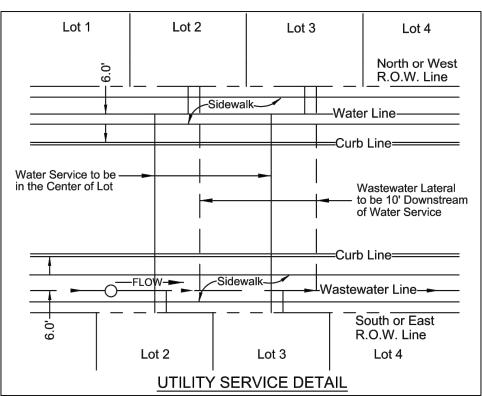


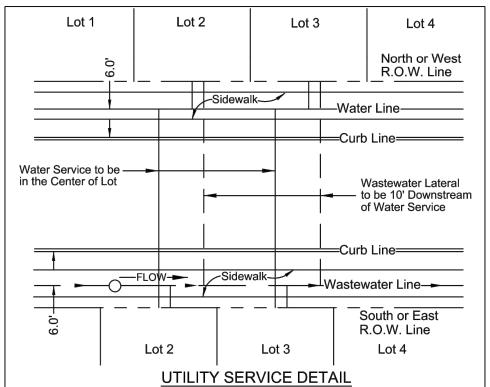
PAPE-DAV ENGINEEN

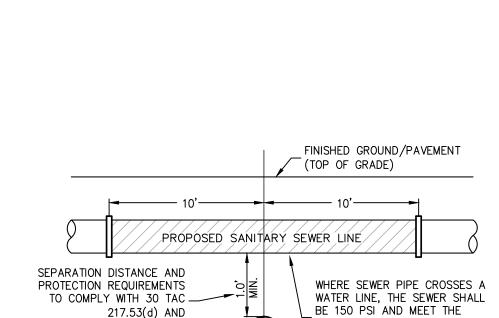
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6126300 June 21 ESIGNER HECKED DRAWN

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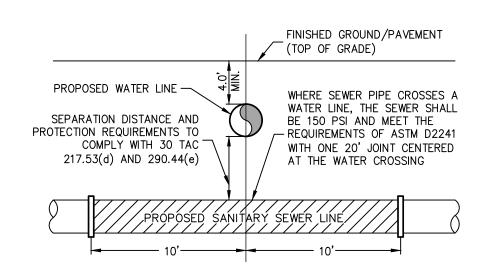


TYPICAL WATER BELOW SANITARY SEWER CROSSING DETAIL NOT-TO-SCALE

REQUIREMENTS OF ASTM D2241

WITH ONE 20' JOINT CENTERED

AT THE WATER CROSSING



TYPICAL WATER ABOVE SANITARY SEWER CROSSING DETAIL NOT-TO-SCALE

TCEQ NOTES

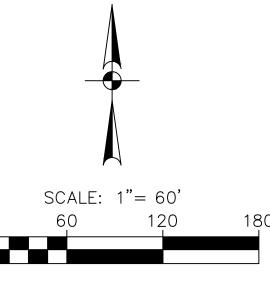
290.44(e)(4)(B)(iii)

DUCTILE IRON OR -

PROPOSED WATER LINE

STEEL PIPE (MIN. 18 LF)

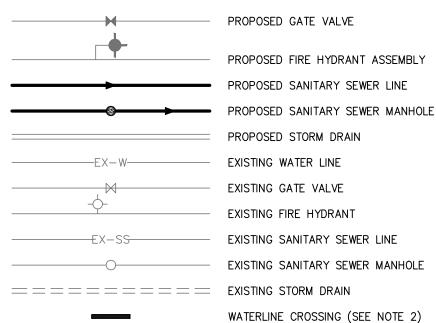
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BENCHMARKS

CENTER OF A CURB INLET IN THE SOUTHWESTERLY CURB LINE OF NORTH 235' SOUTHEST OF THE NORTHWEST N. JOHN KING BOULEVARD AND EAST QUAIL RUN ROAD ELEV: 546.16'

SEWER LEGEND



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03/22/2022

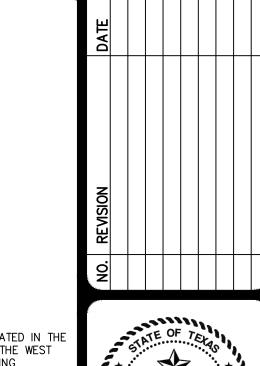
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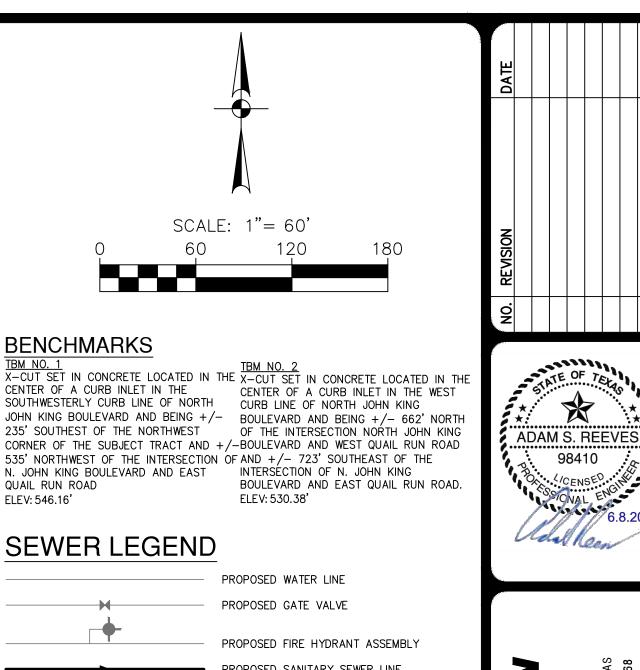
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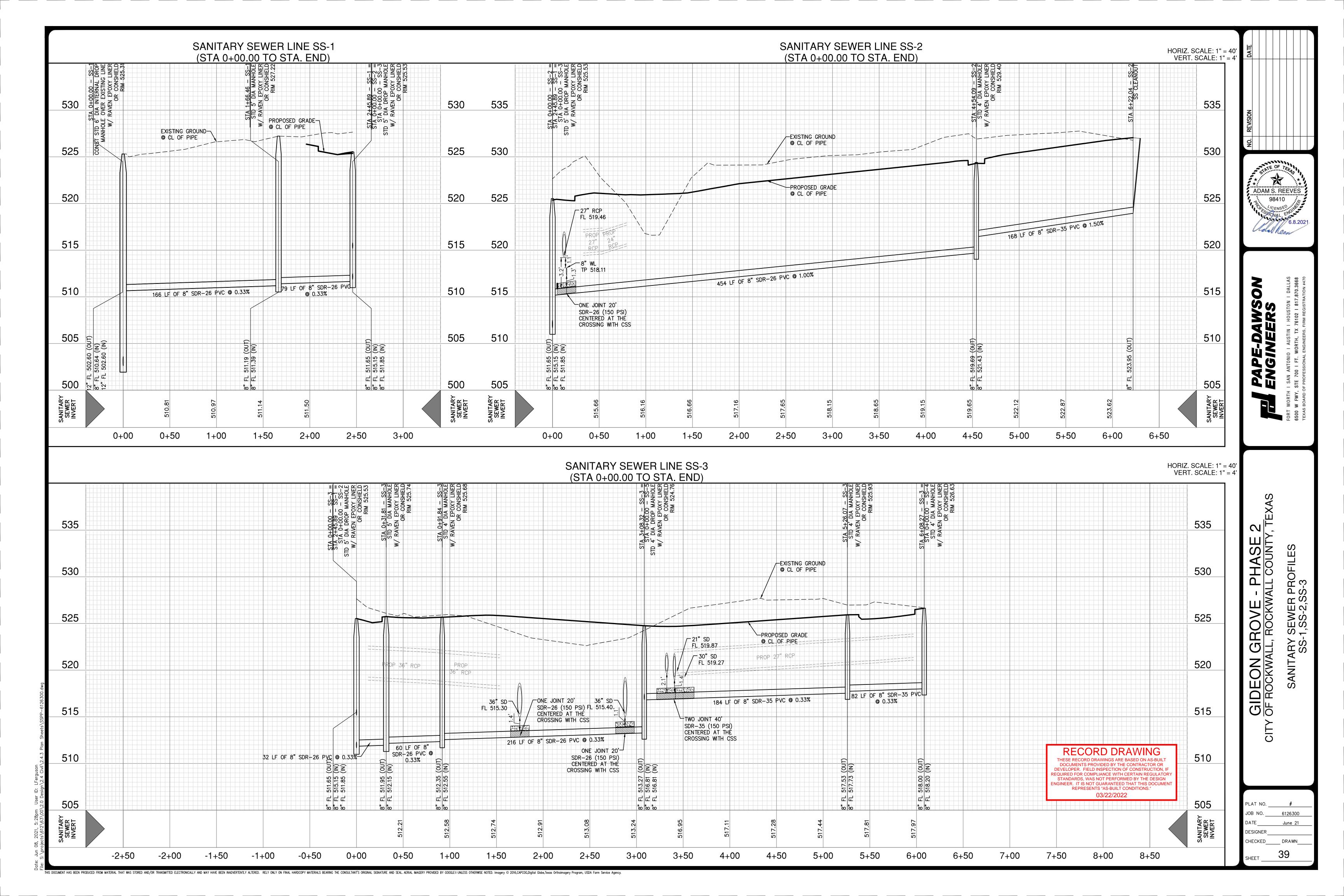
6126300 June 21 ESIGNER HECKED DRAWN 38

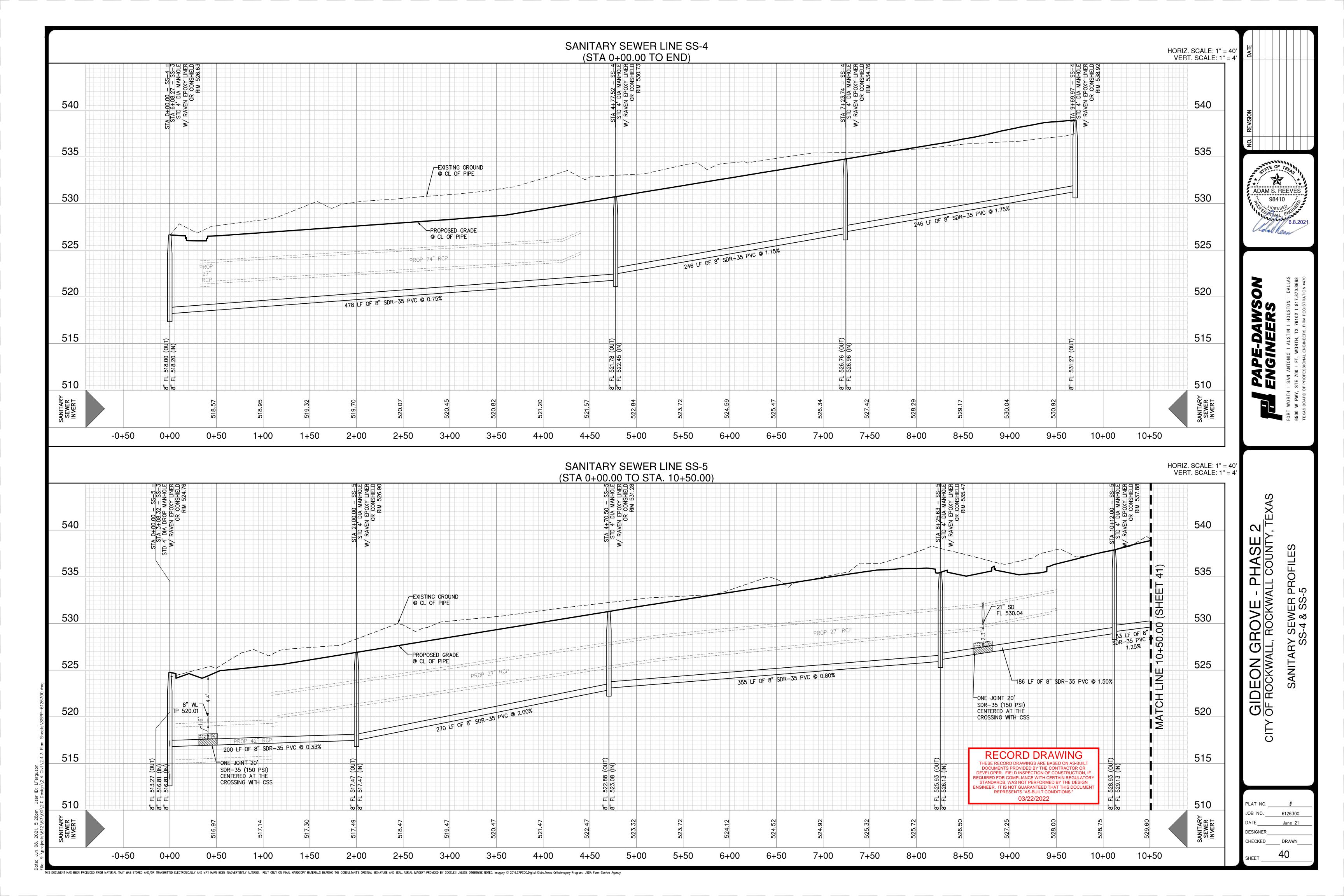


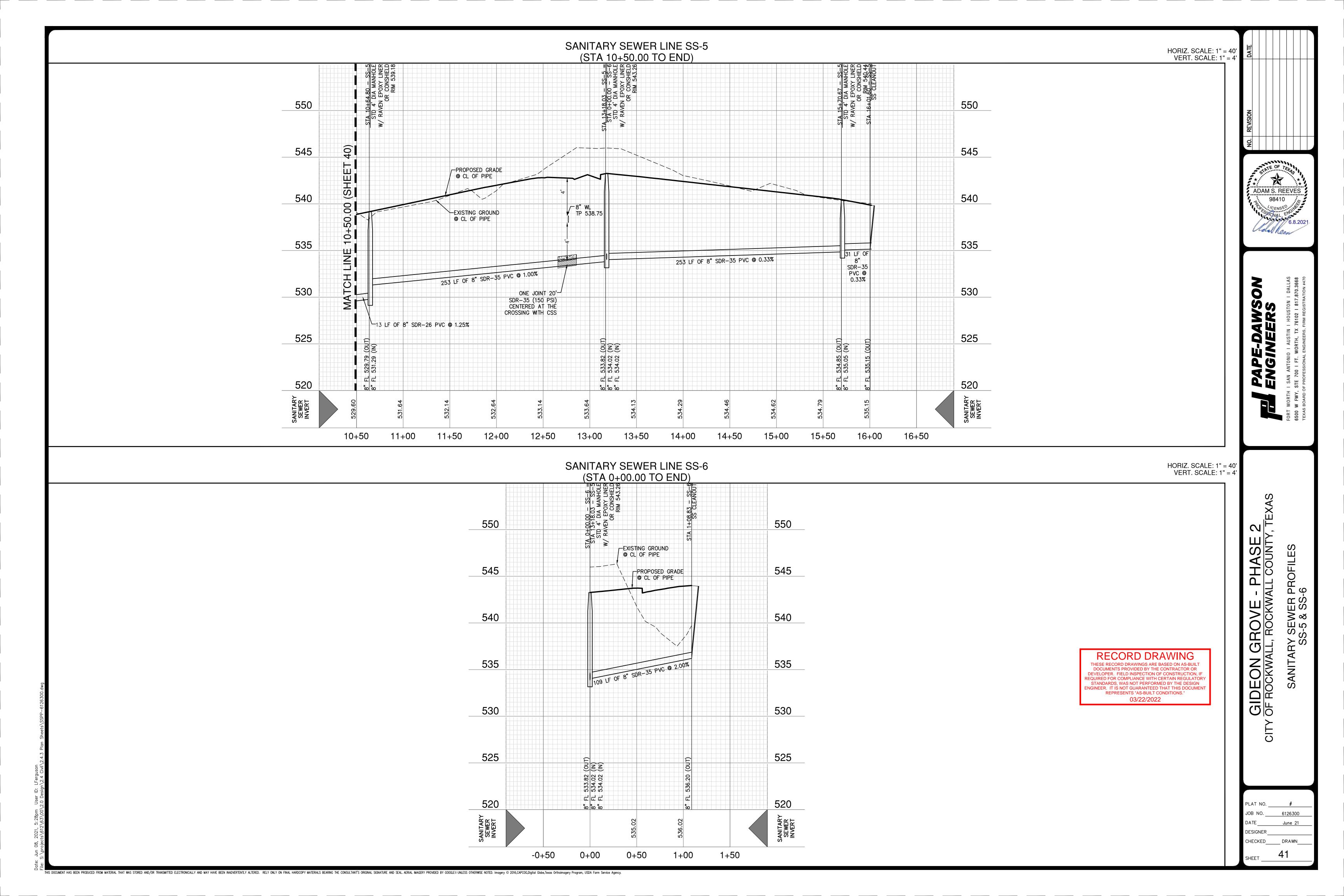
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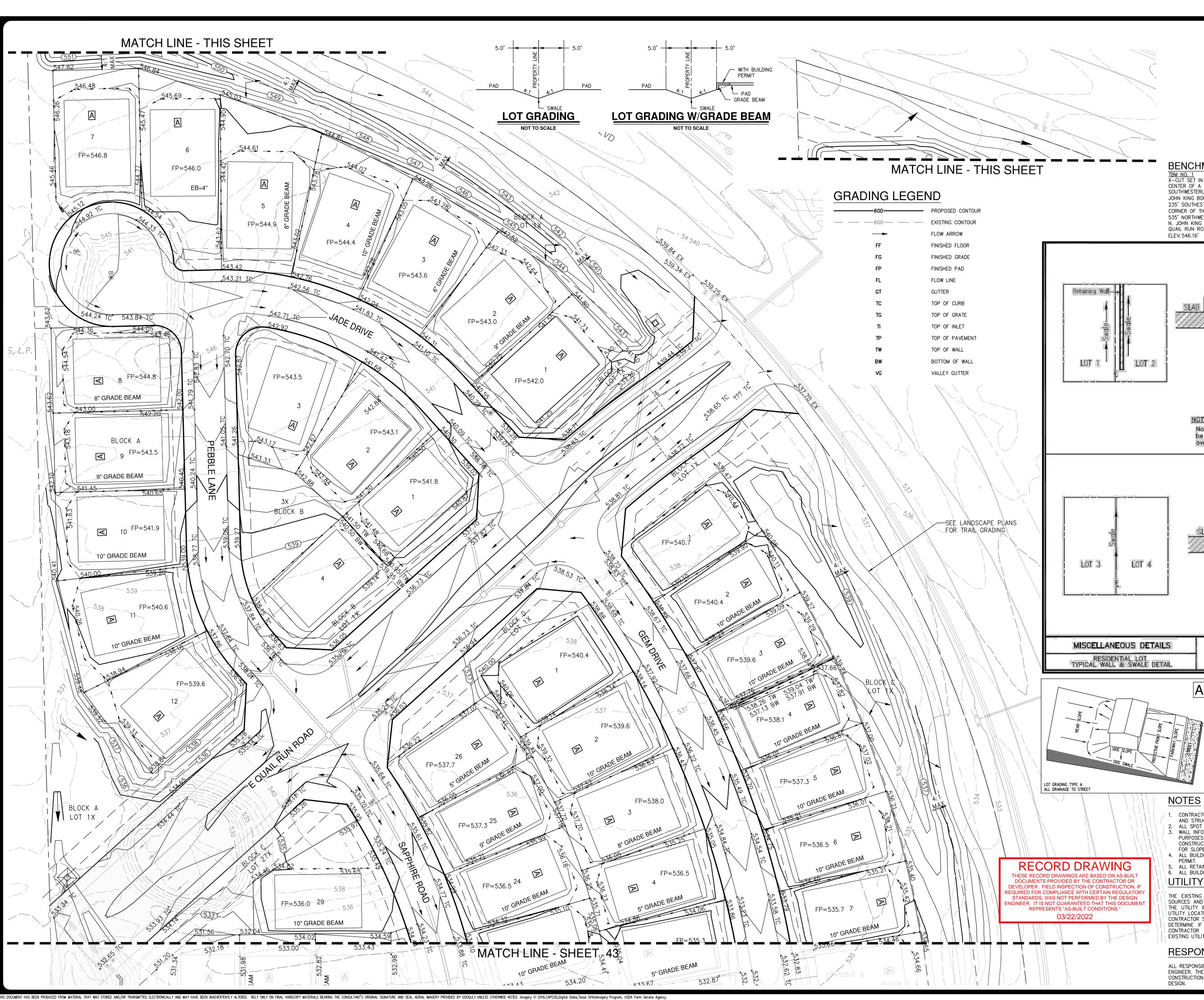
PAPE-DAV ENGINEEN













ELEV: 546.16'

TBM NO. 1

X—CUT SET IN CONCRETE LOCATED IN THE CENTER OF A CURB INLET IN THE SOUTHWESTERLY CURB LINE OF NORTH

CURB LINE OF NORTH CURB LINE OF NORTH CURB LINE OF NORTH CURB LINE OF NORTH CURB LINE OF NORTH CURB LINE OF NORTH CURB LINE OF NORTH CONCRETE LOCATED IN THE CENTER OF A CURB INLET IN THE WEST CURB LINE OF NORTH JOHN KINGS AND CONCRETE LOCATED IN THE CENTER OF A CURB LINE OF NORTH JOHN KINGS AND CONCRETE LOCATED IN THE CENTER OF A CURB LINE OF NORTH JOHN KINGS AND CONCRETE LOCATED IN THE CENTER OF A CURB LINE OF NORTH JOHN KINGS AND CONCRETE LOCATED IN THE CENTER OF A CURB LINE OF NORTH JOHN KINGS AND CONCRETE LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF A CURB LINE OF NORTH LOCATED IN THE CENTER OF NORTH LOCATED IN THE CENTER OF NORTH LOCATED IN THE CENTER OF NORTH LOCATED IN THE C JOHN KING BOULEVARD AND BEING +/- BOULEVARD AND BEING +/- 662' NORTH 235' SOUTHEST OF THE NORTHWEST OF THE INTERSECTION NORTH JOHN KING CORNER OF THE SUBJECT TRACT AND +/-BOULEVARD AND WEST QUAIL RUN ROAD 5.35' NORTHWEST OF THE INTERSECTION OF AND +/- 723' SOUTHEAST OF THE
N. JOHN KING BOULEVARD AND EAST INTERSECTION OF N. JOHN KING
QUAIL RUN ROAD BOULEVARD AND EAST QUAIL RUN ROAD.

No Retaining Walls including the footing shall be placed in the right—of—way, easements, or overlapping property lines.

SCALE: 1"= 40'

ELEV: 530.38'

/ 6" Above

Finish Grade

AUG. 15 R-7020

ADAM S. REEVES 98410

6126300 June 21 DESIGNER HECKED<u>ASR</u> DRAWN<u>RI</u>

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RESPONSIBILITY NOTE

CITY OF ROCKWALL

LOT GRADING TYPE B DRAINAGE BOTH TO STREET & REAR LOT LINE

AND STRUCTURES UNLESS OTHERWISE NOTED.

5. ALL RETAINING WALLS TO BE ROCK OR STONE FACED.
6. ALL BUILDING PERMITS TO INCLUDE A LOT GRADING PLAN

FOR SLOPE AND SOIL STABILITY.

UTILITY NOTE

1. CONTRACTOR SHALL PROTECT ALL EXISTING TREES, FENCES, RETAINING WALLS

ALL SPOT GRADES ARE FINISHED GRADE UNLESS OTHERWISE NOTED.
WALL INFORMATION SHOWN ON PLANS IS TO BE USED FOR HEIGHT AND LOCATION PURPOSES ONLY. ACTUAL WALL AND SLOPE STABILITY DESIGN BY OTHERS. ALL

CONSTRUCTION ON SLOPES EXCEEDING 4:1 SHALL REQUIRE GEOTECHNICAL DESIGN

4. ALL BUILDING PERMITS WILL REQUIRE A GRADING PLAN TO BE SUBMITTED WITH

THE EXISTING UTILITIES SHOWN ON THESE PLANS WERE COMPILED FROM VARIOUS

SOURCES AND ARE INTENDED TO SHOW THE GENERAL EXISTENCE AND LOCATION OF

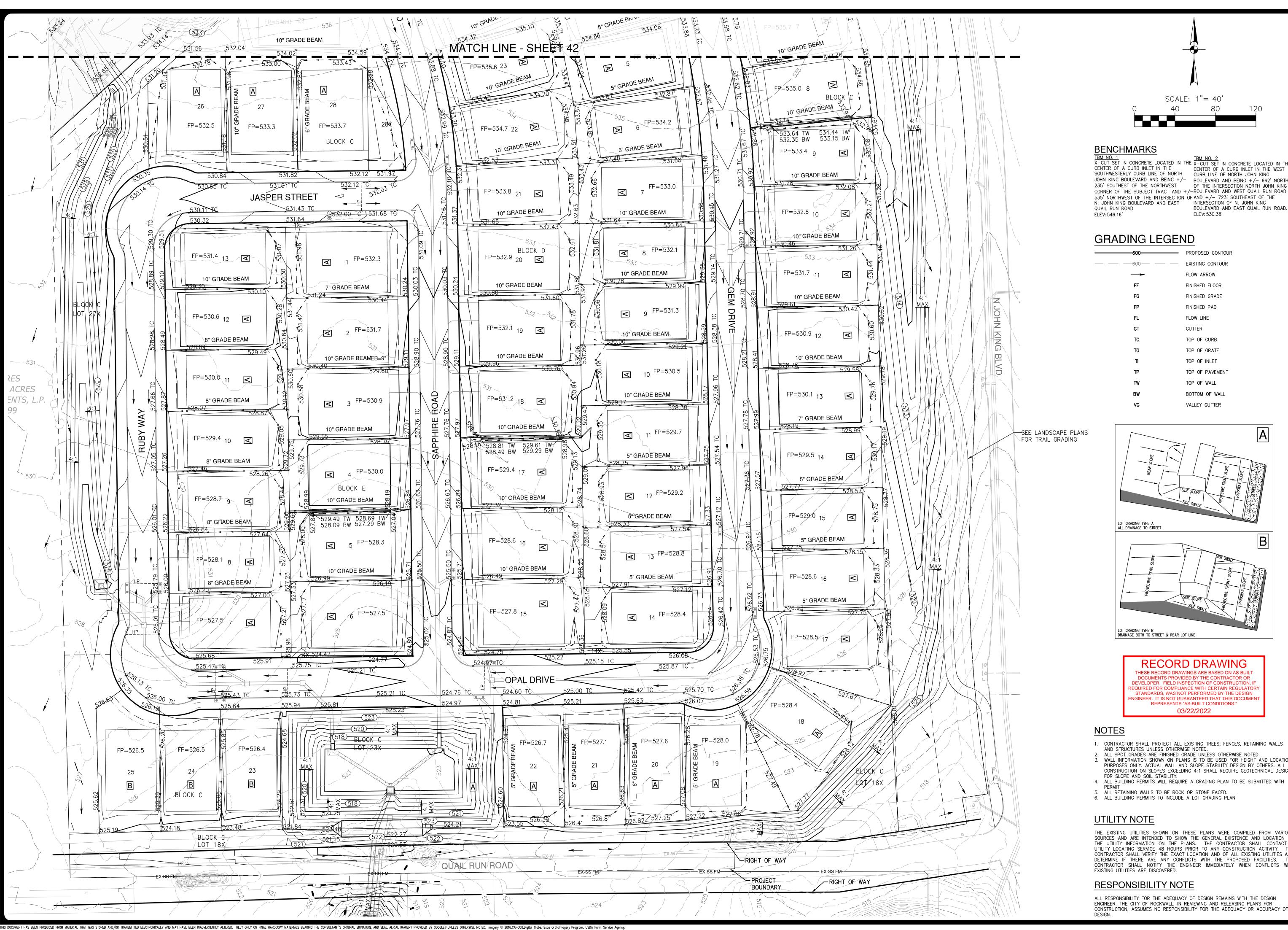
THE UTILITY INFORMATION ON THE PLANS. THE CONTRACTOR SHALL CONTACT A

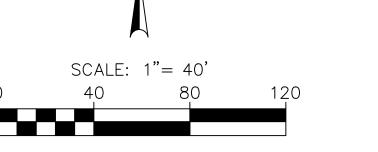
CONTRACTOR SHALL VERIFY THE EXACT LOCATION AND OF ALL EXISTING UTILITIES AND

DETERMINE IF THERE ARE ANY CONFLICTS WITH THE PROPOSED FACILITIES. TH CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY WHEN CONFLICTS WITH EXISTING UTILITIES ARE DISCOVERED.

UTILITY LOCATING SERVICE 48 HOURS PRIOR TO ANY CONSTRUCTION ACTIVITY.

ALL RESPONSIBILITY FOR THE ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR THE ADEQUACY OR ACCURACY OF





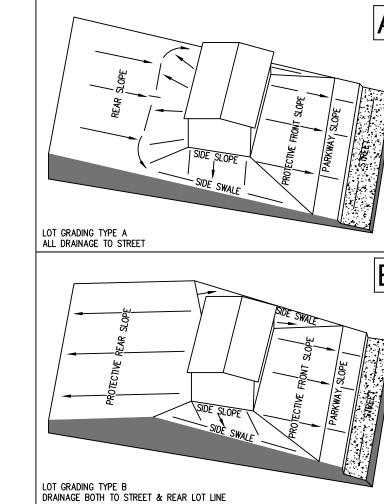
TBM NO. 1

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ELEV: 530.38'

GRADING LEGEND

	PROPOSED CONTOU
<u> </u>	EXISTING CONTOUR
	FLOW ARROW
FF	FINISHED FLOOR
FG	FINISHED GRADE
FP	FINISHED PAD
FL	FLOW LINE
GT	GUTTER
TC	TOP OF CURB
TG	TOP OF GRATE
П	TOP OF INLET
TP	TOP OF PAVEMENT
TW	TOP OF WALL
BW	BOTTOM OF WALL
VG	VALLEY GUTTER



THESE RECORD DRAWINGS ARE BASED ON AS-BUILT DOCUMENTS PROVIDED BY THE CONTRACTOR OR DEVELOPER. FIELD INSPECTION OF CONSTRUCTION, IF REQUIRED FOR COMPLIANCE WITH CERTAIN REGULATOR' STANDARDS, WAS NOT PERFORMED BY THE DESIGN NGINEER. IT IS NOT GUARANTEED THAT THIS DOCUMEN REPRESENTS "AS-BUILT CONDITIONS." 03/22/2022

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RESPONSIBILITY NOTE

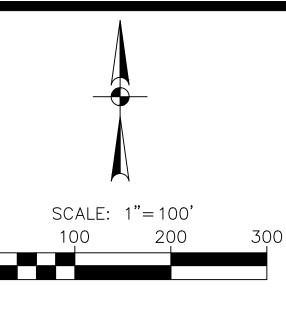
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ADAM S. REEVES 98410

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TBM NO. 1

X-CUT SET IN CONCRETE LOCATED IN THE X-CUT SET IN CONCRETE LOCATED IN THE CENTER OF A CURB INLET IN THE CENTER OF A CURB INLET IN THE WEST SOUTHWESTERLY CURB LINE OF NORTH CURB LINE OF NORTH JOHN KING JOHN KING BOULEVARD AND BEING +/- BOULEVARD AND BEING +/- 662' NORTH 235' SOUTHEST OF THE NORTHWEST OF THE INTERSECTION NORTH JOHN KING CORNER OF THE SUBJECT TRACT AND +/-BOULEVARD AND WEST QUAIL RUN ROAD 535' NORTHWEST OF THE INTERSECTION OF AND +/- 723' SOUTHEAST OF THE N. JOHN KING BOULEVARD AND EAST UNITERSECTION OF N. JOHN KING BOULEVARD AND EAST QUAIL RUN ROAD.

ELEV: 546.16' ELEV: 530.38'

EROSION CONTROL LEGEND

---- EXISTING CONTOUR

PROPOSED CONTOUR FLOW ARROW

INLET PROTECTION STABALIZED CONSTRUCTION

ENTRANCE/EXIT (FIELD LOCATE)

STONE OVERFLOW STRUCTURE

LIMITS OF DISTURBED AREAS

CONCRETE WASH AREA

PROPOSED STORM DRAIN

ROCK CHECK DAM

SLOPE STABILZATION BLANKET WASTE MATERIAL STORAGE

HYDROMULCH W/BINDING AGENT BINDEX BFM MATERIAL RATE FOR SLOPES ≤ 1H:1V

(CONTRACTOR TO DETERMINE IN FIELD). MUST NOT BE ALLOWED TO BE OVERFLOWING. KEEP STORAGE AREA CLEAN.

ULTIMATE BUILD OUT (NOT TO BE CONSTRUCTED WITH THIS CONTRACT)



RECORD DRAWING

THESE RECORD DRAWINGS ARE BASED ON AS-BUILT DOCUMENTS PROVIDED BY THE CONTRACTOR OR DOCUMENTS PROVIDED BY THE CONTRACTOR OR
DEVELOPER. FIELD INSPECTION OF CONSTRUCTION, IF
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NOTES

- CONTRACTOR SHALL PROTECT ALL EXISTING TREES, FENCES, RETAINING WALLS AND STRUCTURES UNLESS OTHERWISE NOTED.
- 2. TOTAL DISTURBED AREA IS 31.7 ACRES

UTILITY NOTE

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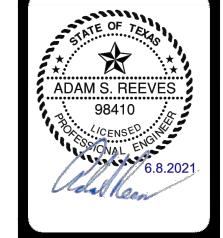
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ADAM S. REEVES

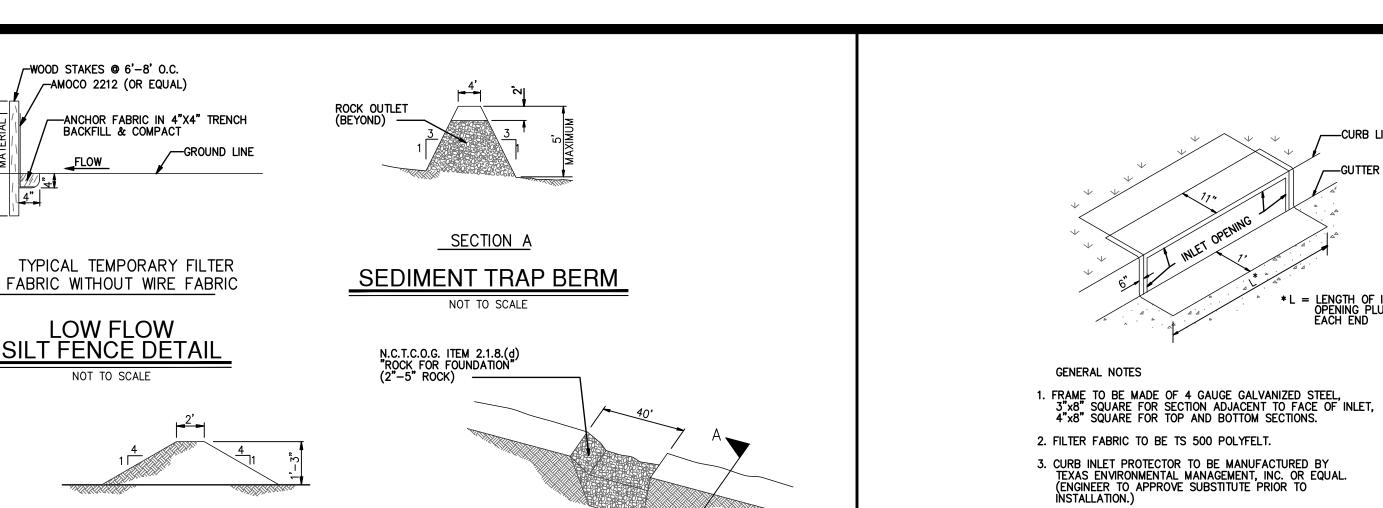




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RESPONSIBILITY NOTE

CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR THE ADEQUACY OR ACCURACY OF

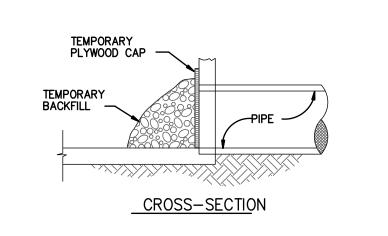


SILT FENCE DETAIL CROSS-SECTION

EARTH CONTAINMENT BERM NOT TO SCALE

SEDIMENT TRAP OUTLET

NOTE: REFERENCE CITY OF ROCKWALL ROCK CHECK DAM GENERAL NOTES FOR ADDITIONAL INFORMATION - WOVEN WIRE SHEATHING WITH MAX 1" OPENING, MIN WIRE DIA 20 GAUGE - REFERENCE CITY OF ROCKWALL ROCK CHECK DAM GENERAL NOTES FOR STONE SIZING SECTION A **ROCK CHECK DAM**

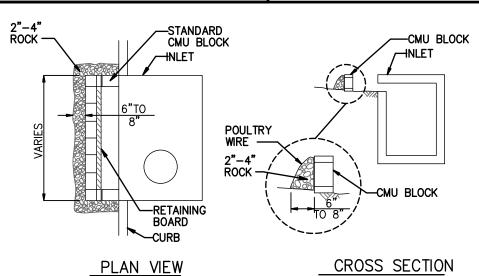


CURB INLET PROTECTION (STAGE ONE) NOT TO SCALE

CURB INLET PROTECTION

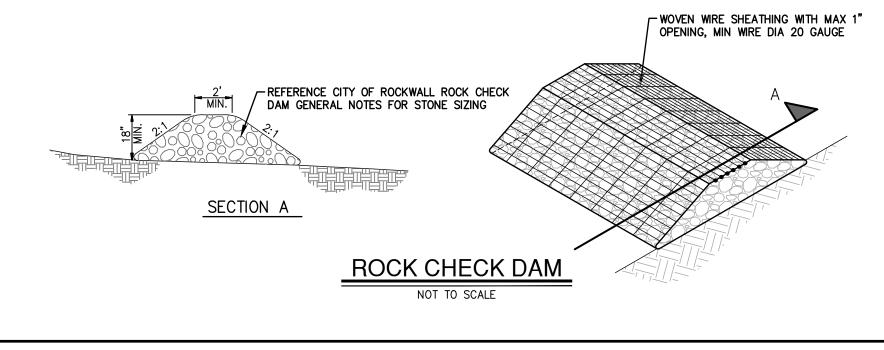
(STAGE TWO)

NOT TO SCALE

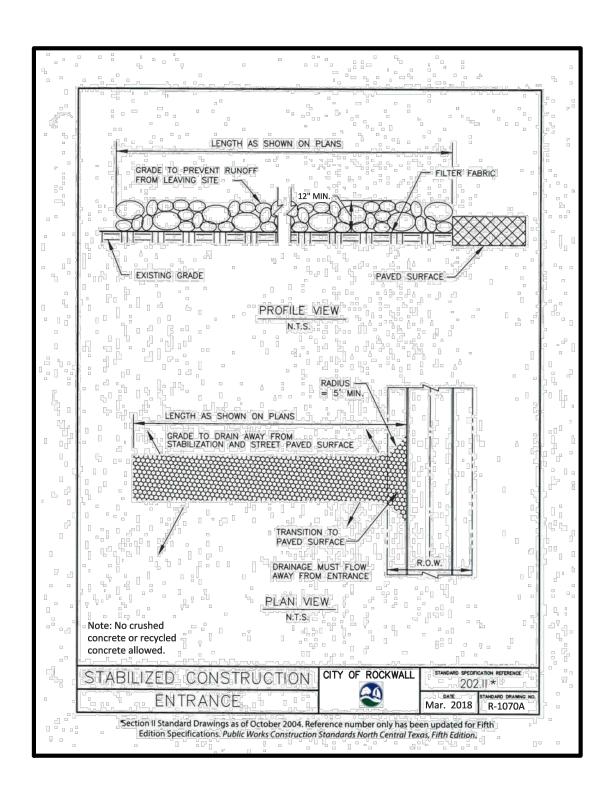


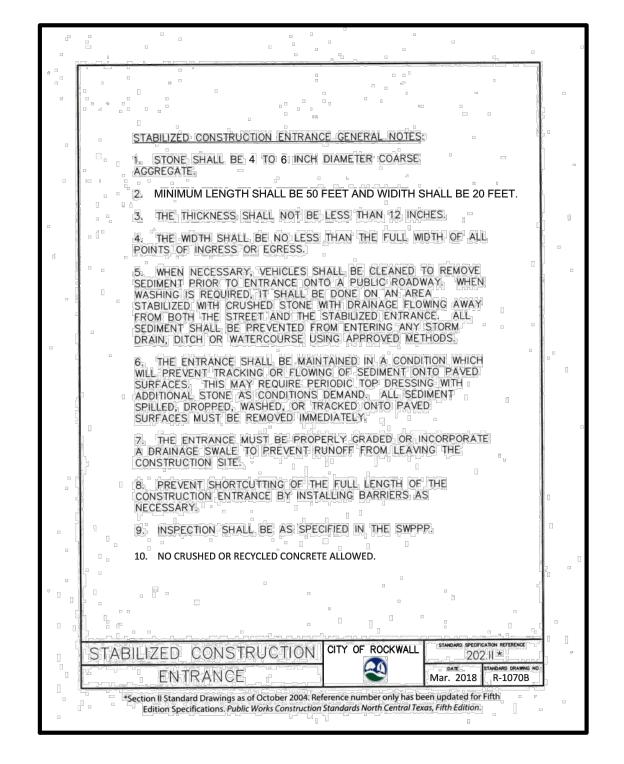
___CURB LINE

ALTERNATE 1 **CURB INLET PROTECTION** (STAGE TWO) NOT TO SCALE



ROCK CHECK DAM GENERAL NOTES: STONE SHALL BE WELL GRADED WITH SIZE RANGE FROM 1½ TO 3½ INCHES IN DIAMETER DEPENDING ON EXPECTED THE CHECK DAM SHALL BE INSPECTED AS SPECIFIED IN THE SWPPP AND SHALL BE REPLACED WHEN THE STRUCTURE CEASES TO FUNCTION AS INTENDED DUE TO SILT ACCUMULATION AMONG THE ROCKS, WASHOUT, CONSTRUCTION TRAFFIC DAMAGE, ETC. 3: WHEN SILT REACHES A DEPTH EQUAL TO ONE THIRD OF THE HEIGHT OF THE CHECK DAM OR ONE FOOT, WHICHEVER IS LESS, THE SILT SHALL BE REMOVED AND DISPOSED OF 4. WHEN THE SITE HAS ACHIEVED FINAL STABILIZATION OR ANOTHER EROSION OR SEDIMENT CONTROL DEVICE US EMPLOYED, THE CHECK DAM AND ACCUMULATED SILT SHALL BE REMOVED AND DISPOSED OF IN AN APPROVED MANNER. 5. FILTER STONE SHALL BE WRAPPED IN APPROPRIATE SIZED WIRE MESH TO CONTAIN STONE AND BURIED SIX (6") INCHES MINIMUM. CITY OF ROCKWALL ROCK CHECK DAM 202.9 * Mar. 2018 R-1060B *Section II Standard Drawings as of October 2004. Reference number only has been updated for Fifth Edition Specifications. Public Works Construction Standards North Central Texas, Fifth Edition.





DEVELOPER. FIELD INSPECTION OF CONSTRUCTION, IF REQUIRED FOR COMPLIANCE WITH CERTAIN REGULATORY STANDARDS, WAS NOT PERFORMED BY THE DESIGN ENGINEER. IT IS NOT GUARANTEED THAT THIS DOCUMEN' REPRESENTS "AS-BUILT CONDITIONS." 03/22/2022

THESE RECORD DRAWINGS ARE BASED ON AS-BUILT

DOCUMENTS PROVIDED BY THE CONTRACTOR OR

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	SEDIMEN				Mar. 2018	R-1090
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	v	nons: rubile, rolls co	46 87.5 ·			~
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SILT FENCE (MIN. HEIGHT 24" ABOVE EXIST. GROUND)

COMPACTED EARTH

OR ROCK BACKFILL

SILT FENCE

SILT FENCE GENERAL NOTES:

DEVICE IS EMPLOYED.

SIX (6") INCHES MINIMUM.

SILT FENCE

STONE OVERFLOW STRUCTURE

*Section II Standard Drawings as of October 2004. Reference number only has been updated for Fifth

Edition Specifications. Public Works Construction Standards North Central Texas, Fifth Edition.

1. POSTS WHICH SUPPORT THE SILT FENCE SHALL BE

INSTALLED ON A SLIGHT ANGLE TOWARD THE ANTICIPATED

RUNOFF SOURCE. POST MUST BE EMBEDDED A MINIMUM OF

2. THE TOE OF THE SILT FENCE SHALL BE TRENCHED IN WITH A SPADE OR MECHANICAL TRENCHER, SO THAT THE DOWNSLOPE FACE OF THE TRENCH IS FLAT AND PERPENDICULAR TO THE LINE OF FLOW. WHERE FENCE CANNOT

BE TRENCHED IN (e.g. PAVEMENT), WEIGHT FABRIC FLAP WITH ROCK ON UPHILL SIDE TO PREVENT FLOW FROM SEEPING UNDER

3. THE TRENCH MUST BE A MINIMUM OF 6 INCHES DEEP AND

6 INCHES WIDE TO ALLOW FOR THE SILT FENCE FABRIC TO BE

LAID IN THE GROUND AND BACKFILLED WITH COMPACTED

4. SILT FENCE SHOULD BE SECURELY FASTENED TO EACH

SUPPORT POST OR TO WIRE BACKING, WHICH IN TURN IS ATTACHED TO THE FENCE POST. THERE SHALL BE A 3 FOOT OVERLAP, SECURELY FASTENED WHERE ENDS OF FABRIC MEET. INSPECTION SHALL BE AS SPECIFIED IN THE SWPPP REPAIR OR REPLACEMENT SHALL BE MADE PROMPTLY AS

SILT FENCE SHALL BE REMOVED WHEN FINAL STABILIZATION IS ACHIEVED OR ANOTHER EROSION OR SEDIMENT CONTROL

7. ACCUMULATED SILT SHALL BE REMOVED WHEN IT REACHES A DEPTH OF HALF THE HEIGHT OF THE FENCE, THE SILT SHALL BE DISPOSED OF AT AN APPROVED SITE AND IN SUCH A MANNER AS TO NOT CONTRIBUTE TO ADDITIONAL SILTATION.

8. FILTER STONE SHALL BE WRAPPED IN FILTER FABRIC AND BURIED

*Section II Standard Drawings as of October 2004. Reference number only has been updated for Fifth

Edition Specifications. Public Works Construction Standards North Central Texas, Fifth Edition.

CITY OF ROCKWALL

CITY OF ROCKWALL

4' LENGTH (MIN.) FENCE POST MAX. 8' SPACING, MIN. EMBEDMENT = 1

WIRE MESH BACKING

POST MUST BE

METAL, NO WOODED

STONE, EACH SIDE OF SILT FENCE

202.5 *

STANDARD SPECIFICATION REFERENCE

SETTLEMENT IF FILE

HIS DOCUMENT HAS BEEN PRODUCED FROM MATERIAL THAT WAS STORED AND/OR TRANSMITTED ELECTRONICALLY AND MAY HAVE BEEN INADVERTENTLY ALTERED. RELY ONLY ON FINAL HARDCOPY MATERIALS BEARING THE CONSULTANT'S ORIGINAL SIGNATURE AND SEAL. AERIAL IMAGERY PROVIDED BY GOOGLE® UNLESS OTHERWISE NOTED. Imagery © 2016,CAPCOG,Digital Globe,Texas Orthoimagery Program, USDA Farm Service Agency.

STAKES ALLOWED.

DESIGNER HECKED<u>PD</u> DRAWN<u>PD</u>

6126300

June 21